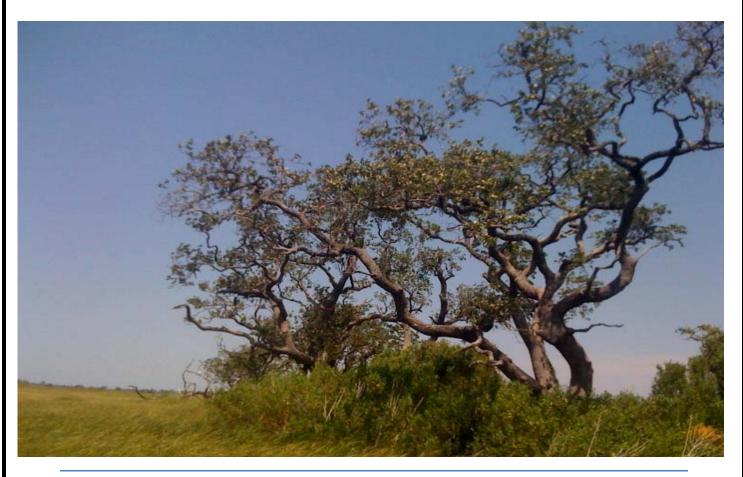
# Preliminary (30%) Design Report

April 2012



# Bayou Bonfouca Marsh Creation Project

St. Tammany Parish, Louisiana

# PO-104







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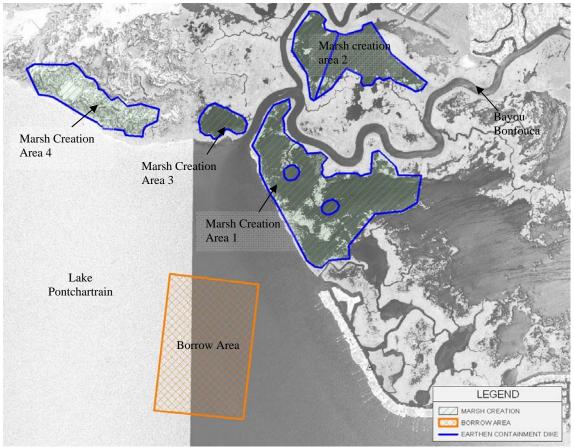
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## **1.0 INTRODUCTION**

The Bayou Bonfouca Marsh Creation Project (PO-104) is located in the Lake Pontchartrain Basin along the northeastern corner of Lake Pontchartrain as shown in Figure 1. The Louisiana Coastal Wetlands Planning, Protection and Restoration Task Force designated PO-104 as part of the 20<sup>th</sup> Priority Project List. The United States Fish and Wildlife Service (USFWS) was designated as the lead federal sponsor with funding approved through the Coastal Wetlands Planning, Protection and Restoration Act (CWPPRA) of 1990 by the United States Congress and the Wetlands Conservation Trust Fund by the State of Louisiana. The Louisiana Coastal Protection and Restoration Authority (CPRA) is serving as the local sponsor and will also be providing engineering and design services.



**Figure 1 – Project Layout** 

The primary goal of PO-104 is to re-create and nourish approximately 639 acres of low salinity brackish marsh in open waters adjacent to Bayou Bonfouca with sediment dredged from Lake Pontchartrain.

The poor condition of this marsh is due to a combination of subsidence, hurricanes causing interior ponding, shoreline erosion. Although the shoreline erosion rates are relatively low, only a narrow strip of shoreline currently exists between Lake

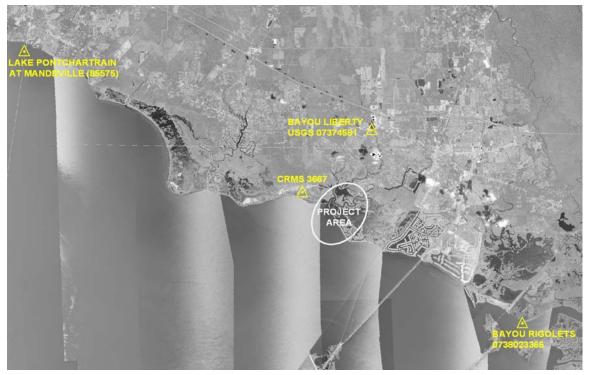
Pontchartrain and the interior ponds. Several breaches are known to exist along the shoreline. Should shoreline breaching and enlargement of tidal channels allow high tidal energy to intrude into the interior ponds of the project area, the interior marshes will experience accelerated loss rates. Restoration of the marsh adjacent to Lake Pontchartrain would provide vital protection to the interior marsh to the north.

The project team, consisting of members of USFWS, CPRA, St. Tammany Parish Government, and Big Branch Marsh National Wildlife Refuge, performed an on-site kick-off meeting on April 21, 2011. Based on that meeting, a plan was developed to identify and address all of the project requirements. The engineering and design, environmental compliance, real estate negotiations, operation/maintenance planning, and cultural resources investigation have been completed to the preliminary (30%) design level as required by the CWPPRA Standard Operating Procedures.

## 2.0 EXISTING CONDITIONS

#### **2.1 Tidal Conditions**

Tidal variations can be considered as being comprised of periodic and apparent secular trends. In order to evaluate these variations, a specific 18.6-year period based upon the Metonic Cycle should be selected so that all tidal datum determinations will have a common reference period. This period is termed a tidal epoch. Unfortunately, Hurricane



**Figure 2: Gage Locations** 

Katrina destroyed all tidal gages in Lake Pontchartrain; therefore, obtaining a continuous up-to-date tidal epoch in the area was not possible. Data from 4 gages was evaluated for calculation of Mean High Water (MHW) and Mean Low Water (MLW). The gage locations are shown in figure 2. They were the United States Geological Survey (USGS) stations at Bayou Rigolets #0738023365 and Bayou Liberty #07374581, CPRA's Coastwide Reference Monitoring Station (CRMS) 3667, and the United States Army Corps of Engineers (USACE) Mandeville gage 85575. The Grand Isle gage is the closest gage that contains a continuous epoch. However, its large tidal ranges would skew the local tidal data in the Bonfouca project area. Therefore, no techniques to standardize the data such as the Range-Ratio method were utilized. A summary of the MHW and MLW levels, adjusted to NAVD 88, for each of the four gages mentioned above is shown in table 1.

Gage	Date Range	MHW (ft)	MLW (ft)
Bayou Rigolets USGS 0738023365	10/94 - 8/05	1.19	0.23
Bayou Liberty USGS 07374581	1/01 - 10/11	1.31	0.64
Lake Pontchartrain-Mandeville USACE			
85575	1/95 - 9/11	0.92	0.37
CRMS 3667	1/07 - 12/10	1.07	0.75
(PO- 33) Goose Point Marsh Creation Project	NA	1.08*	0.48*

#### Table 1: Tidal Gage Data

\*Calculated using the Range-Ratio Method and Grand Isle Gage

The MHW and MLW levels shown in Table 1 demonstrate the tidal variability near the project area. This variability is believed to be related to the gages' location and hydraulic connectivity to the more dynamic Gulf system. The larger ranges occur in areas governed by Gulf tides while the lowest range occurs at the CRMS 3667 gage which is located in the interior marsh. The average MHW and MLW of the four sites is 1.1 feet and 0.5 feet respectively. These values are similar to the values calculated for the PO-33 Goose Point Marsh Creation Project located immediately to the west of this project.

To demonstrate the effects of location on the various tidal gages, a comparison of the tidal heights between the Bayou Rigolets and Bayou Liberty gages during the 48- hour period of October 17 & 18, 2007 is shown in Figure 3. A large difference in amplitude and a tidal lag exists between the two gages. The Bayou Rigolets gage is located in between Lakes Borgne and Pontchartrain and are very tidally influenced. In contrast, the Bayou Liberty gage is located farther away and, therefore, is less controlled by the shifting of the tides.

Bayou Bonfouca Marsh Creation (PO-104) Preliminary Design Report

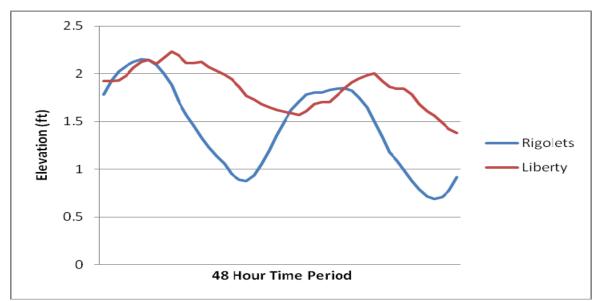


Figure 3: Comparison of Bayous Rigolets and Liberty's tidal graphs

#### 2.2 Winds

Wind data was analyzed from 3 stations around Lake Pontchartrain, including New Orleans Lakefront Airport (KNEW) from 1996-2011, Slidell Airport (KASD) from 1999-2010, Louis Armstrong New Orleans International Airport (KMSY) from 1996-2011. All 3 wind roses, located in Appendix H, show the maximum wind speeds coming from the north and south directions, with some additional strong winds coming from the

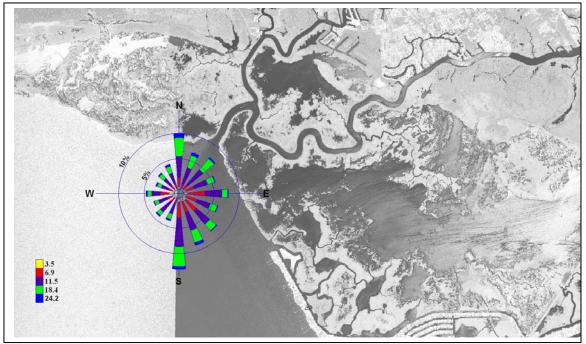


Figure 4: Louis Armstrong Airport Wind Rose

southeast direction. Figure 4 shows how the west and southwest directions, which represent the largest fetch, have some of the lower wind speeds. The maximum wind speed recorded was 69 mph at the Slidell Airport station. This wind speed suggests that the upper range of wind speeds is not collected due to instrumentation limitations since the data time period extended through several major hurricanes. The statistical wind speeds will be lower than actual due to the missing wind data. Based on a statistical analysis of the available hourly wind data, the 90<sup>th</sup> percentile wind direction was determined to be 166.8° clockwise from north (south-southeast). The wind speed associated with the 90<sup>th</sup> percentile wind direction was calculated to be 13.8 miles per hour. The 50<sup>th</sup> percentile wind direction was evaluated to be approximately 170° clockwise from north with an associated wind speed of 9.21 miles per hour. In order to capture the effects of an average daily wind on the project site, the 50% wind speed was used to develop the wave analysis below.

The percentile wind speeds for all these stations were calculated and determined to be consistent as shown in Tables 2 through 4 below.

	130-150	160-180	190-210	220-240	250-280
% Less	Wind Speed				
Than	mi/hr	mi/hr	mi/hr	mi/hr	mi/hr
100(max)	69.06	33.38	32.23	24.17	26.47
95	14.96	17.27	13.81	11.51	12.66
90	13.81	14.96	12.66	10.36	11.51
75	11.51	11.51	10.36	8.06	9.21
50	8.06	9.21	8.06	6.91	6.91
25	5.76	6.91	5.76	4.60	4.60

#### DEGREES FROM TRUE NORTH

 Table 2: Slidell Airport- KASD (Slidell, LA) -- Wind Data (1999-2010)

#### DEGREES FROM TRUE NORTH

	130-150	160-180	190-210	220-240	250-280
% Less	Wind Speed				
Than	mi/hr	mi/hr	mi/hr	mi/hr	mi/hr
100(max)	35.68	40.29	35.68	32.23	37.98
95	17.27	18.42	19.57	17.27	20.79
90	14.96	16.11	17.27	14.96	17.27
75	11.51	11.51	12.66	11.51	12.66
50	8.06	8.06	9.21	9.21	9.21
25	5.76	5.76	5.76	6.91	5.76

 Table 3: New Orleans Lakefront Airport- KNEW (New Orleans, LA – Wind Data (1996-2011)

	130-150	160-180	190-210	220-240	250-280
% Less	Wind Speed				
Than	mi/hr	mi/hr	mi/hr	mi/hr	mi/hr
100(max)	29.93	31.08	37.98	29.93	40.29
95	16.11	18.42	17.27	14.96	17.27
90	13.81	16.11	14.96	12.66	13.81
75	10.36	12.66	11.51	10.36	10.36
50	8.06	9.21	9.21	8.06	6.91
25	5.76	5.76	6.91	5.76	4.60

#### DEGREES FROM TRUE NORTH

#### 2.3 Waves

Three wave generating scenarios were analyzed as shown in Table 5. The wind across the longest fetch was evaluated using the  $50^{\text{th}}$  percentile winds at  $260^{\circ}$  clockwise from north. The direction of fetch associated with the 90th percentile winds was evaluated using both the  $50^{\text{th}}$  and  $90^{\text{th}}$  percentile wind speeds. The U.S. Army Corps of Engineers Coastal Engineering Manual (USACE CEM) was utilized to develop the deep water wave height for all three scenarios.

The maximum height to which a wave will run up onto the shoreline can be estimated taking the sum of the setup, mean high water level and the wave height at a point near the shoreline. This number is identified in Table 5 as absolute wave height. For a conservative estimate the wave height at the -0.1ft contour was used.

Wind Direction (TN)	Wind Speed (MPH)	Wave Height (H) ft	Absolute Wave Height (H + setup + MHW) ft
260°	8.06 (50%)	0.34	1.59
170°	9.21 (50%)	0.49	1.69
170°	20.2 (90%)	1.1	1.99

Table 5: Calculated	Wave Heig	hts using Slidell	Airport Data
I ubic cr Culculated	The second	nes asing shach	I m por t Dutu

As shown in Figure 1, this project requires dredging of a borrow area approximately 3,000 feet from the existing shoreline. As part of the borrow area impact analysis, the numerical model SWAN (Simulating WAves Nearshore) was used to assess both existing and post-dredge wave environments. SWAN is a spectral, two-dimensional wave generation and transformation model. Coast and Harbor Engineering, Inc. (CHE) performed this task for CPRA. Technical information regarding the SWAN modeling is available in Appendix C.

The input conditions for the CHE numerical wave model were developed in coordination with CPRA for a total of 8 different scenarios, which are summarized in Table 6. The conditions consisted of the existing and dredged bathymetry scenarios, two water levels – mean high water (MHW) and mean low water (MLW), and the  $50^{\text{th}}$  percentile wind

Table 4: Louis Armstrong New Orleans International Airport- KMSY (Kenner, LA)

 Wind Data (1996-2011)

speed for two directions -  $170^{\circ}$  and  $260^{\circ}$  from true north (from nearby wind gages). Wave heights were then extracted at 3 points from the SWAN results to quantify the change in magnitude of the wave heights near the shoreline.

Wind Direction (TN)	Wind Speed (MPH)	Water level	Bathymetry Condition
260°	8.06	MHW	Existing
260°	8.06	MHW	Dredged
260°	8.06	MLW	Existing
260°	8.06	MLW	Dredged
170°	9.21	MHW	Existing
170°	9.21	MHW	Dredged
170°	9.21	MLW	Existing
170°	9.21	MLW	Dredged

**Table 6: Scenarios for SWAN Input Conditions** 

Modeling showed that the excavation of the borrow area did not increase wave energy in any appreciable amount at the existing shoreline. The direction of waves around the edges of the borrow area are slightly changed due to wave refraction, but the magnitude of change is small. The maximum increase in wave height occurred at MLW for both wind directions analyzed, which resulted in only a 1 cm increase in wave height (~2% increase). The magnitude of change in wave heights computed by SWAN is determined to be insignificant due to the relative accuracy of bathymetry measuring instruments and

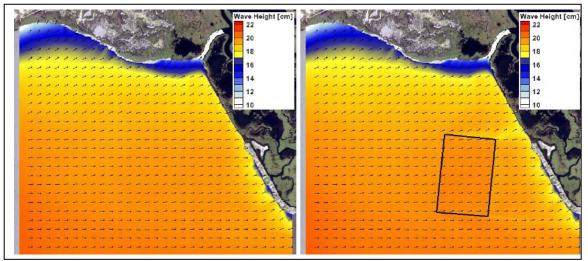


Figure 5: Wave Heights computed by SWAN for runs at MLW and 8.06 MPH winds from 260° for (a) existing and (b) dredged bathymetry conditions

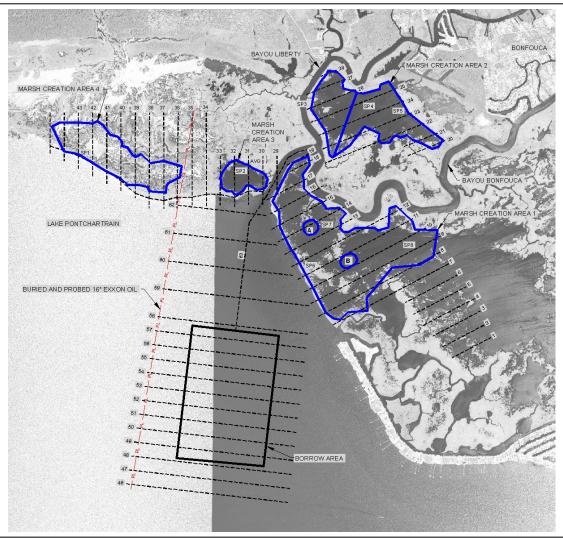
within the natural variability of waves at the project site. Figures 7a and 7b show the predicted wave heights and set up with and without the excavation of the borrow area. The small magnitude of change is not expected to cause any significant shift in the existing morphological conditions at the project site.

When added to the MHW of 1.1 feet, the wave heights without excavation of the borrow area, 0.56 feet (17cm) = 1.66 feet, are consistent with the heights calculated using the USACE CEM.

## 3.0 SURVEYS

#### 3.1 Topographic, Bathymetric and Magnetometer Surveys

C & C Technologies, Inc. performed the surveys of the borrow area, marsh creation areas and pipeline corridor from August 11, 2011 through October 11, 2011. Elevations were



**Figure 6: Survey Transect locations** 

obtained at twenty five foot (25') intervals (approx.) along the prescribed transects as shown in Figure 8.

The hydrographic and magnetometer survey was conducted utilizing C-Nav differentially corrected GPS, Winfrog navigation software, single beam fathometer and cesium

magnetometer to collect data on designated transects. Top of water elevations were recorded utilizing RTK GPS each day and compared to the local tide gage Bayou Bonfouca, Route 433, LA Station Id: 8761473. The magnetometer was towed at the stern of the vessel. The magnetometer dataset was collected at a sampling rate of 10 Hz and a very high sensitivity of less than 0.1 gammas.

The magnetometer identified 218 magnetic anomalies within the borrow area and did not correlate to any existing known infrastructure. The magnetic anomalies exhibit amplitudes between 5 and 1,806 gammas and durations between 11 and 93 feet. The amplitude and duration of a magnetic anomaly are dependent on several factors such as the ferromagnetic content of the object, its shape, size, and distance from the sensor. Some of the magnetic anomalies are relatively small (less than 10 gammas) and are likely geologic in origin or noise. The locations of all magnetic anomalies should be taken into consideration during the planned dredging-related activities.

Geophysical operations were conducted by the M/V *C-Star* on August 11 and 12, 2011. Geophysical instruments utilized for the shallow hazards survey included an Edgetech SB-424 Chirp Seismic Profiler (4 to 24 kHz), an Odom Hydrotrac Fathometer, and a Geometrics 882 Cesium Magnetometer. Survey vessel positioning was accomplished using Leica Geosystems' SmartNet real-time kinematic (RTK) GPS and provided RTK positions in real time with centimeter accuracy. In addition, C & C Technologies' C-NAV<sup>®</sup> L-Band globally corrected differential GPS (DGPS) was employed as a secondary positioning system and provided DGPS positions in real time with sub-meter accuracy and provided vessel positions if RTK signal was lost.

The subbottom profiler transducer was towed from a davit arm mounted on the bow of the vessel. Chesapeake Technology, Inc.'s SonarWiz.SBP software and Seismic Micro Technology's Kingdom Suite 2d/3d PAK program were used for data collection and interpretation of the subbottom dataset, respectively.

One 16 inch Exxon pipeline was identified in the project area (see Figure 8). The magnetometer records the pipeline as high-amplitude monopole and diopoles.

The uppermost lake floor sediments were recorded by the subbottom profiler as one seismic stratigraphic unit which is continuous throughout the study area. The subbottom profiler records show an area exhibiting chaotic bedding reflections. The acoustically chaotic bedded sediment area is interpreted to consist of reworked coarse-grained sediment (e.g., sandy-silty muds). The limited penetration is potentially due to the chaotic sediments. In addition, the subbottom profiler data is also attenuated by the presence of biogenic gas. Biogenic gas occurs in marsh environments and results from the decay of organic matter. The gas consists of low concentrations of carbon dioxide. The low pressure gas attenuates seismic signals and may reduce the shear strength of the soils where present. Bayou Bonfouca may be providing a source of the organic matter being deposited in the borrow area.

Numerous relic submarine channels were identified throughout the project area. The subbottom profiler records show the relic submarine channels as dipping reflectors with an infill of high amplitude reflections. These high amplitude reflections suggest the channel fill is coarser sediment than the surrounding sediment and underlying channel. The relic submarine channels occur just below the mudline with associated thalwegs depths ranging from 1 to 4 feet below the lakefloor.

#### **3.2 Healthy Marsh Elevation Survey**

Elevations from survey transects that appeared to have healthy marsh were utilized to determine an average elevation of healthy marsh. Additionally, elevations were collected from an area that appeared healthy but lacked transects data. This data was combined with the healthy marsh transects to calculate an average elevation of healthy marsh as shown in Table 7 below. The average of all sites is 0.94 ft which is consistent with healthy marsh elevation of the neighboring Goose Point (PO-33) Project. The location of the healthy marsh elevation survey is shown in the preliminary design drawings in Appendix F.

Location	Avg. Elev. (ft)
Transect 17	1.14
Transect 16	0.90
Transect 24	0.91
General Marsh Survey	0.80
Average	0.94

<b>Table 7: Healthy Marsh Elevations</b>
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#### **3.3 Survey Control Monument**

The secondary monument 876 1534 B TIDAL is located to the east of the project site and was utilized as a control for the project. It is located near Carr Drive at coordinates 30°13'44.3989 N, 089°51'05.24823 W. A static GPS session was performed at the monument site during each day. The information was then downloaded into the NOAA Online Positioning User Service (OPUS) to confirm the elevation of the monument. The data sheet for the monument is located in Appendix A.

### 4.0 GEOTECHNICAL ANALYSIS

In order to determine the suitability and physical characteristics of the soils in the PO-104 project area, a geotechnical investigation and analysis was conducted by GeoEngineers Inc. GeoEngineers Inc. was tasked to collect soil borings, perform laboratory tests to determine soil characteristics, perform stability analysis of proposed containment dikes, calculate the settlement of the proposed containment dikes and marsh creation areas, determine an adequate cut-to-fill ratio for the dredge and fill operations, and to identify positions of the Goose Point Fault.

#### 4.1 Soils Investigation

Soil conditions were evaluated in the marsh creation areas by advancing nine (nine) soil borings to depths ranging from approximately 40 to 60 feet below existing mudline and performing two field vane shear tests at each boring location. Five (5) additional soil borings were advanced to an approximate depth of 20 feet below mudline within the confines of the proposed borrow area. The approximate soil boring locations are shown in Figure 9.

The soil borings were performed in 0.5 to 11.5 feet of water. Samples were collected continuously in the upper 20-feet of the soil and on 5-foot centers thereafter to boring completion depths. The borings were completed between August 30 and September 13, 2011 using pontoon- and marsh buggy-mounted drill rigs. A geologist from GeoEngineers managed the drilling on a full time basis, examined and classified the soils encountered, obtained representative samples, and prepared a log of each borehole. Soil characteristics observed during drilling and laboratory test results are located on the soil boring logs in Appendix D.

After transport to GeoEngineers' soil mechanics laboratory, Shelby tube samples were tested for miniature vane shear strength and removed from their tubes. Laboratory tests included soil compressive strength, moisture content, organic content, grain size anaylysis, specific gravity, consolidation with rebound, and Atterberg limits.



**Figure 7 – Soil Borings Locations** 

#### 4.2 General Geologic Evaluation

Subsurface conditions vary widely across the project area. Generally there is very soft organic clay and peat followed by inorganic clay, silt, or sand. Interface depths vary significantly from boring to boring. In all but two of the borings, medium to stiff clays were encountered within the top 20 feet of the soil profile and continued through the completion depth of the boring, except where interrupted by sand and silt layers. This is consistent with geological maps of the area which place the surface of Pleistocene deposits within 20 feet of the ground surface. Clay samples from soil borings B-2 and B-3 remained very soft through the completion depth of the boring the Pleistocene materials which subsequently filled with softer Holocene material.

#### 4.3 Subsidence and Sea Level Rise

To determine a most likely change in sea level over time, CPRA utilized its Louisiana Applied Coastal Engineering and Science (LACES) Division to assist with calculating this value. LACES attempted to bracket this rate by providing a lower and higher value to account for uncertainty. To calculate subsidence LACES used the ranges of subsidence values shown in Figure 10. The figure was created using some of the lowest and highest subsidence rates found in those areas by researchers. The Bayou Bonfouca

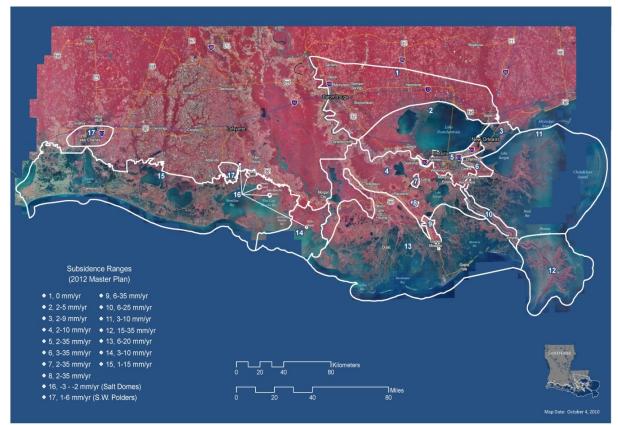


Figure 8: Map of Projected Subsidence Ranges for South Louisiana Generated by the Subsidence Advisory Panel for the Louisiana CPRA Master Plan 2012

area has some of the lowest subsidence rates (2-5mm/yr) seen in coastal Louisiana. The range for possible relative sea-level rise by 2032 calculated by LACES is 0.1304 m – 0.2412 m. This equates to a combined subsidence and sea level rise of approximately 5 to 10 inches over the 20 year design life of this project.

The information provided by LACES represents the relative sea level rise (RSLR) that will occur at a specific location and incorporates both Global Sea Level Rise (GSLR) and subsidence. The RSLR was included in the marsh fill settlement analysis as shown in figure 12. However, studies indicate that historic rates of accretion would likely be sufficient to keep up with the predicted RSLR in the area over the project life (Reed et al. 2009).

#### 4.4 Slope Stability Analysis

Slope stability analyses were performed on proposed earthen containment dikes. The slope stability of any embankment or dike has two types of driving forces: (1) the forces induced by the soil weight, and (2) any seepage forces which tend to cause the soil to slide. In response to these driving forces, the subsurface soils have a resistant force in the form of shear strength, which attempts to keep the slope from sliding. Both the driving forces and the resisting forces are dependent on the geometry of the situation: the "Failure Surface". GeoEngineers, Inc. performed stability analyses that compute factors of safety, against potential failure based on limit equilibrium theory.

As evidenced by the soil properties in the boring logs, the Bayou Bonfouca Marsh Creation Project rests over relatively shallow Pleistocene deposits. The close proximity of low-moisture, low-organic content, and over consolidated soils Pleistocene soils typically present in coastal marsh environments ensures good soil stability and low settlement potential for most of the project area. The profiles from two of the soil borings appear to be exceptions. Soil borings B2 and B3 may be in a historical channel in the Pleistocene deposits, and do not appear to share the design benefits afforded to the other soil boring locations. This is especially noticeable in the settlements estimated for these two borings relative to the estimated settlements at all the other locations.

Table 8 shows the results of slope stability calculations based on proposed construction marsh fill elevations. No geo-textile reinforcement was required to attain the factors of safety.

Soil Boring identification	Proposed Construction Marsh Fill Elevations (ft)	Design Crown Elevation (ft, NAVD 88)	Assumed Foundation Elevation (ft, NAVD 88)	Acceptable Crown Width (ft)	Acceptable Side Slopes	Slope Stability Factor of Safety
B-1	2.5	4.0	0	5	3H:1V	1.84
B-2	3.0	4.0	0	5	3H:1V	1.22
B-3	2.5	5.0	0	5	3H:1V	1.47
B-4	2.5	5.0	-1.5	5	3H:1V	1.13
B-5	2.5	4.0	0	5	3H:1V	2.45
B-6	2.7	4.0	0	5	3H:1V	1.77
B-8	2.7	4.0	-1	1	7H:1V	1.14
B-9	2.7	4.0	-1	5	5H:1V	1.46

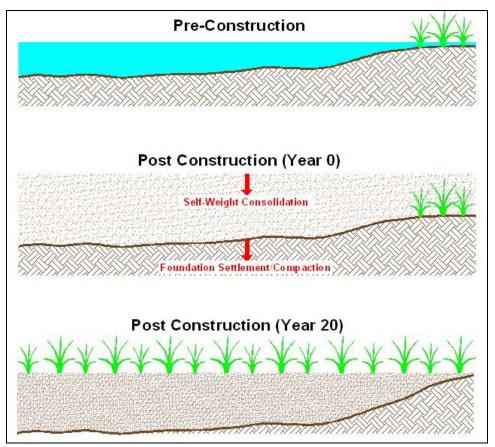
**Table 8: Earthen Containment Dike Slope Stability** 

In order to minimize potential stability problems during construction of the earthen containment dikes, Construction should be performed in two or more lifts to ensure gentle application of pressure on area soils. Stockpiling of fill material in one location to allow it to dewater will not be allowed, as a large pile of soil with steep slopes could result in bearing failure of the foundation soils.

Two duck ponds have been included for Marsh Creation Area 1, at the request of one of the landowners. A fill differential of only 2 feet was used in calculating the stability of the containment dike for these features. Refer to Section 5.3 for the construction process necessary to limit the fill differential.

#### 4.5 Marsh Fill Settlement Analysis

A settlement analysis was performed to determine the construction fill elevation of the marsh creation area and the total volume of material required to fill the area. The final elevation of the marsh fill (at year twenty) is governed by two forms of settlement: (1) the settlement of the underlying soils in the marsh creation areas caused by the loading exerted by the placement of the dredged material, and (2) the self-weight consolidation of the dredged material (See Figure 11). Data from low pressure consolidation tests were used to calculate the time-rate of settlement of the underlying soils of the marsh creation areas. Self-weight consolidation tests were performed on a composite sample from the borrow area material (borings B-10 through B-14) to determine the consolidation of the dredged material.



**Figure 9: Marsh Fill Settlement** 

Settlement curves were developed in 0.5ft increments for proposed construction fill elevations ranging from 2.0 feet to 4.5 feet NAVD 88. These settlement curves show the changes in elevation over the 20 year design life of the project and were used to compare different fill elevations.

The settlement for Boring B-7 which is located on the western portion of Marsh Creation Area 1 is shown in Figure 12. There is very little settlement after 6 months, which is favorable for the project over the 20 year project life. The elevation of each marsh creation area is adjusted to the settlement curves associated with that area.

For Marsh Creation Area 1, the marsh creation area bordering Lake Pontchartrain, a construction fill elevation of 2.7 feet will be sufficient to reach the final target marsh elevation. For Marsh Creation Area 2, a construction fill elevation of +2.5 feet can be used on the east side; however, the western portion of Marsh Creation Area 2 is slightly more complicated. A single lift to a construction fill elevation of +4.0 would be required to reach the final targeted marsh elevation. CPRA tasked GeoEngineers to evaluate a two lift scenario to decrease the volumes necessary for a successful filling of this marsh creation area. Using 2 separate lifts to a construction fill elevation of +2.5, with a minimum retention time of 30 days, would provide the final targeted marsh elevation after settlement. Since this allows for the same containment dike structure as the eastern

side, and a decrease in volume from the single lift system, the two lift scenario will be utilized for Marsh Creation Area 2-West. For Marsh Creation Area 3, a construction marsh fill to elevation +3.0 feet will be sufficient to reach the final target marsh elevation while Marsh Creation Area 4 will require a construction fill elevation of +2.5 feet.

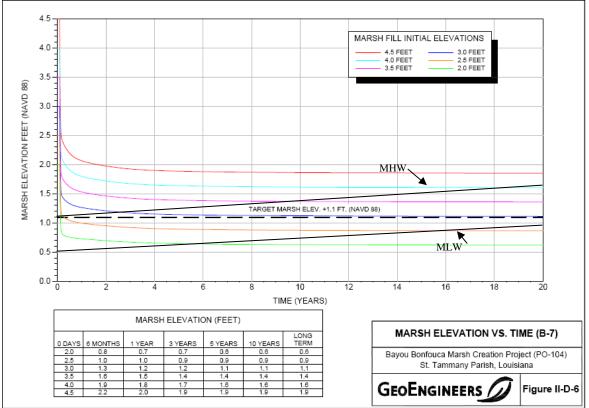


Figure 10: Settlement Curves for Boring 7

#### 4.6 Earthen Containment Dike Settlement Analysis

Settlements of the foundation soils beneath the earthen containment dikes were computed based on the dike geometries determined from the slope stability analyses. Reducing the crown elevation and width will decrease the amount of settlement under the earthen containment dikes. Settlement factors include regional subsidence, self weight consolidation, and elastic settlement of the in situ soils. Self weight consolidation is dependent on several factors, including organic content, natural moisture content, and construction methods. Elastic settlement of the in situ soils will occur quickly and will likely result in an increase in the quantity of fill required to reach the design construction elevation.

Settlement for the containment dikes was performed using an elevation of +4.5 feet NAVD 88. The actual design elevations ranged from +3.2 to +4.0 feet NAVD 88 and are located in figure 10.

#### 4.7 Cut to Fill Ratio Recommendations

A cut to fill ratio was determined in order to account for losses due to dredging, containment, and dewatering. A cut to fill ratio was determined using the settling column test and the method outlined in the United States Army Corps of Engineers' EM-1110-2-5027. However, this method provided a fill to cut ratio of approximately 2.2. Since this project is expected to be consistent with previous marsh creation projects having similar soil types which have yielded cut-to-fill ratio of 1.3, a cut to fill ratio of 1.3 will be applied for all marsh fill sediment. Similarly, previous projects have experienced a cut-to fill ratio of 1.5 to 2.0 on mechanical dredging of containment dikes. The lower ratio of 1.5 will be used for mechanical dredging since that is approximately what was seen on the PO-33 Goose Point Project.

#### 4.8 Goose Point Fault

The borrow area for the PO-104 project lies just north of the Goose Point Fault, which is an extension of the Baton Rouge-Denham Springs Fault system that lies near the north shore of Lake Pontchartrain. The approximate location of the Goose Point fault in relation to the project area is shown in Figure 9. The borrow area does not appear to cross the fault line.

## 5.0 MARSH CREATION DESIGN

The marsh creation design was broken into four (4) components: the marsh creation areas, the dredge borrow area, duck ponds, and the containment dikes. The design of each component is discussed in the sections below.

#### 5.1 Marsh Creation Area Design

The main design component of PO-104 involves the calculation of the marsh creation area volumes. Before this could be accomplished, a construction fill elevation had to be determined. This elevation was governed by several factors including the final target marsh elevation considering healthy marsh elevations obtained, the tidal datum, the physical properties of the borrow material, and the bearing capacity of the foundation soils in each marsh creation area.

Determination of the construction fill elevation involved an examination of the existing marsh conditions. The marsh elevation survey revealed that the average healthy marsh elevation throughout the entire project area is approximately +1.0 ft. NAVD 88 (Section 3.2). The calculated tidal datum (MHW=1.1 ft., MLW=0.5 ft.) verifies that the existing marsh predominantly falls in the upper portion of the project inter-tidal zone, defined as the range of elevations that lie in between the upper and lower extents of the tidal datum.

In order to evaluate the performance of the created marsh over the 20 year project design life the project team decided that the final target marsh elevation would be the criteria to judge the success of the this project. Ideally, biologists from both USFWS and CPRA

would like the created marsh to be as close as possible to the existing marsh conditions and within the inter-tidal zone. This means that the final target marsh elevation (after initial consolidation and long term settlement) would fall within the upper range of the MHW and MLW. To achieve this, the marsh platform will initially have to be pumped to an elevation higher than MHW during construction and settle into the inter-tidal zone over the 20 year design life of the project.

After determining the construction fill elevations, the total volume of each marsh creation area is calculated by using AutoCAD Civil software. The software creates a 3-Dimensional surface based on XYZ coordinate data from the survey cross sections. This surface is known as a Triangulated Irregular Network (TIN). The TIN model represents a surface as a set of contiguous, non-overlapping triangles. Both a TIN surface containing the 2011 survey data from C&C Technologies and a flat TIN surface at the fill construction elevation was generated by AutoCAD. AutoCAD then uses the XYZ differences of each surface to calculate the volume of each marsh creation area. Since the containment borrow must be refilled, the volume required to build containment dikes is then added to the volume required to fill the marsh creation areas. The cut-to-fill ratio of 1.3 is then applied, resulting in a final estimate of volumes for each marsh creation area. Table 9 summarizes the fill volumes for each marsh creation area within the PO-104 project.

Marsh Creation Area	Fill Height (ft)	Area (Acres)	Volume of Fill (yd <sup>3</sup> )
1	2.7	331.6	2,260,636
2 West	2.5	63.4	590,668
2 East	2.5	97.2	647,509
3	3.0	31.6	220,643
4	2.5	115.9	605,115

 Table 9: Summary of Fill Acreage and volume

#### 5.2 Earthen Containment Dike Design

The primary design parameters associated with the earthen containment dike design include crown elevation, crown width, and side slopes. One foot of freeboard will be used to contain the dredge slurry within the marsh creation areas. Therefore, the earthen containment dikes will be constructed to an elevation between 3.5 and 4.0 ft NAVD88.

The width of the crown of the earthen containment dikes provide a minimum factor of safety of 1.2 in regards to bearing capacity. All outer earthen containment dikes will be constructed using crown width of 5 feet.

