Geotechnical Investigation Data Report
Bayou Dupont Restoration Project
Plaquemines and Jefferson Parishes, LA
LDNR (2503-05-44)

For:

Sigma Consulting Group, Inc.

Baton Rouge, Louisiana

July 2007

LOUIS J. CAPOZZOLI & ASSOCIATES, INC.
Geotechnical Engineers

Louis J
CAPOZZOLI
and Associates
a subsidiary of GeoEngineers

16 July 2007

Sigma Consulting Group
10305 Airline Highway
Baton Rouge, Louisiana 70816

Attention: Mr. Gregory P. Sepeda, P.E.

Re: Geotechnical Investigation Data Report
Bayou Dupont Restoration Project
Plaquemines and Jefferson Parishes, Louisiana
LDNR Contract No. 2503-05-44
LJC&A File: 07-110/16715-001-00

Gentlemen:

The following work required for this project was provided in the Louisiana Department of Natural Resources (LDNR) requested scope of work dated 22 February 2007. Work was done per LDNR Contract No. 2503-05-44 for Sigma Consulting Group as prime contractors.

PROJECT LOCATIONS

A project map on sheet 1 of 2 presents the site and boring layout. The boring numbers, depth and boring locations with latitude and longitude are shown on this sheet. A vicinity map on sheet 2 of 2 presents the boring location map and restoration area. These sheets were furnished by LDNR. A portion of a quadrangle sheet of the site depicts the topographic features of the site on sheet 3. Also presented are the cross-sections for borrow area No. 2 which was the area of concern for this project; these are shown on three sheets.

SITE GEOTECHNICAL CONDITIONS

A description of the field and laboratory conduct is depicted in appendix A. A log of boring legend is given on sheet A1. The logs of borings 1, 2, 3, and 5 and 6 are given in Appendix A. Also presented in Appendix A are the results of the laboratory tests on Tables 1 and 2 and Figures 1 through 5. The numerical and graphical presentation of the stress-strain curves are also given in Appendix A.

The soil data, shear strength wet density and moisture content for the jack-and-bore sites and the marsh borings are depicted on sheets 4 through 8. Data used for settlement analyses is presented on sheet 9 for marsh soil. It should be noted that the data for the marsh soil was determined by laboratory tests and borings done by Eustis Engineering Company for a previous report (see Appendix C) and our engineering judgment. The constructed consolidation curves used for the foundation layers of the marsh soil are given on sheets 10 through 14. Laboratory test results for runs No. 7 and 8 for the sand sediment to be pumped into the marsh area are given in graphical form on sheets 15 and 16. Soil parameters used for the computed stability analyses by the U.S. Army Corps of Engineers computer program for primary consolidation, secondary compression, and desiccation of dredged fill (PSDDF) are given in tabular form on sheets 17 and 18. The data provided to us by a subconsultant - Louisiana State University, Geotechnical Laboratory and Research Engineering - are given in

Appendix B. This covers the 1D consolidation tests and the computer (PSDDF) run for 3.5 feet of fill causing settlement at the marsh.

SUBSURFACE CONDITIONS

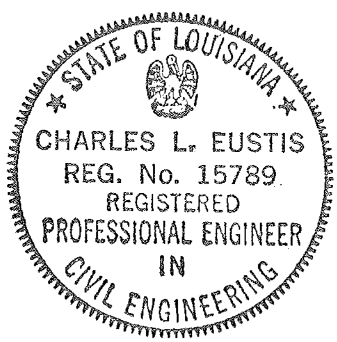
Borings 1, 2, and 3 were done in 51, 51.5, and 57 feet of water at the time of our field investigation. We estimated the water surface to be at elevation 5 feet, NGVD 88. Borings 5 and 6 were done on land for the jack-and-bore pits for receiving the discharge pipe from the dredging operation of the Mississippi River borrow material to be placed as fill in the Bayou Dupont marsh area.

For borings 1, 2, and 3, the borrow material will be sand (SP) with less than 1 percent organic matter from the mudline to 40 feet below existing mud line. See Sheet 18 for a graphical presentation of stratification for borings 1, 2, and 3. Appendix B contains sieve/hydrometer and organic content tests on the sand composite sample. The graphical presentation for the clayey soils encountered at borings 5 and 6 is given in a subsoil profile on sheet 19.

A soil profile of the marsh borings done by others is included in Appendix C. Borings 4 through 7 are for the marsh area to be renourished by hydraulically pumping sand from the Mississippi River borrow area.

SUMMARY

We have provided the requested information for the geotechnical investigation data report. The majority of this information has already been transmitted to Ms. Whitney Thompson, E.I. Project Engineer of LDNR, Coastal Engineering Division.



Very truly yours,

Louis J. Capozzoli & Associates, Inc. -
a subsidiary of GeoEngineers

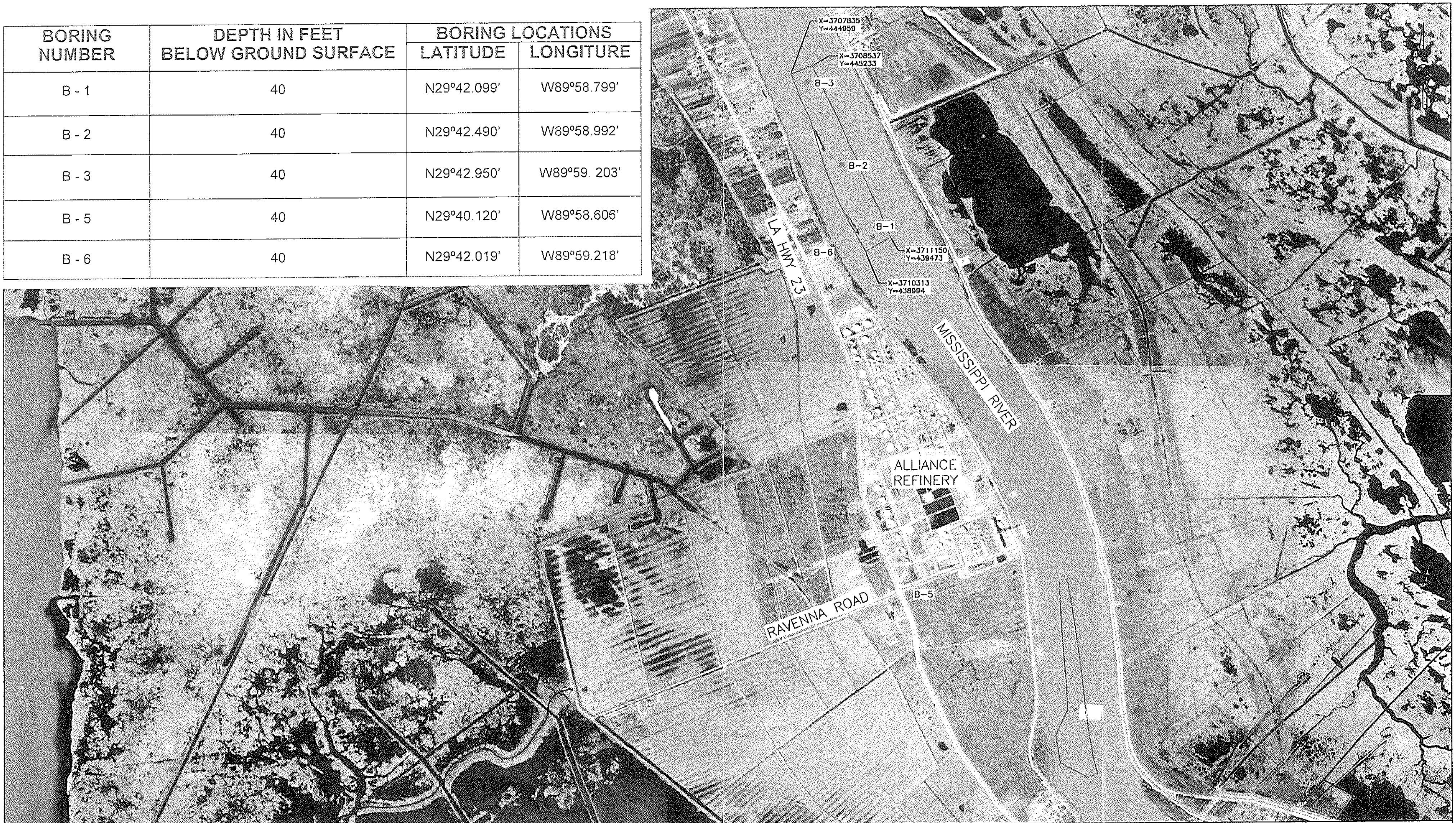
Charles L. Eustis

Charles L. Eustis, P.E.

CLE/cc

Enclosures: Sheets 1 through 19, Site Data
Appendix A, Laboratory Analyses Conduct Description and Laboratory Data
Appendix B, 1D Consolidation Tests and Associated Laboratory Testing done by LSU along with PSDDF computer results
Appendix C, Background Data done in the marsh by Eustis Engineering Company, Inc.