Marsh Creation Area	Design Height (ft)	Side Slopes	Crown Width (ft)
1	3.7	3H:1V*	5
2 West	3.5	3H : 1V	5
2 East	3.5	3H : 1V	5
3	4	3H : 1V	5
4	3.5	3H : 1V	5
Duck Ponds	3.2	2H:1V	2

#### Table 10: Summary of Earthen Containment Dike Design

\*Back segment requires 6H: 1V (refer to Preliminary Design Drawings)

A side slope of 3 feet horizontal for every foot of vertical rise (3H:1V) was utilized for containment of most of the marsh creation areas. One exception is on the eastern side of Marsh Creation Area 1 which will require side slopes of 6H: 1V. Earthen containment dikes for the proposed duck ponds will have 0.5 feet of freeboard, 2 foot crest width and a 2H: 1V side slope.

The earthen containment dikes for all marsh creation areas shall be constructed using insitu material from inside each marsh creation area. For the purposes of slope stability the dike borrow pits will be located at a minimum of 25 feet from the toe of the dike. Furthermore, 3:1 side slopes will be used in the earthen containment dike borrow area with a 10 foot maximum cut.

#### 5.3 Duck Ponds

A stipulation from a landowner for part of Marsh Creation Area 1 was that two duck ponds be provided in the marsh creation area. Two duck ponds, each 5 acres in size, will be created in the western portion of the marsh creation area. In order to accomplish this, the earthen containment dikes will be constructed with two (2) small openings opposite each other. Dredged settlement will be placed into Marsh Creation Area 1 allowing the slurry to fill both inside and outside of the duck ponds. After an elevation +1.5 feet is reached, the gaps in the earthen containment dike will be closed and the remaining portion of Marsh Creation Area 1 will be filled to the construction marsh fill elevation of +2.7 feet. Earthen containment dikes for the duck ponds will only have six (6) inches of freeboard since some overtopping into the duck ponds is permissible.

# 6.0 BORROW AREA DESIGN

According to the Lake Pontchartrain Basin Foundation (LPBF), grass beds in Lake Pontchartrain constitute the lake's most productive underwater habitat. They provide critical food and shelter for juvenile fish and shellfish, and are responsible for about 25 percent of the Lake's fishing industry (LPBF, 2012). In order to protect these critical grass beds, the borrow area was located near the -10 foot contour.

Gulf Sturgeon, a fish listed as threatened under the Endangered Species Act in 1991, are known to exist in this area. The USFWS has identified sand on the surface of Lake Pontchartrain as a beneficial area for Gulf Sturgeon and therefore the borrow area was located to avoid such areas.

In and effort to minimize redundant geotechnical work and cost, a preliminary qualitative sediment review of the lake bed was performed by the CPRA. In the preliminary sediment survey, core samples were collected and classified in several locations where the lake bottom was near the -10 foot contour. The samples were collected using a PVC coring device designed and constructed to sample the top 5 to 10 cm of the lake bottom. The locations of the samples are shown in Figure 12.



Figure 11: Locations of Boring and Lake Bottom Grab Samples

All of the samples can be characterized as primarily "mud" consisting mostly of clay, silts and organic matter. Some sand was present but no attempt was made to determine the ratio of mud to sand in the samples. The sand size component of the samples was evaluated using U.S. Standard Sieve Series #50 and #200, The #50 sieve will not allow particles larger than 0.297 mm to pass through the sieve and the #200 will not allow

particles larger than 0.074 mm to pass the sieve. These two particle size limits correspond in size terms, using the Wentworth grain size classification nomenclature, to "very fine sand" at between 0.0625 mm - 0.125 mm for the #200 sieve and "medium sand" at between 0.25 mm - 0.5 mm for the #50 sieve. If both sieves captured sand then the sample would be termed a "very fine to medium sandy mud". If no sand was recovered in the #50 sieve but was recovered in the #200 sieve, then the sample would be termed a "very fine to medium sandy mud". If no sand was recovered in the #50 sieve but was recovered in the #200 sieve, then the sample would be termed a "very fine to fine sandy mud". Again, using the Wentworth scale, it was assumed that "fine sand", 0.125 mm - 0.25 mm, was captured in the #200 sieve, along with the "very fine sand". The determination of color can be subjective (color charts were not used) but is a basic part of any lithologic classification. Samples that smelled of hydrogen sulfide or contained what appeared to be, fine organic material or both, were termed "organic" mud. The term "slightly" implies the component is a small part of the sample. Bivalve shells, Rangia, and small shell fragments were a relatively common but small component of the samples.

This data was compared with boring data and it was determined that only one boring, boring B-12, contained sand on the lake bottom. More work may be required to delineate the limits of the sand near boring B-12 during the next phase of design.

Table	211			
Desc	Description of Sediment Samples from Site Visit to Lake Pontchartrain, 5/18/11.			
#01	grey, slightly shelly, very fine sandy mud			
#02	grey, very fine to medium sandy mud			
#03	dark grey, slightly very fine sandy, organic mud			
#04	grey, very fine to medium sandy mud (w/ bivalves)			
#05	dark grey and brown, very fine to fine sandy organic mud			
#06	dark grey, very fine to medium sandy, organic mud			
#07	dark grey and brown, very fine to medium sandy, organic mud (w/ included bivalves)			
#08	grey, very fine to medium mud ( w/ bivalves)			
#09	grey, very fine to medium sandy mud			
#10	dark grey to black, slightly very fine sandy, organic mud			
#11	dark grey and brown, very fine to medium sandy, organic mud			
#12	grey, very fine to medium sandy, organic mud			

Table 11

The proposed borrow area is approximately 331 acres with a maximum allowable cut depth to elevation -25 feet. Maintaining side slopes of 3H: 1V will provide approximately 7,752,600 cubic yards of available borrow volume.

# 7.0 ARCHEOLOGICAL REVIEW

According to information received from the Louisiana State Historic Preservation Office (SHPO), at least three archeological sites are known to exist near the PO-104 project area. The sites are located on the lake ridge and along the banks of Bayous Bonfouca and Liberty. These are predominately shell midden sites with a light to moderate artifact

density, at best. Additional information regarding the archeological sites can be found in appendix G. A formal consultation will be requested of SHPO prior to the completion of 95% design.

## 8.0 CONSTRUCTION

#### 8.1 Duration

Due to Gulf Sturgeon habitat issues, the Goose Point Project, (PO-33), was given an initial dredging window from May to September which was later modified to include October. An approximate construction duration was developed using the CDS Dredge Production and Cost Estimation Software. A 25% downtime rate was used for general maintenance and weather, since the dredge window runs through hurricane season. Assuming earthen containment dikes would be built prior to the start of the dredge window and using a 30 inch dredge the dredge time would be approximately 4 months as compared 6.5 months with a 24 inch dredge.

#### 8.2 Cost Estimate

A cost estimate for the PO-104 project was prepared using the CWPPRA PPL 21 spread sheet. Adjustments will be made prior to completion of 95% design that will represent the most up to date quantities cost.

Bayou Bonfouca Marsh Creation	2/6/2012	Date	Revised:	14-Mar-12
Work or Material	Quantity	Unit	Unit Cost	Amount
Mobilization/Demobilization	1	LS	\$2,042,900	\$2,042,900
Marsh Creation	4,324,571	CY	\$3.75	\$16,217,141
Earthen Containment Dikes	175,941	CY	\$3.25	\$571,808
Settlement Plates	8	EA	\$3,500	\$28,000
Gapping Containment Dikes	3,014	CY	\$3.25	\$9,796
Construction Surveys	1	LS	\$394,373.99	\$394,374

#### Table 12: Preliminary Construction

ESTIMATED CONSTRUCTION COST ESTIMATED CONSTRUCTION COST + 25%

\$19,264,019 \$24,080,024

# 9.0 MODIFICATIONS TO APPROVED PHASE 0 PROJECT

As a result of Phase 1 activities, a few changes have been made to the approved Phase 0 project. During further consultation with the USFWS staff at Big Branch Marsh National Wildlife Refuge, the design team was informed of a new breach along the rim of Lake Pontchartrain, west of the Phase 0 project footprint. The PO-104 project team recognized that adjustments should be made to protect the lake rim. Therefore, a new marsh creation

area, Marsh Creation Area 4, was added and a comparable area was removed from the eastern side of Marsh Creation Area 1. The modified footprint is now 639 acres as compared to 591 acres proposed in Phase 0. The changes to the size and placement of the marsh creation areas will increase the overall size of the project by 48 acres.

During consultation with the landowners of Marsh Creation Area 1, one of the land owners was concerned about turning all open water into land because he leases the land to duck hunters. Therefore two shallow duck ponds were included in Marsh Creation Area 1 that will maintain a hydraulic connection with Bayou Bonfouca.

The borrow area was moved from its Phase 0 location to the current location farther into the lake to eliminate damage to grass beds on the lake bottom.

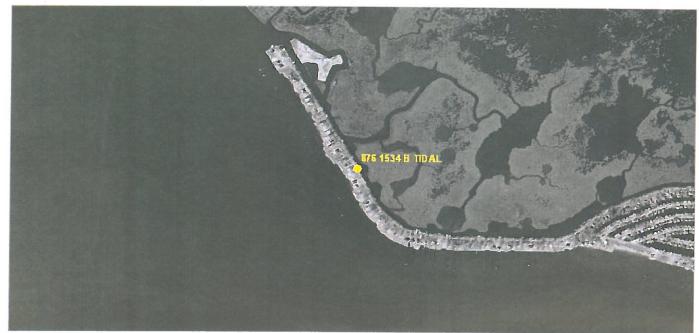
#### REFERENCES

- GeoEngineers, Inc. (Geotechnical Engineering Report for Bayou Bonfouca Marsh Creation Project (PO-104). Baton Rouge, LA. March 2012
- C&C Technologies Survey Services (C&C). Survey Report for OCPR Bayou Bonfouca Marsh Creation (PO-104), Lafayette, LA. November 2011
- Reed, D.J.; Commagere, A., and Hester, M., 2009. Marsh Elevation Response to Hurricanes Katrina and Rita and the Effect of Altered Nutrient Regimes. *Journal* of Coastal Research, SI(54), 166-173. West Palm Beach (Florida) ISSN 0749-0208
- United States Army Corps of Engineers, EM 1110-2-5027. Confined Disposal of Dredged Material. Washington, D.C. 1987
- Lake Pontchartrain Basin Foundation (LPBF), *Environmental Changes around the Basin*, Retrieved April 4, 2012, from {HYPERLINK "http://www.saveourlake.org/basinissues.php"}

Louisiana's Coastal Master Plan 2012, March 2012

DeMarco, K. E., J. Mouton., J. W. Pahl. (January 2012 Version). Recommendations for Anticipating Sea-level Rise impacts on Louisiana Coastal Resources on Project Planning and Design: Technical Report {HYPERLINK ""http://www.lacpra.org/assets/docs/LACES/LACEStech02\_06\_.pdf"}

# **Appendix A:** Secondary Monument Data Sheet



#### VICINITY MAP Not to Scale

Reproduced from Louisiana 2005 DOQQ

#### Station Name: "876 1534 B TIDAL"

Location: Described by National Ocean and Atmospheric Administration 1982, to reach the site proceed west on Carr Drive approximately 2.4 mi (3.9 km) from its intersection with US Highway 11 on the north shore of Lake Ponchartrain to the private residence of Ms. Lucille R. Mocklin, 387 Carr Drive. The station is on the private dock at the rear of the residence. The benchmark is located 261 ft (79.6 m) ease-northeast of the north shore of Lake Ponchartrain, 31 ft (9.4 m) north-northwest of the centerline of a boat ramp north of Carr Drive, 19 ft (5.8 m) east-northeast of the centerline of Carr Drive, 15 ft (4.6 m) west-southwest of the high water mark on the southwest shore of Facine Canal.

Monument Description: Tidal Station Disk on top of stainless steel rod driven 65.6 feet to refusal encased in a PVC pipe.

Stamping: 1534 B 1982

Installation Date: 1982 Date of Survey: August 2007

Monument Established By: NOAA

For: NOAA

Adjusted NAD83 Geodetic Position (NSRS2007) Lat. 30°13'44.39891" N Long. 089°51'05.24823" W

Adjusted NAD83 Datum LSZ (1702) Ft (NSRS2007) N= 631,798.38 E= 3,748,804.16

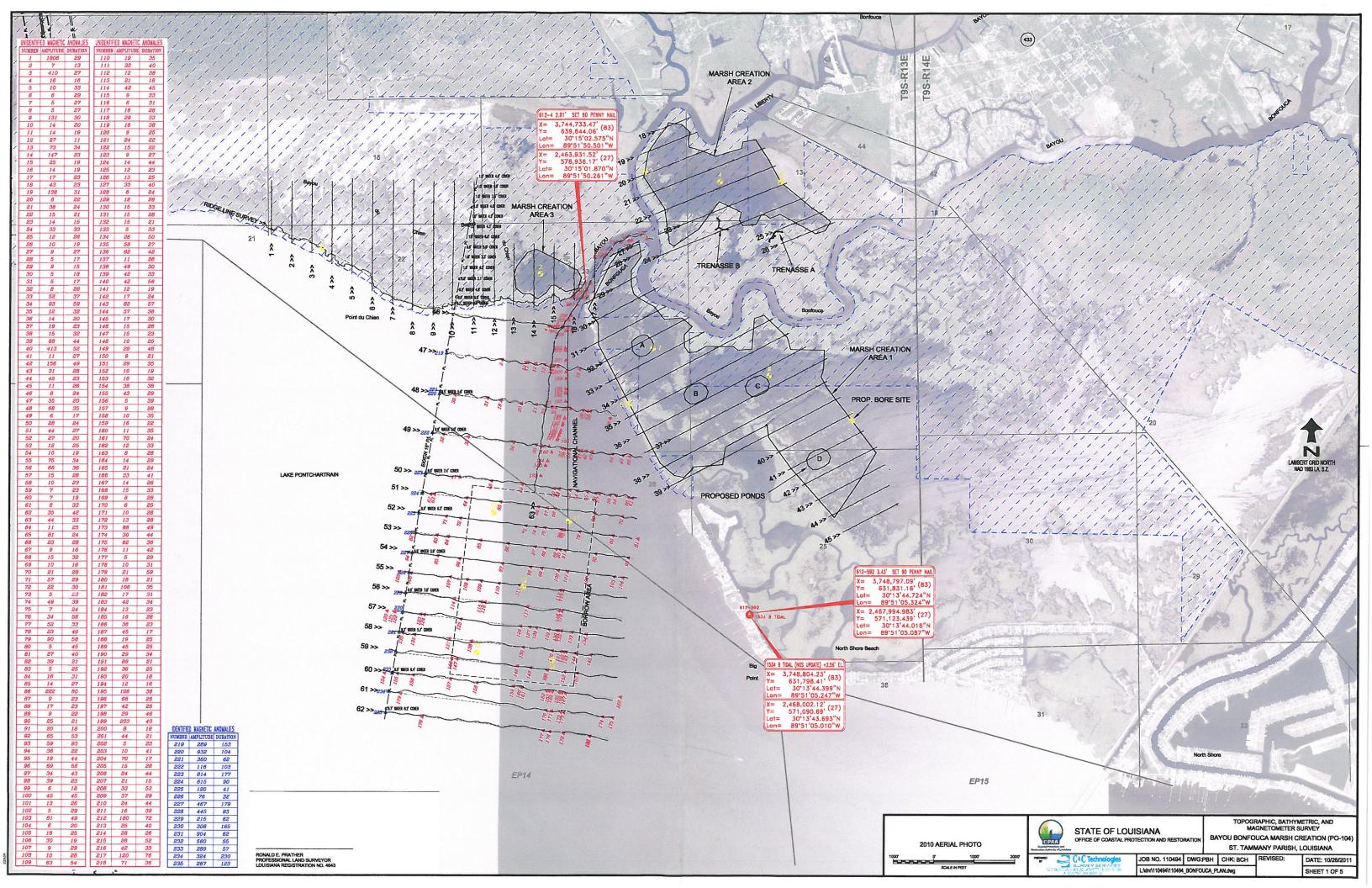
Adjusted NAVD88 Height (2006.81) Elevation = 2.56 feet (0.780 mtrs)

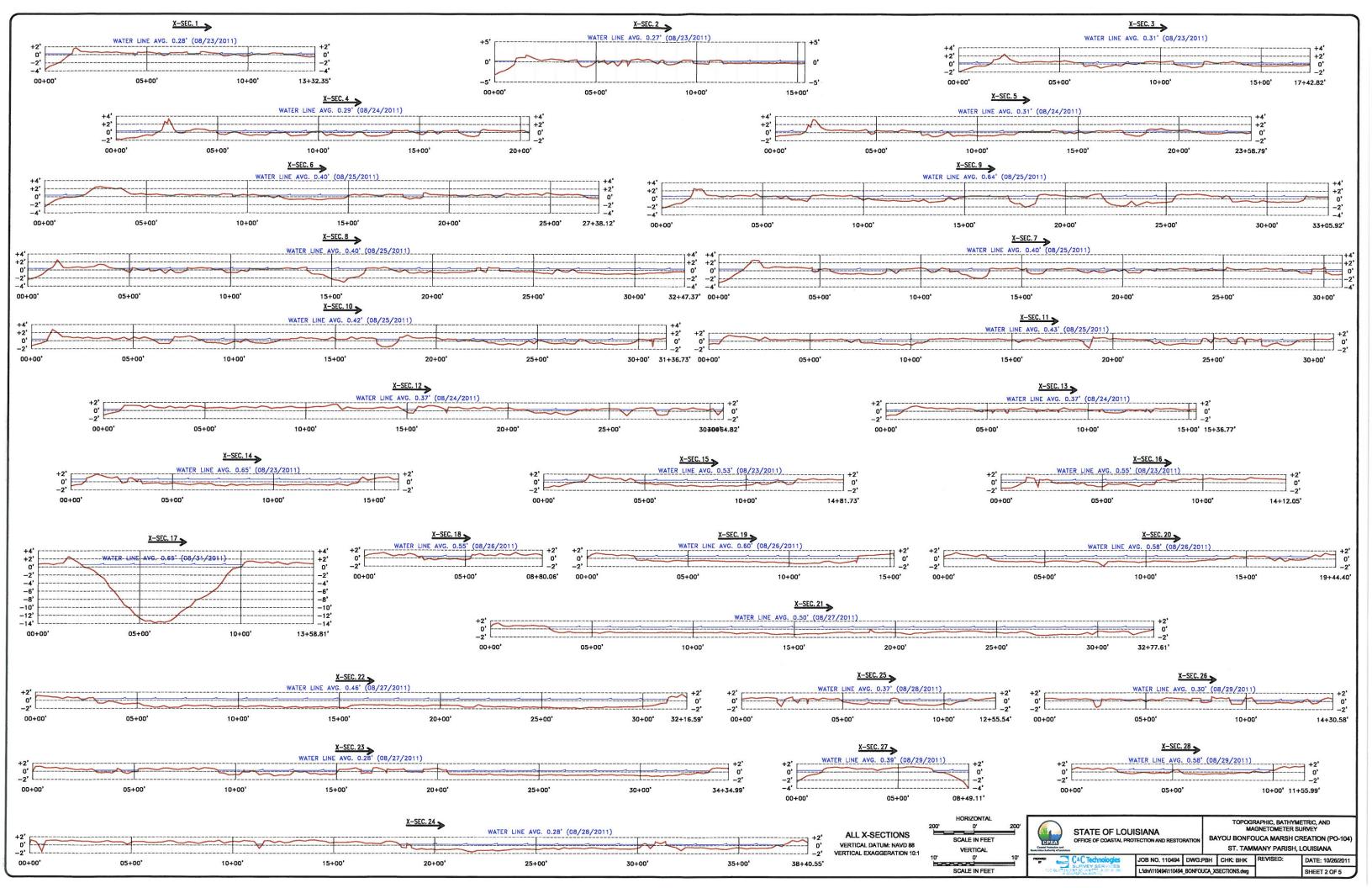
Ellipsoid Height = -25.879 mtrs. Geoid03 Height = -26.659 mtrs. (2004.65)

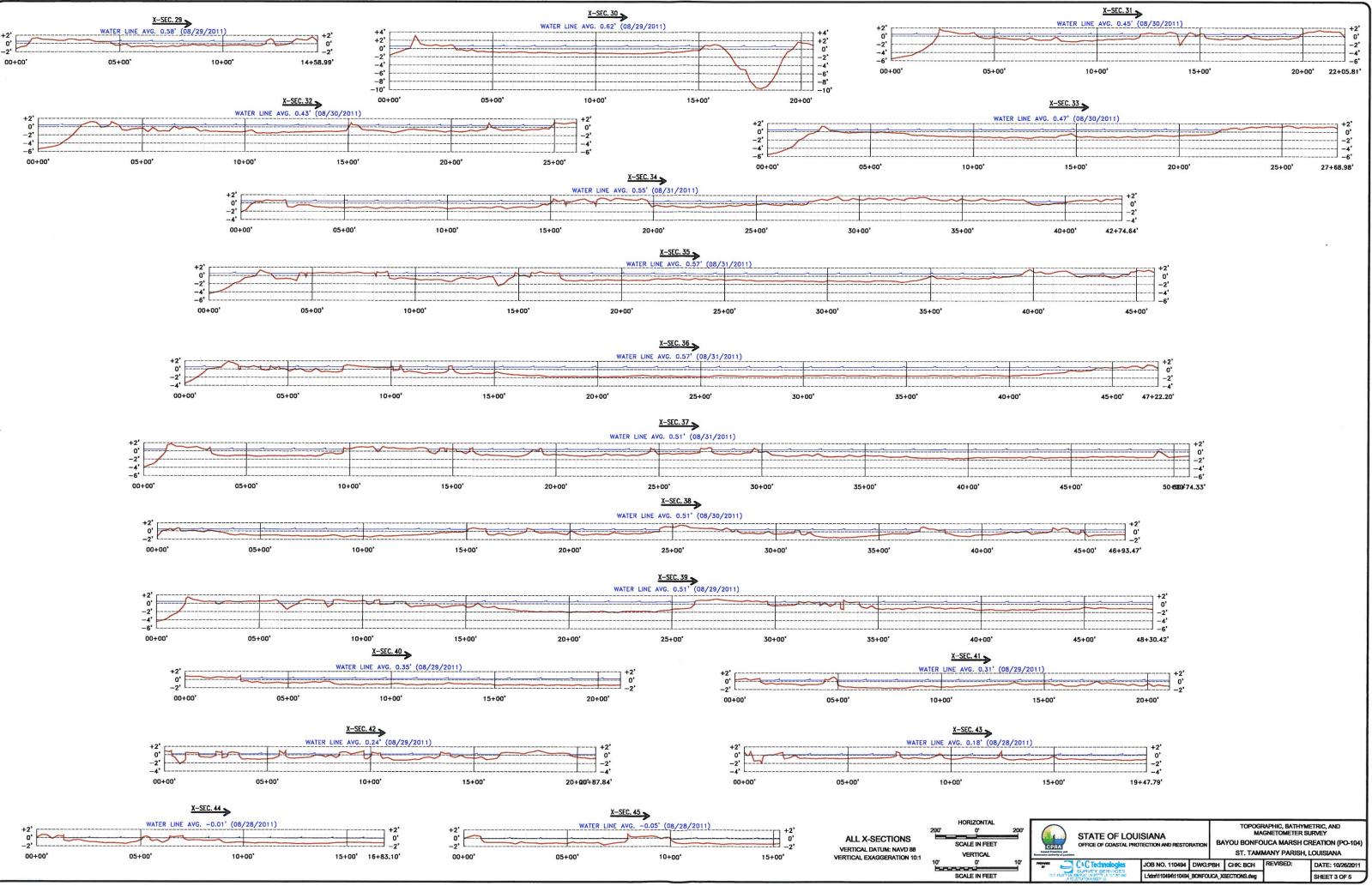


Adjusted Position Established John Chance Land Surveys, Inc. for the Coastal Protection & Restoration Authority of Louisiana, OCPR

# Appendix B: C&C Technologies, Inc. Survey Drawings







# Appendix C: Coast & Harbor Engineering, Inc. SWAN Model Technical Letter



January 31, 2012

Joseph Guillory 450 Laurel Street, 11<sup>th</sup> Floor Baton Rouge, LA 70804

Dear Mr. Guillory:

This technical letter summarizes the results of the work conducted by Coast & Harbor Engineering, Inc. (CHE) under Task 1 of the Scope of Work in accordance with the Coastal Protection and Restoration Authority of Louisiana (CPRA) Contract No. 2503-12-16 for the Bayou Bonfuca Marsh Creation Project (PO-104) – Numerical Wave Modeling Assistance.

#### Background

The project involves dredging of a borrow area approximately 3,000 feet offshore from the existing shoreline and marsh creation site at Bayou Bonfuca. A borrow area impact analysis was conducted using the numerical wave model SWAN to assess the impact of the borrow area on local wave energies impacting the existing shoreline. This technical letter describes the results of the wave modeling conducted for this project.

#### Input Conditions

The input conditions for the numerical wave model were developed in coordination with CPRA for a total of 8 different scenarios, which are summarized in Table 1. The conditions consisted of the existing and dredged bathymetry scenarios, two water levels – mean higher high water (MHHW) and mean lower low water (MLLW), and the 50% wind speed for two directions - 170° and 260° TN (from nearby wind gages).

Wind Direction [TN]	Wind Speed [mph]	Water Level	<b>Bathymetry Condition</b>
260°	8.06	MHHW	Existing
260°	8.06	MHHW	Dredged
260°	8.06	MLLW	Existing
260°	8.06	MLLW	Dredged
170°	9.21	MHHW	Existing
170°	9.21	MHHW	Dredged
170°	9.21	MLLW	Existing
170°	9.21	MLLW	Dredged

Table 1. Modeling scenarios evaluated using the numerical wave model

#### Numerical Wave Modeling

The borrow area impact analysis was conducted using the spectral, two-dimensional wave generation and transformation model SWAN (Simulating WAves Nearshore). Bathymetry and wind grids were used to generate and transform waves into the nearshore environment. The SWAN modeling results for the existing and dredged bathymetry conditions run at MLLW for 8.06 mph winds from 260°TN are shown in Figure 1a and b respectively. The

PACIFIC NW OFFICE 110 MAIN STREET, SUITE 103 EDMONDS, WA 98020 GULF COAST OFFICE 3410 FAR WEST BLVD, SUITE 210 AUSTIN, TX 78731 CALIFORNIA OFFICE 155 MONTGOMERY ST, SUITE 301 SAN FRANCISCO, CA 94104 Mr. Joe Guillory January 31, 2012 Page 2

wave modeling results show that the creation of dredged pit increases wave heights less than approximately 1 cm over the borrow area. The direction of waves around the edges of the borrow area are slightly changed due to wave refraction, but the magnitude of change is small.

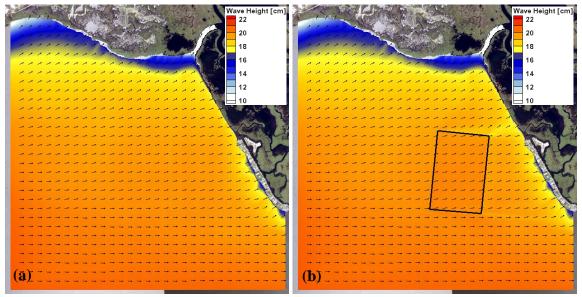


Figure 1. Wave heights computed by SWAN for runs at MLLW and 8.06 mph winds from 260°TN for (a) existing and (b) dredged bathymetry conditions.

Wave heights were extracted at 3 points from the SWAN results to quantify the change in magnitude of the wave heights near the shoreline. Figure 2a and b shows the choice of extraction points for the runs with winds from 260°TN and 170°TN respectively. Table 2 summarizes the measured wave heights at the extraction points for all the SWAN runs. Wave modeling indicated that for any modeling scenario, the change in the wave height due to the creation of the borrow area was 1 cm or less.

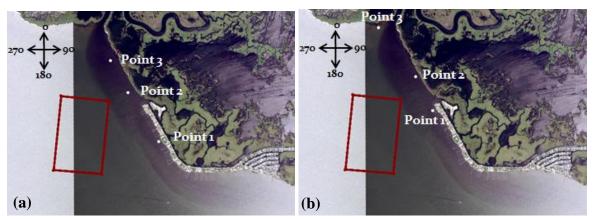


Figure 2. Extraction points used for comparison of nearshore wave heights for cases with winds from (a) 260°TN and (b) 170°TN

Mr. Joe Guillory January 31, 2012 Page 3

Wind	Water Level	Bathymetry		Wave Height [cm]	
wind	Waler Lever	Condition	Point 1	Point 2	Point 3
	MHHW	Existing	18	18	18
8.06 mph from		Dredged	18	17	18
260°TN	MLLW	Existing	16	18	18
		Dredged	16	17	18
	MHHW	Existing	14	15	16
9.21 mph from		Dredged	14	15	16
170°TN	MLLW	Existing	13	15	15
		Dredged	14	14	16

#### Conclusions

The borrow area did not increase wave energy in any appreciable amounts at the existing shoreline. The maximum increase in wave height occurred at MLLW for both wind directions, which resulted in only 1 cm in wave height increase (~2% increase). The magnitude of change in wave heights computed by SWAN is determined to be insignificant due to the relative accuracy of bathymetry measuring instruments (+/- 0.5 feet) and within the natural variability of waves at the project site. Overall the change in wave heights due to the dredged pit are so small in magnitude that it is not expected to cause any significant shift in existing morphological conditions at site.

Thank you for the opportunity to provide this analysis. If you have questions or need more information, please contact me at <u>matt@coastharboreng.com</u> or by phone at 512.342.9716.

Sincerely,

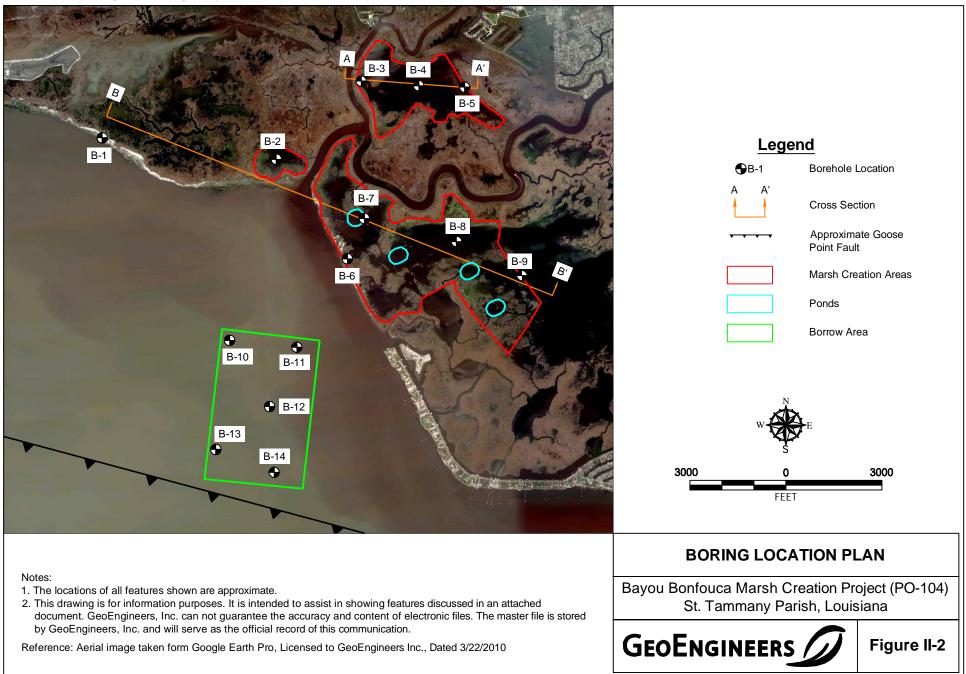
COAST & HARBOR ENGINEERING, INC.

Matt Campbell, P.E. Coastal Engineer Bayou Bonfouca Marsh Creation (PO-104) Preliminary Design Report

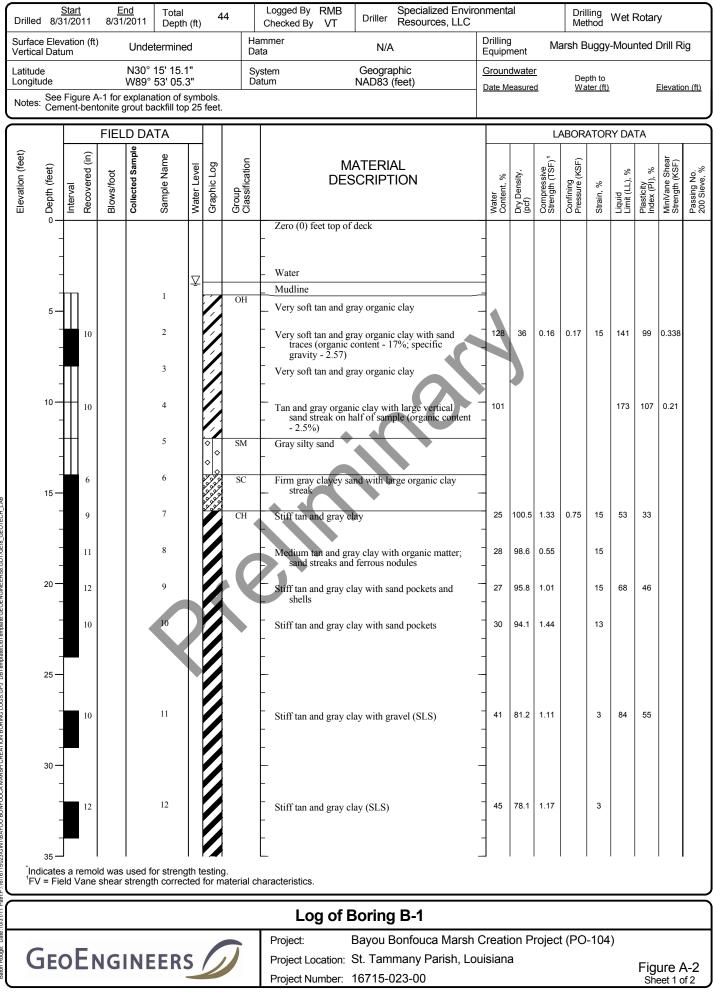
# **Appendix D: GeoEngineers Boring Logs**

P:\16\16715023\00\CAD\Boring Location Plan.dwg\TAB:Layout1 modified on Dec 22, 2011 - 7:57am

JMP: KMC



	SO	IL CLASSIF		CHART		ADDITI		MATERIAL SYMBOLS
м	AJOR DIVIS	IONS				SYME GRAPH		TYPICAL DESCRIPTIONS
					RIPTIONS RAVELS. GRAVEL -		LEITER	DESCRIPTIONS
	GRAVEL AND	CLEAN GRAVELS	GV				CC	Cement Concrete
	GRAVELLY SOILS	(LITTLE OR NO FINES)	GI	GRAVEL - SAND I			AC	Asphalt Concrete
COARSE GRAINED SOILS	MORE THAN 50% OF COARSE FRACTION RETAINED ON NO.	GRAVELS WITH FINES	GI	SILTY GRAVELS, SILT MIXTURES	GRAVEL - SAND -		CR	Crushed Rock/ Quarry Spalls
	4 SIEVE	(APPRECIABLE AMOUNT OF FINES)	G	CLAYEY GRAVEL CLAY MIXTURES	S, GRAVEL - SAND -		тѕ	Topsoil/ Forest Duff/Sod
MORE THAN 50% RETAINED ON NO.	SAND	CLEAN SANDS	SV	V WELL-GRADED S SANDS	ANDS, GRAVELLY			
200 SIEVE	AND SANDY SOILS		SF	POORLY-GRADEI GRAVELLY SAND	D SANDS,		Measure noted on	d groundwater level at time log
	MORE THAN 50% OF COARSE FRACTION PASSING NO. 4	SANDS WITH FINES		SILTY SANDS, SA MIXTURES	ND - SILT			undwater level observed at cploration
	SIEVE	(APPRECIABLE AMOUNT OF FINES)		MIXTORED				water observed at time of
			М	PLASTICITY	ITH SLIGHT			
FINE GRAINED	SILTS AND CLAYS	LIQUID LIMIT LESS THAN 50	СІ	INORGANIC CLAY MEDIUM PLASTIC CLAYS, SANDY C LEAN CLAYS				Log Contact
SOILS			OI	ORGANIC SILTS / SILTY CLAYS OF	AND ORGANIC LOW PLASTICITY		geologic	
MORE THAN 50% PASSING NO. 200 SIEVE			MI	H INORGANIC SILTS	S, MICACEOUS OR SILTY SOILS			nate location of soil strata /ithin a geologic soil unit
	SILTS AND CLAYS	LIQUID LIMIT GREATER THAN 50	CH	INORGANIC CLAY	/S OF HIGH			scription Contact
			OF	H ORGANIC CLAYS MEDIUM TO HIGH	AND SILTS OF PLASTICITY		geologic	
HI	GHLY ORGANIC S	SOILS	P	PEAT, HUMUS, S HIGH ORGANIC C	WAMP SOILS WITH CONTENTS	(	change w	rithin a geologic soil unit
IOTE: Multiple		r Symbol D						
	She	ndard Penetra elby tube ton ect-Push lk or grab	tion Test (SP	Γ)		%F AL CA CP CS DS HA MC MD	Percent f Atterberg Chemica Laborato Consolid Direct sh Hydrome Moisture	I limits I analysis ry compaction test ation test ear ter analysis content content and dry density
of blo dista and o	ows required nce noted). Irop. ' indicates sa	orded for drive I to advance sa See exploratio ampler pusheo	ampler 12 incl on log for ham	nes (or mer weight		PM PP SA UU UC	Permeab Pocket p Sieve ana Triaxial c	ility or hydraulic conductivit enetrometer alysis ompression ed compression
		oply only at the sp		n locations and at				subsurface conditions. re not warranted to be
Description		ce conditions at o						
Description		ce conditions at o	KEY	TO EXPLO	RATION LO	GS		



on Rouge: Date:10/31/11 Path:P:/16/16715023/GINT/BAYOU BONFOUCA MARSH CREATION BORING LOGS.GPJ DBTemplate/LibTemplat

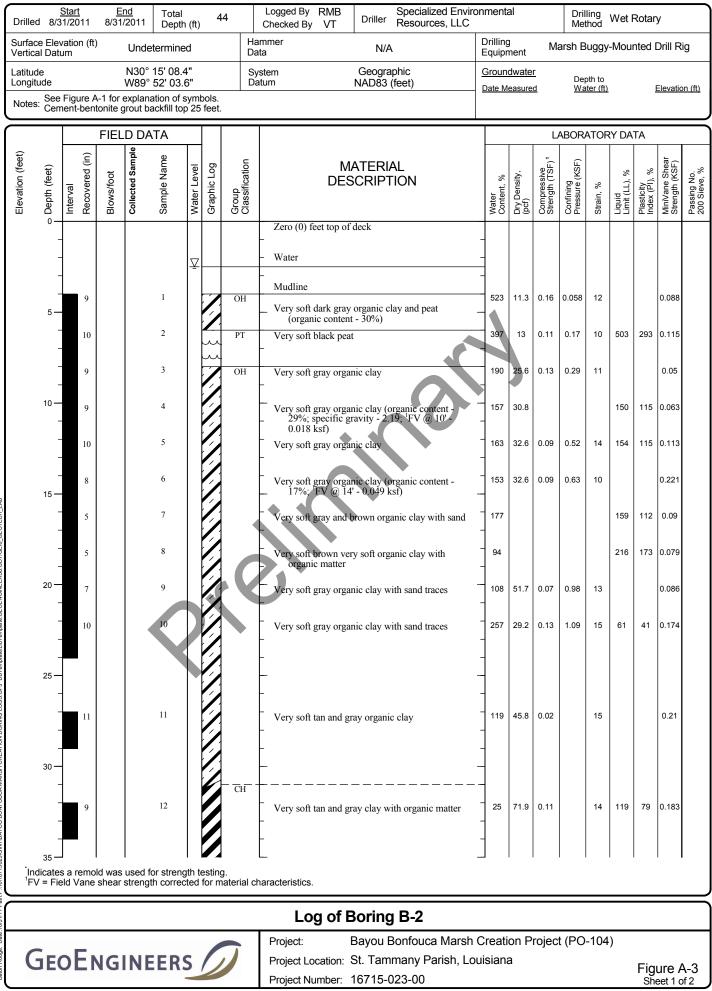
33       6       13       5       Stiff tan and gray clay       46       82       50         40       14       <	(feet) ∍t)		-	LD D	ATA								ABOR/	ATOR	ry da'	TA		
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14       14 <td< td=""><td>-</td><td></td><td>6</td><td></td><td>13</td><td></td><td></td><td>-</td><td>Stiff tan and gray clay</td><td>46</td><td></td><td></td><td></td><td></td><td>82</td><td>50</td><td></td><td></td></td<>	-		6		13			-	Stiff tan and gray clay	46					82	50		
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Bonfouca Marsh Creation Project (PO-104) RS Project Location: St. Tammany Parish, Louisiana

Project Number: 16715-023-00

Figure A-2 Sheet 2 of 2



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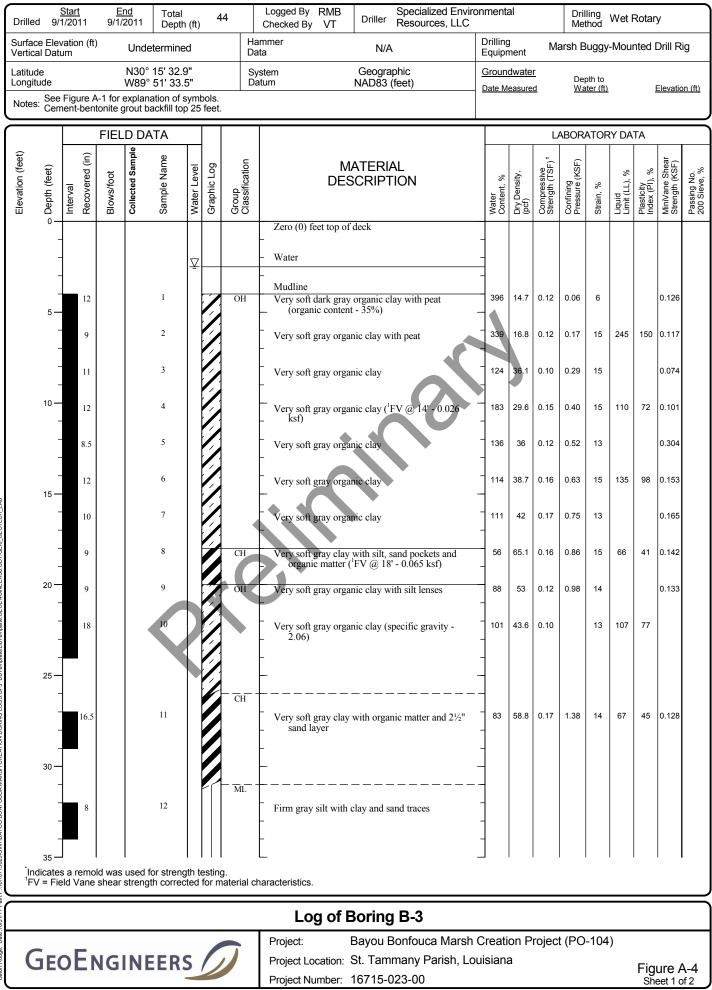
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	Elevation (feet)	ଝ Depth (feet) I	Interval	Recovered (in)	Blows/foot	Collected Sample	Sample Name	Water Level	Graphic Log	Group Classification	MATERIAL DESCRIPTION	Water Content, %	Dry Density, (pcf)	Compressive Strength (TSF) <sup>1</sup>	Confining Pressure (KSF)	Strain, %	Liquid Limit (LL), %	Plasticity Index (PI), %	MiniVane Shear Strength (KSF)	Passing No. 200 Sieve, %
		-		10			13				Very soft tan and gray clay with organic matter	42	72.6	0.06		15			0.223	
		40		8			14			CL	Very soft gray very silty clay with organic matter	24	100.7	0.13		15	29	15	0.295	
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11 Path:P:\16	F\	v = ⊢I€		ane	snear	sireng	ui corre	cied	ior m											—
ate:10/31/										L	og of Boring B-2 (continued)					101	<b>`</b>			

Bayou Bonfouca Marsh Creation Project (PO-104) Project: Project Location: St. Tammany Parish, Louisiana Project Number: 16715-023-00

aton Rouge: Date

GEOENGINEERS

Figure A-3 Sheet 2 of 2

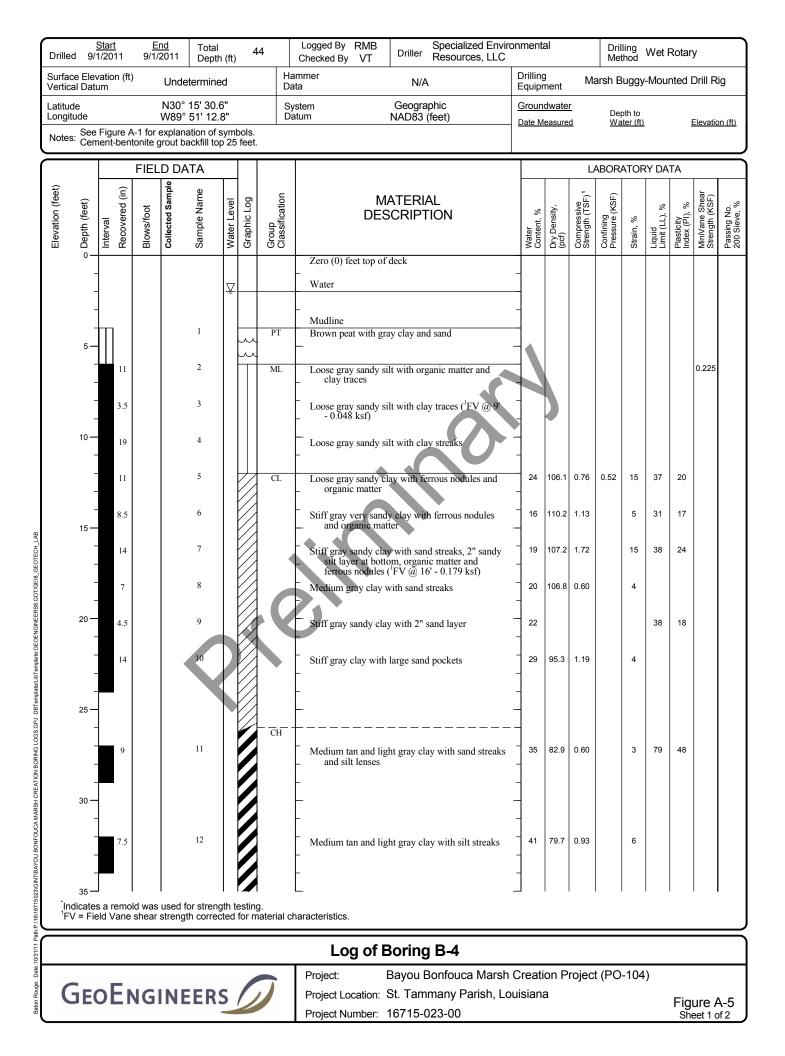


				FIEL	D DATA	4						1	L	ABOR	ATOF	RY DA	ΓA		
	Elevation (feet)	Interval	Recovered (in)	Blows/foot	Collected Sample		Water Level	- Graphic Log	Group Classification	MATERIAL DESCRIPTION	Water Content, %	Dry Density, (pcf)	Compressive Strength (TSF) <sup>1</sup>	Confining Pressure (KSF)	Strain, %	Liquid Limit (LL), %	Plasticity Index (PI), %	MiniVane Shear Strength (KSF)	Passing No. 200 Sieve, %
		-	19		13	3				Firm gray sandy silt									61
<sup>1</sup> Indicates a remold was used for strength testing. <sup>1</sup> FV = Field Vane shear strength corrected for material characteristics.	40	) — - - -	11		14	4				Firm gray sandy silt with clay streaks									
Log of Boring B-3 (continued)	P://bifr15023/GINTBAYOU BONFOUCA MARSH CREATION BORING LOGS OPJ DBTemplateLIbTemplate GEOENGINEERS9.GDT/GE18_GEOTECH_LAB = Caji = Caji	ates a Field	remo Vane	ld was shear	s used for s	streng	th te	esting	g. aterial ch	aracteristics									
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Figure A-4 Sheet 2 of 2

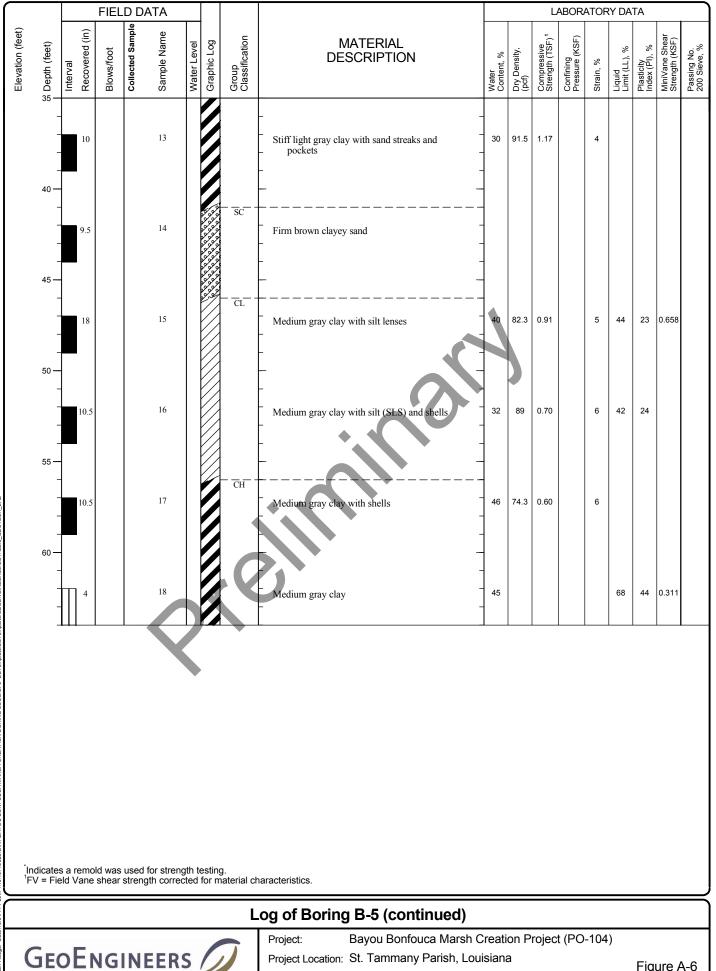
GEOENGINEERS Project: Bayou Bonfouca Marsh Creatio Project Location: St. Tammany Parish, Louisiana Project Number: 16715-023-00

Baton Rouge: [



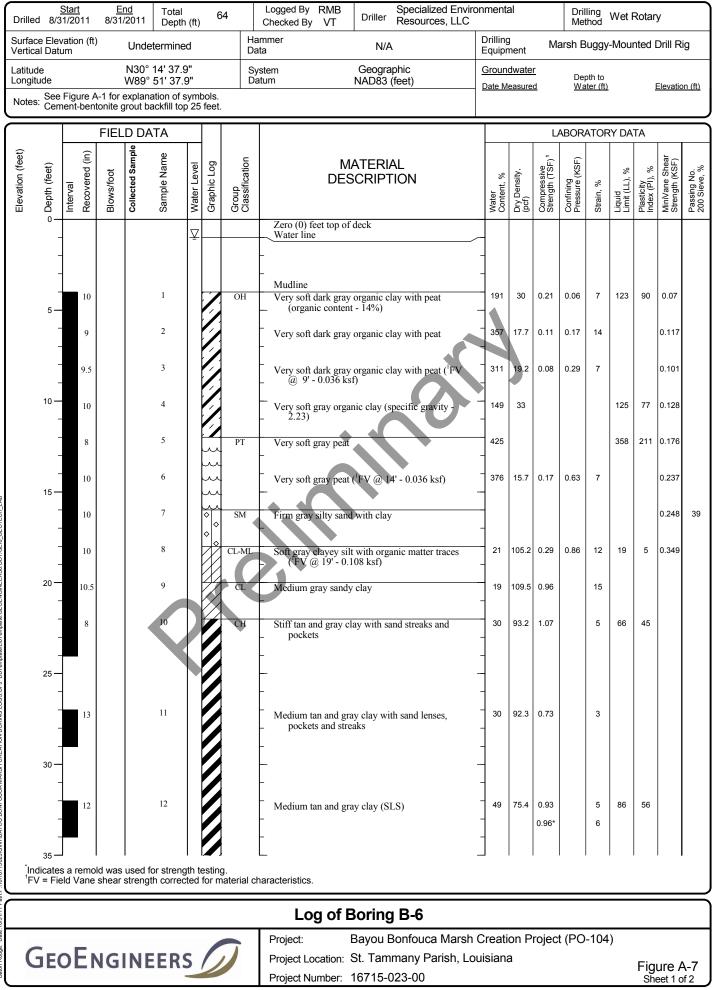
feet)		FIEL	D DATA				LABORATORY	DATA		
Elevation (feet) 얈 Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample Sample Name	Water Level	Graphic Log	Group Classification	Water	Limit (LL), % Plasticity Index (PI), %	MiniVane Shear Strength (KSF)	Passing No. 200 Sieve, %
	15.5		13				-       -	2 45		
40 — - - -	9		14				Medium tan and gray clay with 3" sand layer 41 80.7 0.93 6			
Indicate	es a remo eld Vane	ld was shear	used for stre strength corr	ngth ta	esting for ma	aterial ch	aracteristics.			
Indicate	es a remo eld Vane	ld was shear	used for stre strength corr	ngth te	esting for ma	aterial ch	aracteristics.			

Drille	d 9/	Start 1/20 <sup>-</sup>			<u>ind</u> /2011	Total Deptl		6	4		Logged By RMB Checked By VT		alized Enviro Irces, LLC	onment	al		Dril Met	ling thod	Wet I	Rotar	y	
Surfac Vertica			n (ft)		Unde	termine	ed			Ha Da	ammer ata	N/A		Drilling Equipr		М	arsh B	luggy	-Moui	nted [	Drill R	.ig
Latituo Longit						15' 30. 50' 55.					/stem atum	Geographic NAD83 (feet)		<u>Groun</u> Date M		_		oth to iter (ft)			Elevatio	on (ft)
Notes	See Cerr	Figu nent-	ire A bent	-1 for onite	explana grout ba	ation of ackfill to	sym p 25	bols. feet.				<u>.</u>		Date IM	casure	<u>iu</u>	<u>vva</u>			<u>1</u>		<u>/// (IL)</u>
_				FIEL	D DA	TA										L	ABOR	ATOF	ry da	TA		
Elevation (feet)	o Depth (feet) I	Interval	Recovered (in)	Blows/foot	Collected Sample	Sample Name	Water Level	Graphic Log	Group	Classification	DES	ATERIAL CRIPTION		Water Content, %	Dry Density, (pcf)	Compressive Strength (TSF) <sup>1</sup>	Confining Pressure (KSF)	Strain, %	Liquid Limit (LL), %	Plasticity Index (PI), %	MiniVane Shear Strength (KSF)	Passing No. 200 Sieve, %
	-						l₽				Zero (0) feet top of o Water line	leck		-								
	- - 5		10 10.5			1 2		<ul> <li></li> <li><!--</td--><td>SM</td><td></td><td>- Mudline Loose gray silty sand Loose gray clayey sa</td><td>ç</td><td>tter</td><td>- - 28 - 38</td><td>96.7 91.8</td><td>0.31</td><td>0.058</td><td>9 9</td><td>26</td><td>7</td><td>0.16 0.223</td><td></td></li></ul>	SM		- Mudline Loose gray silty sand Loose gray clayey sa	ç	tter	- - 28 - 38	96.7 91.8	0.31	0.058	9 9	26	7	0.16 0.223	
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	-		10 14			5			C	L	Loose gray clayey sa Soft tan and gray ve	y sandy clay with	1 ferrous	- 20 - 24	109.5 101.4		0.52	10 15	17 33	5 19		
	15 <del>-</del> -		10			7					gravity - 2.63; <sup>1</sup> F	content - 2.4%; s V @ 14' - 0.168 I	ksf)	- - 23 -	103.3	0.29	0.75	15				
	-		10.5			8					Very soft gray very nodules	silty clay with ferr	ous	- 22 -	102	0.22		15	22	8		
	20 —		9			9					Medium gray sandy ( <sup>1</sup> FV @ 20' - 0.2	clay with ferrous : 4 ksf)	nodules	22 	100.4	0.50		15				
	-		10			10			CI	н	Medium tan and gra lenses (SLS)	y clay with sand p	oockets and	32	91.3	0.98		6	69	48		
	25 — - - -		12			11					<ul> <li>Stiff gray clay with :</li> <li>clay layer at bott</li> </ul>		and with	- - 22 -	98.9	1.26 1.17*		6 14				
	30 — - - -		13			12					<ul> <li>Stiff gray clay with</li> </ul>	sand pockets		- - 29 -	94.3	1.43		8	69	48		
					used for strengt					al ch	L haracteristics.			_	I	I	I		I		I	1
											Log of B	oring B-5										
C	ΞEO	b	ĒN	١G	INE	ER	S		J		Project: E Project Location: S Project Number: 1	-	Parish, Lo			rojec	t (PO	-104	)	Fig	ure A	<b>A-6</b>

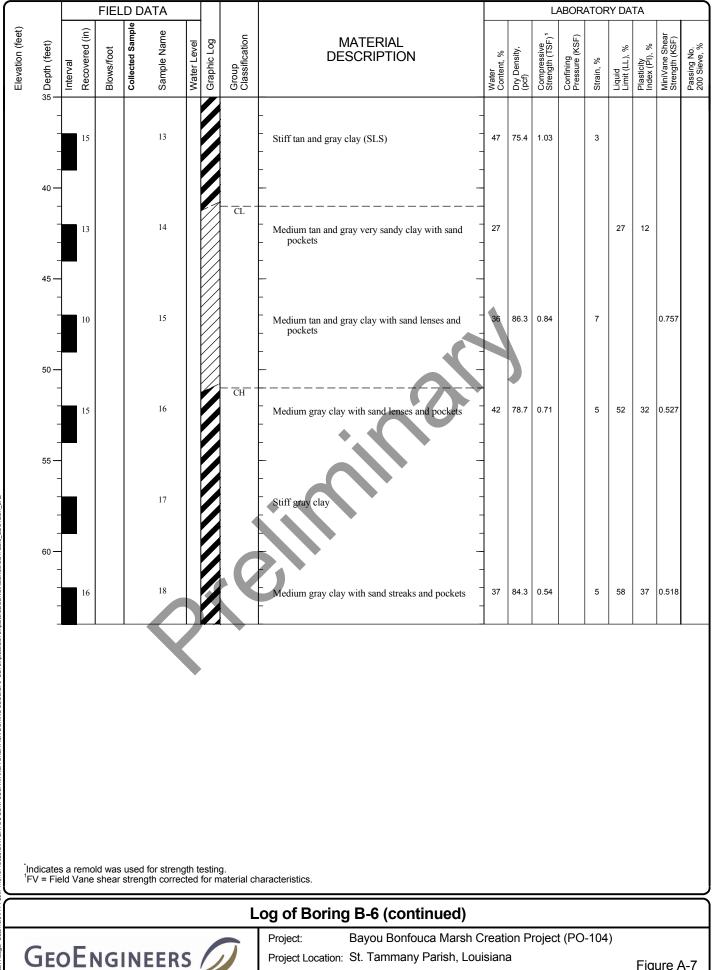


Project Number: 16715-023-00

Figure A-6 Sheet 2 of 2

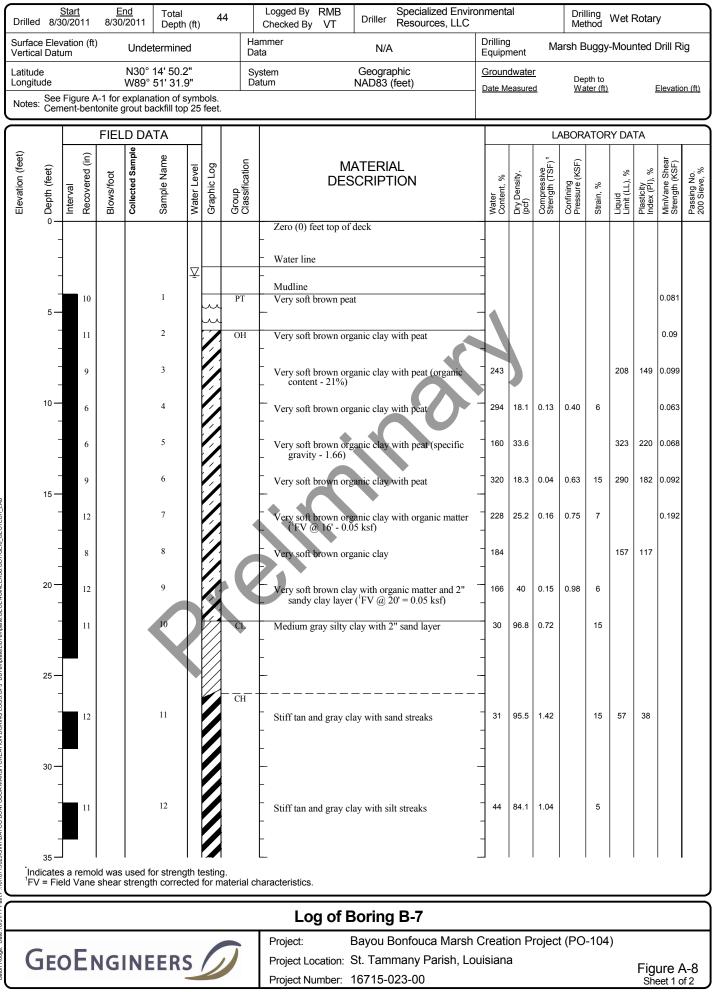


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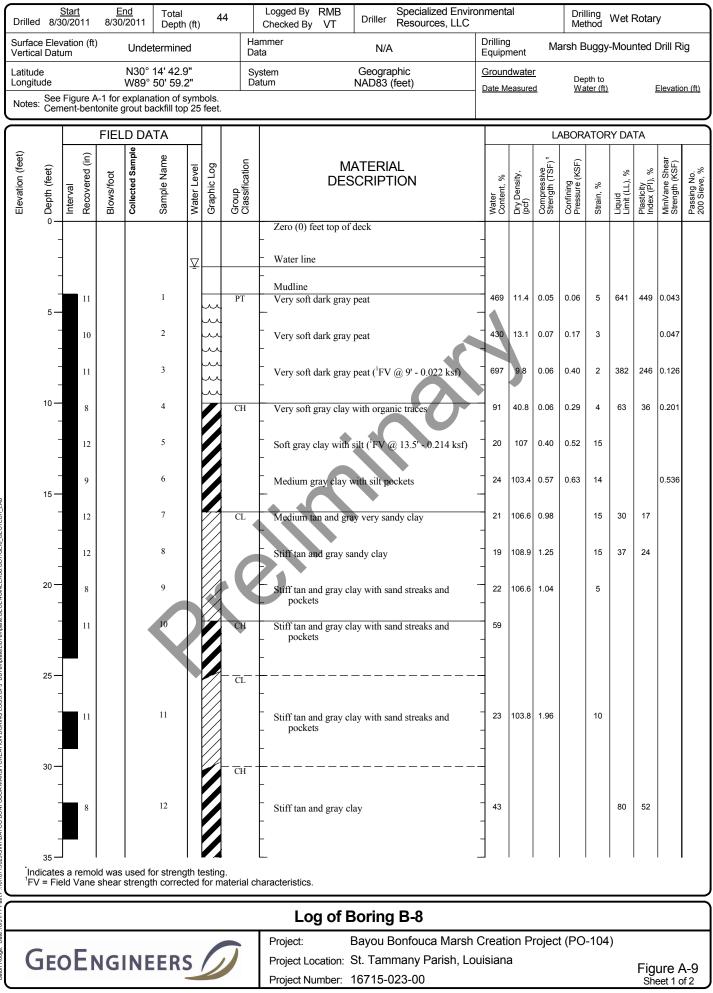
Project Number: 16715-023-00

Figure A-7 Sheet 2 of 2



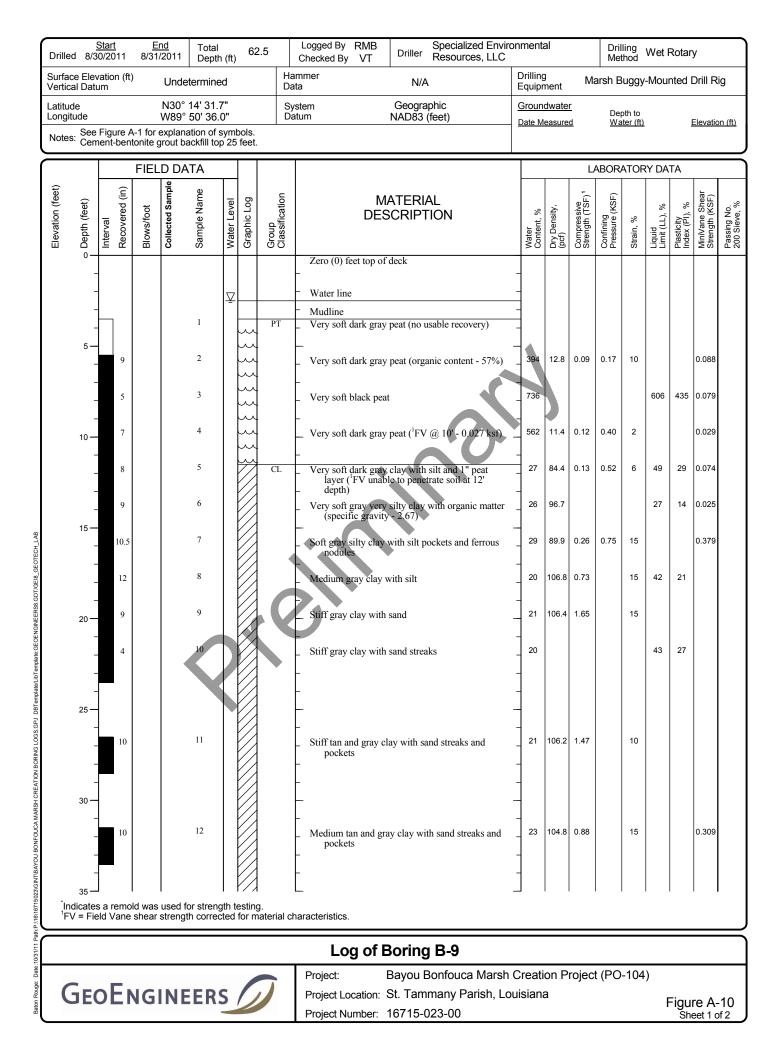
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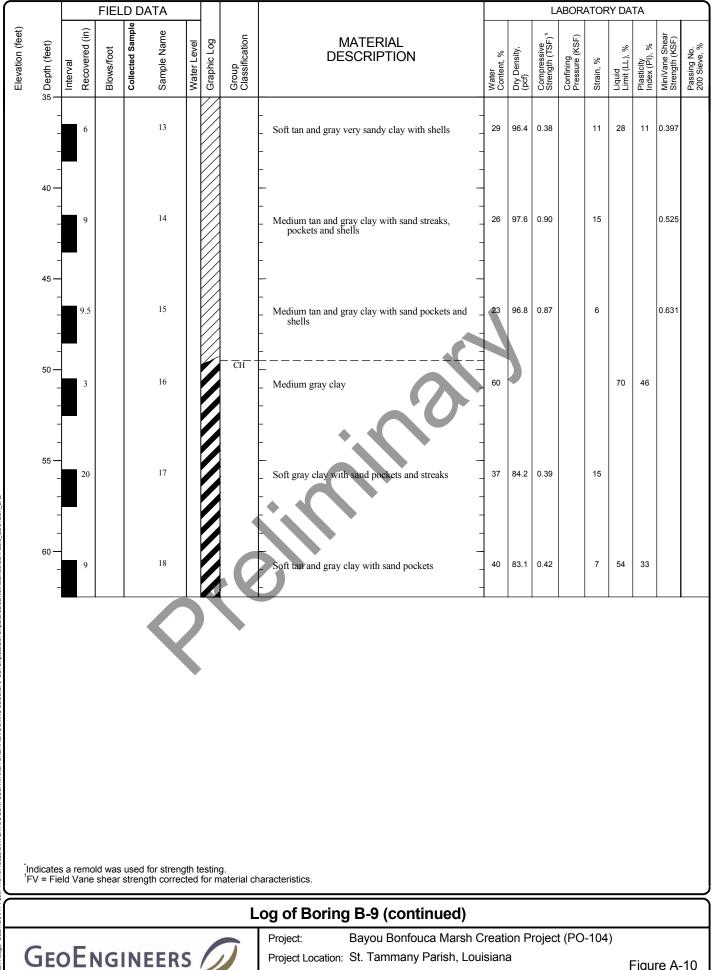
-			FIEL	D DATA				LABORATOF	RY DAT	ΓA	i	
Elevation (feet)	양 Depth (feet) I	Interval Recovered (in)	Blows/foot	Collected Sample Sample Name	Water Level	Graphic Log	Group Classification	Water Water Content, % (pcd) Content, % (pcd) Confining	Liquid Limit (LL), %	Plasticity Index (PI), %	MiniVane Shear Strength (KSF)	Passing No. 200 Sieve, %
	-	11		13								
	40 — - - -	9		14				Medium tan and gray clay 43 77.5 0.58 15	88	60		
emplare.GE.OENGINEERS8.GDT/GE.IB_GEOTECH_LAB												
ahrp://site7150236.INTBAYOU BONFOUCA MARSH CREATION BORING LOGS 6PJ DBTemplateLUDT	ndicates V = Fie	s a remo	old was	used for stre strength cor	ngth	testing I for m	). aterial ch	aracteristics.				
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÷			FIEL	D DA	TA				LABORATORY	DATA		
Elevation (feet)	ଜ Depth (feet) 	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name	Water Level	Graphic Log	Group Classification	Water Water     Water Content, %       Dby Density.     Doy Density.       Confining     Confining       Confining     Strain, %	Limit (LL), % Plasticity Index (Pl), %	MiniVane Shear Strength (KSF)	Passing No. 200 Sieve, %
	-	12			13							
	40	11			14				Stiff tan and gray clay (SLS)         37         82.2         0.88         6           1.02*         4		0.618	
								Q				
בייניינייני איינייני איינייני איינייני איינייני אייניא אייניא אייניא אייניא אייניא אייניאן אייניאן אייניאן איי	licates ' = Fie	s a remo	old was ∋ shear	used for strength	or streng	gth te	esting for m	l. aterial ch	aracteristics.			
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Project Number: 16715-023-00

SORING LOGS

Figure A-10 Sheet 2 of 2

Drille		<u>Start</u> 2/2011		<u>End</u> 2/2011	Tota Dep	th (ft)	31		Logged By SM Checked By VT	Driller Specialized Envi Resources, LLC	;			Dril Met	ling thod	Wet F	Rotar	y	
	ce Elev al Datu	/ation ( um	ft)	Und	etermin	ed			Hammer Data	N/A	Drillin Equip		Po	ntoon	Barge	e-Mou	Inted	Drill F	٦ig
Latitu Longi Note:	tude	Figure	A-1 fc	W89	° 14' 13 ° 52' 20 nation of	).0"	bols.		System Datum [	Geographic NAD83 (feet)		idwate leasure 2011	_	Wa	oth to i <u>ter (ft)</u> .0		I	Elevatio	<u>)n (ft)</u>
			FIE	LD D	ATA								L	ABOR	ATOF	ry da	TA		
Elevation (feet)	Depth (feet)	Interval Recovered (in)	Blows/foot	Collected Sample	Sample Name	Water Level	Graphic Log	Group Classification	MA DES	ATERIAL CRIPTION	Water Content, %	Dry Density, (pcf)	Compressive Strength (TSF) <sup>1</sup>	Confining Pressure (KSF)	Strain, %	Liquid Limit (LL), %	Plasticity Index (PI), %	MiniVane Shear Strength (KSF)	Passing No.
	0					¥			Zero (0) feet top of d Water line	leck									
	- 5— -								-										
	- - 10	3			1			CL	 Mudline Very soft dark gray o	lay with silt	30	5				44	27		
	- - 15 — -	12			2 3					ay with ferrous nodules lay with ferrous nodules	- - 25					26	12	0.284	
	- 20 — -	11			4				<ul> <li>Stiff gray silty clay v</li> <li>Stiff gray silty clay v</li> <li>Stiff gray silty clay v</li> </ul>	vith ferrous nodules	- _ 24 _					40	22		
	- - 25 —	3			7			СН	-		- - 38					53	34		
	-	9	,		8 9			CL CH	-	ay with ferrous nodules	 					84	55		
	- 30 —	13			10			CL	Medium gray silty cl	ay with ferrous nodules									

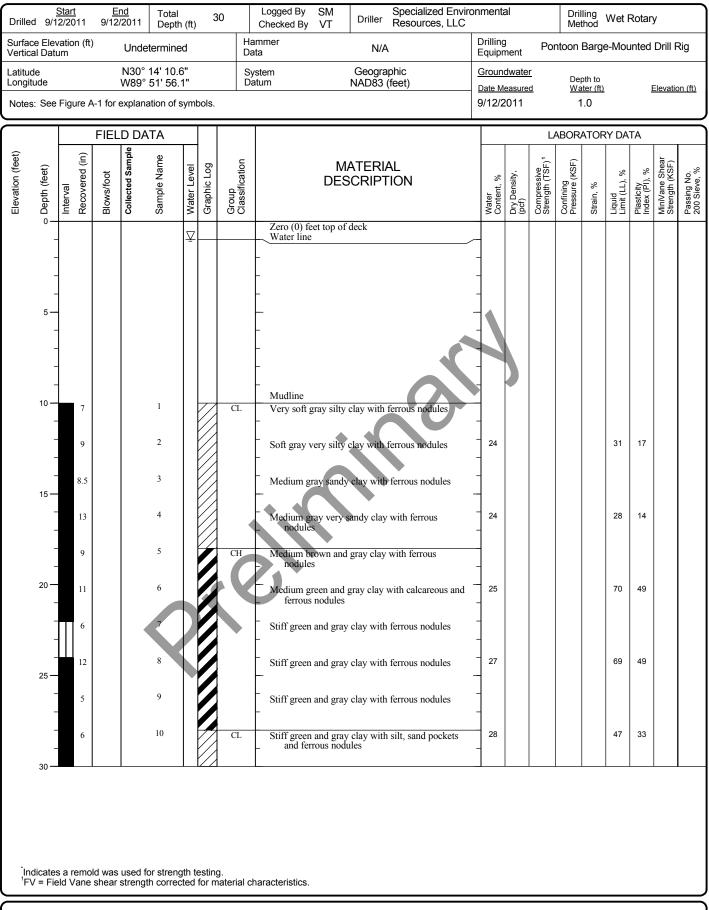
Indicates a remold was used for strength testing. <sup>1</sup>FV = Field Vane shear strength corrected for material characteristics.