BORING NUMBER	DEPTH IN FEET BELOW GROUND SURFACE	BORING LOCATIONS	
		LATITUDE	LONGITURE
B - 1	40	N29°42.099'	W89°58.799'
B - 2	40	N29°42.490'	W89°58.992'
B - 3	40	N29°42.950'	W89°59 203'
B - 5	40	N29°40.120'	W89°58.606'
B - 6	40	N29°42.019'	W89°59.218'



3000' 1500' 0' 3000' 6000'

REV.	DATE	DESCRIPTION	BY

LOUISIANA DEPARTMENT OF NATURAL RESOURCES
COASTAL ENGINEERING DIVISION
617 NORTH 3RD STREET
BATON ROUGE, LOUISIANA 70802

DRAWN BY: W. THOMPSON, E.I.
DESIGNED BY: W. THOMPSON, E.I.

MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM-BAYOU DUPONT

STATE PROJECT NUMBER: BA-39

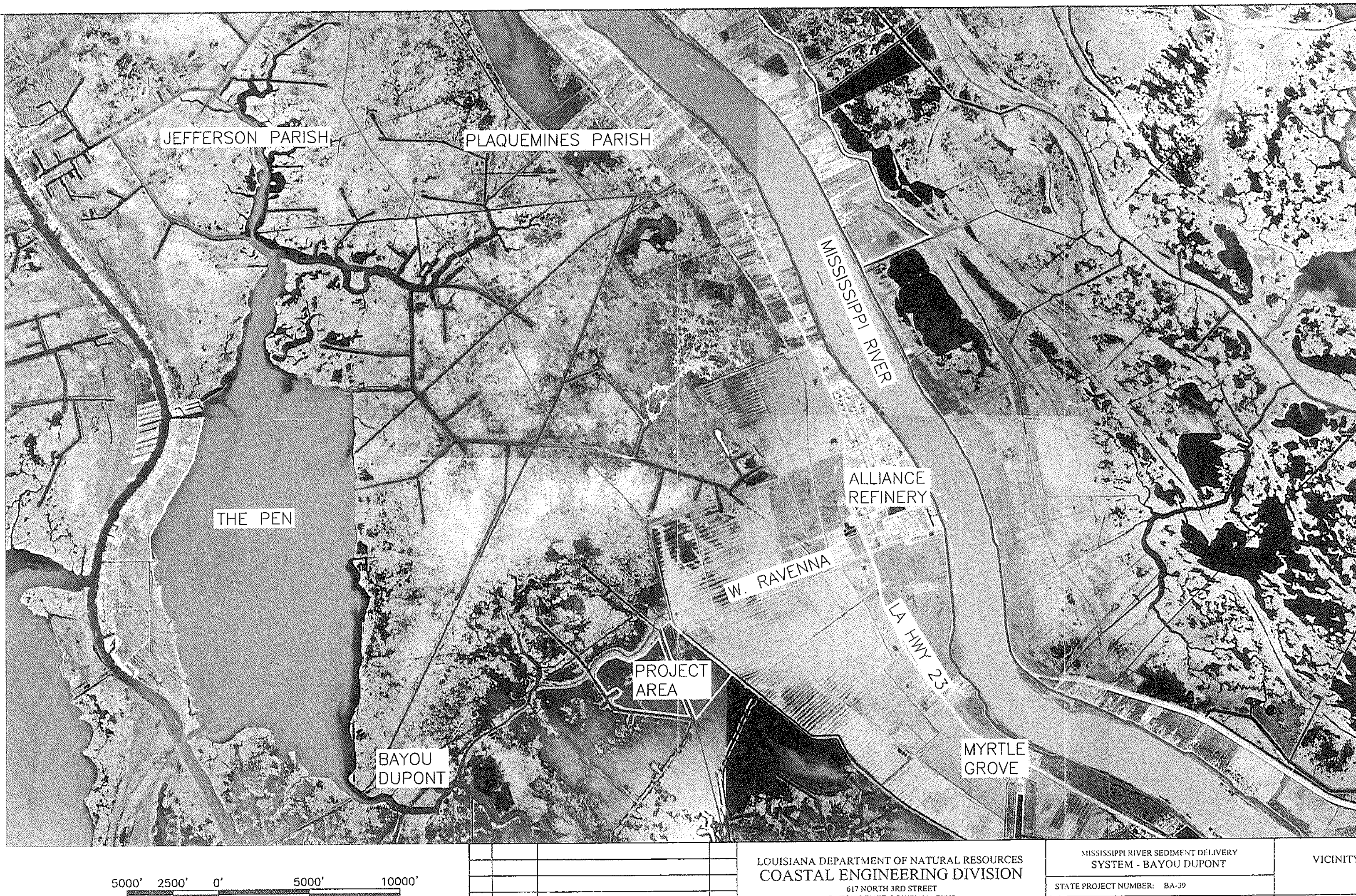
FEDERAL PROJECT NUMBER: BA-39

APPROVED BY: LUKE LEBAS, P.E.

BORING LAYOUT

DATE: MARCH 2007

SHEET 1 OF 2



REV.	DATE	DESCRIPTION	BY

LOUISIANA DEPARTMENT OF NATURAL RESOURCES
COASTAL ENGINEERING DIVISION
617 NORTH 3RD STREET
BATON ROUGE, LOUISIANA 70802

DRAWN BY: W. THOMPSON, E.I. DESIGNED BY: W. THOMPSON, E.I.

MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM - BAYOU DUPONT

STATE PROJECT NUMBER: BA-39

FEDERAL PROJECT NUMBER: BA-39

APPROVED BY: LUKE LEBAS, P.E.

VICINITY MAP

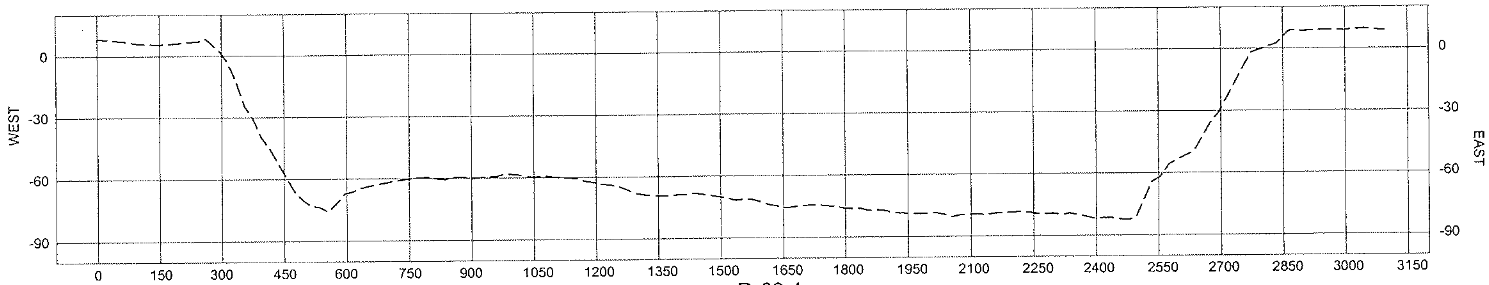
DATE: FEBRUARY 2007

SHEET 2 OF 2

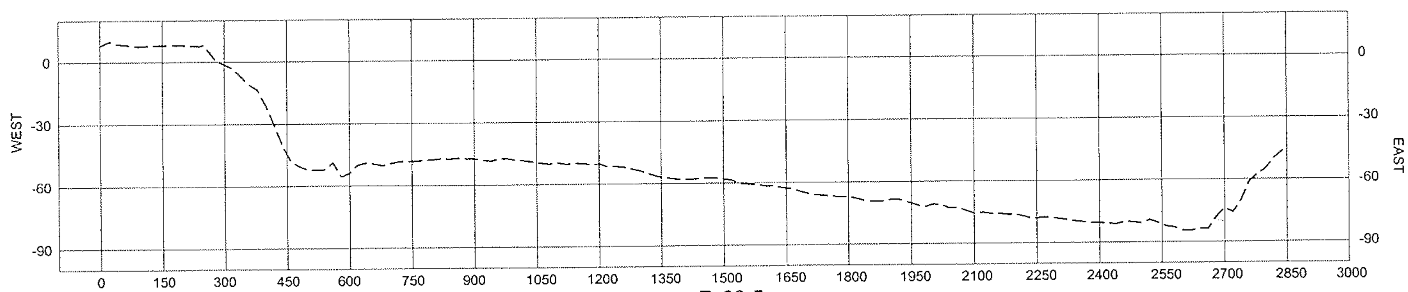
Bayou Dupont Plaquemines Parish, Louisiana LDNR (2503-05-44))		
Sigma Consulting Group Baton Rouge, Louisiana		
Louis J. Capozzoli & Associates, Inc. Geotechnical Engineers Baton Rouge, Louisiana		
Scale Shown	Dwn. CLE Ckd. 11 Jun 07	File No. 07-110
SITE TOPOGRAPHIC MAP		Sheet No. 3

LEGEND

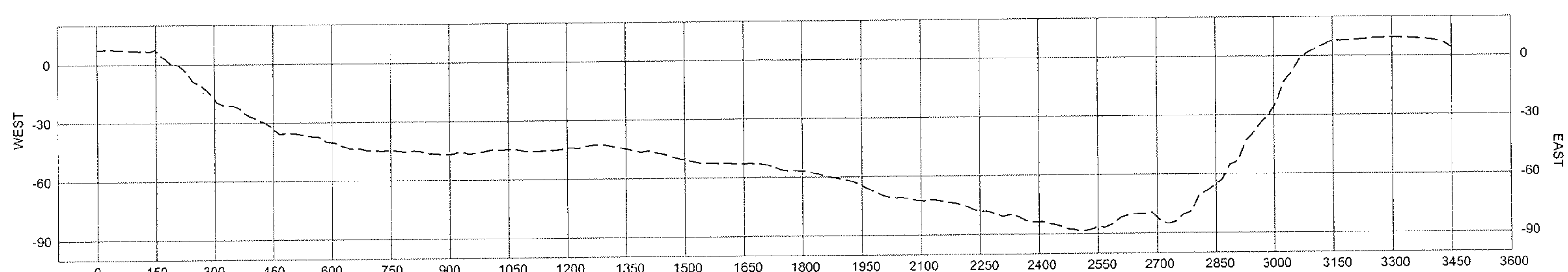
--- EXISTING BOTTOM



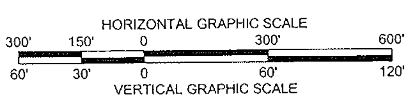
R-63.4



R-63.5



R-63.7



REV.	DATE	DESCRIPTION	BY

LOUISIANA DEPARTMENT OF NATURAL RESOURCES
COASTAL ENGINEERING DIVISION
617 NORTH 3RD STREET
BATON ROUGE, LOUISIANA 70802

DRAWN BY: KRISTI CANTU | DESIGNED BY: W. THOMPSON, E.I.

MISSISSIPPI RIVER SEDIMENT DELIVERY
BAYOU DUPONT

STATE PROJECT NUMBER: BA-39

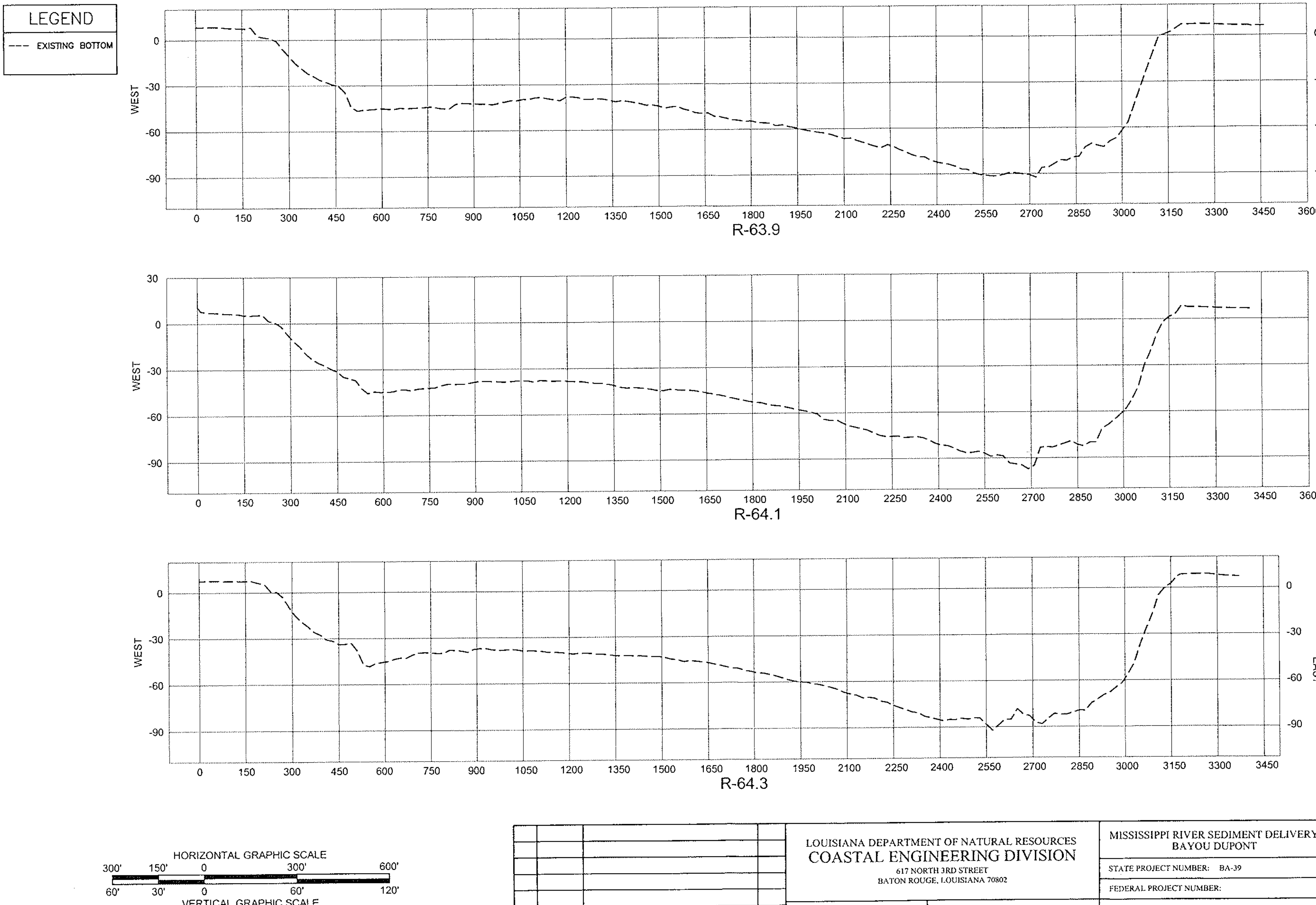
FEDERAL PROJECT NUMBER:

APPROVED BY: LUKE LEBAS, P.E.

BORROW AREA 2
CROSS SECTIONS

DATE: MARCH 2007

SHEET 1 OF 3



REV.	DATE	DESCRIPTION	BY

LOUISIANA DEPARTMENT OF NATURAL RESOURCES
COASTAL ENGINEERING DIVISION
617 NORTH 3RD STREET
BATON ROUGE, LOUISIANA 70802

MISSISSIPPI RIVER SEDIMENT DELIVERY
BAYOU DUPONT

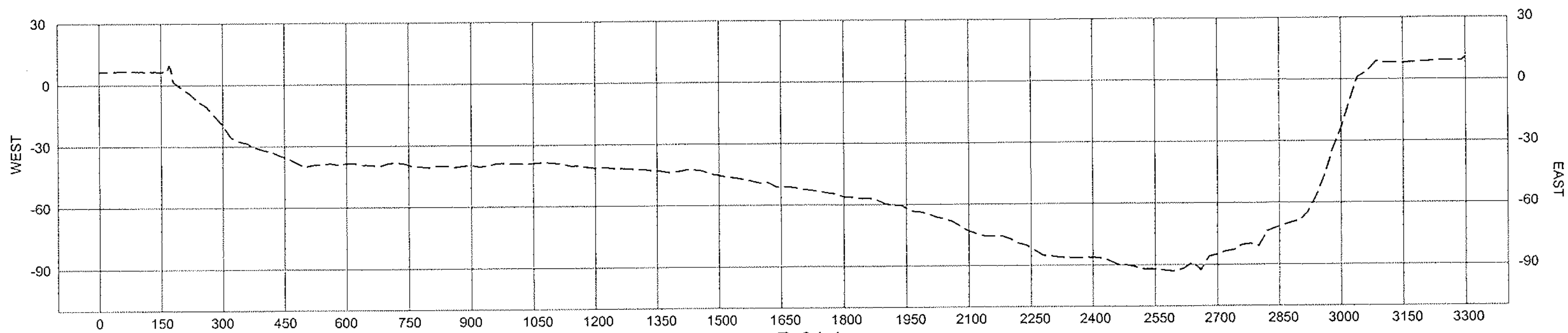
BORROW AREA 2
CROSS SECTIONS

STATE PROJECT NUMBER: BA-39
FEDERAL PROJECT NUMBER:
DATE: MARCH 2007

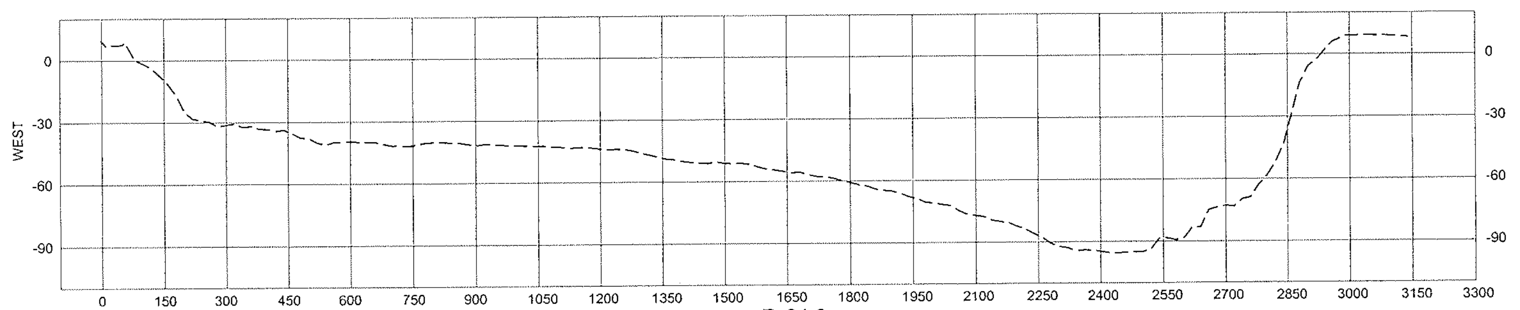
DRAWN BY: KRISTI CANTU
DESIGNED BY: W. THOMPSON, E.I.
APPROVED BY: LUKE LEBAS, P.E.
SHEET 2 OF 3