## Log of Boring B-10



Project:Bayou Bonfouca Marsh Creation Project (PO-104)Project Location:St. Tammany Parish, LouisianaProject Number:16715-023-00Figure A-11<br/>Sheet 1 of 1

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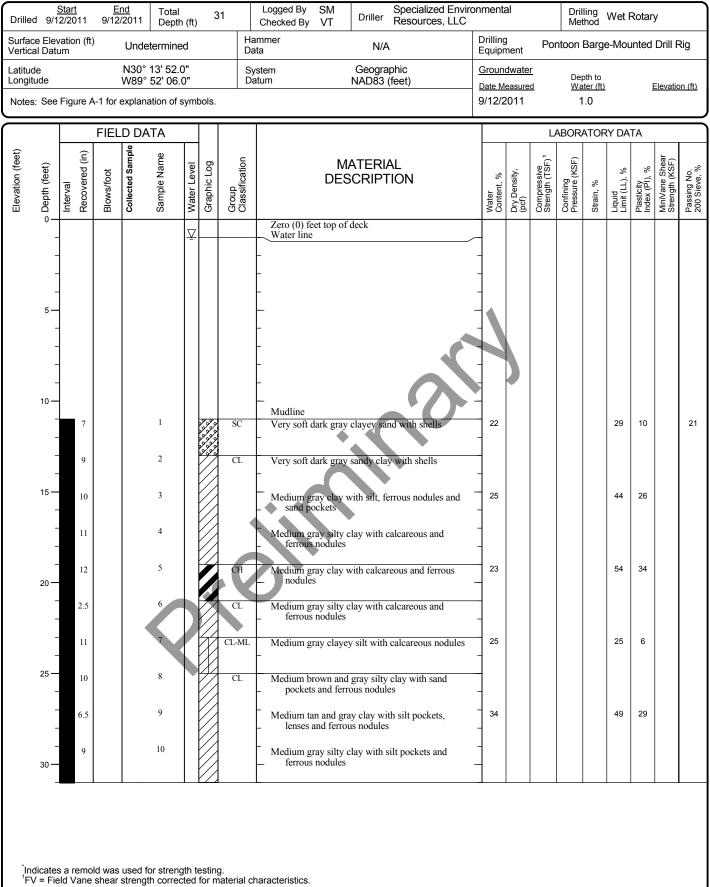
Log of Boring B-11

 Project:
 Bayou Bonfouca Marsh Creation Project (PO-104)

 Project Location:
 St. Tammany Parish, Louisiana

 Project Number:
 16715-023-00

Figure A-12
Sheet 1 of 1

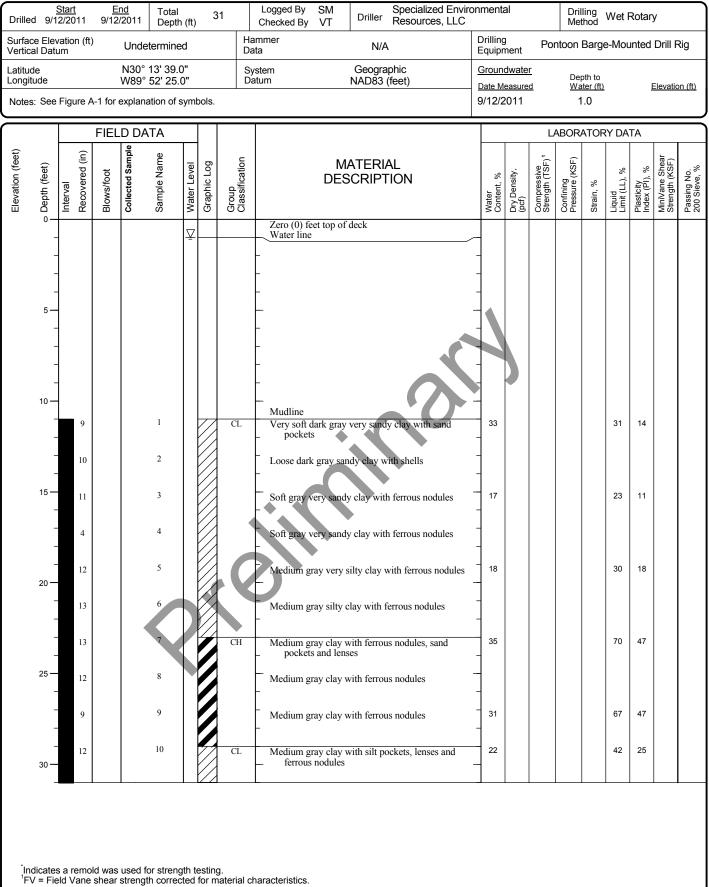


### Log of Boring B-12

Project:Bayou Bonfouca Marsh Creation Project (PO-104)Project Location:St. Tammany Parish, LouisianaProject Number:16715-023-00

GEOENGINEERS

Figure A-13 Sheet 1 of 1

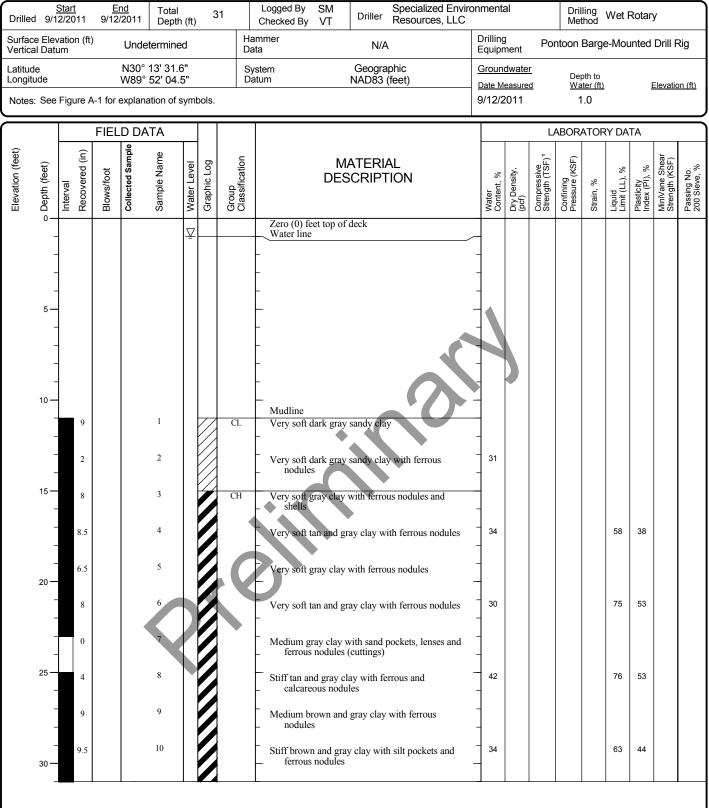


### Log of Boring B-13

Project:Bayou Bonfouca Marsh Creation Project (PO-104)Project Location:St. Tammany Parish, LouisianaProject Number:16715-023-00

GEOENGINEERS

Figure A-14 Sheet 1 of 1



Indicates a remold was used for strength testing. <sup>1</sup>FV = Field Vane shear strength corrected for material characteristics.

GEOENGINEERS

#### Log of Boring B-14

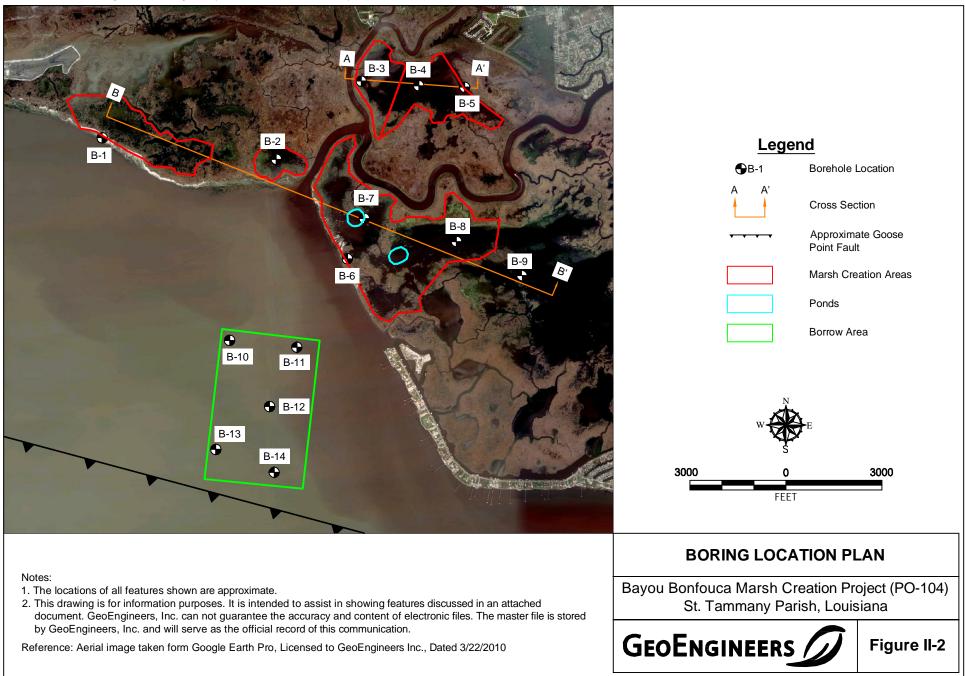
Project:Bayou Bonfouca Marsh Creation Project (PO-104)Project Location:St. Tammany Parish, LouisianaProject Number:16715-023-00

Figure A-15 Sheet 1 of 1 Bayou Bonfouca Marsh Creation (PO-104) Preliminary Design Report

# **Appendix E: GeoEngineers Settlement Curves**

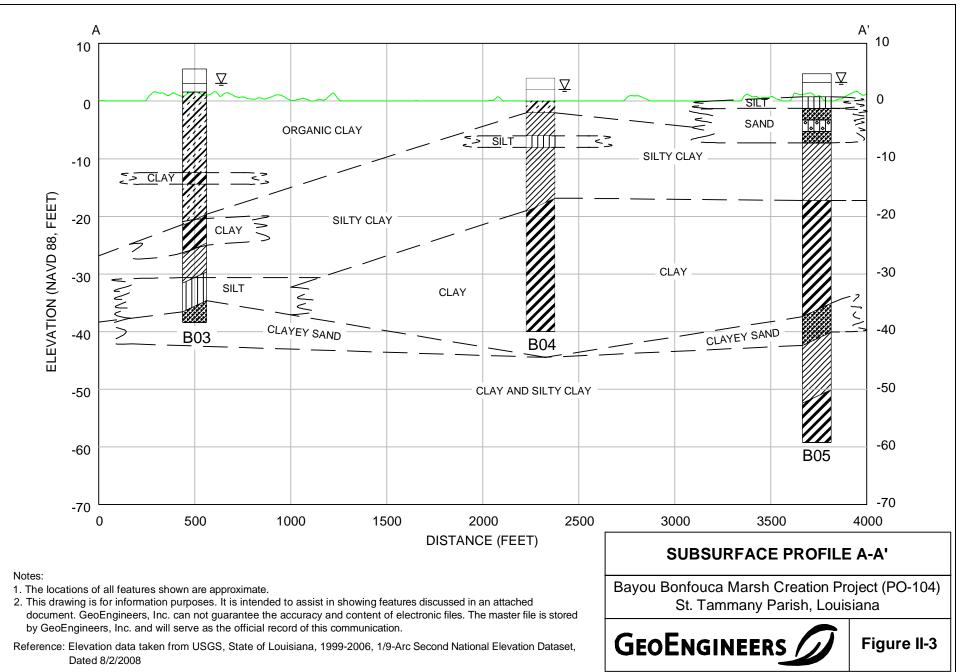
P:\16\16715023\00\CAD\Boring Location Plan.dwg\TAB:Layout1 modified on Mar 21, 2012 - 2:25pm

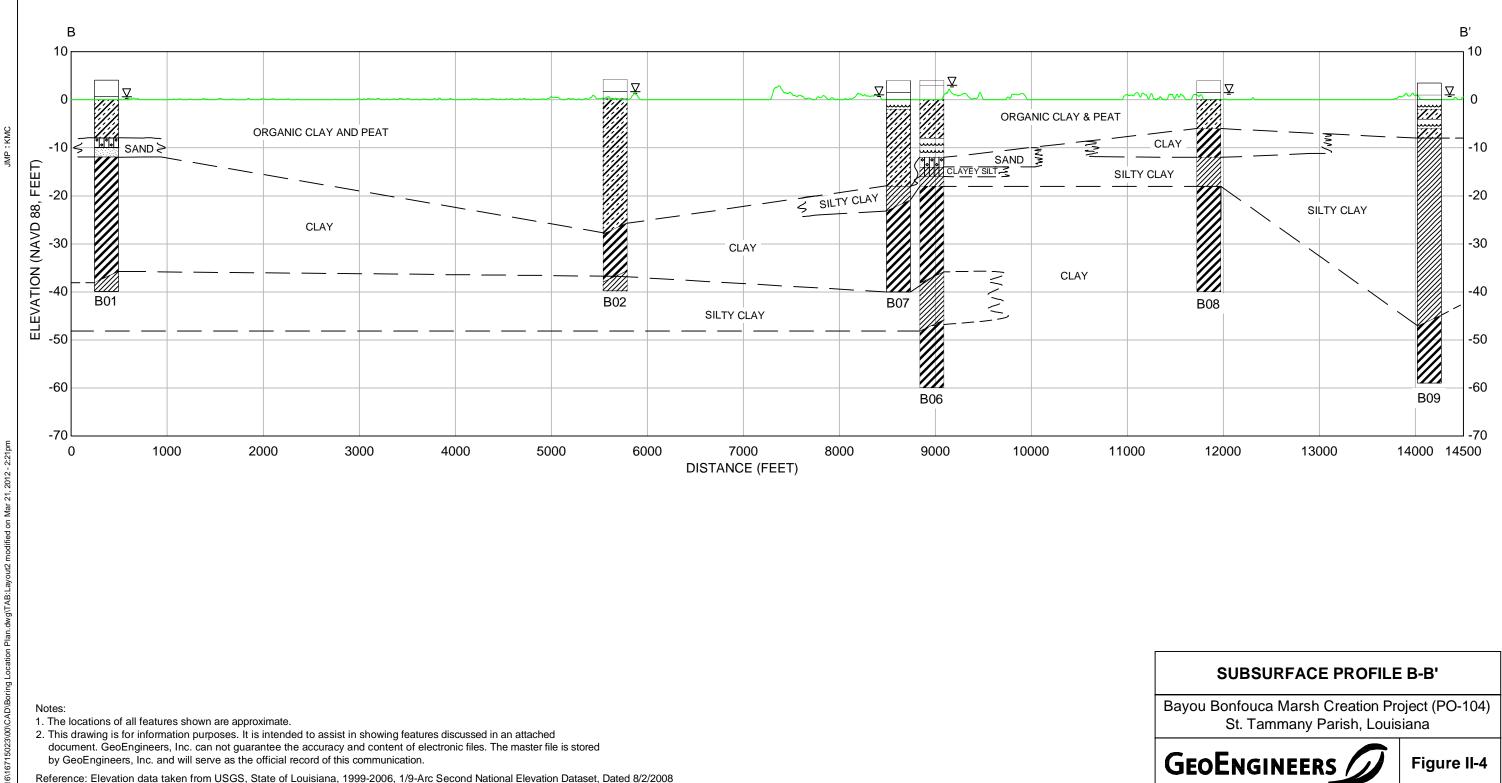
JMP: KMC











Reference: Elevation data taken from USGS, State of Louisiana, 1999-2006, 1/9-Arc Second National Elevation Dataset, Dated 8/2/2008

#### Settlement Calculation Approach for the Dredged Fill Marsh Creation Area Bayou Bonfouca Marsh Creation (PO-104)

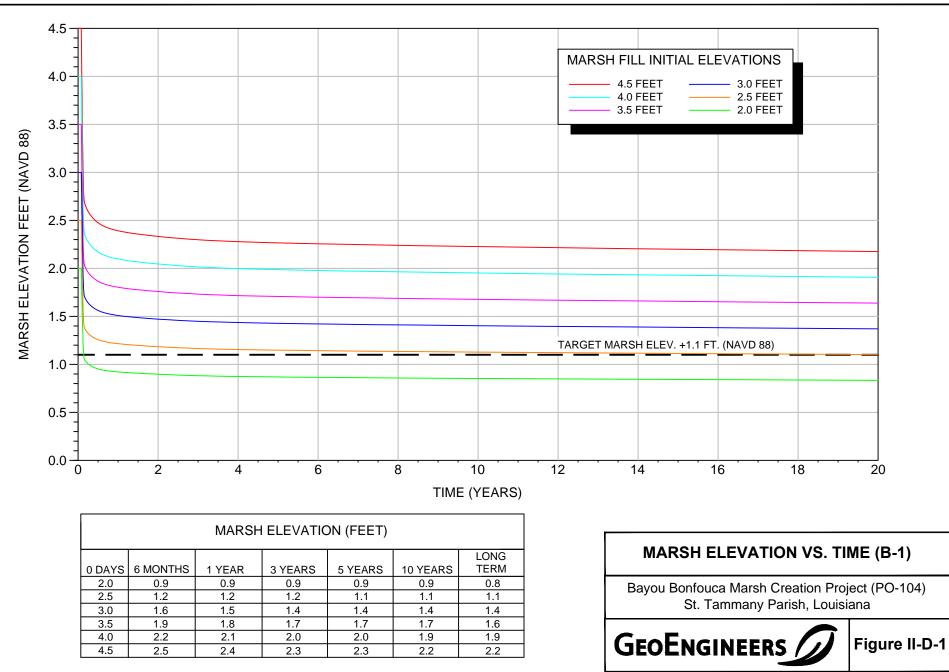
- Settlement parameters were developed for each soil layer for all borings as shown in the parameter spreadsheets provided in this Appendix. Settlement parameters for Borings B-1 through B-5 and B-7 through B-9 were used for settlement estimates for the marsh creation area. Settlement parameters were developed as follows.
  - (a) One consolidation test was done for each of the above mentioned soil borings and the samples for the consolidation tests were selected from varying depths and materials.
  - (b) Consolidation test results were analyzed and graphs were reconstructed to determine compression ( $C_c$ ), recompression ( $C_r$ ), and vertical consolidation ( $C_v$ ) coefficients, initial void ratios ( $e_0$ ) and maximum past pressures ( $P_c$ ).
  - (c) Correlations presented in equations 1 through 3 (shown in the spreadsheets in Appendix III-E) were used to calculate  $e_0$  and  $C_c$  for all the soil layers.
  - (d) GeoEngineers developed different correlations based on the analyses of the consolidation test results as follows:
    - (i) Void Ratio (e<sub>o</sub>) was estimated based on water content test results for various samples and the best fit curve drawn through plotted points from consolidation test results.
    - (ii) Moisture Content (w) Vs.  $C_v$ : A best fit curve was drawn through the plotted points from this and other coastal projects and  $C_v$  for the soil layers were obtained depending upon the moisture content.
    - (iii) w Vs. C<sub>c</sub>: C<sub>c</sub>=0.0054\*((w\*S.G.)-35) was found to provide sufficient accuracy based on the test data for this and other projects for all compressible soil types; C<sub>c</sub> was obtained for the soil layers based on the moisture content.
    - (iv)  $C_r$  was taken to be 10% of  $C_c$  for all cohesive and semi-cohesive soils.
  - (e) For the soil layers without a representative consolidation test, the above mentioned correlations were used to estimate  $C_c$ ,  $C_r$ ,  $C_v$ , and  $e_0$ .
  - (f) Past previous pressure (P<sub>c</sub>) were obtained from the consolidation test curves for the soil layers with a representative consolidation test. For other soil layers, the overconsolidation ratio (OCR) was estimated from the equation OCR =  $(c/(P_0' * 0.22))^{(1/0.8)}$ . This equation was taken from Figure 7.1 of "Recommended practice for soft ground site characterization," by Charles Ladd and Don DeGroot. P<sub>c</sub> was estimated by multiplying the overburden pressure (P<sub>0</sub>) by OCR.
  - (g) In cases where P'<sub>0</sub> was greater than P<sub>c</sub>, P<sub>0</sub>' was used as the maximum past pressure under the assumption that the soil is normally consolidated.
- 2. In this area, clay shear strength for a normally consolidated soil profile will be approximately 22% of the effective overburden pressure. This relationship is shown as the C/P line on the shear strength profiles. Based on this relationship, it appears that the top 20 feet of the soil profile is over-consolidated throughout the design profiles. For all but two of the borings (B-2 and B-3), the top 40 feet are overconsolidated. Beyond 40 feet depth, some of the layers are still overconsolidated. This affects the settlement parameters selected for design.
- 3. Due to the broad fill area, the drainage is vertical for all the soil layers. Drainage to the phreatic surface or to the nearest granular soil layer has been considered for these soil layers. The presence of small sand and silt layers within clay was considered in the drainage path evaluation.

Settlement of the marsh creation area consists primarily of two separate processes: consolidation of the dredged fill and consolidation of the foundation soils. Consolidation of the dredged fill was modeled using PSDDF (Primary Consolidation, Secondary Compression, and Desiccation of Dredged Fill), a program created for the United States Army Corps of Engineers to simulate finite strain consolidation in dredged fill materials. Consolidation of the foundations soils was modeled iteratively using a one-dimensional consolidation program. To account for the effects of progressive dredged fill densification and submergence below the waterline caused by foundation soil settlement, we re-computed effective vertical stress and corresponding settlement at various time intervals after fill placement. The typical steps at some time = t were as follows:

- 1. Calculate settlement for soil beneath the fill based on the elapsed time and the effective stress calculated for the application of a single lift of fill and determine the new mudline elevation.
- 2. From PSDDF determine the change in thickness of the dredged fill to calculate the fill density and the new fill surface elevation. The new fill surface elevation is influenced by both the foundation settlement and the change in fill thickness computed by PSDDF.
- 3. Re-compute the effective vertical stress based on the new elevations of the fill surface and mudline, and a constant water elevation of 0.8 feet NAVD 88.
- 4. Use the new lower effective stress to re-compute settlement.

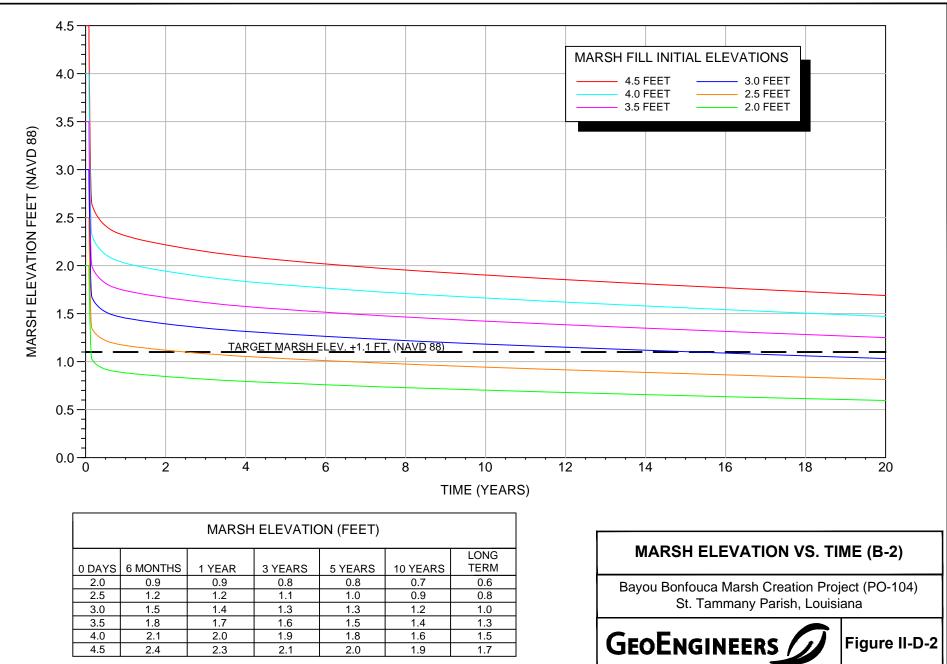
This was repeated at days 45, 60, 90, 180, 365, 1095 (3 years), 1825 (5 years), 3650 (10 years), and 7300 (20 years). Day 1 of the PSDDF calculation was taken as 30 days after the start of filling, allowing 30 days to complete placement of the hydraulic fill. Therefore, day 30 for foundation soil settlement calculations is day 1 for PSDDF calculations.

The sum of the dredged fill settlement and the underlying soil settlement was used to determine the total settlement at the surface of the dredge fill area after filling is complete. Settlement of dredged fill evaluations were performed for a single lift scenario with fill placed in a range of elevations from +2.0 to +4.5 feet. Results were plotted at 0.5-foot intervals (based on initial constructed fill elevation) alongside a line representing the marsh target elevation (+1.1 ft) to establish the best estimate for initial fill elevation.



P:\16\16715023\00\CAD\Settlement.dwg\TAB:B-1 modified on Mar 21, 2012 - 7:57am

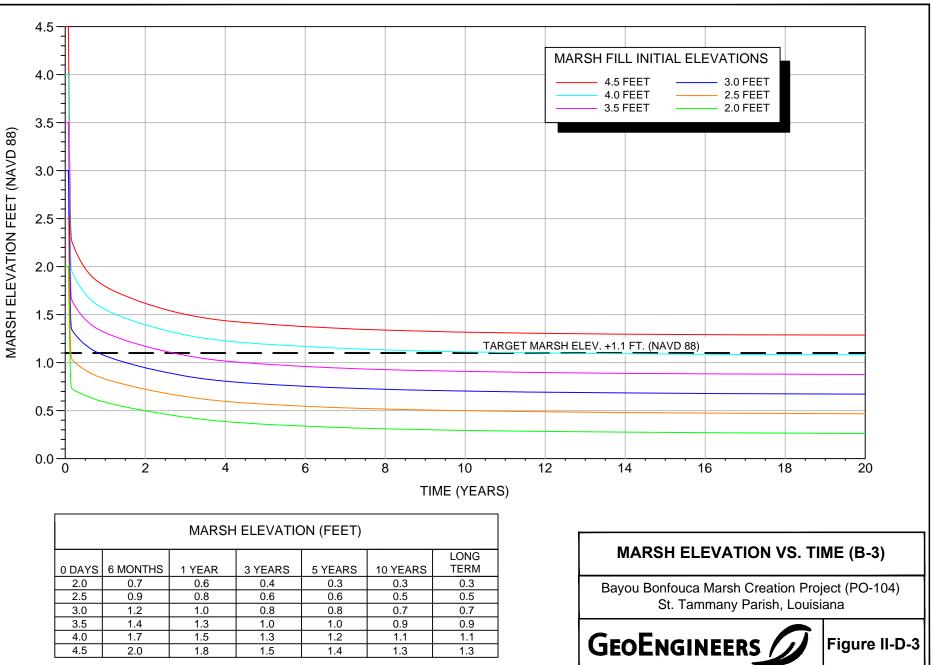
JMP : KMC



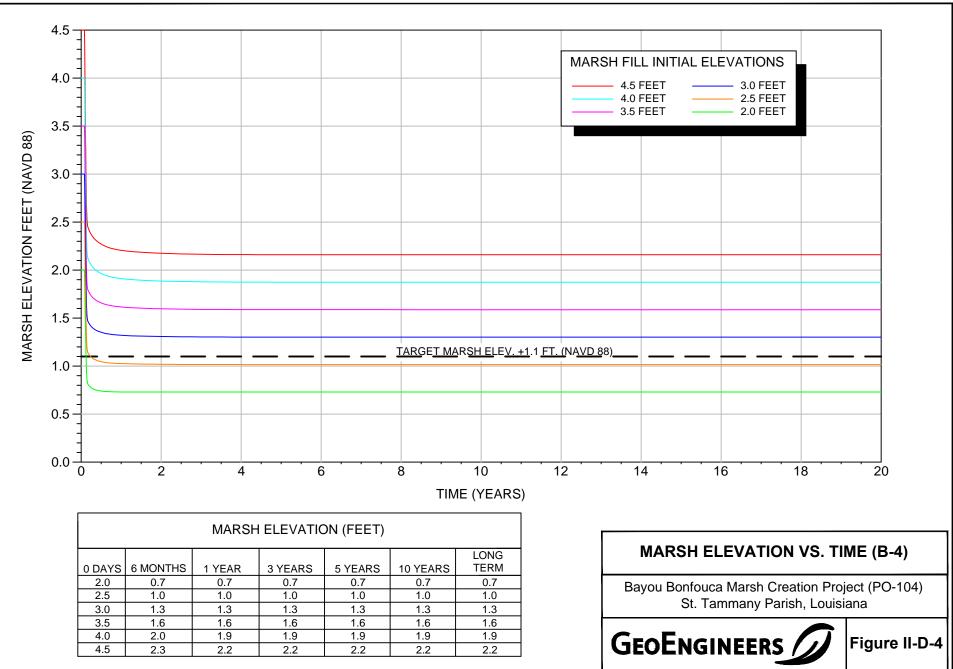
P:\16\16715023\00\CAD\Settlement.dwg\TAB:B-2 modified on Mar 21, 2012 - 7:57am

JMP : KMC





JMP : KMC

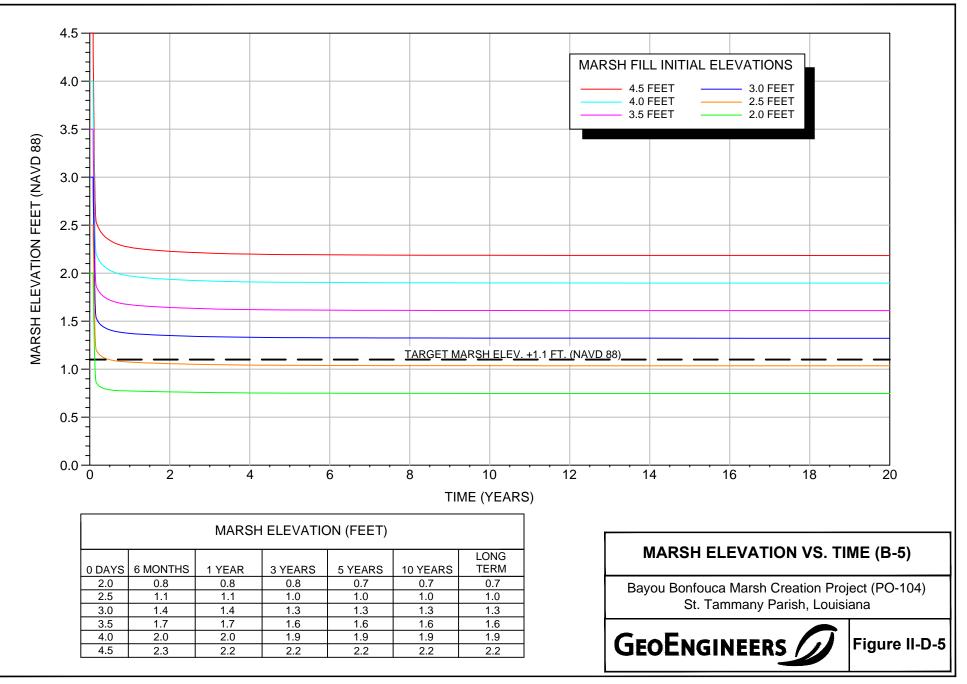


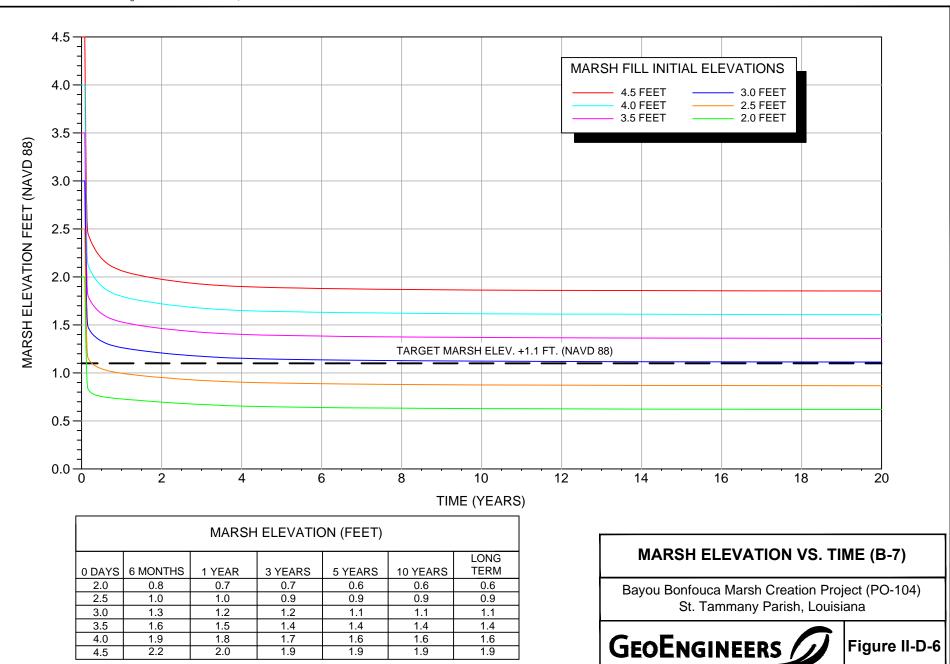
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JMP : KMC



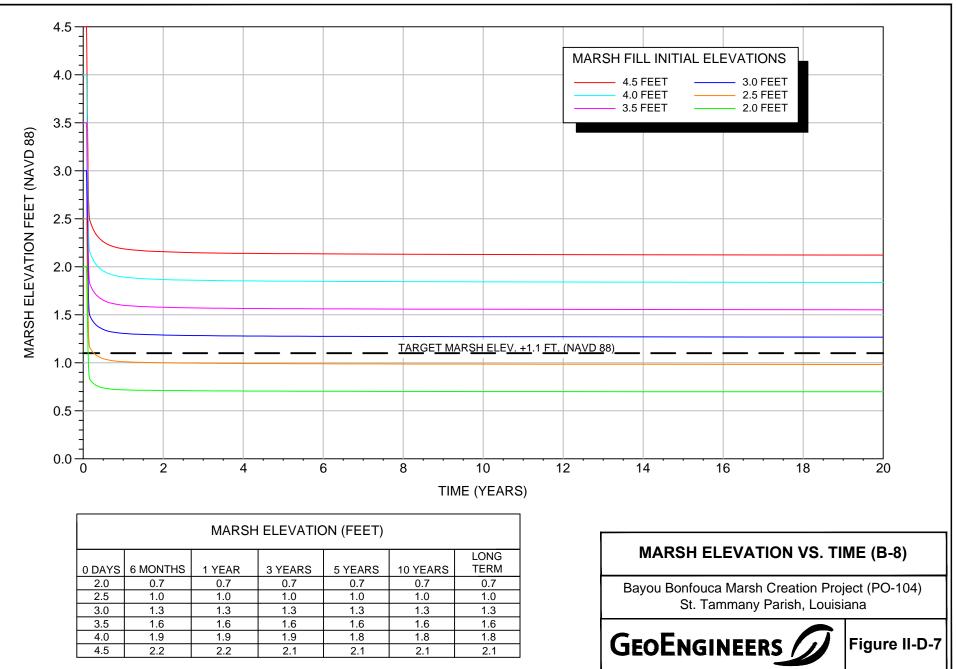






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JMP : KMC

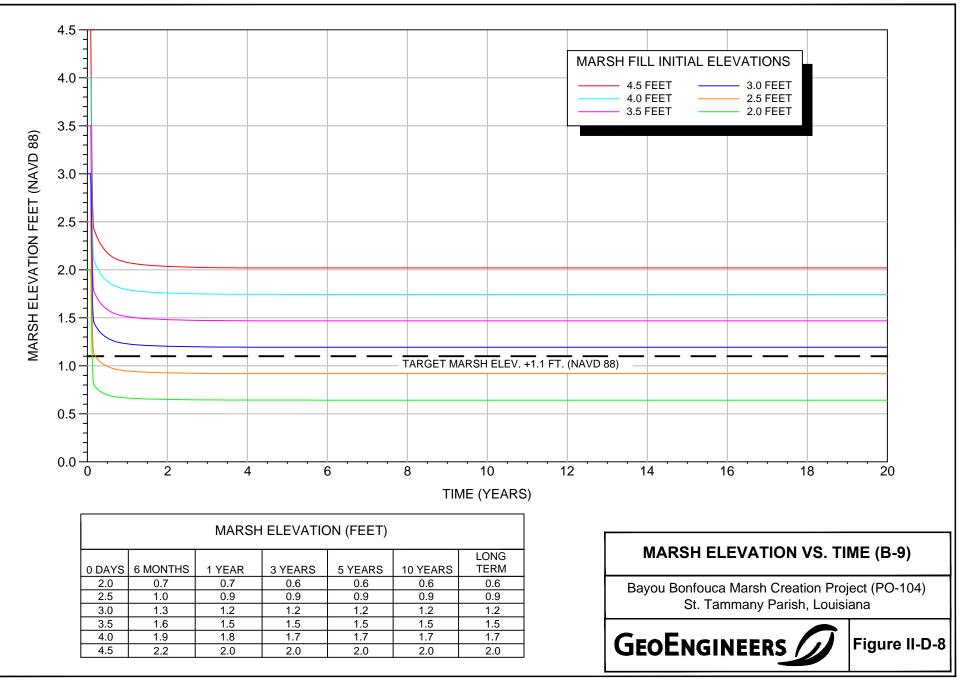


#### P:\16\16715023\00\CAD\Settlement.dwg\TAB:B-8 modified on Mar 21, 2012 - 7:57am

JMP : KMC







# **Appendix F: Archeological Information**

#### SITE RECORD FORM

/ite Name: Bonfouca 2 Instructions for Reaching Site: Shell midden at the mouth of Other Site Designations: Bonfouca on Lake Pontchatrain Parish St Tammany USGS Quad (name, date, series): North Shore, 7.5 ' series  $(32 \ Qb)$ of Section 23 Township 9S Range 13E of the of the Northing 33479780 UTM Coordinates: Zone 15 Easting 224070

#### PHYSICAL SETTING

Slope: none Land Form: beach ridge Elevation: 0-5 ft AMSL Geologic Processes: wave washed, subsidence Site Position with Respect to Terrain: in the lake and along bayou banks Nearest Water: Lake Pontchatrain Flooding: frequent Soil Characteristics: Lafitte. level, very poorly drained soils that have a mucky surface layer and clayey and mucky underlying matrial; in brackish marshes Floral Communities: plants and animals associated with lake shore

Faunal Communities: Other Potential Resources: Nearest Known Site: 16ST3/27

#### SITE DESCRIPTION

Plan:

ite Size: c. 60 x 90 m(200 x 300 ft) Orientation: Artifact Density; light to moderate

Stratigraphy: shell midden Artifact Distribution: very scattered by wave action

Cultural Affiliation: Marksville (based on Marksville Incised) and Plaquemine/Mississippian (based on possible Pontchetrain Checked Stamp and shell tempered sherd) Presumed Function: 'camp?