LEGEND

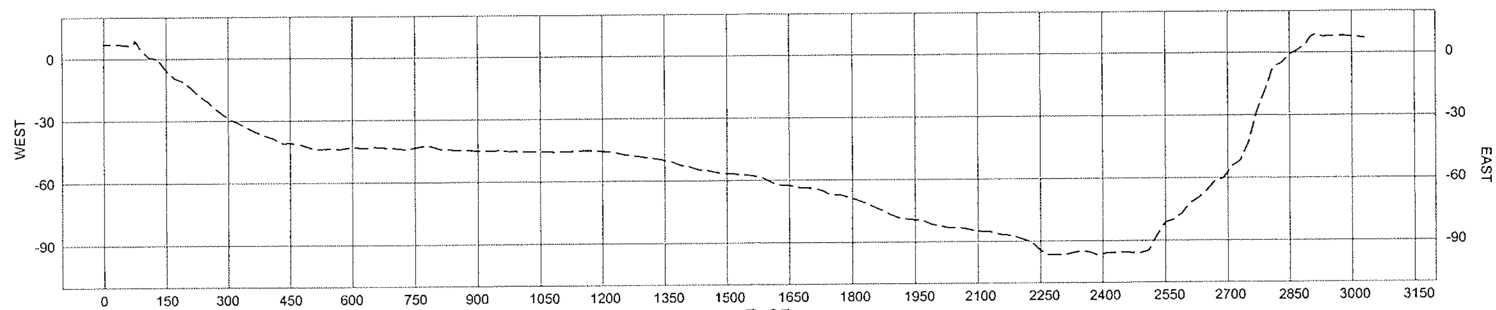
--- EXISTING BOTTOM



R-64.4



R-64.8



R-65

HORIZONTAL GRAPHIC SCALE

300' 150' 0 300' 600'

60' 30' 0 60' 120'

VERTICAL GRAPHIC SCALE

REV	DATE	DESCRIPTION	BY

LOUISIANA DEPARTMENT OF NATURAL RESOURCES
COASTAL ENGINEERING DIVISION
617 NORTH 3RD STREET
BATON ROUGE, LOUISIANA 70802

MISSISSIPPI RIVER SEDIMENT DELIVERY
BAYOU DUPONT

BORROW AREA 2
CROSS SECTIONS

STATE PROJECT NUMBER: BA-39

FEDERAL PROJECT NUMBER:

DATE: MARCH 2007

DRAWN BY: KRISTI CANTU | DESIGNED BY: W. THOMPSON, E.I. | APPROVED BY: LUKE LEBAS, P.E.

SHEET 3 OF 3

Subject SOIL DATA

BORING 5

File No. 07-110

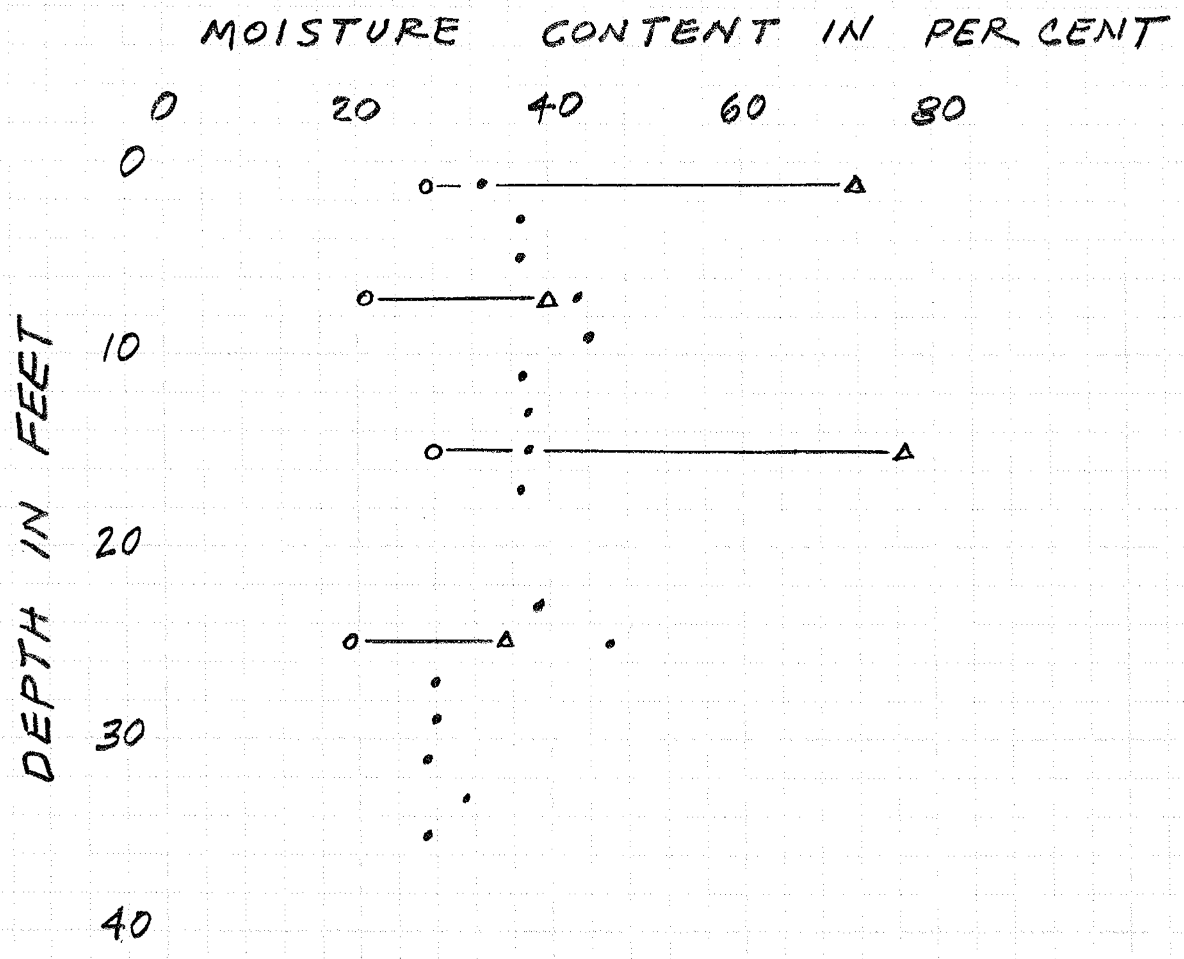
Sheet 4 Of

Made by GFE Date 1JUN07

Checked by Date

LOUIS J. CAPOZZOLI & ASSOCIATES, INC. Geotechnical Engineers

JACK & BORE SITE



LEGEND:

• = WATER CONTENT IN %

o = PLASTIC LIMIT IN %

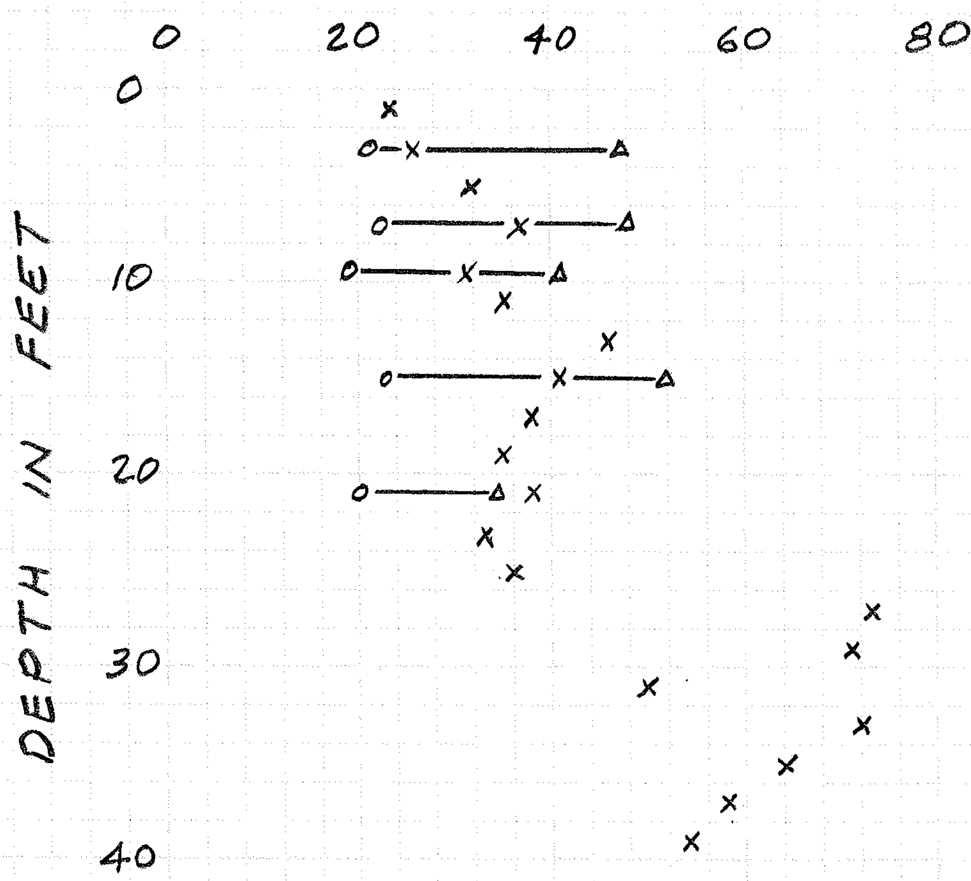
Δ = LIQUID LIMIT IN %

Subject: SOIL DATA
BORING 6
Sheet: 5 Of:
Made by: GJE Date: 1JUN07
File No.: 07-110
Checked by: Date:

LOUIS J. CAPOZZOLI & ASSOCIATES, INC. Geotechnical Engineers

JACK & BORE SITE

MOISTURE CONTENT IN PER CENT



LEGEND:

• = WATER CONTENT IN %
O = PLASTIC LIMIT IN %
Δ = LIQUID LIMIT IN %

Subject SHEAR STRENGTH MARSH BORINGS

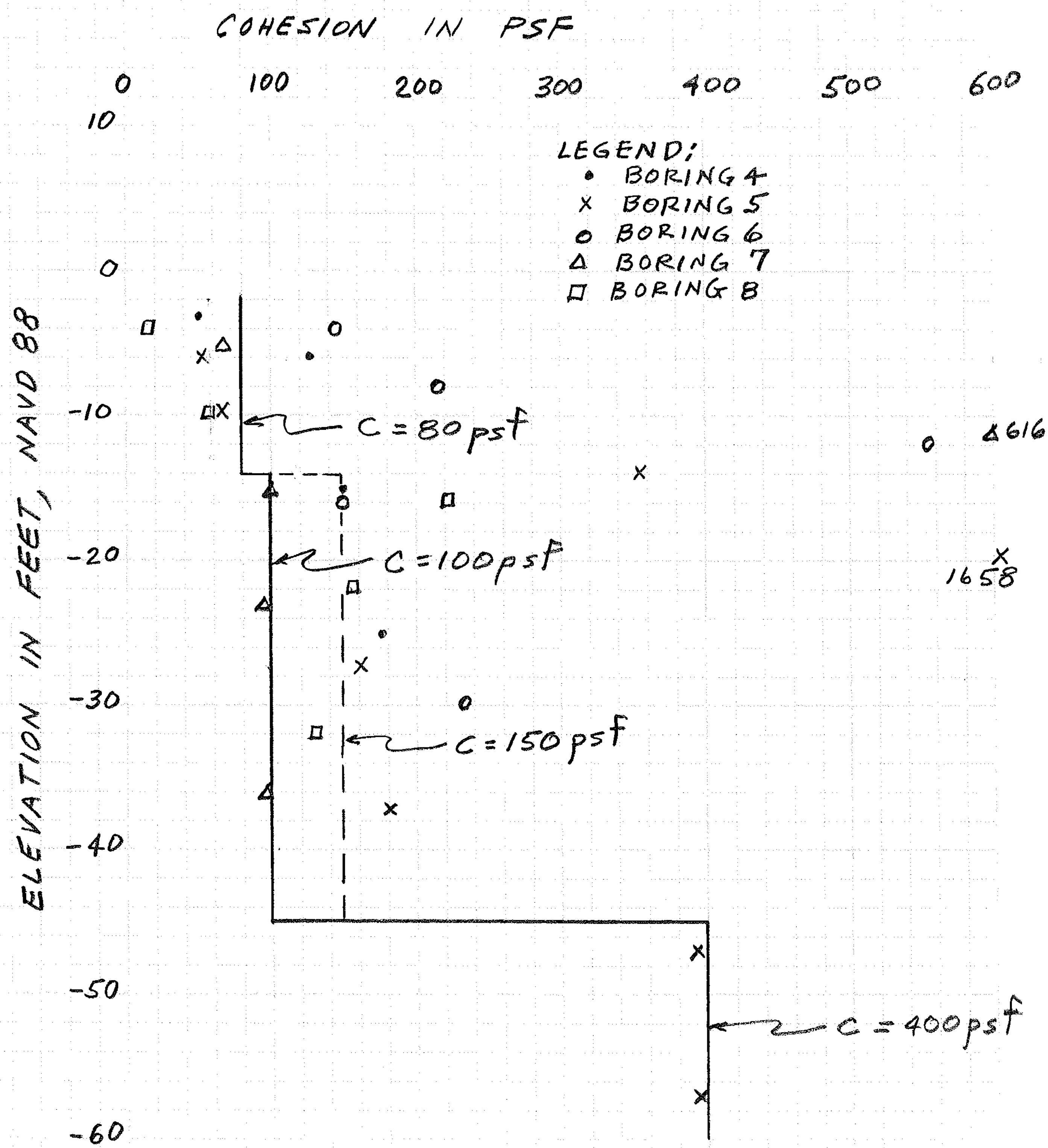
Sheet 6 Of

Made by [illegible] Date 11MAY07

File No. 07-110

Checked by Date

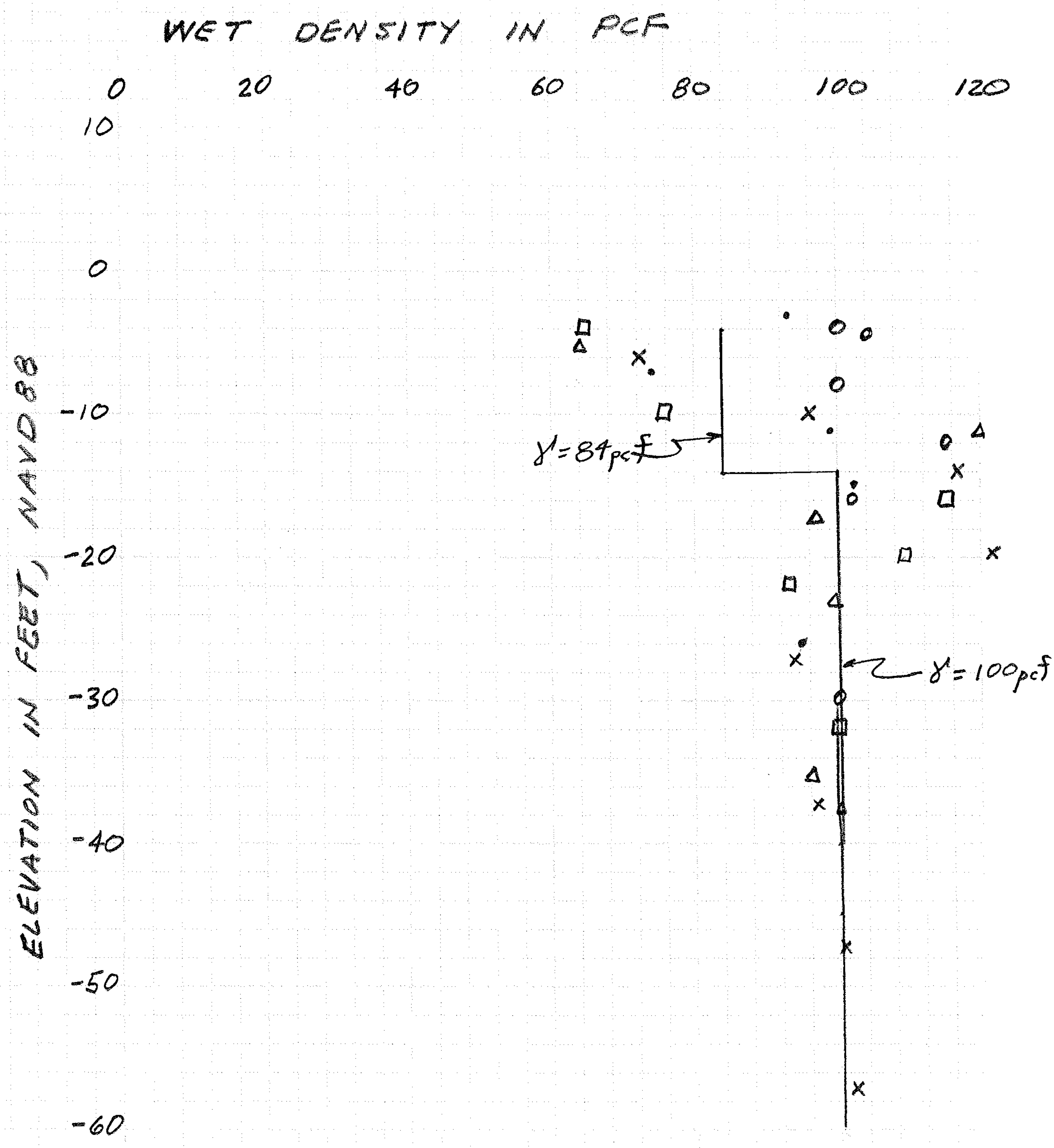
LOUIS J. CAPOZZOLI & ASSOCIATES, INC. Geotechnical Engineers