#### COLLECTIONS

Survey Method: grab surface

**Cultural Features:** 

Assessment of Collecting Conditions: moderate to good, depending on depth of lake and bayou Description of Material: 33 sherds including 3 possible Pontchrartrain Check Stamp, 1 Mazique Incised, 1 Marksville Incised, and 1 shell tempered sherd; 2 Gary and 2 Kent points, 1 drill

#### CONDITIONS

Erosion or Disturbance: highly eroded by lake wave action, Present Use also area may be used to dump historic debris (many bricks on site) likely destruction through subsidence and wave action Probable Future Destruction:

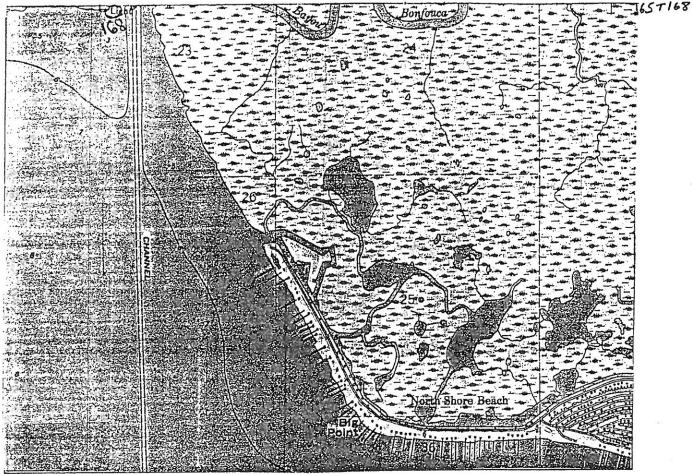
State Survey No.:16ST168

(2 of 3) 1657**1**68

#### SITE EVALUATION

Research Potential: probably poor given its eroded condition State or National Register Eligibility (Criteria D): unknown Recommendations: monitor

#### **QUAD MAP OF SITE AREA**



#### RECORDS

Owner and Address: Tenant and Address: Informants: Joey Grubaugh Previous Investigations: Previous Collections and Availability: collection with J. Grubaugh References: 22 - |9|3Photographs and Maps: photograph of site (see Continuation form) and photograph of Grubaugh collection: LSU H95-L1-11 thru 13

Remarks:

Recorded by: Chris Hays (RAP-LSU) based on interview with J. Grubauagh

Date: 7/12/95

## STATE OF LOUISIANA SITE RECORD UPDATE FORM

#### Site Name: Point au Chien

Site Number: 16ST33

NAD: NAD 83

Northing: <u>3350130</u>

Other Site Designations: none

Parish: St. Tammany

DRAFT

Instructions for Reaching the Site: Launch into Bayou Liberty at the St. Genevieve Church ramp in Bonfouca La. Travel SW down Bayou Liberty into Lake Pontchartrain, travel west/northwest along the north shore of Lake Pontchartrain approximately 0.94 miles, site is located on the north shore of Lake Pontchartrain at this point. 7 5? USGS Quadrangle (name, date): Lacombe, 1971 (1994); Slidell, 1971 (1994)

 7.5' USGS Quadrangle (name, date): Lacombe, 1971 (1994); Slidell, 1971 (1994)

 \_\_\_\_\_¼ of the \_\_\_\_¼ of the \_\_\_\_¼ of Irreg Section: 21 and 22 Township: 9S

 Range: 13E

 UTM CP Coordinates

(Beach Ridge): Zone: <u>16</u>

UTM CP Coordinates					
(Shell midden):	Zone: <u>16</u>	Easting: 222858	Northing:	3350134	NAD: <u>NAD 83</u>

Easting: <u>223029</u>

Geographical Coordinates:

Latitude: \_\_\_\_\_ Longitude: \_\_\_\_\_

#### Geographical Setting

Landform: marsh/relic beach ridge

Distance and Direction to Nearest Water: on north shore of Lake Pontchartrain Soil Series: Lafitte Muck

#### Site Investigation and Description

Survey Method(s): visual inspection

Site Size: shell midden stretches approximately 1,866 meters along north shore of Lake Pontchartrain Site Shape/Plan: linear along the shore of Lake Pontchartrain

Representative Stratigraphy: none observed

Depth of Deposit: unknown

Cultural Features: shell midden on lake shore/earth and shell midden on relic beach ridge

Cultural Affiliation: unknown prehistoric; Coles Creek

Site Function: camp/extraction locale

Description of Material: shell, aboriginal pottery, projectile point, and faunal bone observed but not collected

#### Site Condition

Present Use: open marsh land Disturbance: Yes 🛛 No 🗌 please explain in the Narrative

#### Site Evaluation

Research Potential: good Recommend Further Work: Yes 🛛 No 🗌 please explain in the Narrative

#### Records

Owner and Address/Contact Info: Big Branch National Wildlife Refuge, 61389 Hwy. 434 Lacombe, LA 70445. Phone Number: 985-882-2000 References: 22-1380 Permanent Disposition of Current Collection: no collections made

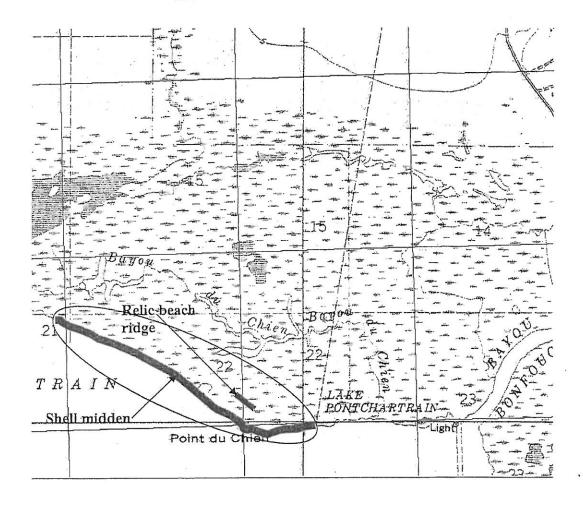
Recorded By: R. Mann Company/Organization Contact Info: LSU Date: 09/21/2010

## STATE OF LOUISIANA MAP PAGE

Site Name: Point au Chien

Site Number: 16ST33

## USGS 7.5' Quadrangle Map of Site Area



#### STATE OF LOUISIANA NARRATIVE PAGE

Site Name: Point au Chien

Site Number: 16ST33

Please provide a brief summary of the geographical setting and site condition. This information may include site elevation, slope, other potential resources, other nearby sites, past/current environmental information, site orientation on the landscape, collecting conditions such as ground visibility, and any possible future threats to the site. Also use this page to elaborate on any of the sections on the site form, including additional UTM coordinates for the site boundaries.

My visit to the Big Branch Marsh NWR was spurred by reports of an unrecorded mound associated with a large shell midden site along the north shore of Lake Pontchartrain. In conversations with Mr. Bill Baker, a local collector and fisherman, I initially thought he was referring to Bonfouca 2 (16ST168), a shell midden located where the mouth of Bayous Bonfouca/Liberty empty into Lake Pontchartrain. This site was recorded by Chris Hays in 1995 based on an interview with a local collector and LAS Delta Chapter member named Joey Grubaugh. Hays (1995:33) did not visit the site but did inventory and photograph Grubaugh's surface collection from the site.

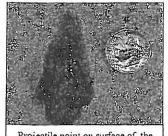
Because the site had not been visited by a professional archaeologist and Mr. Baker was concerned about the amount of erosion occurring along the north shore of Lake Pontchartrain, especially in the years following Hurricane Katrina, I made arrangements with Mr. Daniel Breaux, Manager of the Big Branch NWR, to visit the area. We put in at Boufouca and traveled down Bayou Liberty to Lake Pontchartrain. At this point it became clear that the site Mr. Baker was referring to was not Bonfouca 2 (16ST168). In fact, we saw no evidence of this site at its plotted location. It may be that is it now submerged or has completely eroded away. However, since this turned out not to be the primary area of interest, we did not make a systematic effort to locate this site. An additional trip to the area should be able to clear up the present status of the site.

The site Mr. Baker was concerned with turned out to be Point au Chien (16ST33). This site was also originally recorded by Saucier and Gagliano in 1958. As originally recorded the site was described as an elongated shell (rangia) midden located on a relic beach ridge approximately 500 feet (152.4 m) north of the shore of Lake Pontchartrain. The site was updated by Beavers, Lamb, and Greene in 1988. They also note that the site is located on a "beach ridge extending above the marsh" and likewise plotted the site north of the shore of Lake Pontchartrain. It is unclear if they saw midden deposits or artifacts eroding from the marsh on the shore of Lake Pontchartrain. Presently, however, an extensive shell midden is actively eroding from the marsh on the north shore of Lake Pontchartrain. The relict beach ridge, which rises slightly above the marsh, is covered with scrub and several large live oaks. The elongated midden reported by Saucier and Gagliano and Beavers, Lamb, and Greene is still present here and I saw artifacts (pottery) and rangia shell in the root ball of a recently uprooted live oak. The relic beach ridge is what Mr. Baker was referring to as a mound. This is the site recorded as Point au Chien (16ST33). However, the site seems to be much more extensive than previously recorded if the shell (rangia and some oyster) midden now eroding from the shore line of Lake Pontchartrain is considered part of the site. Based on aerial images of the area, the shell midden extends for approximately1866.8 m along the north shore of Lake Pontchartrain. We were able to only inspect a small portion of the midden during my visit, but it is evident that artifact density is fairly high throughout the midden. Diagnostic pottery types that I observed (e.g. Pontchartrain Check Stamped, var. unspecified, Mazique Incised, var. unspecified) suggest a strong Coles Creek component present on the site. A stemmed dart point was also observed (see figure). Mr. Baker reports that he has found Tchefuncte pottery and microdrills at the site, but I have not seen

his collection. Faunal bone was also present on the surface; I saw no obvious human bone. No collections were made during our visit, artifacts were photographed on site.

My visit to the Point au Chien site (16ST33) has revealed a much more extensive shell

midden than previously recorded as being present at this location. It seems possible, given the amount of shore line erosion that has taken place since Hurricane Katrina, that this midden deposit has only recently become so prominently exposed. Investigators visiting the site in 1988 certainly did not give the impression that such an extensive shell midden was visible on the shore of Lake Pontchartrain. Additional survey and testing of this midden is recommended in order to accurately record the nature and extent of this deposit, its cultural affiliation(s), and its precise relationship to the midden deposits located on the relic beach ridge. These



Projectile point on surface of the Point au Chien site.

investigations are necessary before the NRHP status of this impressive site can be determined.

## (1 of 2)

STATE OF LOUISIANA

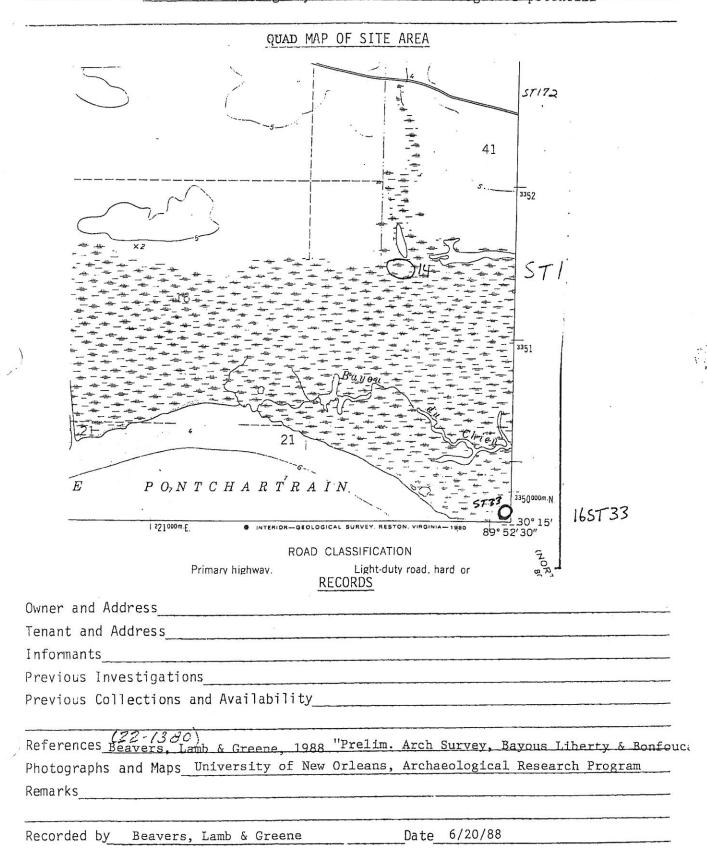
SITE RECORD UPDATE FORM

Site Name Point au Chien	State Survey No. 16 ST 33
ther Site Designations	
	n Bayou Bonfouca to Lake Pontchartrain, west
along lake shore to Bayou du Chien	
along_lake_Shore_lo_bayou_ou_chizen	Parish <u>St. Tammany</u>
USGS Quad: (name, date series) Slidell,	
of the sw ½ of the sw ½ of Sect	
UTM Coordinates: ZoneEasting	
Geographical Coordinates: Latitude 30° 15	
PHYSICAL SE	
Land FormBeach ridge	Geologic Processes <u>Erosion</u>
	Elevation
Slope Site Position with Re	spect to Terrain Beach ridge extending above
marsh	
Nearest Water Lake Pontchartrain & B. du Ch	nien Flooding <sub>Yes</sub>
Soil Characteristics <u>Sand</u>	
Floral Communities	
Saunal Communities	
Jther Potential Resources	
Nearest Known Site 16 ST 34	
SITE DESCRI	
Site Size 450' x 45' Pla	n
Orientationeast_west along shore	Stratigraphy
Artifact Density Sparse Art	
Cultural Feat	ures
Cultural Affiliation Late Marksville	
	tation
COLLECTI	
Assessment of Collecting Conditions <u>Good</u> -	
Description of Material <u>Baytown Plain var</u>	0 0
	arksville Incised Var. Unspecified
	er, turtle, alligator and fish
	· · · · · · · · · · · · · · · · · · ·
CONDITIC	INS
	on or Disturbance_ <u>Erosion</u>
Probable Future Destruction Continued ero	•

#### SITE EVALUATION

Research Potential Fair

State or National Register Eligibility Probable state eligibility Recommendations Further testing may indicate National Register potential



(20f2)

STATE OF LOUISIANA SITE RECORD FORM

State Survey No: 16ST33 ite Name: Point Auchien Parish: St. Tammany Other Site Designation: Instructions for Reaching Site: On Point Auchien on N shore of Lake Pontchartrain about 500' N of shore and about 440 W of mouth of Bayou Bonfouca USGS Quad (Name, date, series): Slidell (1950) 15' Quad No: 31-Q E quarter of the IR quarter of Section 22 Township 9S Range: 13E Zone: 16 Easting: Northing: UTM Coordinates: Latitude: Longitude: Geographical Coordinates: PHYSICAL SETTING

Land Form: Beach ridge Slope: Geologic Processes: Elev. ft AMSL: -2 Site Position: Subsided beneath marsh on a beach ridge Near Water: Flooding: Soil Characteristics: Medium silty sand w/ some shell below midden Floral Communities: Live oaks, hackberry Faunal Communities: Other Potential Resources: Nearest Known Site:

#### SITE DESCRIPTION

Site Size: 450' x 45' x 2'high Plan: )rientation: E-W Artifact Density: Cultural Features: Elongated shell midden - Rangia cuneata Cultural Affiliation: Neo-Indian (unknown) Presumed Function: Prehistoric (unknown)

#### COLLECTIONS

Survey Meth: Grab surface collection, test units Assessment of Collecting Conditions: Description of Material: Bone sample collected for Cl4 dating, pottery, shell

#### CONDITIONS

1 (C. 1997)

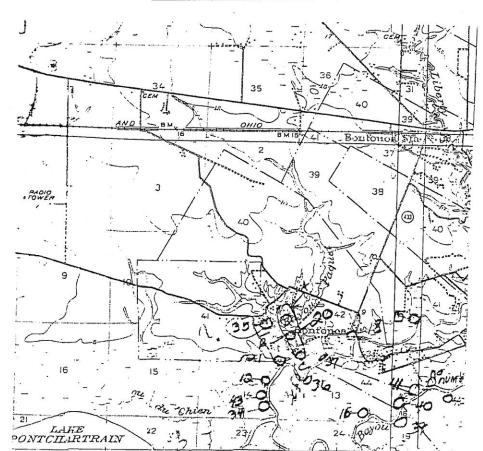
Present Use: Erosion or Disturbance: Subsidence Probable Future Destruction:

والمجرورة الأحال الإيراد جرري الا

#### Research Potential:

State/National Register Eligibility: Unknown Recommendations:

QUAD MAP OF SITE AREA



RECORDS

#### Owner and Address:

Tenant and Address:

Informants: Prev. Invest: Previous Collections and Availability: Fair collections LSU Cat. No. 58-594 References: Heartfield, et al (1981); Neuman (1977); Saltus (1988) Photos and Maps: Remarks:

Recorder: Saucier & Gagliano

Date: 3/58

· · · · ·

## (1 of 2)

STATE OF LOUISIANA

SITE RECORD UPDATE FORM	SITE RECORD U	PDATE	FORM
-------------------------	---------------	-------	------

Site Name	State Survey No. <u>16 ST 43</u>
Other Site Designations	
Instructions for Reaching Site By boat along B	ayou Liberty
	Parish <u>St. Tammany</u>
USGS Quad: (name, date series) <u>Slidell</u> , La.	(31Qd)
of theof theof Section	
UTM Coordinates: ZoneEasting	Northing
Geographical Coordinates: Latitude 30° 15' 52	" N Longitude 89° 51'19" W
PHYSICAL SETTI	
Land Form <u>Marsh</u>	Geologic Processes erosion, subsidence
	Elevation
SlopeSite Position with Respec	ct to Terrain right descending bank of bayou
Nearest Water Bayou Liberty	Flooding
Soil Characteristics	
Floral Communities marsh grasses	
Faunal Communities	
Other Potential Resources	
Nearest Known Site 16 ST 34	
SITE DESCRIPTI	ON
Site Size Unknown Plan s	Site appears to be totally subsided
OrientationStra	atigraphy
Artifact DensityArtifa	ct Distribution
Cultural Feature	S
Cultural Affiliation	
Presumed Function	
COLLECTIONS	
Survey Method close order pedestrian	
Assessment of Collecting Conditions Poor - sit	te is completely subsided below marsh
Description of Material <u>None recovered</u> . No evi	idence of site located
CONDITIONS	
	or Disturbance Destroyed
Hesene use None	
Probable Future Destruction	×

4

#### SITE EVALUATION

Research Potential None - site is subsided below marsh.

State or National Register Eligibility\_\_\_\_\_ Recommendations

QUAD MAP OF SITE AREA 165T43 PONTCHARTRAIN

RECORDS

Owner and Address\_\_\_\_\_\_ Tenant and Address\_\_\_\_\_\_ Informants

Previous Investigations <u>Saucier and Gagliano 7/25/54</u> Previous Collections and Availability

 References
 (22-13 & O) Beavers, Lamb & Greene, 1988, "Prelim Arch Survey, Bayous Liberty & Bonfou

 Photographs and Maps
 University of New Orleans, Archaeological Research Program

 Remarks
 Originally noted in 1954 as visible only at extreme low water, this site has apparently continued to subside for the last 24 years and is now inexcessible

 Recorded by
 Beavers, Lamb and Greene
 Date 6/9/88

. .

(2-5)

165T43

(10/3)

#### STATE OF LOUISIANA SITE RECORD FORM

Site Name: Bayou Liberty State Survey No: 16ST43 Other Site Designation: Parish: St. Tammany Instructions for Reaching Site: About 880' SW from site 16ST12 in marsh by bayou USGS Quad (Name, date, series): Slidell (1950) 15' Quad No: 31-Q E quarter of the IR quarter of Section 14 Township 8S Range: 13E UTM Coordinates: Zone: 16 Easting: Northing: Geographical Coordinates: Latitude: Longitude:

#### PHYSICAL SETTING

Land Form: Buried beach ridge Geologic Processes: Site Position: In marsh by bayou Near Water: Soil Characteristics: Marsh Floral Communities: Marsh grass Faunal Communities: Other Potential Resources: Nearest Known Site: Slope:

Elev, ft AMSL:

Flooding:

#### SITE DESCRIPTION

Site Size: Plan: Orientation: Stratigraphy: Cream (cont) Artifact Density: Artifact Distribution: Cultural Features: 1/5' organic matter over shell, shell mixed with sand and about l' thick Cultural Affiliation: Prehistoric (unknown) Presumed Function: Prehistoric (unknown)

#### COLLECTIONS

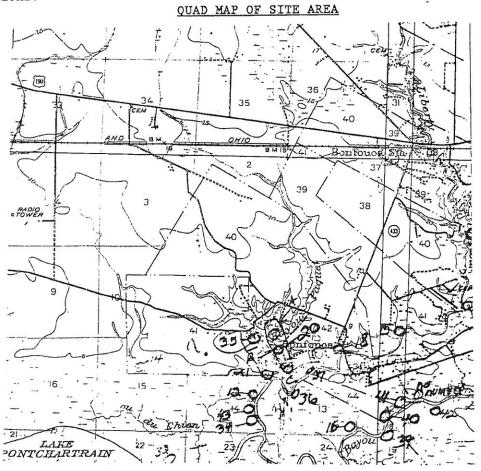
Survey Meth: Auger testing Assessment of Collecting Conditions: During extreme low water, shell visible in bank Description of Material: Shell

#### CONDITIONS

Present Use: Erosion or Disturbance: Underwater Probable Future Destruction:

#### Research Potential:

State/National Register Eligibility: Unknown Recommendations:



RECORDS

Owner and Address:

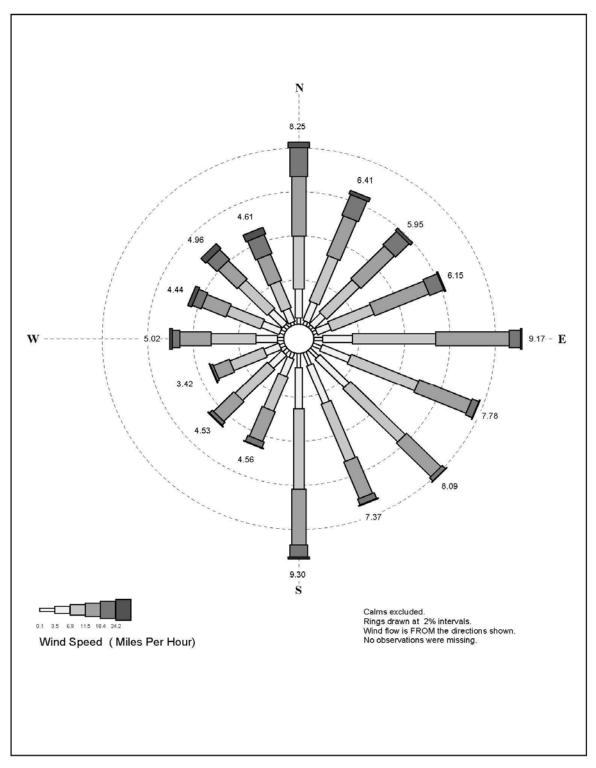
Tenant and Address:

Informants: Prev. Invest: Previous Collections and Availability: LSU Cat. No. 58-604 References: Heartfield, et al (1981); Neuman (1977) Photos and Maps: Remarks:

Recorder: Saucier & Gagliano

Date: 7/25/54

> Appendix G: Wind Roses



New Orleans Lakefront 1996-2011

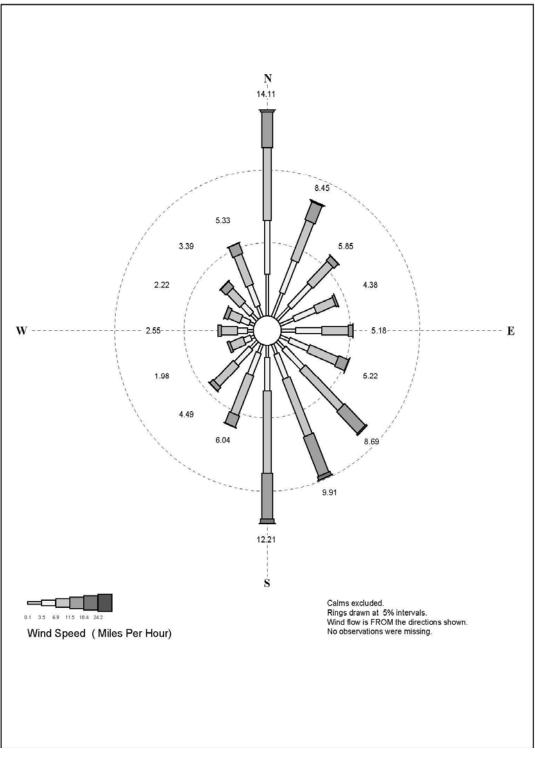


Figure 5: Slidell Airport 1999-2010

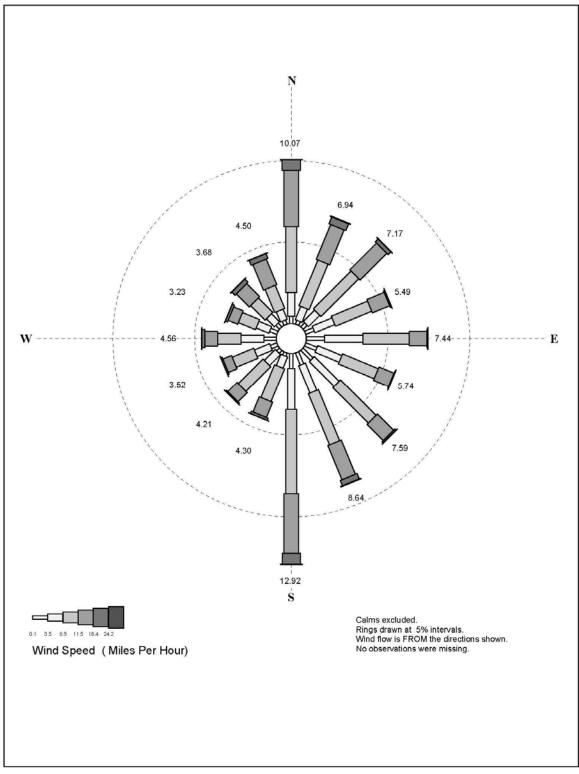


Figure 6: Louis Armstrong Airport 1996-2010

# **Appendix H: Preliminary Design Drawings**

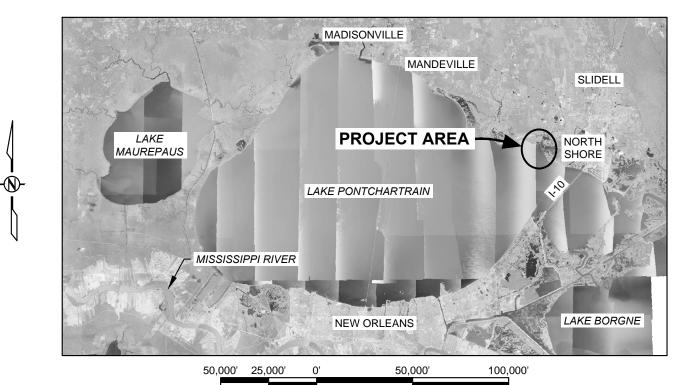
#### INDEX TO SHEETS

#### SHEET NO. DESCRIPTION

- 1 TITLE SHEET
- 2 GENERAL NOTES
- 3 PROJECT LAYOUT
- 4 BORROW AREA LAYOUT
- 5 MARSH CREATION AREA 1 LAYOUT
- 6 MARSH CREATION AREA 2 LAYOUT
- 7 MARSH CREATION AREA 3 LAYOUT
- 8 MARSH CREATION AREA 4 LAYOUT
- 9 10 TYPICAL SECTIONS
- 11 12 TYPICAL DETAILS
- 13 SURVEY LAYOUT
- 14 27 SECTIONS

# STATE OF LOUISIANA COASTAL PROTECTION AND RESTORATION AUTHORITY

# BAYOU BONFOUCA MARSH CREATION PROJECT P0-104 ST. TAMMANY PARISH



PRELIMINARY

DOCUMENTS ARE NOT TO BE USED FOR CONSTRUCTION, BIDDING, RECORDATION, CONVEYANCE, SALES, OR AS THE BASIS FOR THE ISSUANCE OF A PERMIT.

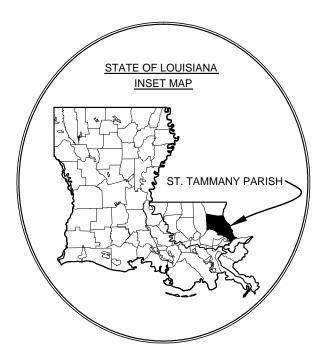




# REV. DATE DESCRIPTION BY DRAWN BY: KRISTI CANTU DESIGNED BY: JOSEPH GUILLORY, E.I.

#### LICENSURE CLASSIFICATION REQUIREMENTS

MAJOR CATEGORY: HEAVY CONSTRUCTION SUBCLASSIFICATION: DREDGING



#### RESTORATION ENGINEERING CHIEF

#### ENGINEER MANAGER

#### PROJECT ENGINEER

BAYOU BONFOUCA MARSH CREATION PROJECT	TITLE SHEET
STATE PROJECT NUMBER: PO-104	
FEDERAL PROJECT NUMBER:	DATE: APRIL 2012
APPROVED BY: SHANNON HAYNES, P.E.	SHEET 1 OF 27

# **GENERAL NOTES**

1. ALL ELEVATIONS ARE GIVEN IN THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88) U.S. SURVEY FEET. ALL HORIZONTAL COORDINATES ARE GIVEN IN THE NORTH AMERICAN DATUM OF 1983 (NAD 83, LOUISIANA STATE PLANE SOUTH ZONE U.S. FEET). ELEVATIONS ARE BASED ON THE FOLLOWING SECONDARY MONUMENT:

SECONDARY MONUMENT	ELEVATION	NORTHING	EASTING
876 1534 B TIDAL	2.60'	631,798.38	3,748,804.16

- 2. MARSH CREATION AND BORROW AREA ELEVATIONS SHOWN ON THE PLANS ARE BASED ON THE SURVEYS PERFORMED BETWEEN AUGUST 11, 2011 AND OCTOBER 11, 2011, BY C&C TECHNOLOGIES, INC. FOR CPRA.
- 3. MEAN HIGH WATER (MHW) AND MEAN LOW WATER (MLW) WERE CALCULATED FROM THE GAGE CR3667 LOCATED IN LAKE PONTCHARTRAIN. DATA FROM JANUARY 2007 TO DECEMBER 2010 WAS USED. ELEVATIONS ARE REFERENCED TO NAVD 88, US FEET. MHW=1.10' AND MLW =0.50'.
- 4. THE CONTRACTOR SHALL FOLLOW THE SPECIFIED DREDGE PIPELINE CORRIDOR AND SHALL NOT, AT ANYTIME, TRAVERSE EXISTING MARSH OR VEGETATIVE WETLANDS OUTSIDE MARSH CREATION AREAS UNLESS OTHERWISE DIRECTED BY THE CONSTRUCTION ENGINEER.
- 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR IDENTIFYING SAFE ACCESS TO THE PROJECT SITE AND FOR NAVIGATING WITHIN THE LIMITS OF THE PROJECT SITE. ALL EQUIPMENT SHALL BE FLOATING AT ALL TIMES DURING TRANSIT TO AND FROM THE PROJECT SITE. THE ENGINEER OR HIS REPRESENTATIVE SHALL MONITOR THE LOCATION OF EQUIPMENT DURING CONSTRUCTION.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING PIPELINE OPERATORS FIVE (5) WORKING DAYS IN ADVANCE OF THE WORK. ALL PIPELINES SHALL BE MARKED WITH BUOYS BY THE CONTRACTOR. THE CONTRACTOR SHALL MAINTAIN BUOYS DURING CONSTRUCTION OR HAVE ADEQUATE NAVIGATIONAL EQUIPMENT ON THE DREDGE TO AVOID DREDGING IN RESTRICTED AREAS. THE CONTRACTOR SHALL NOT ANCHOR OR EXCAVATE WITHIN 500 FEET OF ANY PIPELINE. CALL LOUISIANA ONE CALL AT 1-800-272-3020 48 HOURS PRIOR TO ANY EXCAVATION AND/OR DREDGING TO LOCATE ALL PIPELINES OR UTILITIES.

EXXONMOBILE PIPELINE COMPANY MR. DOOLEY G. OUBRE **RIGHT OF WAY & CLAIMS** P.O. BOX 1989 DENHAM SPRINGS, LA 70727 OFFICE PHONE: (225) 271-3914 CELL PHONE: (225) 715-9381 FAX: (225) 271-3848

- 7. THE CONTRACTOR SHALL CONTACT MR. DANNY BREAUX OF THE BIG BRANCH MARSH NATIONAL WILDLIFE REFUGE AT LEAST FIVE (5) WORKING DAYS PRIOR TO ACCESSING THE SITE TO OBTAIN A SPECIAL USE PERMIT. MR. BREAUX CAN BE REACHED AT (985) 882-2030.
- 8. PLANS AND SPECIFICATIONS ARE COMPLEMENTARY; WHAT IS REQUIRED BY ONE IS BINDING AS IF REQUIRED BY ALL. CLARIFICATIONS AND INTERPRETATIONS OF, OR NOTIFICATIONS OF MINOR VARIATIONS AND DEVIATIONS IN THE CONTRACT DOCUMENTS, WILL BE ISSUED IN WRITING BY THE ENGINEER.
- ANY DAMAGE TO EXISTING U.S. COAST GUARD NAVIGATION AIDS OR PRIVATE NAVIGATION AIDS SHALL BE REPAIRED BY 9. THE CONTRACTOR TO U.S. COAST GUARD STANDARDS AT THE EXPENSE OF THE CONTRACTOR.
- 10. THE MARSH CREATION AREAS AND BORROW AREA MAY BE REVISED BY ENGINEER DURING CONSTRUCTION TO REFLECT CHANGES IN FIELD CONDITIONS.
- 11. ESTIMATED HYDRAULIC DREDGING QUANTITIES SHOWN ARE FOR BIDDING PURPOSES ONLY AND WERE CALCULATED ACCORDING TO CONDITIONS SURVEYED FROM AUGUST 11, 2011 AND OCTOBER 11, 2011. QUANTITIES WERE CALCULATED USING AUTOCAD AND AERIAL PHOTOGRAPHY. THE OWNER RESERVES THE RIGHT TO ADJUST QUANTITIES HIGHER OR LOWER WITHOUT ADJUSTMENT OF THE UNIT PRICE.

- 12. THE CONTRACTOR SHALL PERFORM A MAGNETOMETER SURVEY OF THE BORROW AREA AND ALL PROPOSED ACCESS CORRIDORS PRIOR TO MOBILIZATION FOR CONSTRUCTION. DRAWINGS SHOWING THE TRACK LINES, ANY MAGNETOMETER HITS, COORDINATES, AMPLITUDE, SIGNATURE TYPE, AND SIGNATURE WIDTH OF ALL MAGNETOMETER HITS SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO MOBILIZATION. ALSO, MAGNETOMETER LINES SHALL BE RUN ALONG THE CENTERLINE ALIGNMENT OF THE PROPOSED EARTHEN CONTAINMENT DIKES BORROW AREAS. ADDITIONAL MAGNETOMETER LINES SHALL BE RUN PERPENDICULAR TO THE EARTHEN CONTAINMENT DIKE LOCATIONS. THESE LINES SHALL BEGIN AT THE PROPOSED CENTERLINE OF THE EARTHEN CONTAINMENT DIKE AND EXTEND 25' PAST THE EARTHEN CONTAINMENT DIKES BORROW PIT AND SHALL BE SPACED A MAXIMUM OF 250' APART. MAGNETOMETER LINES IN THE BORROW AREA SHALL FORM A GRID PATTERN WITH A MAXIMUM OFFSET OF 500' APART AND SHALL BE ORIENTED NORTH/SOUTH AND EAST/WEST. THE MAGNETOMETER DRAWINGS SHALL BE STAMPED BY A REGISTERED PROFESSIONAL SURVEYOR LICENSED IN LOUISIANA.
- 13. THE CONTRACTOR IS RESPONSIBLE FOR CONTAINING ALL DREDGED MATERIAL WITHIN THE BOUNDARIES OF THE PROPOSED MARSH CREATION AREAS.