NOTE: BORINGS AND LABORATORY TEST DONE BY EUSTIS ENGINEERING COMPANY, INC. (EECO)

LOUIS J. CAPOZZOLI & ASSOCIATES, INC. Geotechnical Engineers

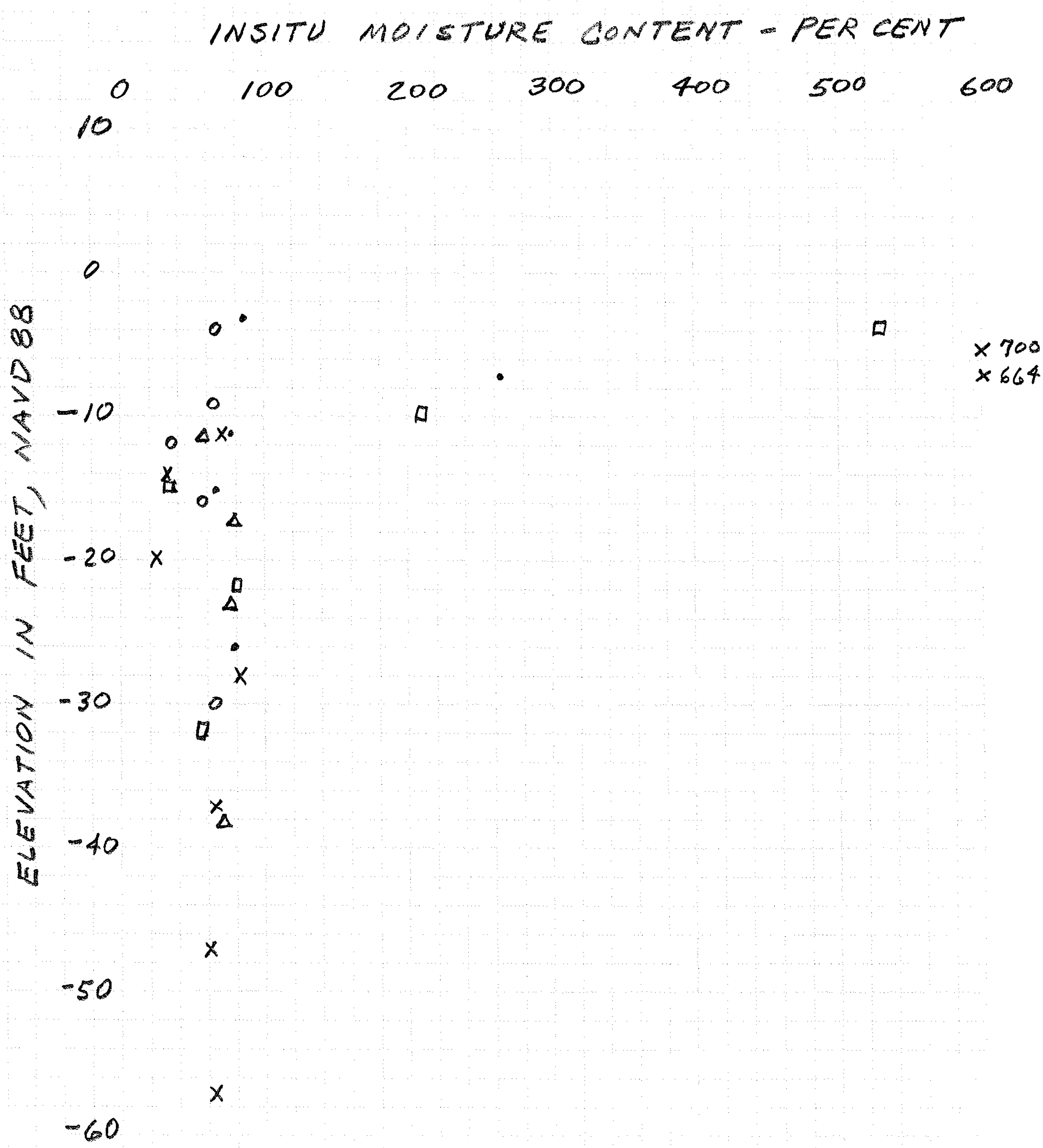
Subject: WET DENSITY MARSH BORINGS
Sheet: 7 Of:
Made by: [illegible] Date: 12 MAY 07
File No.: 07-110
Checked by: Date:



FOR NOTES AND LEGEND, SEE "SHEAR STRENGTH" SHEET FOR MARSH BORINGS

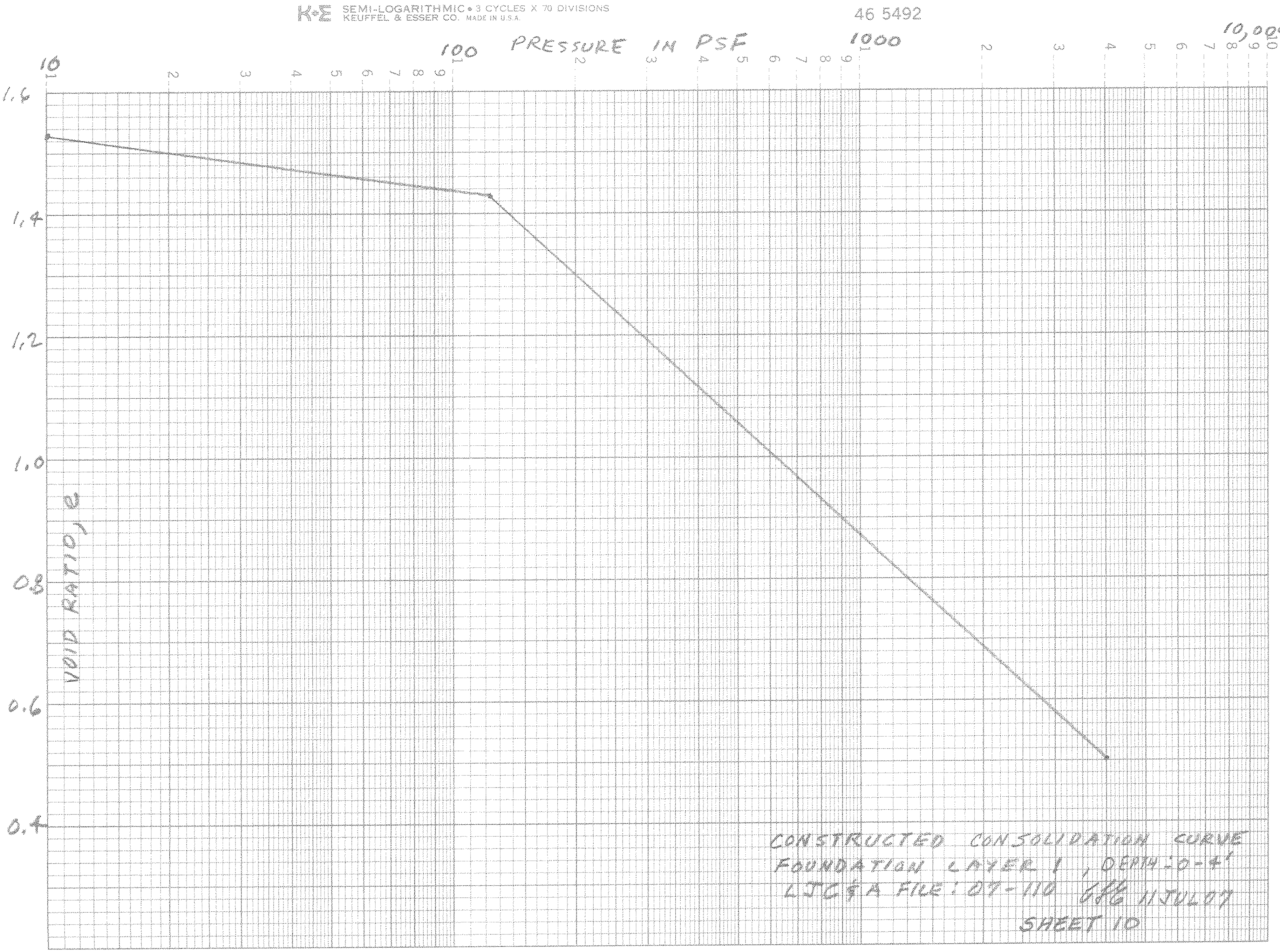
Subject MOISTURE CONTENT MARSH BORINGS | Sheet 8 | Of
Made by [illegible] | Date 12 MAY 07
File No. 07-110 | Checked by | Date