# SUMMARY OF ESTIMATED QUANTITIES

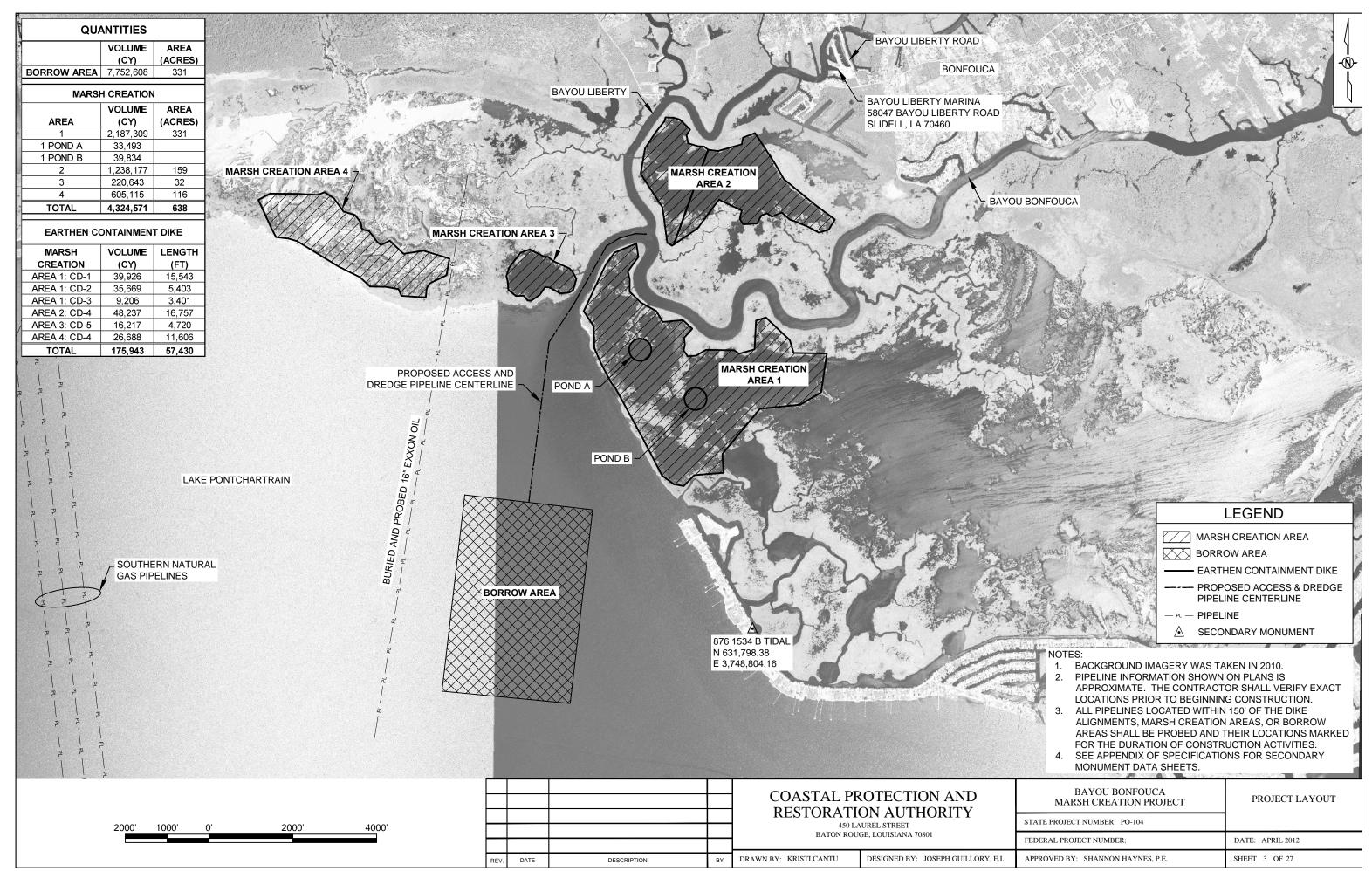
ITEM No.	DESCRIPTION	UNIT	ESTIMATED QUANTITY
1	MOBILIZATION AND DEMOBILIZATION	LUMP SUM	1
2	HYDRAULIC DREDGING (BORROW CUT VOLUME)*	CUBIC YARD	4,324,571
3	EARTHEN CONTAINMENT DIKES (CD-1)**	LINEAR FOOT	15,543
4	EARTHEN CONTAINMENT DIKES (CD-2)**	LINEAR FOOT	5,403
5	EARTHEN CONTAINMENT DIKES (CD-3)**	LINEAR FOOT	3,401
6	EARTHEN CONTAINMENT DIKES (CD-4)**	LINEAR FOOT	28,363
7	EARTHEN CONTAINMENT DIKES (CD-5)**	LINEAR FOOT	4,720
8	GRADE STAKES	EACH	113
9	SURVEYS	LUMP SUM	1
10	SETTLEMENT PLATES	EACH	8
11	GAPPING DIKES	CUBIC YARD	3,014

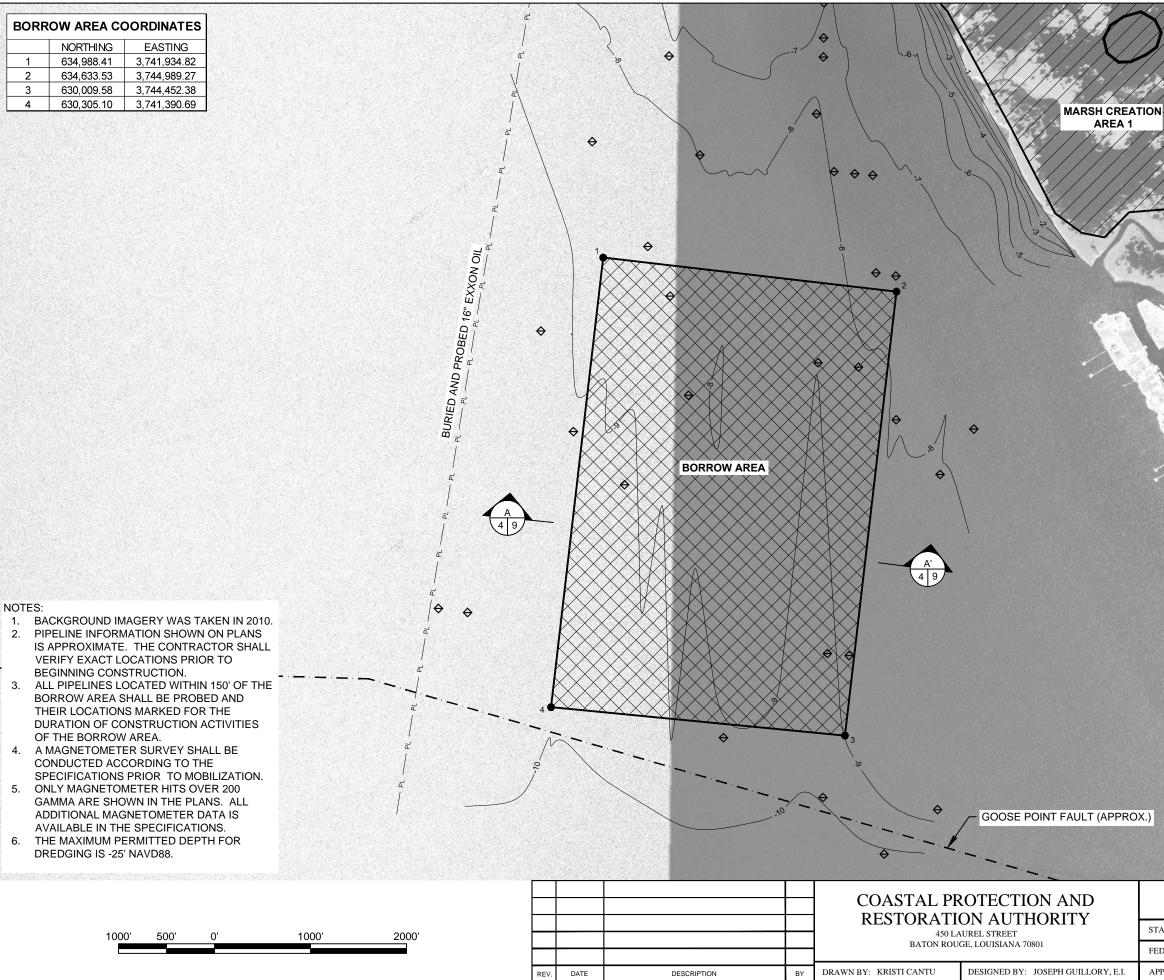
\* A SPECIAL DREDGING WINDOW EXISTS FOR THIS PROJECT TO PROTECT ENDANGERED SPECIES. THE DREDGING WINDOW FOR THIS PROJECT SHALL BE MAY 1ST TO SEPTEMBER 30TH (153 DAYS) OF A CALENDAR YEAR. NO DREDGING IS TO OCCUR OUTSIDE THIS DESIGNATED TIMEFRAME UNLESS OTHERWISE APPROVED.

\*\*THE TOTAL LINEAR FEET OF CONTAINMENT DIKES WAS ESTIMATED FOR CONSTRUCTION USING AERIAL PHOTOGRAPHY, FIELD SURVEYS, AND AUTOCAD.

				RESTORATIO 450 LAUI	DTECTION AND N AUTHORITY REL STREET , LOUISIANA 70801
REV.	DATE	DESCRIPTION	BY	DRAWN BY: KRISTI CANTU	DESIGNED BY: JOSEPH GUILLORY, E

BAYOU BONFOUCA MARSH CREATION PROJECT	GENERAL NOTES
STATE PROJECT NUMBER: PO-104	
FEDERAL PROJECT NUMBER:	DATE: APRIL 2012
APPROVED BY: SHANNON HAYNES, P.E.	SHEET 2 OF 27
APPROVED BY: SHANNON HAYNES, P.E.	SHEET 2 OF 27





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	$\boxtimes$	BORROW AREA
		EARTHEN CONTAINMENT DIKE
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22	— -8 —	CONTOUR
	$\land$	SECONDARY MONUMENT
	♦	MAGNETOMETER ANOMALY

∿

BAYOU BONFOUCA MARSH CREATION PROJECT	BORROW AREA LAYOUT	
STATE PROJECT NUMBER: PO-104		
FEDERAL PROJECT NUMBER:	DATE: APRIL 2012	
APPROVED BY: SHANNON HAYNES, P.E.	SHEET 4 OF 27	

#### EARTHEN CONTAINMENT DIKES CENTERLINE COORDINATES

				0001		
		NORTHING	EASTING		NORTHING	EASTING
-	1	638,363.12	3,750,579.49	22	640,320.23	3,745,823.95
	2	637,437.17	3,750,442.43	23	640,132.51	3,745,871.26
-3	3	637,142.34	3,749,948.11	24	639,842.16	3,745,868.03
3	4	637,045.07	3,748,980.11	25	639,581.74	3,746,133.30
	5	635,966.27	3,748,084.75	26	639,377.26	3,746,507.46
29	6	635,517.94	3,748,112.43	27	639,118.45	3,746,657.46
	7	635,460.31	3,747,413.73	28	639,159.36	3,747,140.58
1	8	635,199.15	3,747,156.99	29	638,957.46	3,747,364.00
X	9	635,246.50	3,746,919.70	30	638,740.63	3,747,172.80
	10	635,448.82	3,746,643.35	31	638,566.53	3,747,187.06
de	11	636,336.58	3,746,183.73	32	638,435.88	3,747,157.22
de	12	637,552.25	3,745,520.92	33	638,297.37	3,747,258.87
1	13	637,640.01	3,745,442.89	34	638,311.22	3,747,452.93
1	14	638,080.70	3,745,212.23	35	638,450.41	3,747,692.63
1. E.	15	638,487.06	3,745,026.45	36	638,441.09	3,748,367.94
24	16	639,015.56	3,745,011.55	37	638,357.68	3,748,482.62
-	17	639,372.85	3,744,773.18	38	638,463.54	3,748,855.39
	18	640,491.43	3,745,302.50	39	639,148.63	3,748,888.70
15	19	640,774.29	3,745,511.08	40	639,223.63	3,749,124.60
5	20	640,990.16	3,746,032.53	41	638,678.83	3,749,696.19
1	21	640,670.08	3,746,077.22	42	638,376.54	3,749,784.89
1	1523	2 2 1 C 3				

#### NOTES:

- 1. BACKGROUND IMAGERY WAS TAKEN IN 2010.
- 2. PIPELINE INFORMATION SHOWN ON PLANS IS APPROXIMATE. THE CONTRACTOR SHALL VERIFY EXACT LOCATIONS PRIOR TO BEGINNING CONSTRUCTION.
- 3. ALL PIPELINES LOCATED WITHIN 150' OF THE DIKE ALIGNMENTS, MARSH CREATION AREAS, OR BORROW AREAS SHALL BE PROBED AND THEIR LOCATIONS MARKED FOR THE DURATION OF CONSTRUCTION ACTIVITIES.
- 4. THE EARTHEN CONTAINMENT DIKES ON THE PERIMETER OF MARSH CREATION AREA 1 SHALL BE CONSTRUCTED AND MAINTAINED TO AN ELEVATION OF 3.7' NAVD88. SEE DETAILS CD-1 AND CD-2 ON SHEET 11.
- 5. THE EARTHEN CONTAINMENT DIKES ON THE PERIMETER OF DUCK PONDS A AND B SHALL BE CONSTRUCTED AND MAINTAINED TO AN ELEVATION OF 3.2' NAVD88. SEE DETAIL CD-3 ON SHEET 11.
- 6. GEOTECHNICAL PROPERTIES OF THE IN-SITU MATERIAL MAY VARY THROUGHOUT THE PROJECT SITE, AND AS A RESULT, THE GEOMETRY OF EARTHEN CONTAINMENT DIKES MAY VARY. THE CONTRACTOR SHALL FULLY EVALUATE ALL GEOTECHNICAL DATA AND CONSIDER THE COSTS ASSOCIATED WITH THE EQUIPMENT AND LABOR NECESSARY TO CONSTRUCT AND MAINTAIN THE EARTHEN CONTAINMENT DIKES TO THE REQUIRED ELEVATIONS AS SPECIFIED.
- FINAL DISCHARGE FROM DEWATERING OF FILL MATERIAL MUST NOT BE DIRECTED TO BAYOU BONFOUCA AND LAKE PONTCHARTRAIN.
- 8. INTERNAL TRAINING DIKES MAY BE USED AS NECESSARY AT ANY LOCATION WITHIN THE MARSH CREATION AREA TO PROPERLY CONTAIN AND DEWATER FILL MATERIAL AND ACHIEVE THE TARGET ELEVATION.
- 9. THE CONSTRUCTION MARSH FILL ELEVATION FOR MARSH CREATION AREA 1 SHALL BE 2.7' NAVD88 WITH A ±0.5' TOLERANCE.
- 10. A MAGNETOMETER SURVEY SHALL BE PERFORMED ON THE EARTHEN CONTAINMENT DIKE BORROW AREAS ACCORDING TO THE SPECIFICATIONS PRIOR TO MOBILIZATION.
- 11. SEE SPECIFICATIONS FOR SOIL BORING LOG INFORMATION.



										COLECTION AND
										ON AUTHORITY
600'	300'	0'	600'	1200'						AUREL STREET GE, LOUISIANA 70801
					REV.	DATE	DESCRIPTION	BY	DRAWN BY: KRISTI CANTU	DESIGNED BY: JOSEPH GUILLORY, E.I.

B8	3	B' 5 9	
	SOIL I	NORTHING	EASTING 3,745,796.16
	B6 B7	NORTHING 637,022.60 638,401.16	EASTING 3,745,796.16 3,746,372.55
6	B6 B7 B8	NORTHING 637,022.60 638,401.16 637,726.33	EASTING 3,745,796.16
6	B6 B7 B8	NORTHING 637,022.60 638,401.16 637,726.33 EGEND	EASTING 3,745,796.16 3,746,372.55 3,749,256.73
	B6 B7 B8 L ARSH CF	NORTHING 637,022.60 638,401.16 637,726.33 EGEND REATION ARE	EASTING 3,745,796.16 3,746,372.55 3,749,256.73
Ξ	B6 B7 B8 L ARSH CF	NORTHING 637,022.60 638,401.16 637,726.33 EGEND REATION ARI CONTAINME	EASTING 3,745,796.16 3,746,372.55 3,749,256.73 EA NT DIKE (CD-1)
е/ е/	B6 B7 B8 L ARSH CF ARTHEN	NORTHING 637,022.60 638,401.16 637,726.33 EGEND REATION ARI CONTAINME	EASTING 3,745,796.16 3,746,372.55 3,749,256.73
— ел — ел — ел — ел	B6 B7 B8 L ARSH CF ARTHEN	NORTHING 637,022.60 638,401.16 637,726.33 EGEND REATION ARI CONTAINME CONTAINME	EASTING 3,745,796.16 3,746,372.55 3,749,256.73 EA NT DIKE (CD-1) NT DIKE (CD-2)
E/ E/ €/ So	B6 B7 B8 L ARSH CF ARTHEN ARTHEN	NORTHING 637,022.60 638,401.16 637,726.33 EGEND REATION ARI CONTAINME CONTAINME	EASTING 3,745,796.16 3,746,372.55 3,749,256.73 EA NT DIKE (CD-1) NT DIKE (CD-2)
— ел — ел — ел — ел	B6 B7 B8 L ARSH CF ARTHEN ARTHEN	NORTHING 637,022.60 638,401.16 637,726.33 EGEND REATION ARI CONTAINME CONTAINME CONTAINME ING	EASTING 3,745,796.16 3,746,372.55 3,749,256.73 EA NT DIKE (CD-1) NT DIKE (CD-2)
E/ E/ €/ €/ BAYOU BONFOUCA	B6 B7 B8 L ARSH CF ARTHEN ARTHEN	NORTHING 637,022.60 638,401.16 637,726.33 EGEND REATION ARI CONTAINME CONTAINME CONTAINME ING MARSH AREA	EASTING 3,745,796.16 3,746,372.55 3,749,256.73 EA NT DIKE (CD-1) NT DIKE (CD-2) NT DIKE (CD-2) NT DIKE (CD-3)
BAYOU BONFOUCA MARSH CREATION PROJECT	B6 B7 B8 L ARSH CF ARTHEN ARTHEN	NORTHING 637,022.60 638,401.16 637,726.33 EGEND REATION ARI CONTAINME CONTAINME CONTAINME ING	EASTING 3,745,796.16 3,746,372.55 3,749,256.73 EA NT DIKE (CD-1) NT DIKE (CD-2) NT DIKE (CD-2) NT DIKE (CD-3)

APPROVED BY: SHANNON HAYNES, P.E.

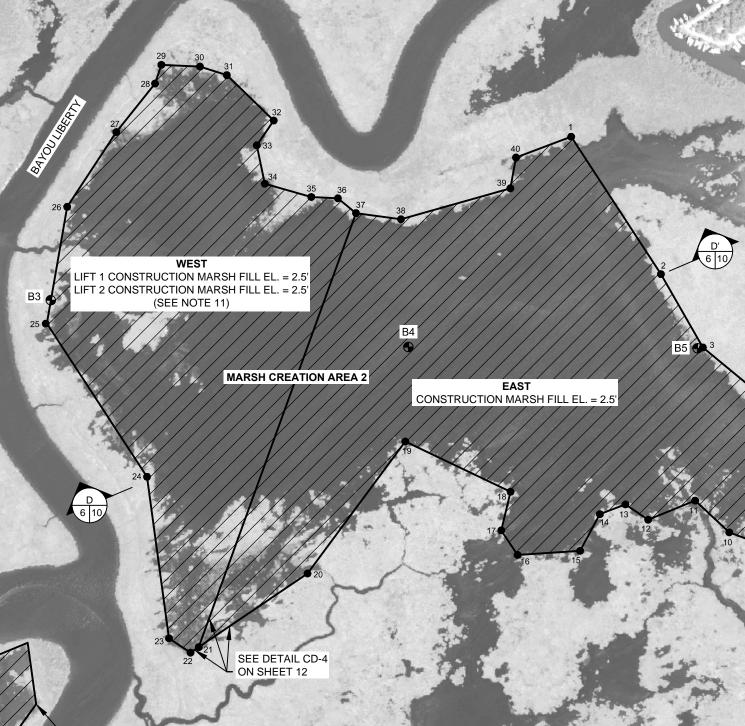
SHEET 5 OF 27

#### EARTHEN CONTAINMENT DIKE CENTERLINE COORDINATES

100		Ľ		JUUR	DINATES	
		NORTHING	EASTING		NORTHING	EASTING
	1	643,625.28	3,748,863.77	21	640,967.13	3,746,924.14
×.	2	642,909.67	3,749,331.31	22	640,937.98	3,746,881.05
	3	642,527.80	3,749,549.99	23	641,013.07	3,746,769.15
2	4	641,930.10	3,750,274.44	24	641,854.35	3,746,654.81
111	5	641,562.21	3,750,770.26	25	642,651.85	3,746,127.20
23	6	641,318.48	3,750,621.23	26	643,260.38	3,746,238.95
	7	641,248.75	3,750,384.02	27	643,649.57	3,746,494.90
1	8	641,386.50	3,750,134.79	28	643,902.55	3,746,696.36
	9	641,503.55	3,749,851.52	29	643,999.68	3,746,729.21
	10	641,564.82	3,749,687.17	30	643,990.83	3,746,930.67
	11	641,730.29	3,749,509.51	31	643,946.11	3,747,070.37
	12	641,630.55	3,749,264.50	32	643,709.68	3,747,314.56
	13	641,710.60	3,749,146.59	33	643,580.85	3,747,226.34
	14	641,659.87	3,749,013.41	34	643,380.95	3,747,267.90
der.	15	641,468.76	3,748,910.20	35	643,311.81	3,747,510.11
	16	641,447.39	3,748,584.80	36	643,304.77	3,747,648.69
2	17	641,575.28	3,748,499.48	37	643,227.76	3,747,742.96
14	18	641,776.26	3,748,548.23	38	643,195.59	3,747,977.36
S. F. m	19	642,038.14	3,747,999.70	39	643,356.82	3,748,547.15
200	20	641,350.42	3,747,490.71	40	643,517.54	3,748,576.39
7.2	173			all plays		

NOTES:

- 1. BACKGROUND IMAGERY WAS TAKEN IN 2010.
- 2. PIPELINE INFORMATION SHOWN ON PLANS IS APPROXIMATE. THE CONTRACTOR SHALL VERIFY EXACT LOCATIONS PRIOR TO BEGINNING CONSTRUCTION.
- 3. ALL PIPELINES LOCATED WITHIN 150' OF THE DIKE ALIGNMENTS, MARSH CREATION AREAS, OR BORROW AREAS SHALL BE PROBED AND THEIR LOCATIONS MARKED FOR THE DURATION OF CONSTRUCTION ACTIVITIES.
- 4. GEOTECHNICAL PROPERTIES OF THE IN-SITU MATERIAL MAY VARY THROUGHOUT THE PROJECT SITE, AND AS A RESULT, THE GEOMETRY OF EARTHEN CONTAINMENT DIKES MAY VARY. THE CONTRACTOR SHALL FULLY EVALUATE ALL GEOTECHNICAL DATA AND CONSIDER THE COSTS ASSOCIATED WITH THE EQUIPMENT AND LABOR NECESSARY TO CONSTRUCT AND MAINTAIN THE EARTHEN CONTAINMENT DIKES TO THE REQUIRED ELEVATIONS AS SPECIFIED.
- 5. THE EARTHEN CONTAINMENT DIKES SHALL BE CONSTRUCTED AND MAINTAINED TO AN ELEVATION OF 3.5' NAVD88. SEE DETAIL CD-4 ON SHEET 12.
- 6. THE CONSTRUCTION MARSH FILL ELEVATION FOR MARSH CREATION AREA 2 SHALL BE 2.5' NAVD88 WITH A ±0.5 TOLERANCE.
- 7. FINAL DISCHARGE FROM DEWATERING OF FILL MATERIAL MUST NOT BE DIRECTED TO BAYOU BONFOUCA AND LAKE PONTCHARTRAIN.
- 8. INTERNAL TRAINING DIKES MAY BE USED AS NECESSARY AT ANY LOCATION WITHIN THE MARSH CREATION AREA TO PROPERLY CONTAIN AND DEWATER FILL MATERIAL AND ACHIEVE THE TARGET ELEVATION.
- 9. A MAGNETOMETER SURVEY SHALL BE PERFORMED ON THE EARTHEN CONTAINMENT DIKE BORROW AREAS ACCORDING TO THE SPECIFICATIONS PRIOR TO MOBILIZATION.
- SEE SPECIFICATIONS FOR SOIL BORING LOG INFORMATION.
   MINIMUM RETENTION AND DEWATERING TIME IN BETWEEN LIFTS
- IN MARSH CREATION AREA 2 WEST IS 30 DAYS.



MARSH CREATION AREA 1

500' 250' 0' 500' 1000'					RESTORATIO	ROTECTION AND ON AUTHORITY AUREL STREET
					BATON ROU	GE, LOUISIANA 70801
					DRAMMA DV - KDIGTI CANTU	
	REV.	DATE	DESCRIPTION	BY	DRAWN BY: KRISTI CANTU	DESIGNED BY: JOSEPH GUILLORY, E.I.

l	Contraction of the second	ORDINATES	BORING CC	SOIL
-	and and	EASTING	NORTHING	
		3,746,154.06	642,774.12	B3
	E.	3,748,016.03	642,529.59	B4
		3,749,522.16	642,523.22	B5
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LEGEND	
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/	MARSH CREATION AREA	1
_	EARTHEN CONTAINMENT DIKE (CD-4)	
	SOIL BORING	

BAYOU BONFOUCA MARSH CREATION PROJECT	MARSH CREATION AREA 2 LAYOUT
STATE PROJECT NUMBER: PO-104	
FEDERAL PROJECT NUMBER:	DATE: APRIL 2012
APPROVED BY: SHANNON HAYNES, P.E.	SHEET 6 OF 27

### EARTHEN CONTAINMENT DIKE CENTERLINE COORDINATES

		NORTHING	EASTING		NORTHING	EASTING
1000	1	640,848.45	3,743,481.33	12	639,946.11	3,743,850.76
Marrie 1	2	640,684.33	3,743,813.99	13	639,763.22	3,743,724.28
	3	640,670.73	3,743,923.71	14	639,619.21	3,743,515.55
	4	640,566.79	3,744,052.60	15	639,629.07	3,743,430.26
	5	640,397.78	3,744,426.55	16	639,743.80	3,743,341.42
	6	640,335.29	3,744,494.27	17	639,737.93	3,743,157.47
	7	640,147.72	3,744,596.98	18	639,882.64	3,742,975.47
	8	639,875.21	3,744,522.60	19	640,080.14	3,742,961.77
1	9	639,821.31	3,744,426.17	20	640,206.54	3,742,926.05
	10	639,795.50	3,744,216.27	21	640,417.45	3,742,993.52
č	11	639,942.87	3,743,958.79	22	640,767.02	3,743,265.51
i	Section States	and the second second	AND REAL PROPERTY OF			

#### NOTES:

- 1. BACKGROUND IMAGERY WAS TAKEN IN 2010.
- 2. PIPELINE INFORMATION SHOWN ON PLANS IS APPROXIMATE. THE CONTRACTOR SHALL VERIFY EXACT LOCATIONS PRIOR TO BEGINNING CONSTRUCTION.
- 3. ALL PIPELINES LOCATED WITHIN 150' OF THE DIKE ALIGNMENTS, MARSH CREATION AREAS, OR BORROW AREA SHALL BE PROBED AND THEIR LOCATIONS MARKED FOR THE DURATION OF CONSTRUCTION ACTIVITIES.
- 4. GEOTECHNICAL PROPERTIES OF THE IN-SITU MATERIAL MAY VARY THROUGHOUT THE PROJECT SITE, AND AS A RESULT, THE GEOMETRY OF EARTHEN CONTAINMENT DIKES MAY VARY. THE CONTRACTOR SHALL FULLY EVALUATE ALL GEOTECHNICAL DATA AND CONSIDER THE COSTS ASSOCIATED WITH THE EQUIPMENT AND LABOR NECESSARY TO CONSTRUCT AND MAINTAIN THE EARTHEN CONTAINMENT DIKES TO THE REQUIRED ELEVATIONS AS SPECIFIED.
- THE EARTHEN CONTAINMENT DIKES SHALL BE 5. CONSTRUCTED AND MAINTAINED TO AN ELEVATION OF 4.0' NAVD88. SEE DETAIL CD-5 ON SHEET 12.
- THE CONSTRUCTION MARSH FILL ELEVATION 6. FOR MARSH CREATION AREA 3 SHALL BE 3.5' NAVD88 WITH A ±0.5' TOLERANCE.
- 7. FINAL DISCHARGE FROM DEWATERING OF FILL MATERIAL MUST NOT BE DIRECTED TO BAYOU BONFOUCA AND LAKE PONTCHARTRAIN.
- 8. INTERNAL TRAINING DIKES MAY BE USED AS NECESSARY AT ANY LOCATION WITHIN THE FILL CELL TO PROPERLY CONTAIN AND DEWATER FILL MATERIAL AND ACHIEVE THE TARGET ELEVATION.
- 9. A MAGNETOMETER SURVEY SHALL BE PERFORMED ON THE EARTHEN CONTAINMENT DIKE BORROW AREAS ACCORDING TO THE SPECIFICATIONS PRIOR TO MOBILIZATION.
- 10. SEE SPECIFICATIONS FOR SOIL BORING LOG INFORMATION.

**MARSH CREATION AREA 3** CONSTRUCTION MARSH FILL EL. = 3.0'

# B2

LAKE PONTCHARTRAIN

200' 100' 0' 200' 400'					RESTORATIO 450 LA	COTECTION AND ON AUTHORITY UREL STREET GE, LOUISIANA 70801
	REV.	DATE	DESCRIPTION	BY	DRAWN BY: KRISTI CANTU	DESIGNED BY: JOSEPH GUILLORY, E.I.

s	OIL BORI	NG COO	RDINATES
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•	SOIL BORI	NG	
BAYOU BONFOUCA	and the second second		
MARSH CREATION PROJE	СТ		RSH CREATIC EA 3 LAYOU
ATE PROJECT NUMBER: PO-104			LA J LA IOU
ERAL PROJECT NUMBER:		DATE: AI	PRIL 2012

APPROVED BY: SHANNON HAYNES, P.E.

SHEET 7 OF 27

EARTHEN CONTAINMENT DIKE **CENTERLINE COORDINATES** 

						1200
	NORTHING	EASTING		NORTHING	EASTING	. 4
1	640,630.79	3,741,568.30	18	642,179.27	3,737,284.13	£.,
2	640,234.59	3,741,501.52	19	642,173.03	3,737,929.37	í.
3	640,046.65	3,741,112.65	20	642,148.61	3,738,429.79	
4	639,980.19	3,741,054.40	21	642,213.16	3,738,588.43	
5	639,902.59	3,741,022.63	22	642,063.92	3,738,812.36	
6	639,698.75	3,741,004.60	23	641,749.01	3,739,040.53	
7	639,740.81	3,740,422.93	24	641,670.66	3,739,314.72	
8	639,904.87	3,740,352.33	25	641,489.30	3,739,457.10	
9	639,955.55	3,740,041.34	26	641,354.15	3,739,471.06	
10	639,961.92	3,739,672.64	27	641,315.04	3,739,538.53	82
11	640,186.05	3,739,430.13	28	641,290.14	3,739,646.90	10
12	640,436.40	3,738,926.02	29	641,022.88	3,739,849.20	55
13	641,047.50	3,737,928.91	30	640,953.88	3,739,934.31	
14	641,106.92	3,737,726.90	31	641,056.25	3,740,143.34	
15	641,143.27	3,737,427.78	32	640,742.10	3,740,473.22	
16	641,355.54	3,737,317.04	33	640,818.58	3,740,939.19	28
17	642,030.79	3,737,006.95				100

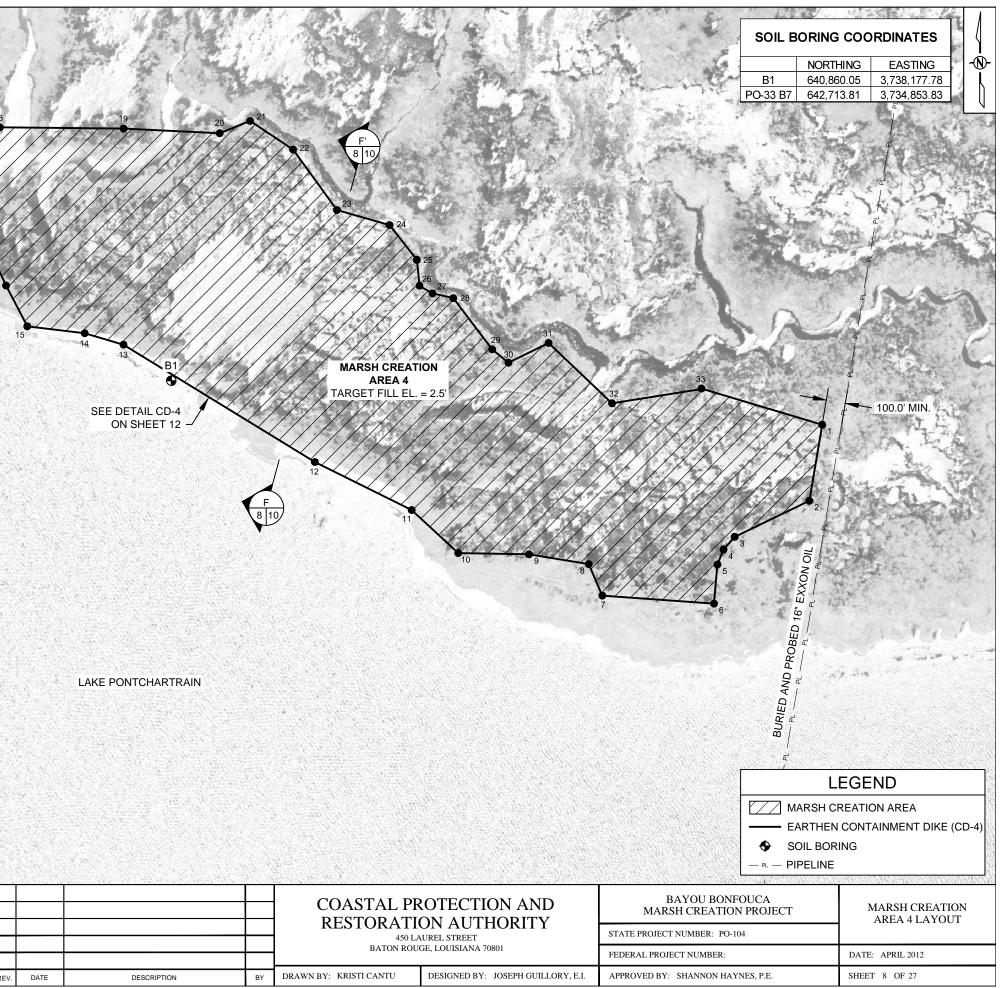


PO-33 6

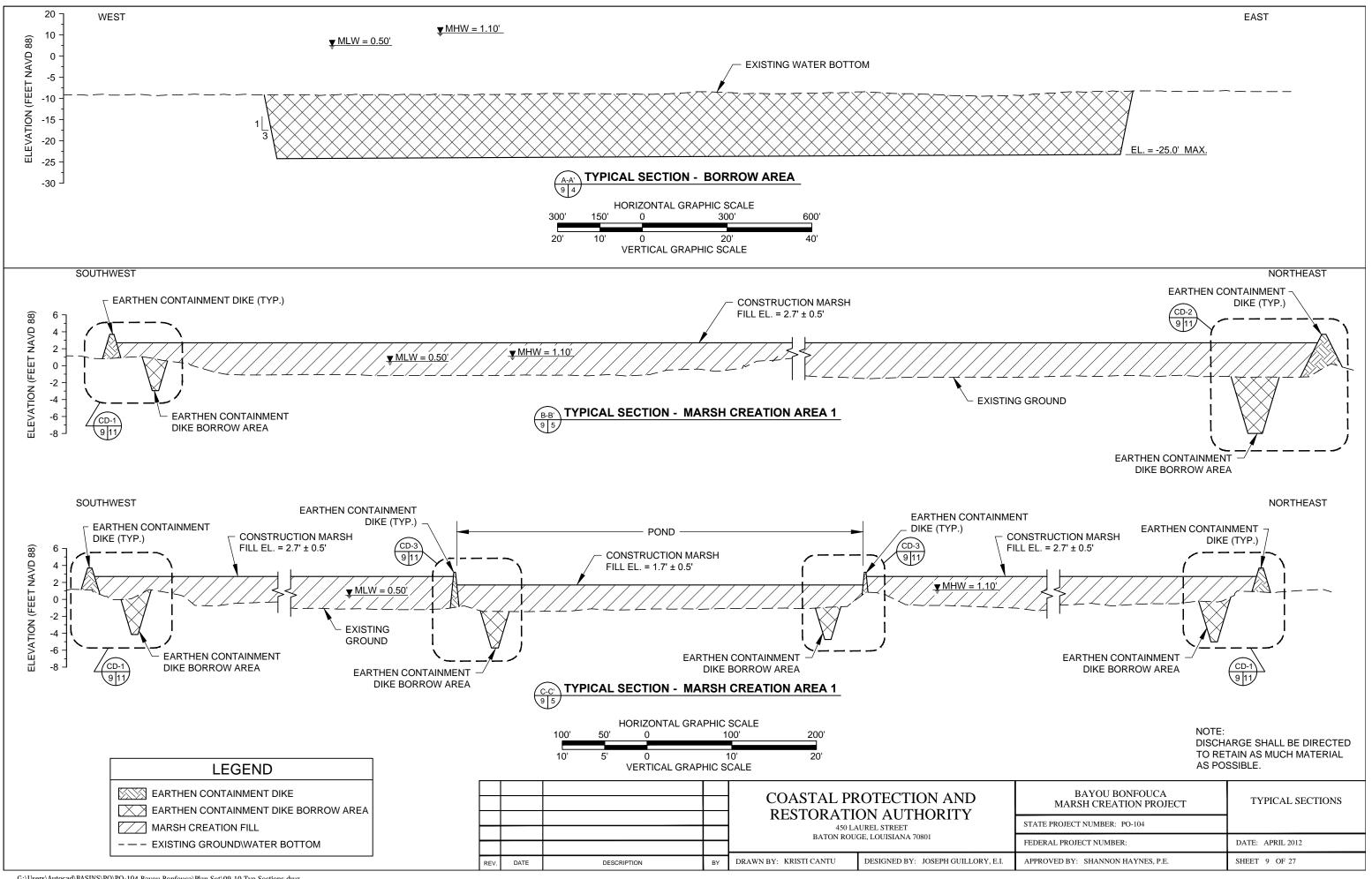
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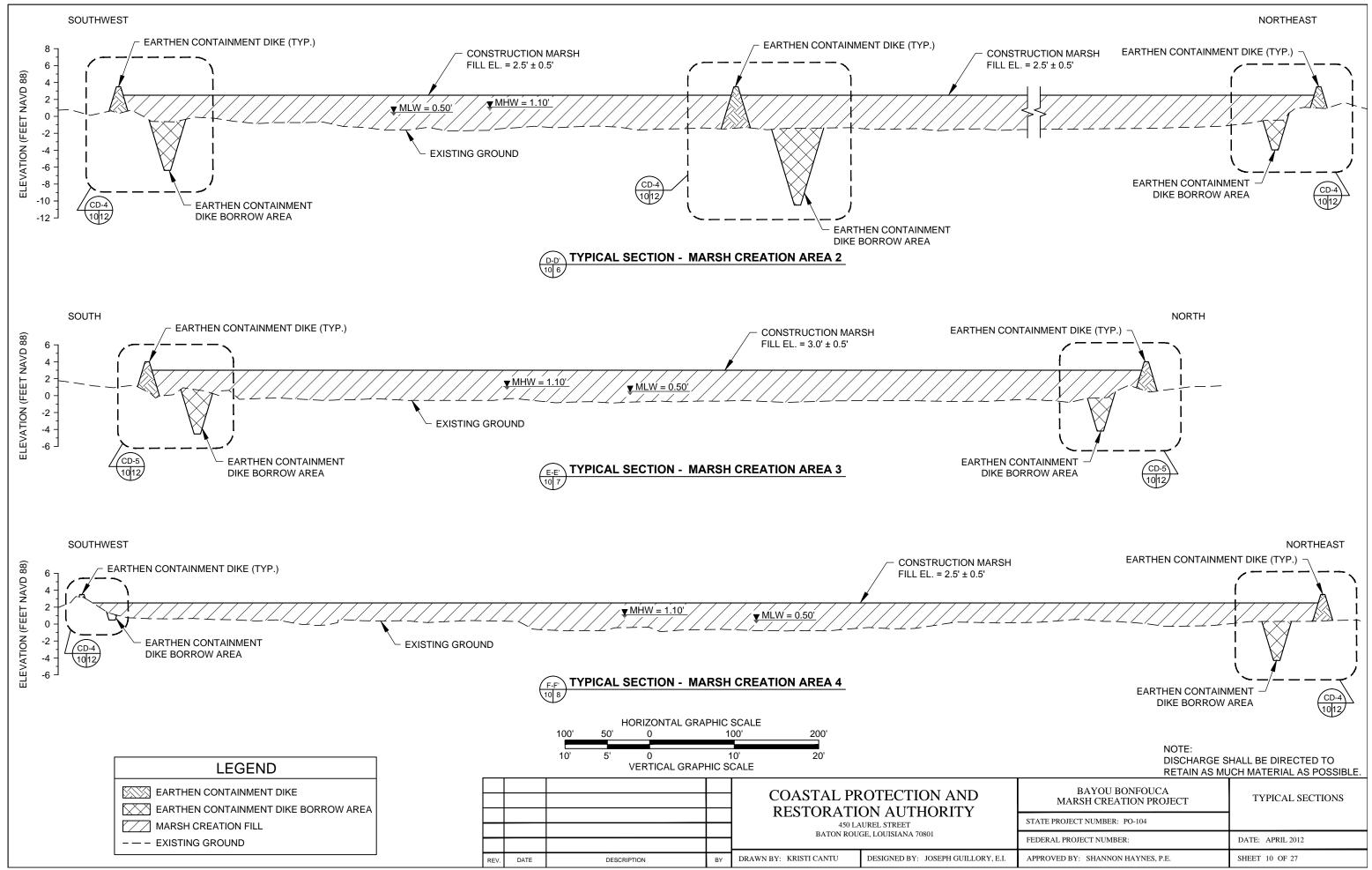
- 2. PIPELINE INFORMATION SHOWN ON PLANS IS APPROXIMATE. THE CONTRACTOR SHALL VERIFY EXACT LOCATIONS PRIOR TO **BEGINNING CONSTRUCTION.**
- ALL PIPELINES LOCATED WITHIN 150' OF THE DIKE ALIGNMENTS, 3. MARSH CREATION AREAS, OR BORROW AREAS SHALL BE PROBED AND THEIR LOCATIONS MARKED FOR THE DURATION OF CONSTRUCTION ACTIVITIES.
- GEOTECHNICAL PROPERTIES OF THE IN-SITU MATERIAL MAY 4. VARY THROUGHOUT THE PROJECT SITE, AND AS A RESULT, THE GEOMETRY OF EARTEN CONTAINMENT DIKES MAY VARY. THE CONTRACTOR SHALL FULLY EVALUATE ALL GEOTECHNICAL DATA AND CONSIDER THE COSTS ASSOCIATED WITH THE EQUIPMENT AND LABOR NECESSARY TO CONSTRUCT AND MAINTAIN THE EARTHEN CONTAINMENT DIKES TO THE REQUIRED ELEVATIONS AS SPECIFIED.
- THE EARTHEN CONTAINMENT DIKES SHALL BE CONSTRUCTED 5. AND MAINTAINED TO AN ELEVATION OF 3.5' NAVD88. SEE DETAIL CD-4 ON SHEET 12.
- THE CONSTRUCTION MARSH FILL ELEVATION FOR MARSH 6. CREATION AREA 4 SHALL BE 2.5' NAVD88 WITH A ±0.5' TOLERANCE.
- 7. FINAL DISCHARGE FROM DEWATERING OF FILL MATERIAL MUST NOT BE DIRECTED TO BAYOU BONFOUCA AND LAKE PONTCHARTRAIN.
- 8. INTERNAL TRAINING DIKES MAY BE USED AS NECESSARY AT ANY LOCATION WITHIN THE FILL CELL TO PROPERLY CONTAIN AND DEWATER FILL MATERIAL AND ACHIEVE THE TARGET ELEVATION.
- A MAGNETOMETER SURVEY SHALL BE PERFORMED ON THE 9. EARTHEN CONTAINMENT DIKE BORROW AREAS ACCORDING TO THE SPECIFICATIONS PRIOR TO MOBILIZATION.
- 10. SEE SPECIFICATIONS FOR SOIL BORING LOG INFORMATION.



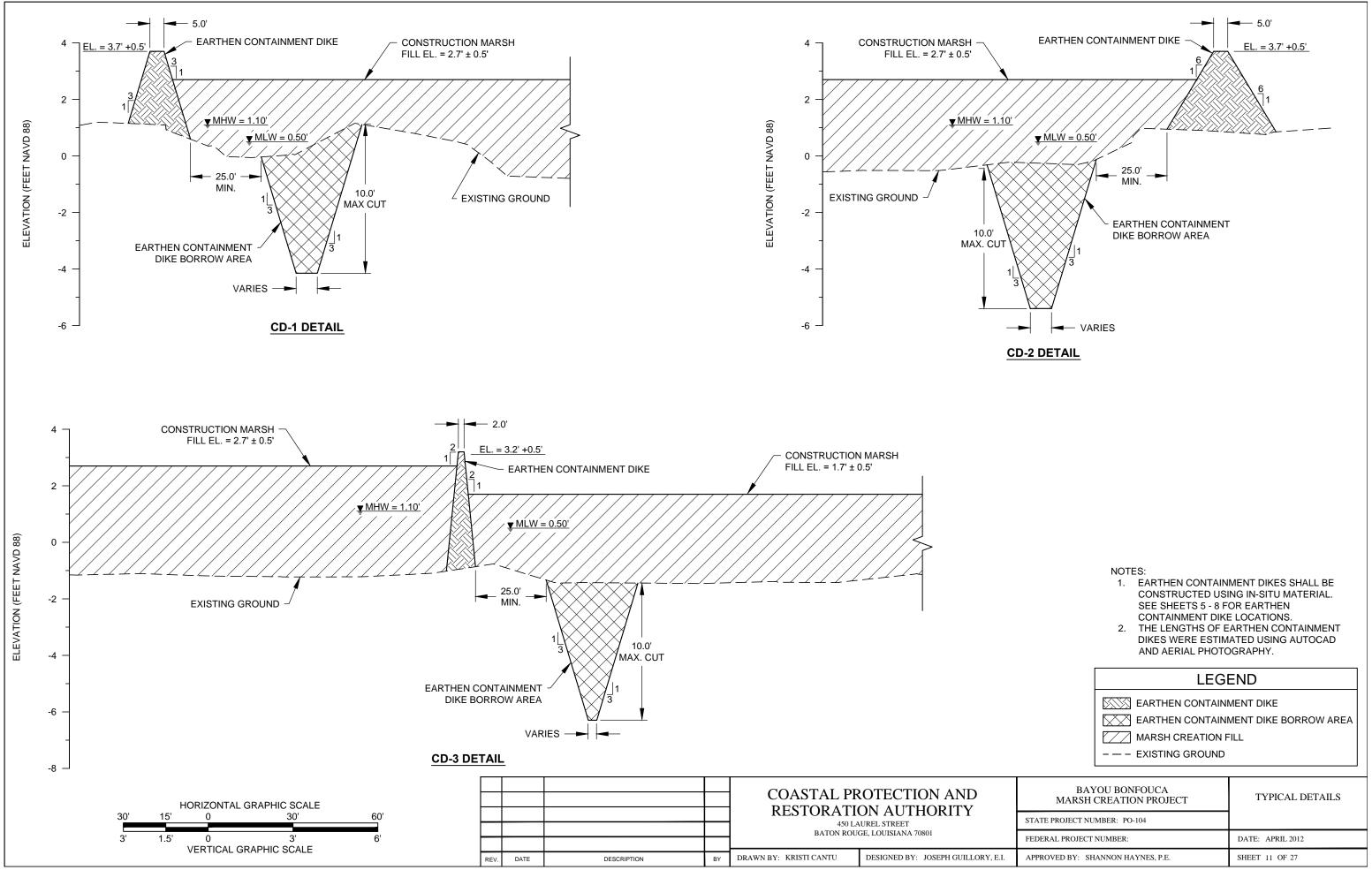
					COASTAL PROTECTION	
						ON AUTHORITY
500' 250' 0' 500' 1000'				1	450 LAUREL STREET BATON ROUGE, LOUISIANA 70801	
	REV.	DATE	DESCRIPTION	BY	DRAWN BY: KRISTI CANTU	DESIGNED BY: JOSEPH GUILLORY, E.I.



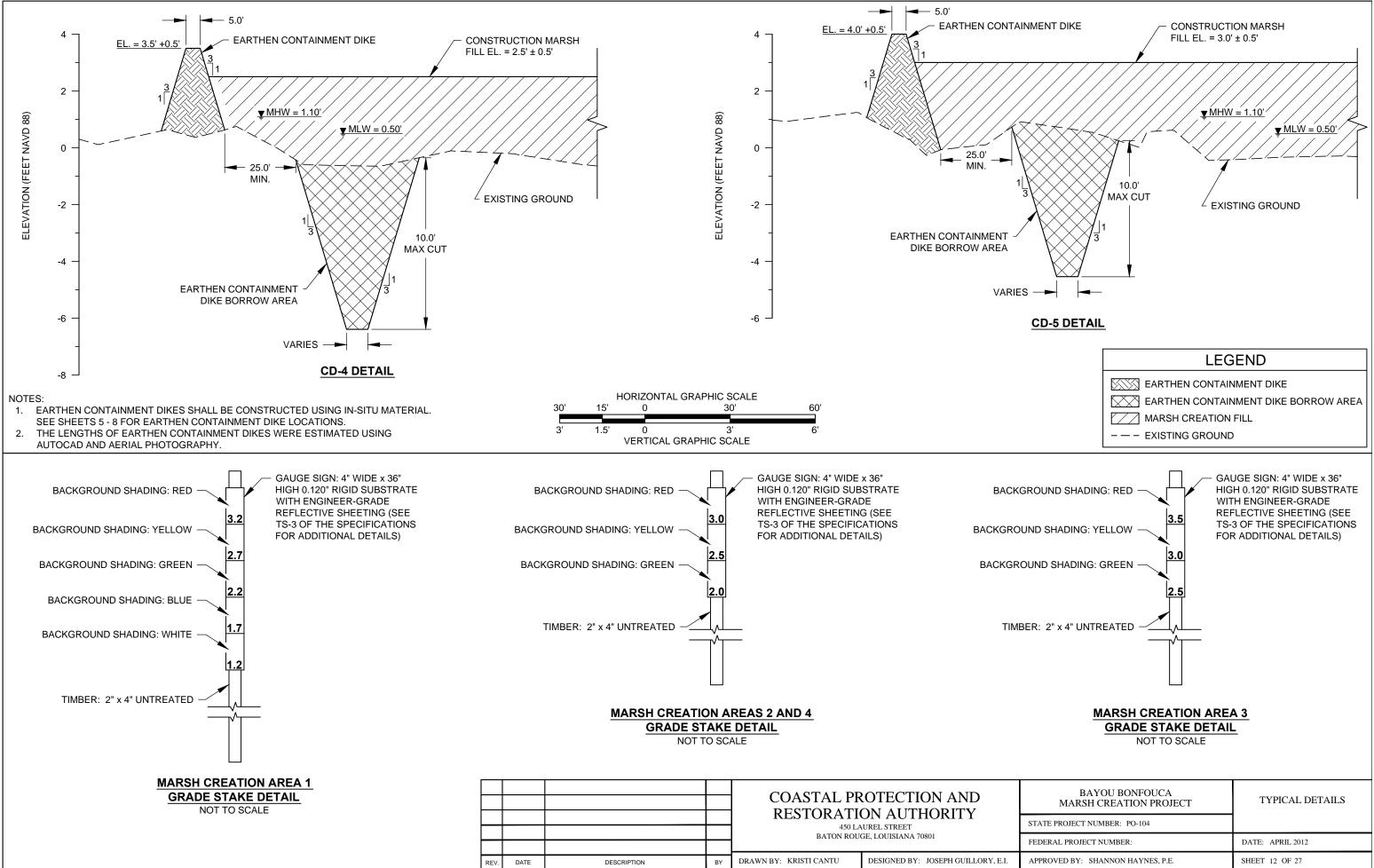
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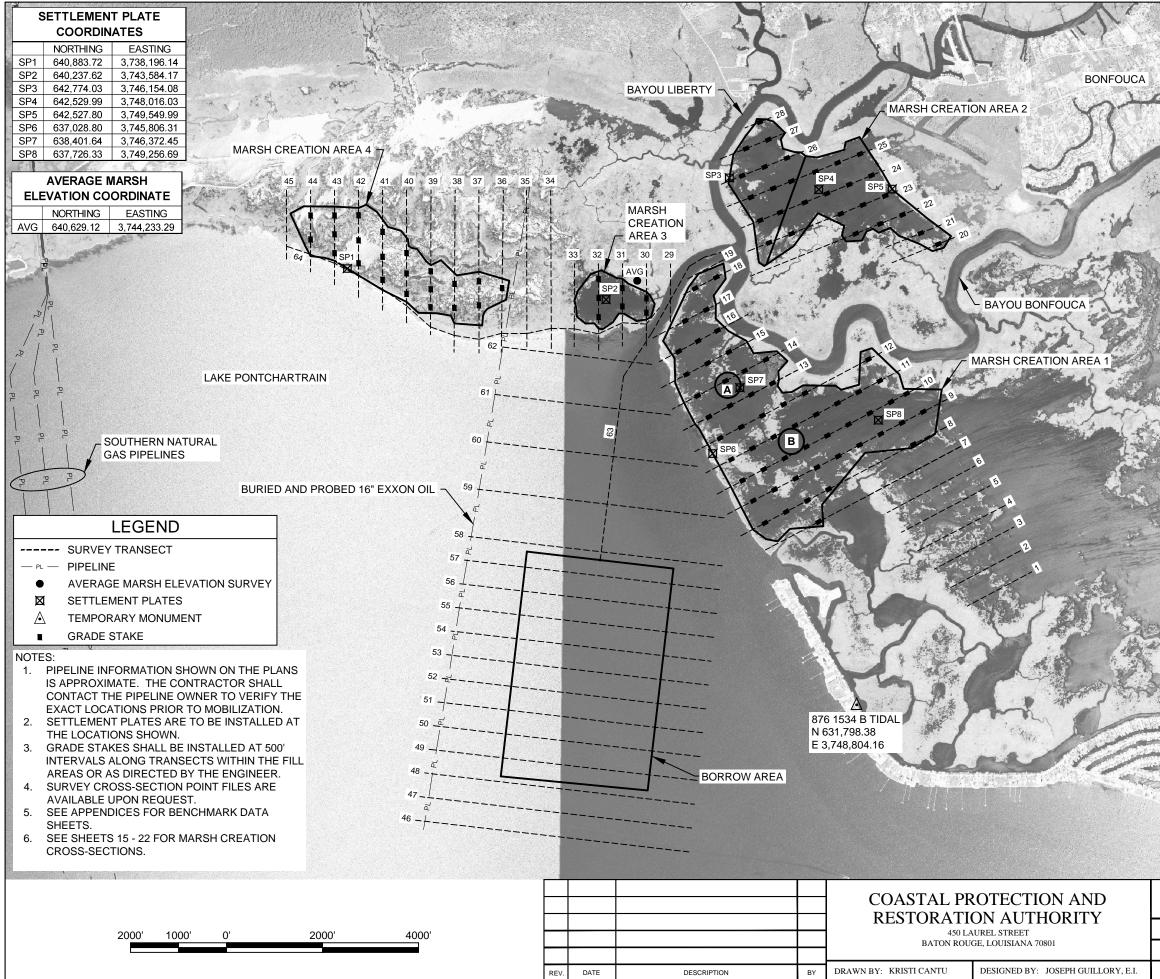


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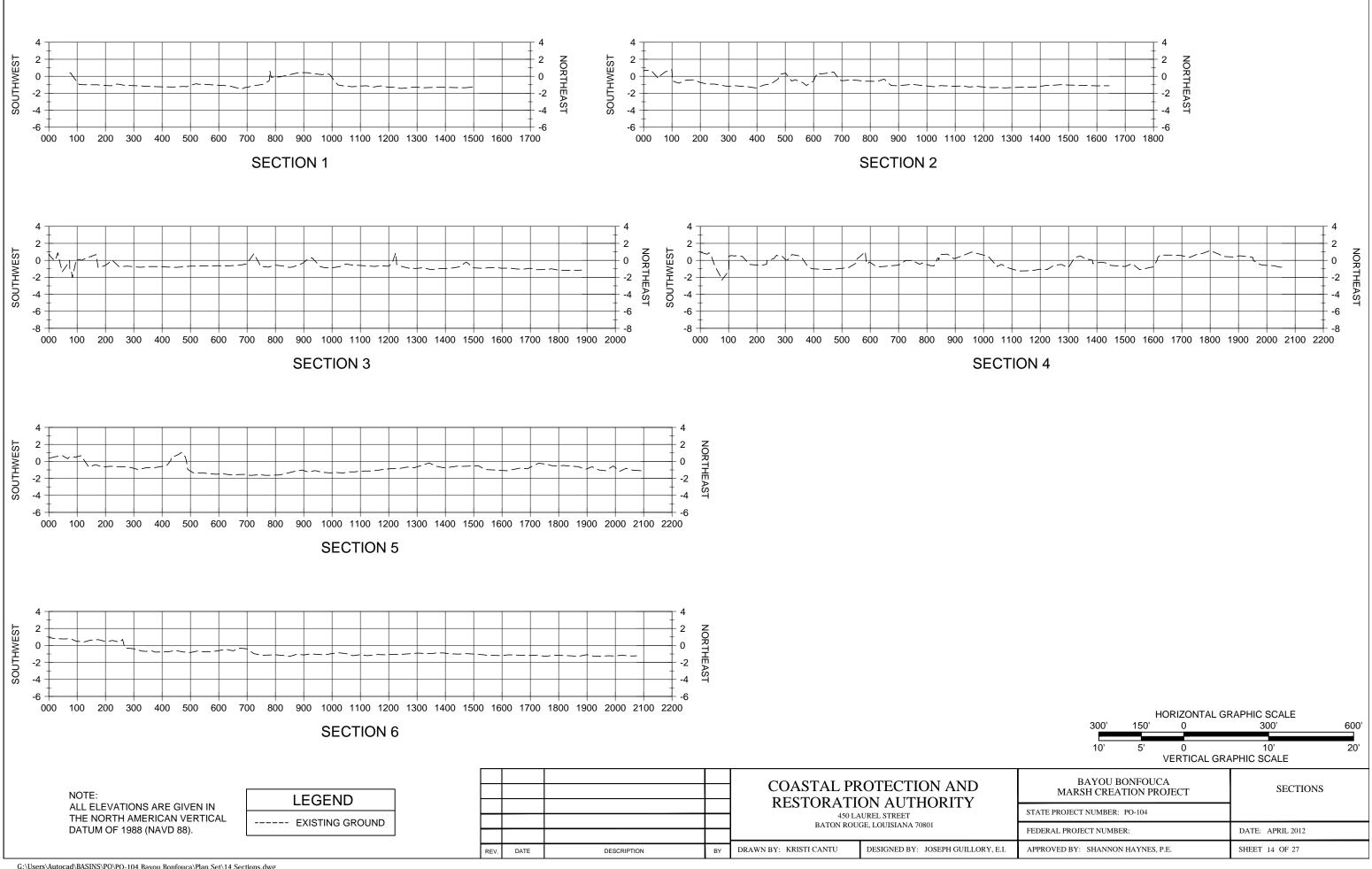
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BAYOU BONFOUCA MARSH CREATION PROJECT	TYPICAL DETAILS
STATE PROJECT NUMBER: PO-104	
FEDERAL PROJECT NUMBER:	DATE: APRIL 2012
APPROVED BY: SHANNON HAYNES, P.E.	SHEET 12 OF 27

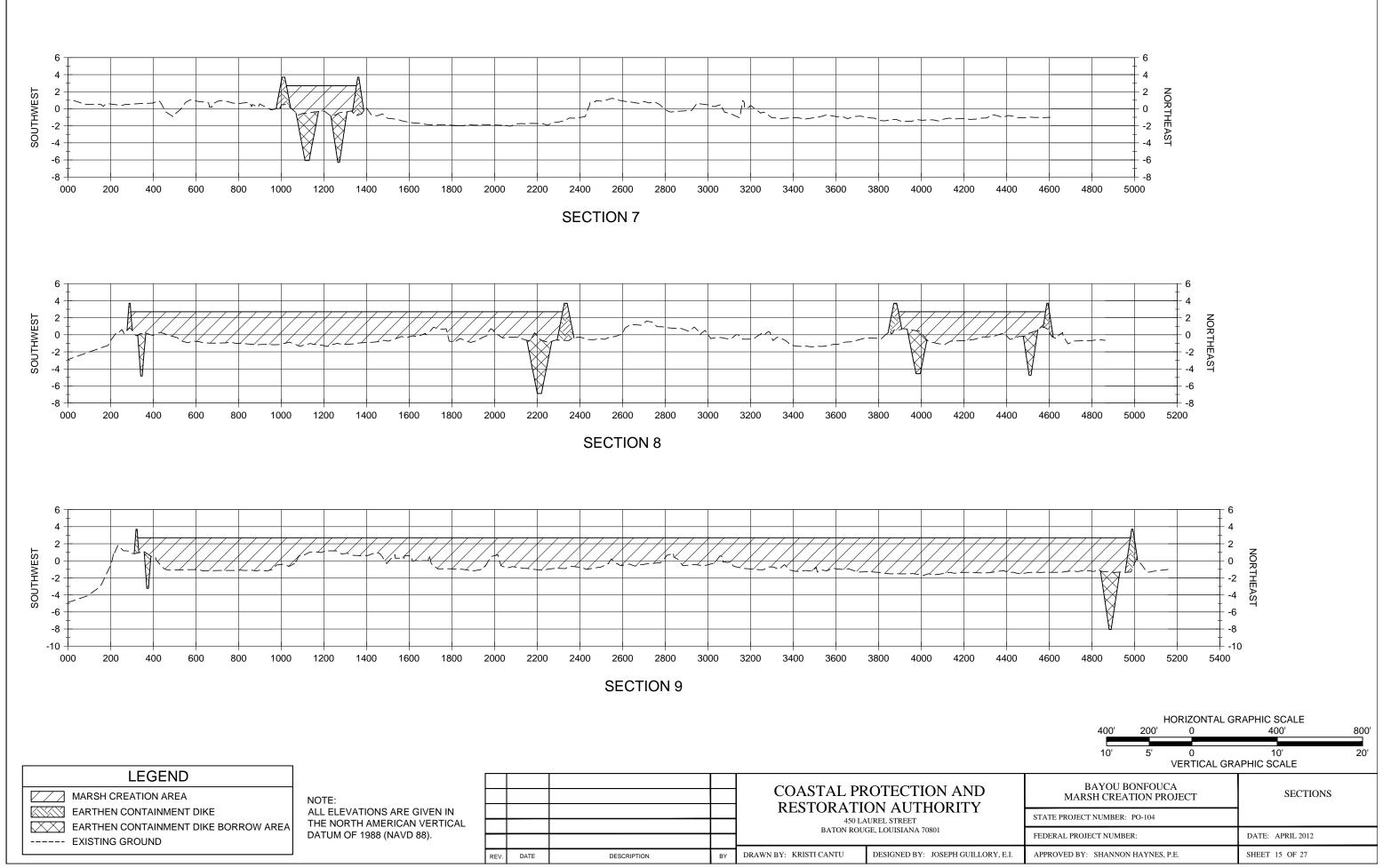


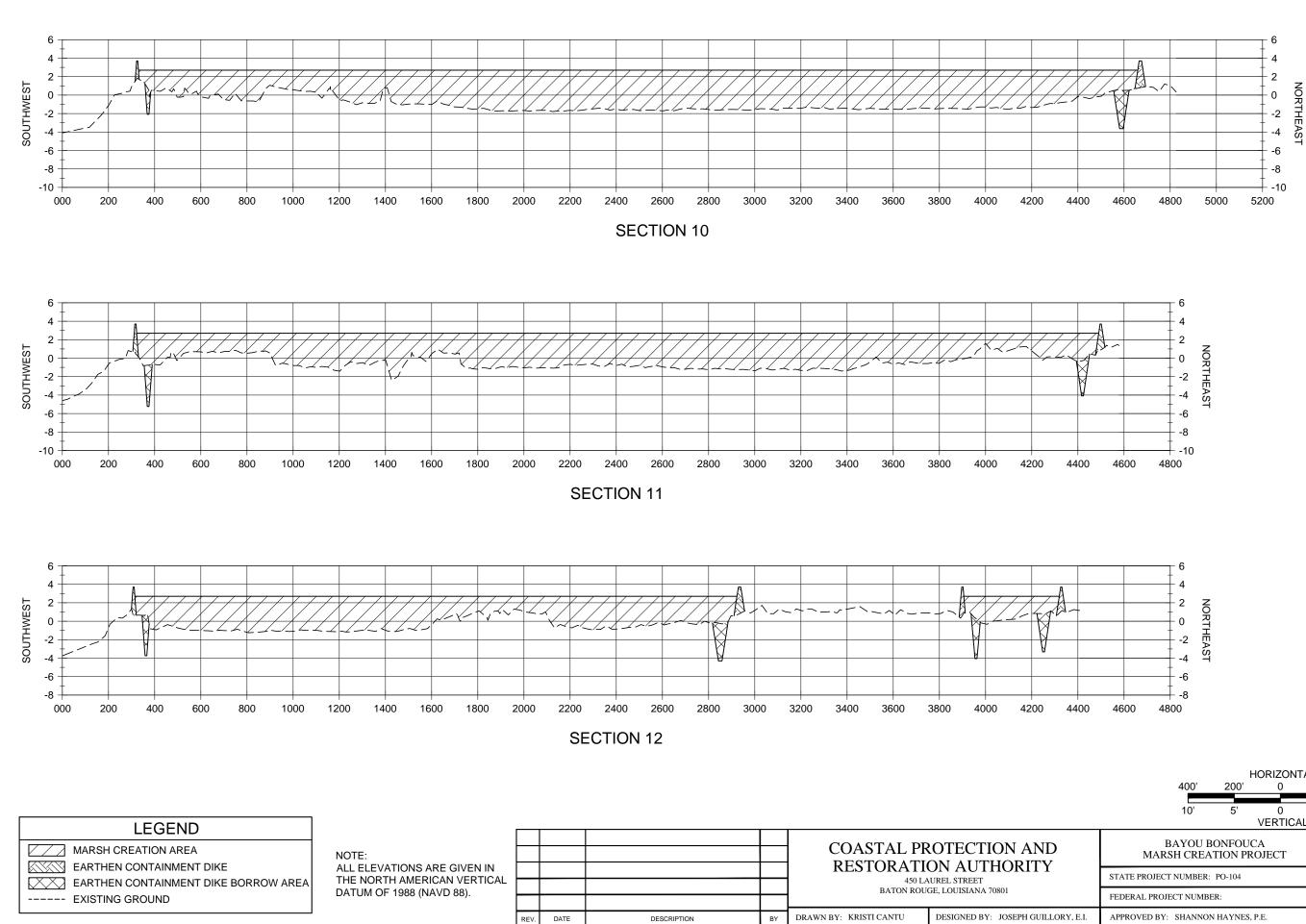
G:\Users\Autocad\BASINS\PO\PO-104 Bayou Bonfouca\Plan Set\13 survey layout.dwg

	TRANSEC			T COORDINATES			
				_			
	1	NORTHING 633,852.11	EASTING 3,751,108.4	0	NORTHING 634,563.35	EASTING 3,752,451.94	
-®-	2	634,236.24	3,750,765.3	5	635,005.25	3,752,218.01	
	3	634,620.38	3,750,422.3		635,500.53	3,752,084.92 3,751,902.32	
~2/	4 5	635,003.23 635,389.00	3,750,080.4 3,749,738.2		635,949.84 636,360.79	3,751,589.77	
	6	635,799.86	3,749,443.7	5	636,772.98	3,751,276.28	
	7	635,019.42	3,746,901.3		637,173.17	3,750,971.92	
Num	8 9	635,315.12 635,767.89	3,746,387.7 3,746,116.6		637,589.37 638,158.39	3,750,685.58 3,750,686.71	
	10	636,184.96	3,745,897.4	1	638,445.06	3,750,161.16	
in the state	11	636,627.86	3,745,664.6		638,767.86	3,749,711.49	
the prove	12 13	637,069.97 637,512.93	3,745,432.2		639,136.42 638,768.58	3,749,324.80 3,747,571.43	
al rai	14	637,955.53	3,744,966.8		639,186.61	3,747,292.42	
	15	638,398.13	3,744,734.2		639,419.56	3,746,663.78	
and the second second	16 17	638,944.40 639,438.42	3,744,698.3 3,744,562.0		639,927.88 640,175.85	3,746,539.67 3,745,955.04	
10	18	640,342.73	3,745,201.6		640,858.75	3,746,176.36	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	19	640,668.91	3,745,283.4 3,749,539.9		641,067.77	3,746,036.85	
The .	20 21	641,002.38 641,323.58	3,749,539.9		641,529.41 641,793.35	3,750,839.71 3,750,568.60	
	22	640,734.90	3,746,617.0		642,141.73	3,750,100.73	
	23	641,242.34	3,746,538.3		642,504.18	3,749,662.96	
H AND .	24 25	641,726.81 642,175.82	3,746,402.6 3,746,115.5		642,931.33 643,367.31	3,749,385.43 3,749,120.26	
	26	642,641.36	3,745,984.7		643,325.77	3,747,697.23	
1 DEN SAME	27	643,165.43	3,745,959.2		643,717.07	3,747,334.43	
Start Contract	28 29	644,029.97 639,629.98	3,747,019.8 3,744,923.8		643,812.37 640,981.97	3,746,477.35 3,744,927.73	
	30	639,268.94	3,744,923.8		640,983.39	3,744,927.73	
	31	639,270.36	3,743,922.8		640,984.82	3,743,927.74	
	32 33	639,182.54 639,252.80	3,743,430.4 3,742,920.4		640,984.82 640,984.82	3,743,427.74 3,742,927.74	
Monthand His	34	639,217.67	3,742,445.5		642,517.16	3,742,430.75	
V. Marker Const	35	639,252.80	3,741,935.5	8	642,517.16	3,741,929.03	
the state of the	36 37	639,142.05 639,124.19	3,741,401.8		642,517.16	3,741,444.96	
Carles Martin	38	639,124.19	3,740,936.8 3,740,418.2		642,517.16 642,517.16	3,740,933.71 3,740,439.69	
and the state	39	639,505.39	3,739,927.7		642,517.16	3,739,936.99	
	40	639,752.39	3,739,427.7		642,517.16	3,739,427.74	
Press and	41 42	640,017.79 640,283.20	3,738,927.7 3,738,427.7		642,517.16 642,517.16	3,738,927.74 3,738,427.74	
R incontract	43	640,548.60	3,737,927.7	4	642,517.16	3,737,927.74	
	44	640,814.01	3,737,427.7		642,517.16	3,737,427.74	
	45 46	641,079.41 629,391.19	3,736,927.7 3,739,615.3		642,517.16 628,729.53	3,736,927.74 3,745,310.22	
and the second s	47	629,883.66	3,739,709.0		629,226.19	3,745,367.92	
and the training	48	630,376.14	3,739,802.7		629,722.85	3,745,425.62	
IJ.	49 50	630,868.62 631,361.33	3,739,896.4 3,739,988.1		630,219.51 630,716.17	3,745,483.33 3,745,541.03	
A A A A A	51	631,854.04	3,740,079.8		631,212.83	3,745,598.74	
A DECEMBER OF THE PARTY OF THE	52	632,346.76	3,740,171.4		631,709.49	3,745,656.44	
	53 54	632,839.47 633,332.19	3,740,263.1 3,740,354.7		632,206.15 632,702.81	3,745,714.14 3,745,771.85	
AL AN	55	633,824.90	3,740,334.7		633,199.47	3,745,829.55	
and the second s	56	634,317.61	3,740,538.0	9	633,696.12	3,745,887.26	
the factor in	57 58	634,810.33 635,303.04	3,740,629.7 3,740,721.4		634,192.78 634,689.44	3,745,944.96 3,746,002.66	
2321 MAR	50 59	636,288.47	3,740,721.4		635,691.52	3,746,002.88	
	60	637,273.90	3,741,088.0	3	636,763.42	3,745,481.77	
	61	638,259.33 639,246.87	3,741,271.3		637,835.32 638,886.42	3,744,920.81	
	62 63	639,246.87 641,212.70	3,741,436.4 3,746,271.5		638,886.42 641,224.63	3,744,538.89 3,746,002.78	
A CON		641,123.17	3,745,638.4	3	640,930.44	3,745,317.97	
		639,562.30	3,744,548.1		638,638.58	3,743,947.61	
	64	634,810.97 641,346.36	3,743,462.0 3,736,927.7	_	639,532.84	3,741,406.86	
The second	- /	641,179.19	3,737,427.7	4	639,532.84	3,741,935.02	
17 March March		640,993.84	3,737,927.7		639,637.93	3,742,443.70	
No.		640,726.34 640,456.61	3,738,427.7 3,738,927.7		639,626.67 639,523.44	3,742,922.00 3,743,429.92	
· · · · · /		640,106.95	3,739,427.7		639,754.83	3,743,924.23	
mu )		639,666.03	3,739,928.2		639,727.58	3,744,424.16	
A second second		639,491.30 639,410,10	3,740,420.4 3,740,936.6		640,066.00	3,744,693.82	
				<u> </u>			
BAYOU BONFOUCA				c	URVEY LA	YOUT	
MARSH CREATION PROJECT				5	UNVET LA	1001	
STATE PROJECT NUMBER: PO-104							
FEDERAL PROJECT NUMBER:				DATE: APRIL 2012			
		SPE					
APPROVED BY: SHANNON HAYNES, P.E.			58	SHEET 13 OF 27			

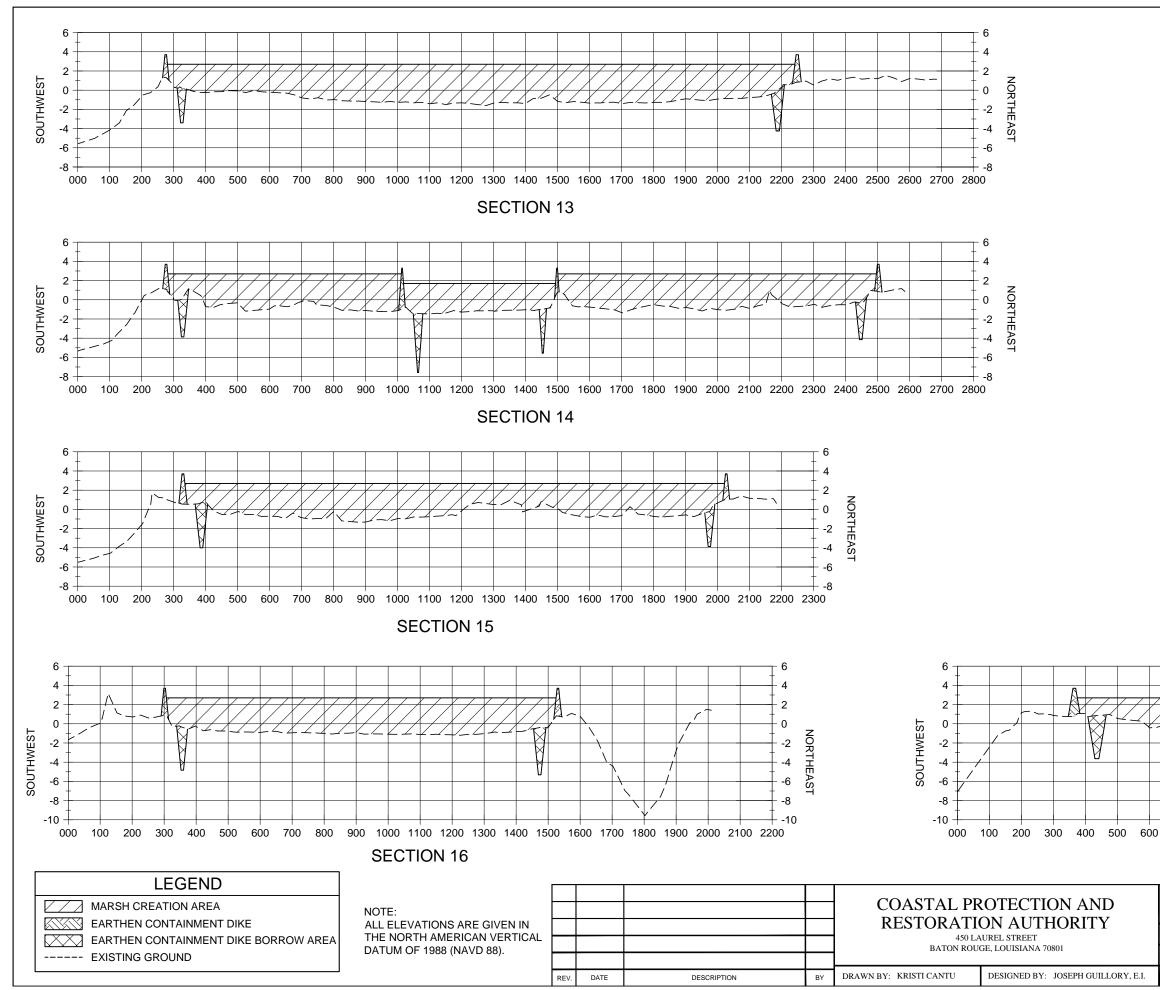


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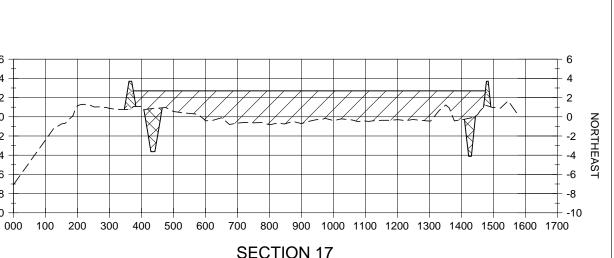




	HORIZONTAL GRAPHIC SCALE					
	400'	200'	0		400'	800'
	_					
	10'	5'	0		10'	20'
	VERTICAL GRAPHIC SCALE					
BAYOU BONFOUCA MARSH CREATION PROJECT		SECTIONS				
STATE PROJECT N	UMBER:	PO-104				
FEDERAL PROJEC	T NUMBE	ER:			DATE: APRIL 2012	
APPROVED BY: SHANNON HAYNES, P.E.			SHEET 16 OF 27			



SECTIONS
DATE: APRIL 2012
SHEET 17 OF 27



HORIZONTAL GRAPHIC SCALE

VERTICAL GRAPHIC SCALE

300'

10'

150

5'