LOUIS J. CAPOZZOLI & ASSOCIATES, INC. Geotechnical Engineers

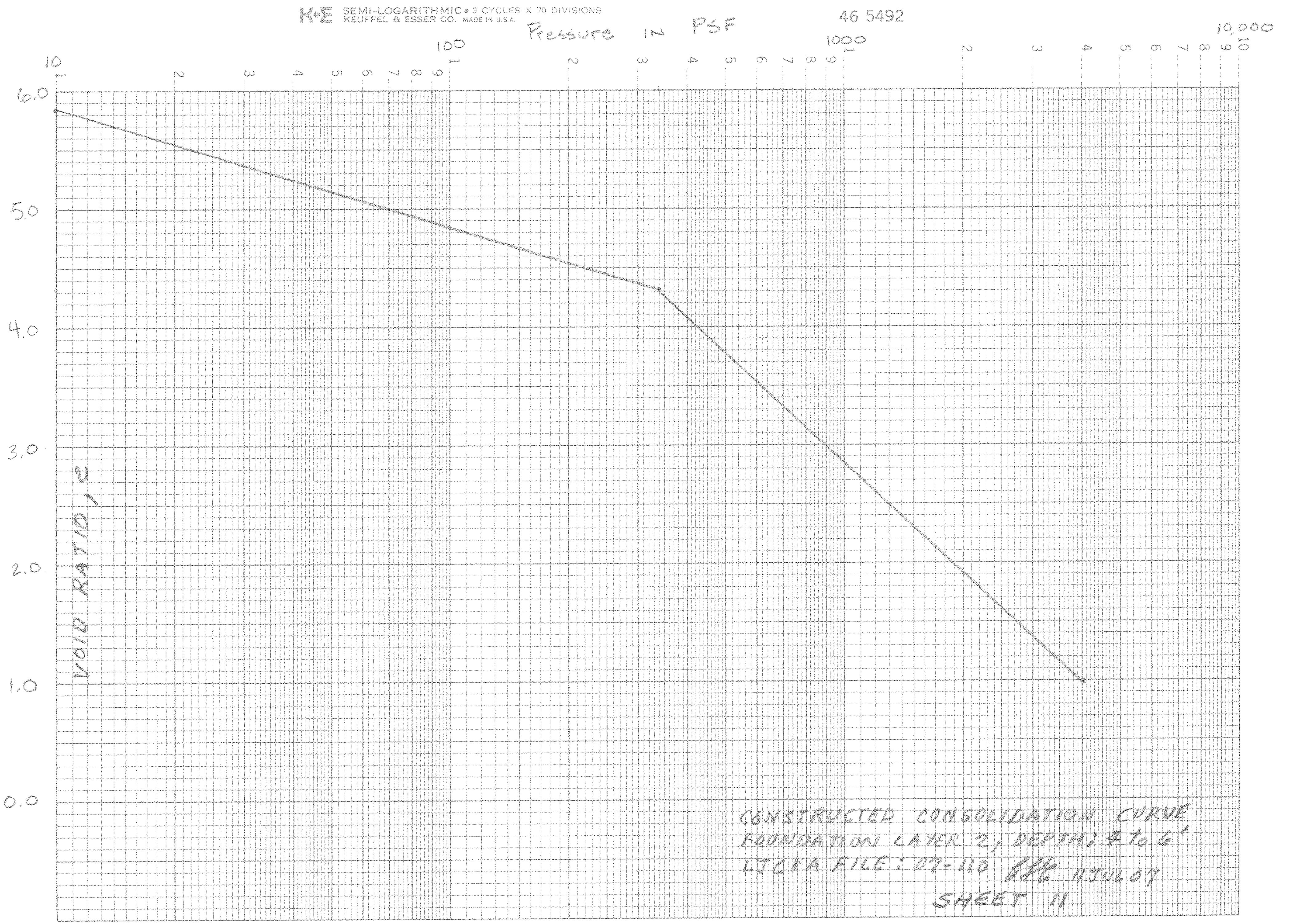


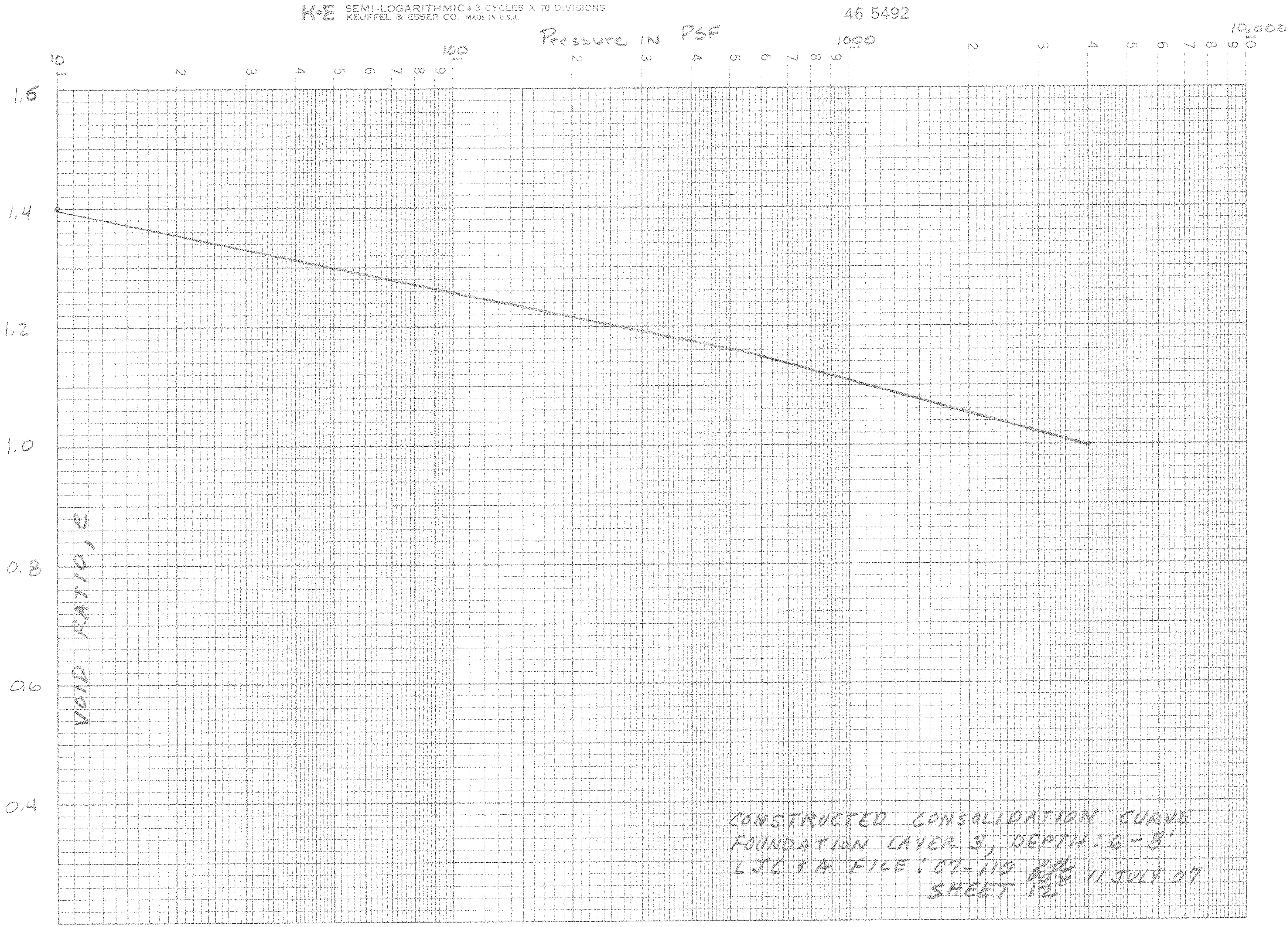
FOR NOTES AND LEGEND, SEE "SHEAR STRENGTH" SHEET FOR MARSH BORINGS

BAYOU DUPONT, PLAQUEMINES PARISH, LA LDNR (2503-05-44) SETTLEMENT DATA MARSH SOIL LJC&A FILE: 07-110						
FOUNDATION LAYER	ELEV. (Ft.), NAVD88	LAYER HEIGHT (FT.)	C_r	C_c	P_c (psf)	C_v (ft^2/day)
1	-1.3 to –5.3	4	010	0.62	122	0.040
2	-5.3 to –7.3	2	1.00	3.10	340	0.025
3	-7.3 to –9.3	2	.014	0.20	600	2.000
4	-9.3 to –41.3	32	0.10	0.80	2500	0.040
5	-41.3 to –61.3	20	0.14	0.68	1260	0.040


K+E SEMI-LOGARITHMIC • 3 CYCLES X 70 DIVISIONS
KEUFFEL & ESSER CO. MADE IN U.S.A.
46 5492
PRESSURE IN PSF
10
100
1000
10,000
VOID RATIO, e
1.6
1.4
1.2
1.0
0.8
0.6
0.4
CONSTRUCTED CONSOLIDATION CURVE
FOUNDATION LAYER 1, DEPTH 0-4'
LJC&A FILE: 07-110 11JUL07
SHEET 10