0

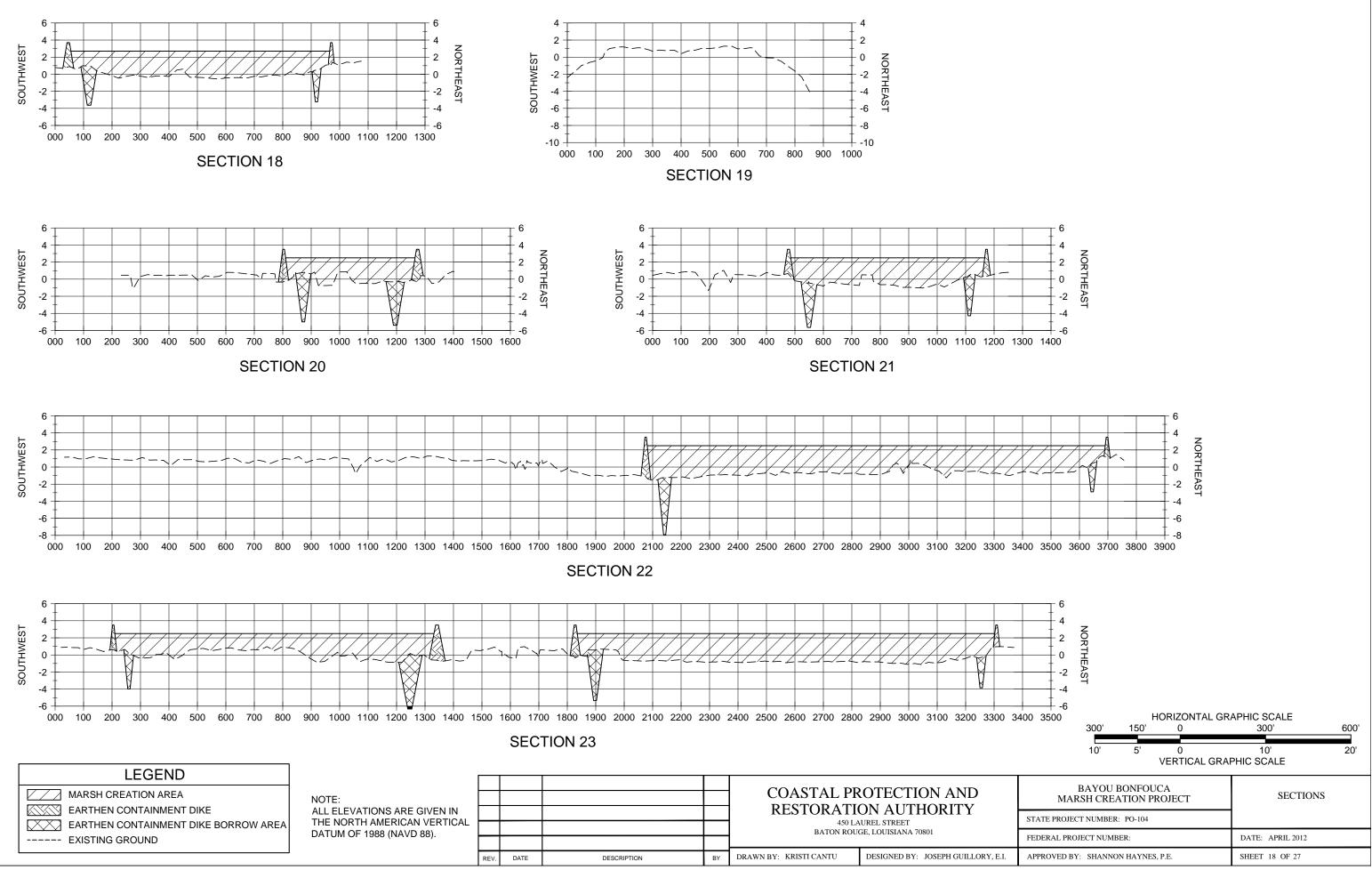
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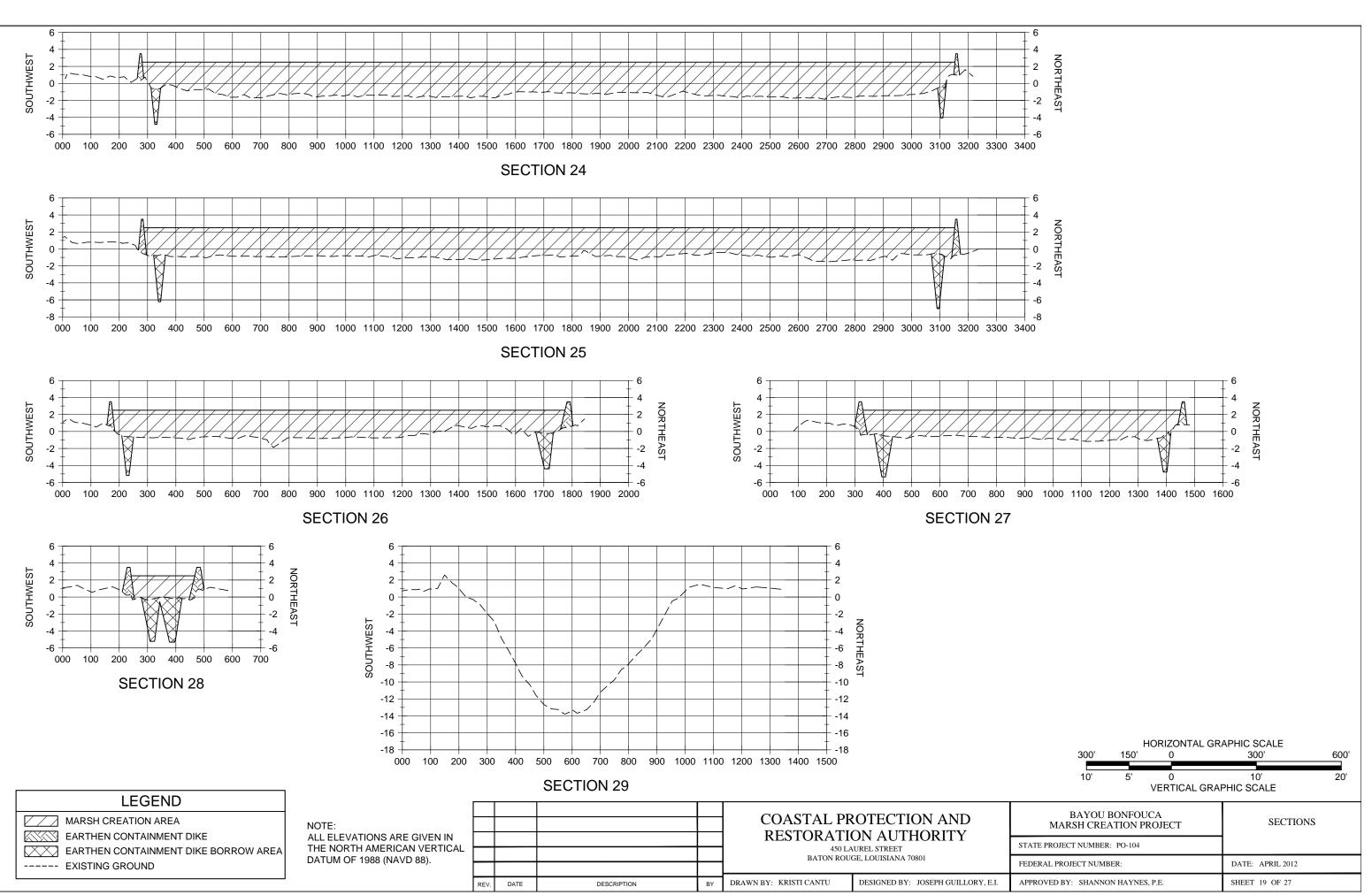
300'

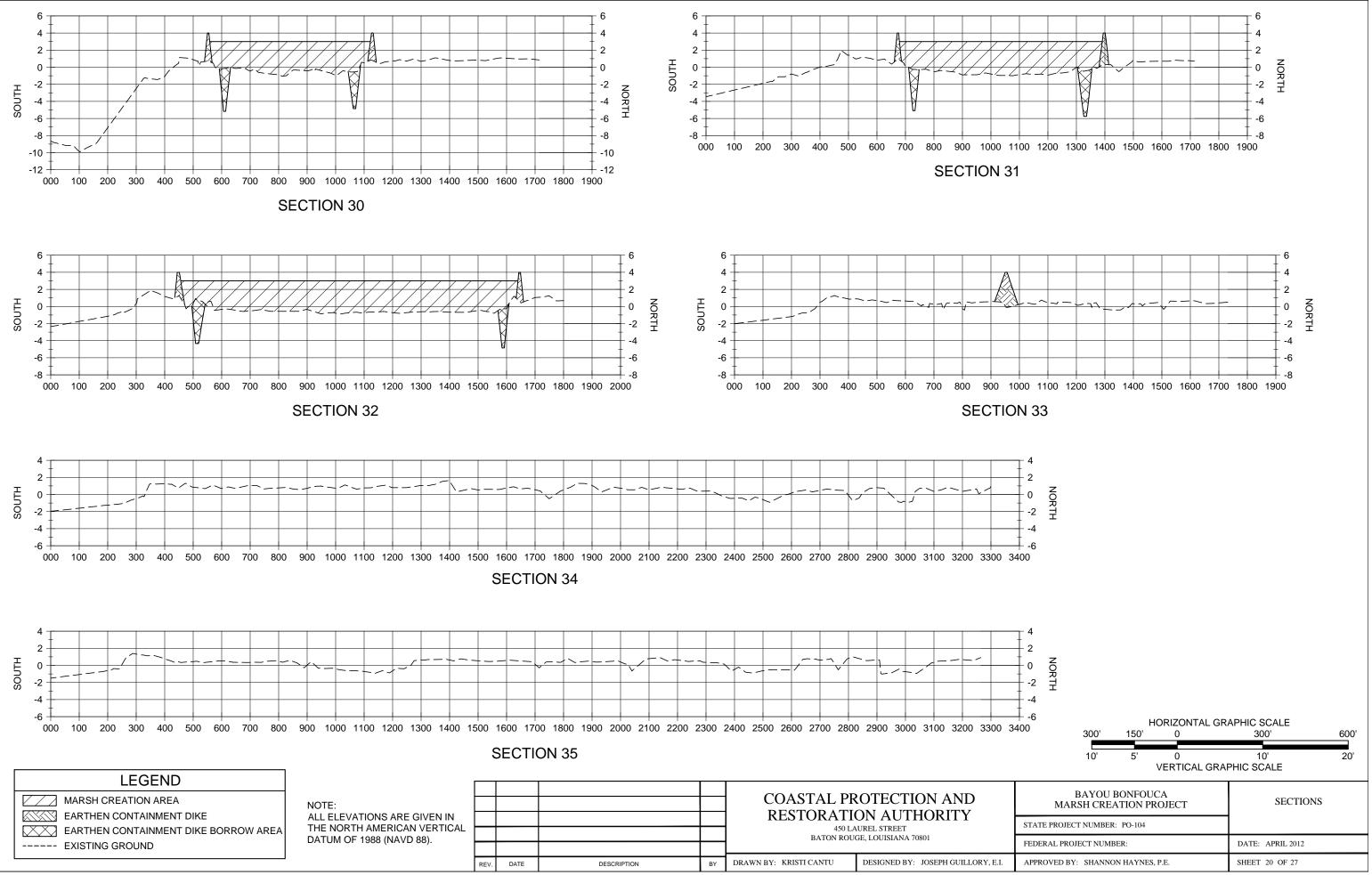
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600'

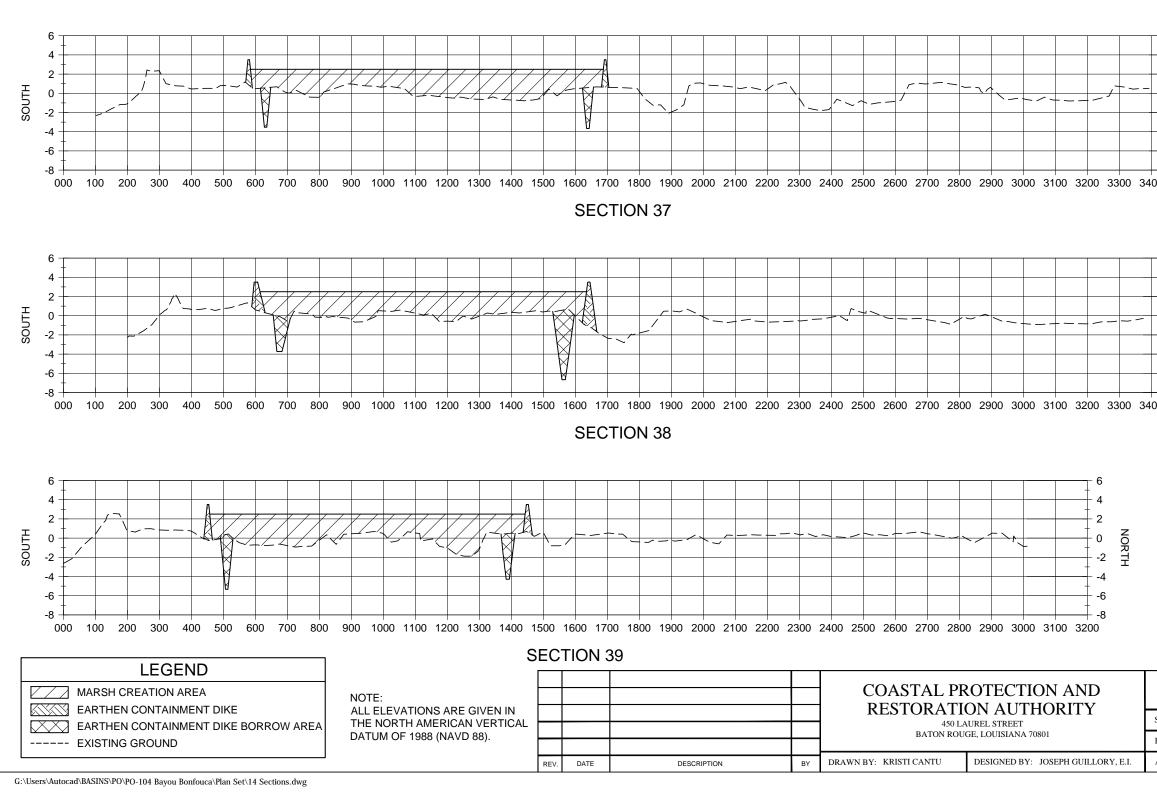
20'



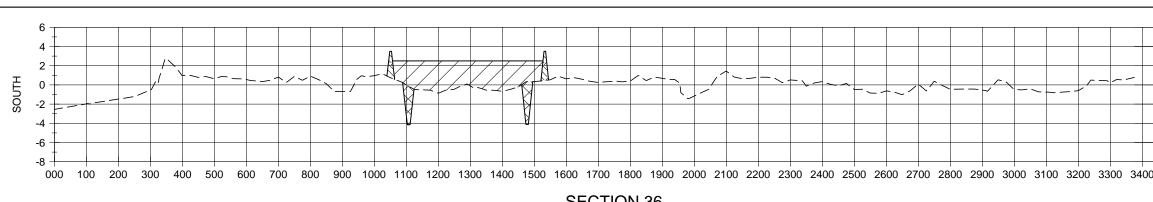




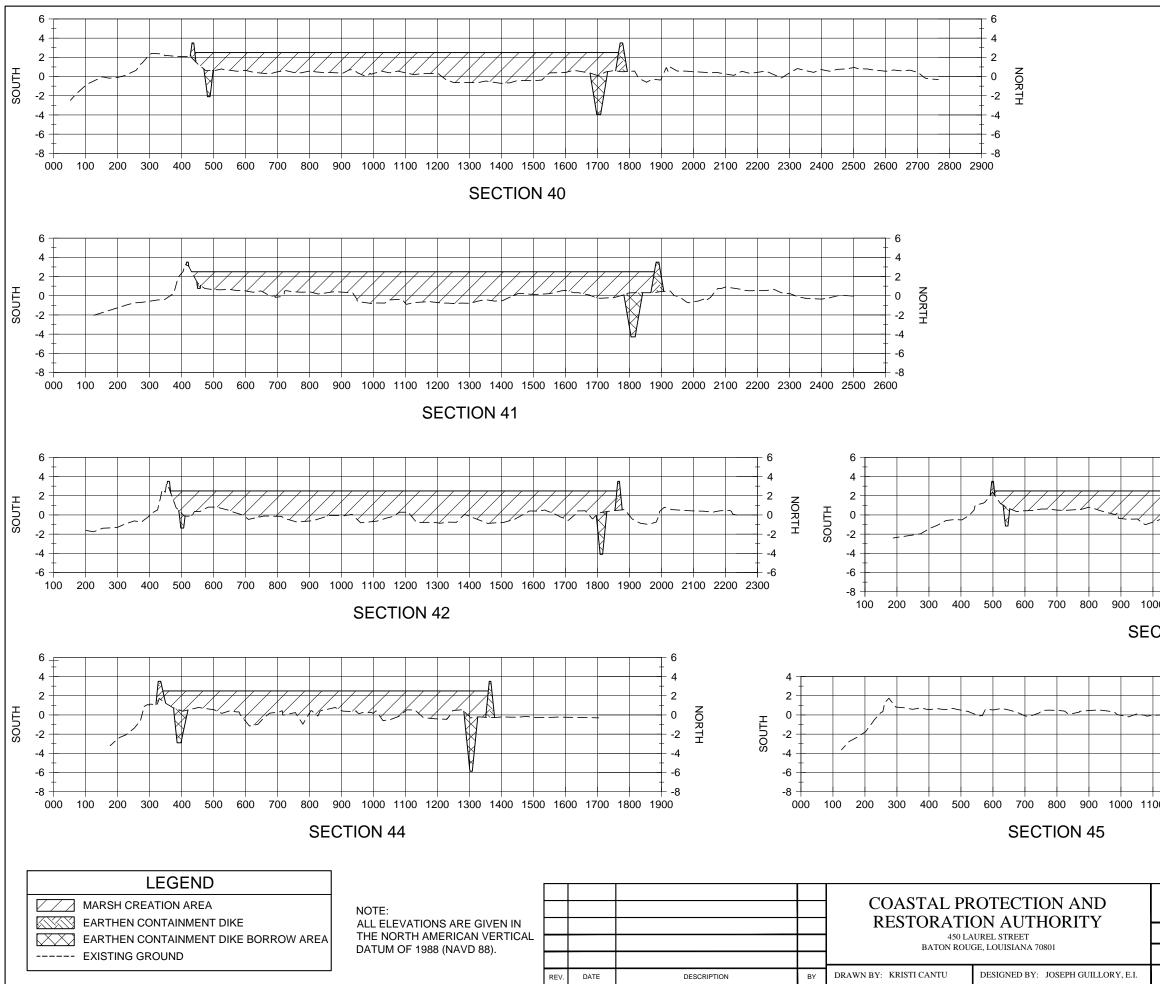
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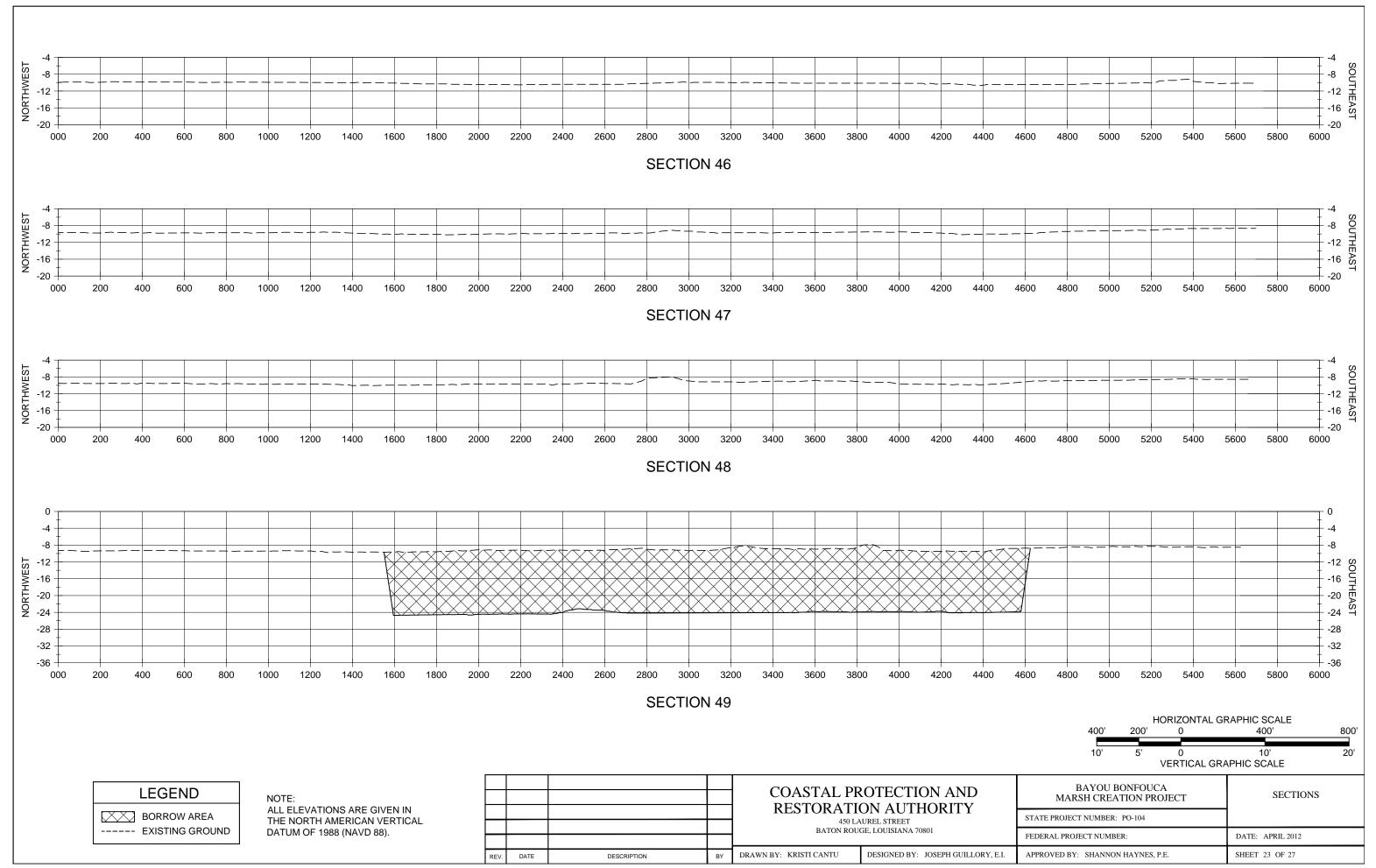
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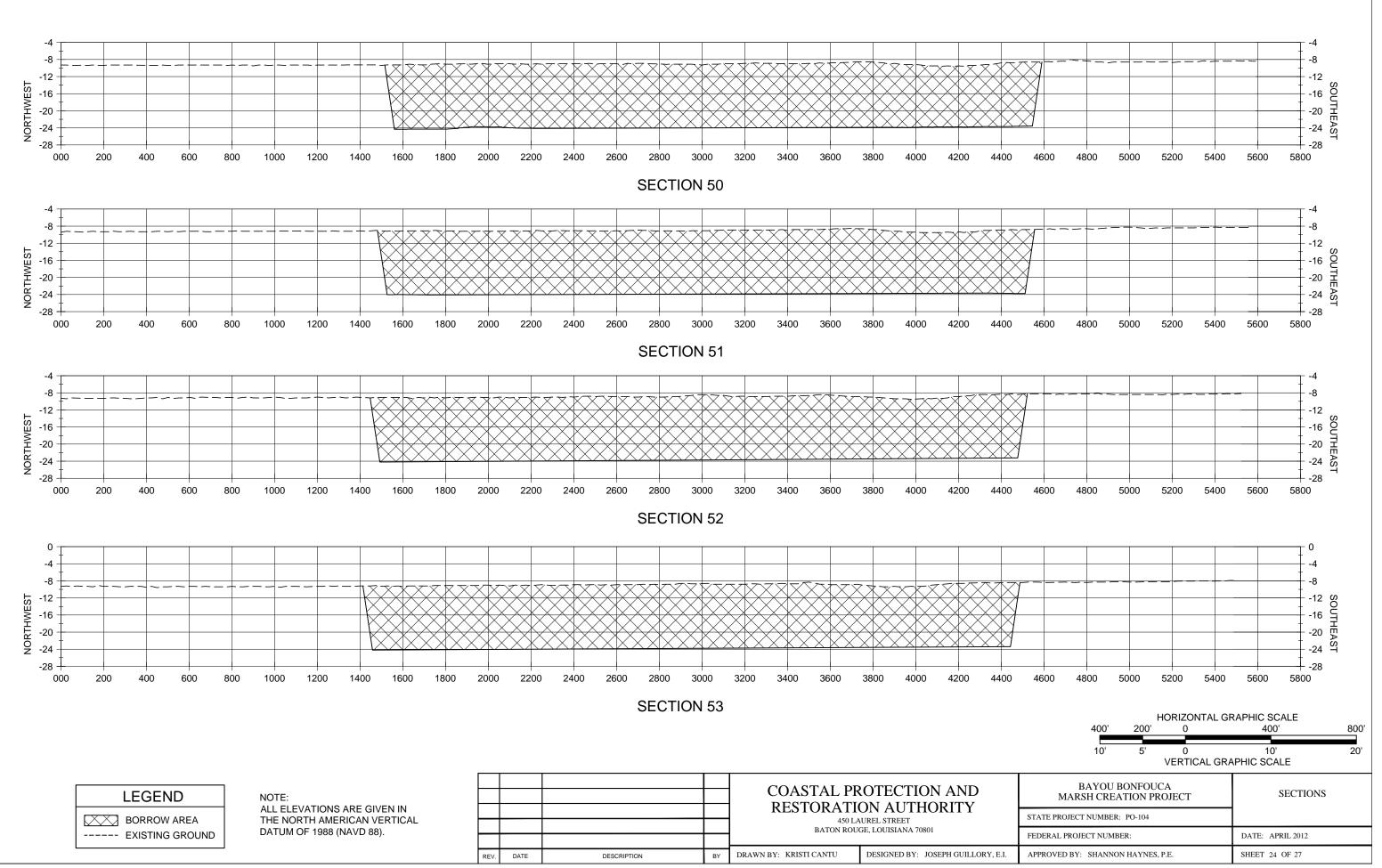


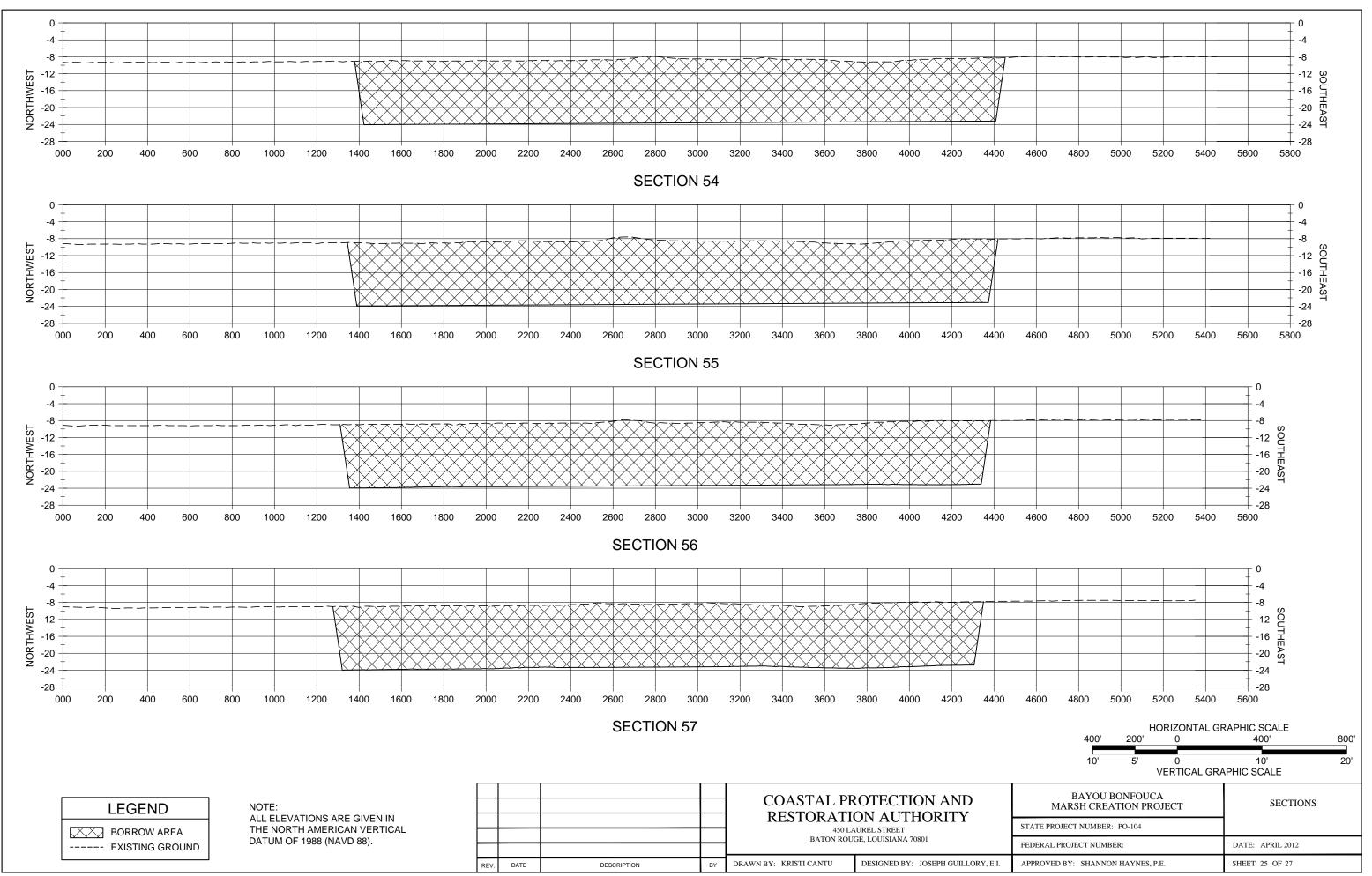
6 4 2 0 -2 -4 -6 400 3500	NORTH				
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	NORTH				
6 4 2 0 -2 -4 -4 -6 400 3500	NORTH				
	300'	HORIZC 150' 0	DNTAL GR.	APHIC SCALE 300'	600'
	10'	5' 0		10'	20'
			ICAL GRAF	PHIC SCALE	
MA	BAYOU BO RSH CREAT	ONFOUCA	Т	SECTIONS	
	CT NUMBER:				
FEDERAL PROJECT NUMBER: APPROVED BY: SHANNON HAYNES, P.E.			DATE: APRIL 2012		
A DDD OV TET	V 077132202			SHEET 21 OF 27	



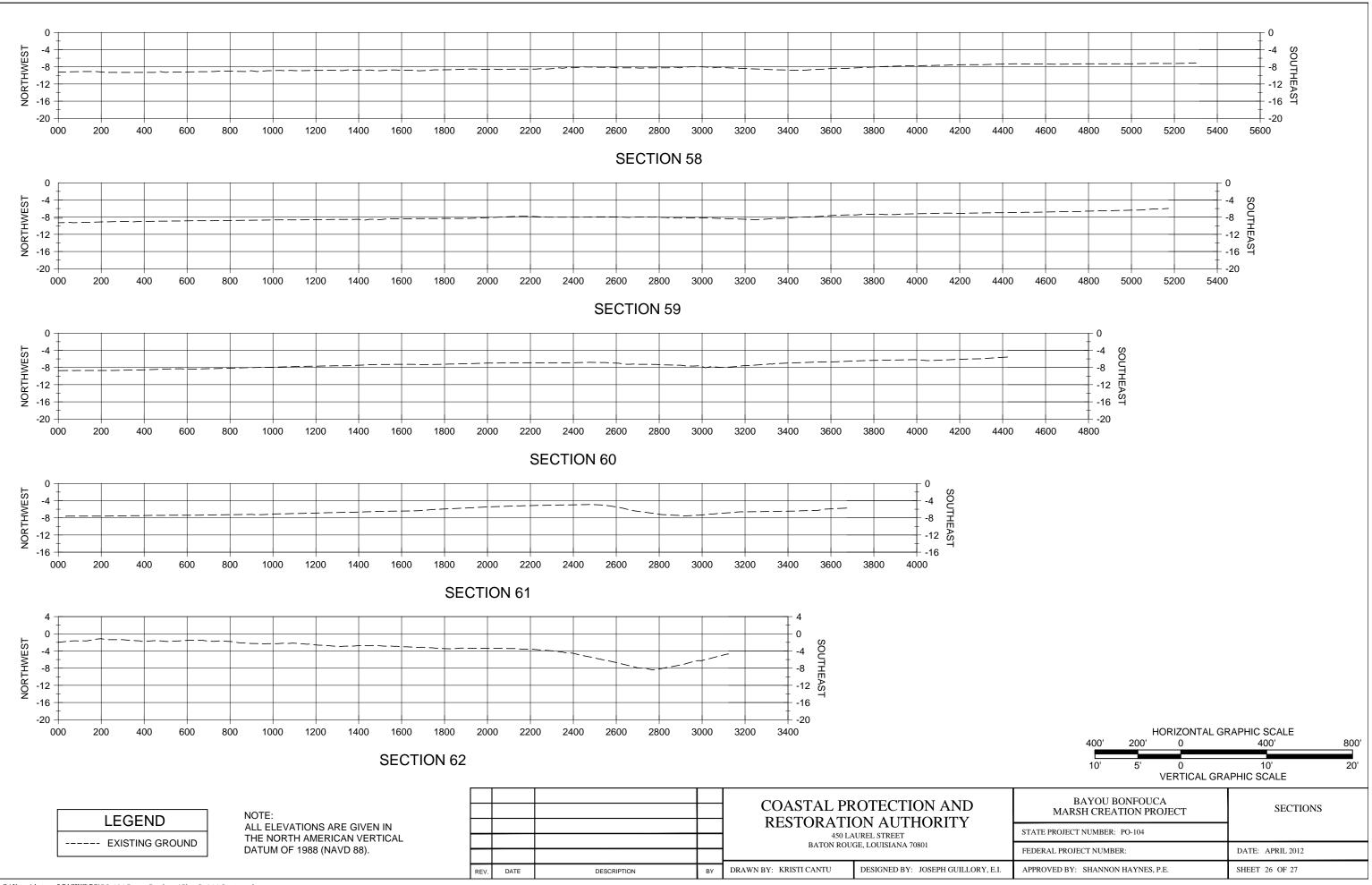
	6 4 2 0 
CTION 43	
4 2 0 -2 -4 0 -2 -4 0 -2 -4 0 -2 -4 0 -2 -4 -4 0 -4 -4 0 -4 -6 -8 00 1200 1300 1400 1500 1600	
HORIZONTAL GF 300' 150' 0	RAPHIC SCALE 300' 600'
10' 5' 0 VERTICAL GRA	10' 20' APHIC SCALE
BAYOU BONFOUCA MARSH CREATION PROJECT	SECTIONS
STATE PROJECT NUMBER: PO-104	
FEDERAL PROJECT NUMBER:	DATE: APRIL 2012
APPROVED BY: SHANNON HAYNES, P.E.	SHEET 22 OF 27

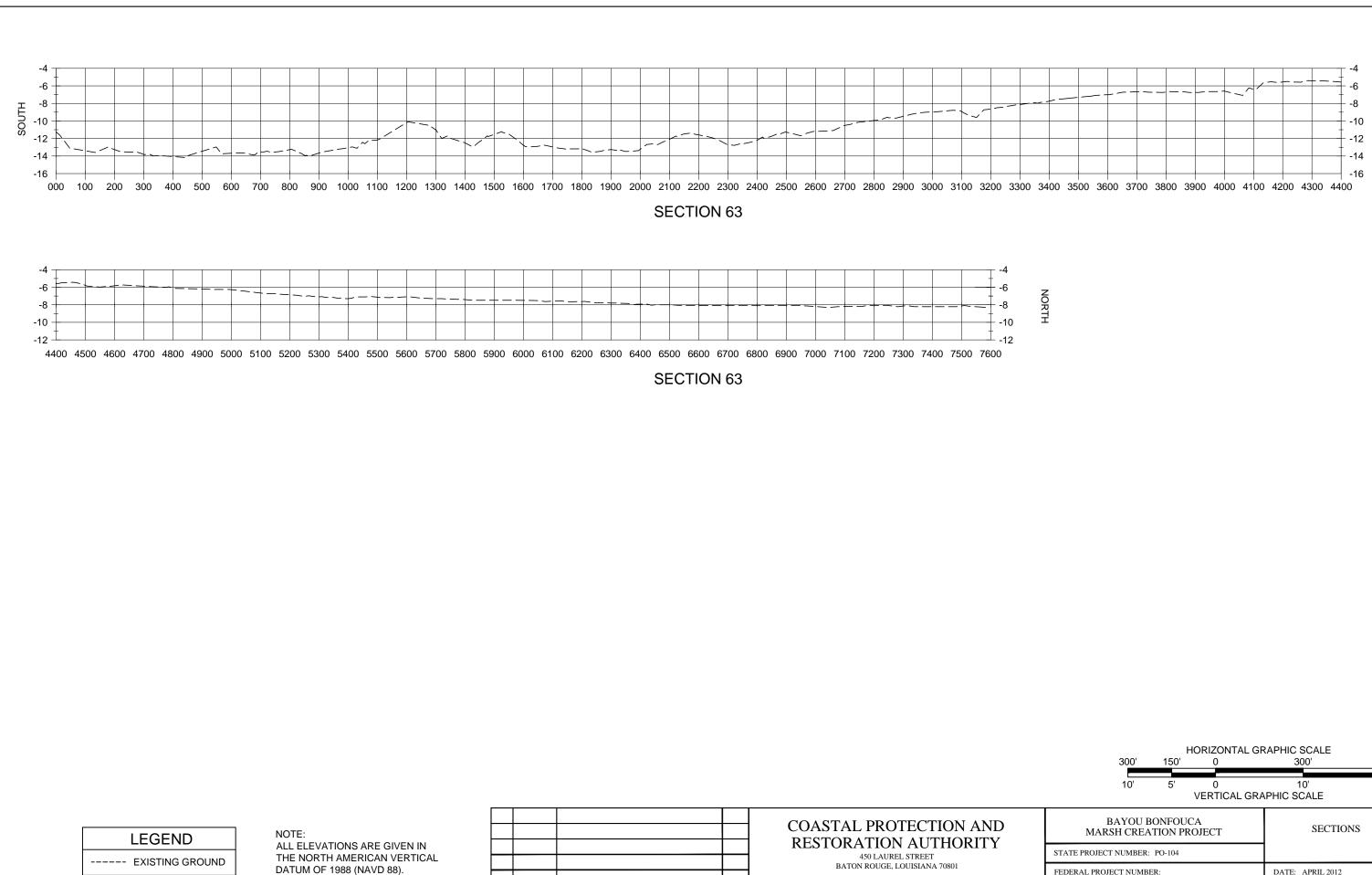






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REV.

DATE

DESCRIPTION

DRAWN BY: KRISTI CANTU

BY

DESIGNED BY: JOSEPH GUILLORY, E.I.

HORIZONTAL GR 30 <u>0' 150' 0</u>	8APHIC SCALE 300' 600'				
10' 5' 0 10' 20' VERTICAL GRAPHIC SCALE					
BAYOU BONFOUCA MARSH CREATION PROJECT	SECTIONS				
STATE PROJECT NUMBER: PO-104					
FEDERAL PROJECT NUMBER:	DATE: APRIL 2012				
APPROVED BY: SHANNON HAYNES, P.E.	SHEET 27 OF 27				