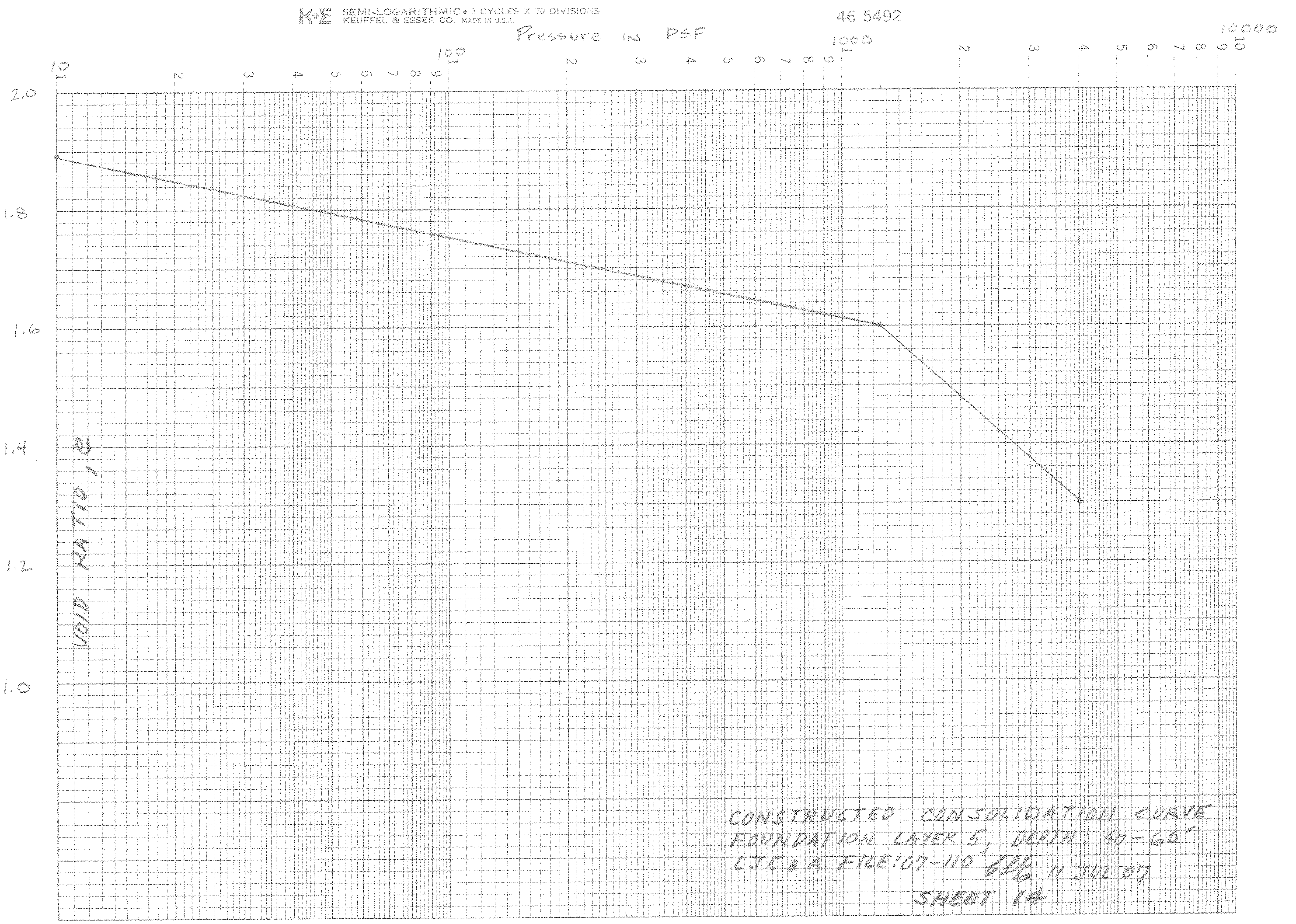
#3


K+E SEMI-LOGARITHMIC 3 CYCLES X 70 DIVISIONS
KEUFFEL & ESSER CO. MADE IN U.S.A.
46 5492
Pressure in PSF
10
100
1000
10,000
6.0
5.0
4.0
3.0
2.0
1.0
0.0
VOID RATIO, e
CONSTRUCTED CONSOLIDATION CURVE
FOUNDATION LAYER 2, DEPTH: 4 to 6'
LJC&A FILE: 07-110 11JUL07
SHEET 11

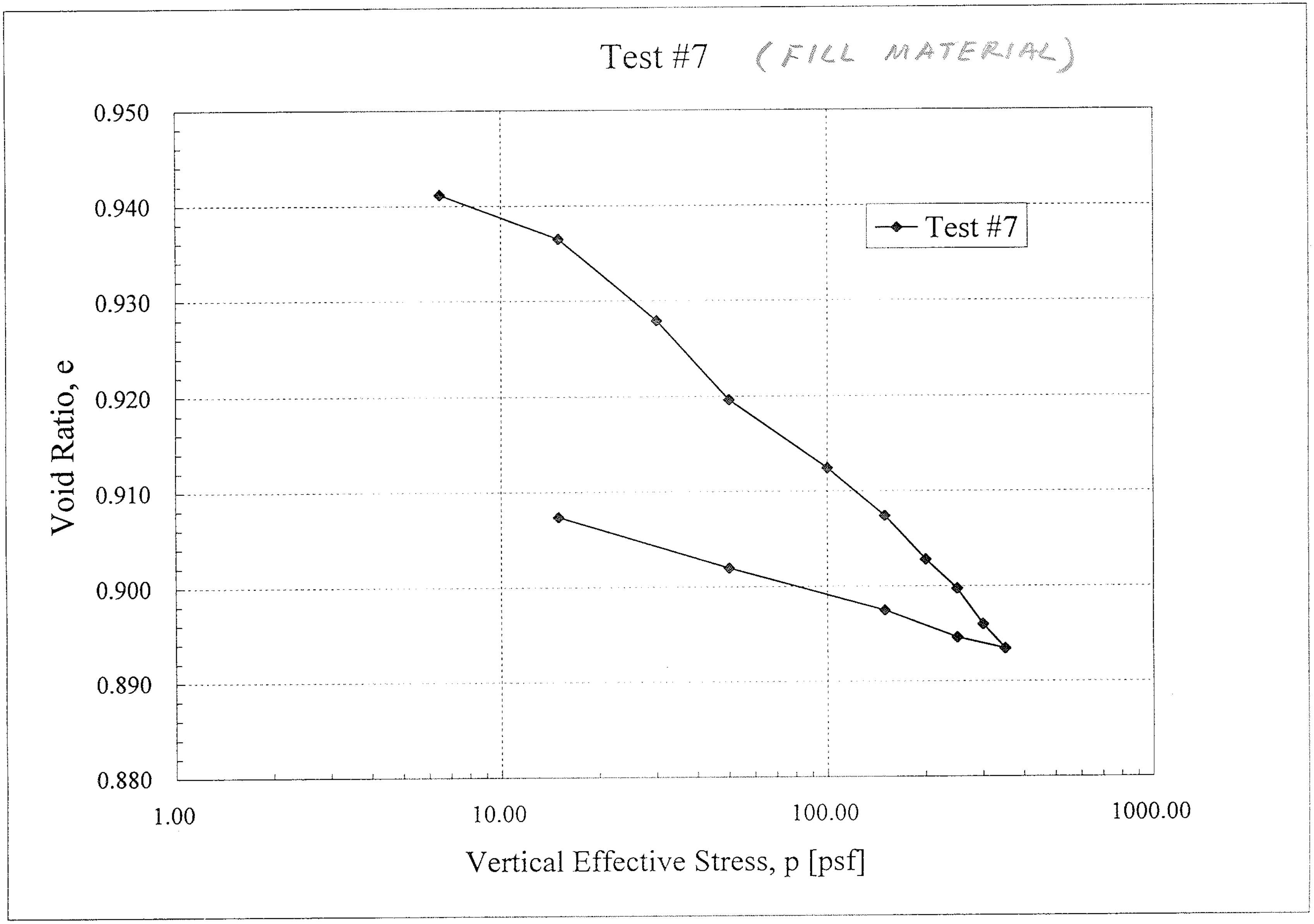

K+E SEMI-LOGARITHMIC • 3 CYCLES X 70 DIVISIONS
KEUFFEL & ESSER CO. MADE IN U.S.A.
46 5492
PRESSURE IN PSF
10
100
1000
10,000
VOID RATIO, e
1.6
1.4
1.2
1.0
0.8
0.6
0.4
CONSTRUCTED CONSOLIDATION CURVE
FOUNDATION LAYER 3, DEPTH: 6-8'
LJC & A FILE: 07-110 11 JULY 07
SHEET 12


K+E SEMI-LOGARITHMIC • 3 CYCLES X 70 DIVISIONS
KEUFFEL & ESSER CO. MADE IN U.S.A.
46 5492
PRESSURE IN PSF
10
100
1000
10,000
2.2
2.0
1.8
1.6
1.4
1.2
1.0
VOID RATIO, e
CONSTRUCTED CONSOLIDATION CURVE,
FOUNDATION LAYER 4, DEPTH: 8-40'
LJCEA FILE: 07-122 11 JUL 07
SHEET 13

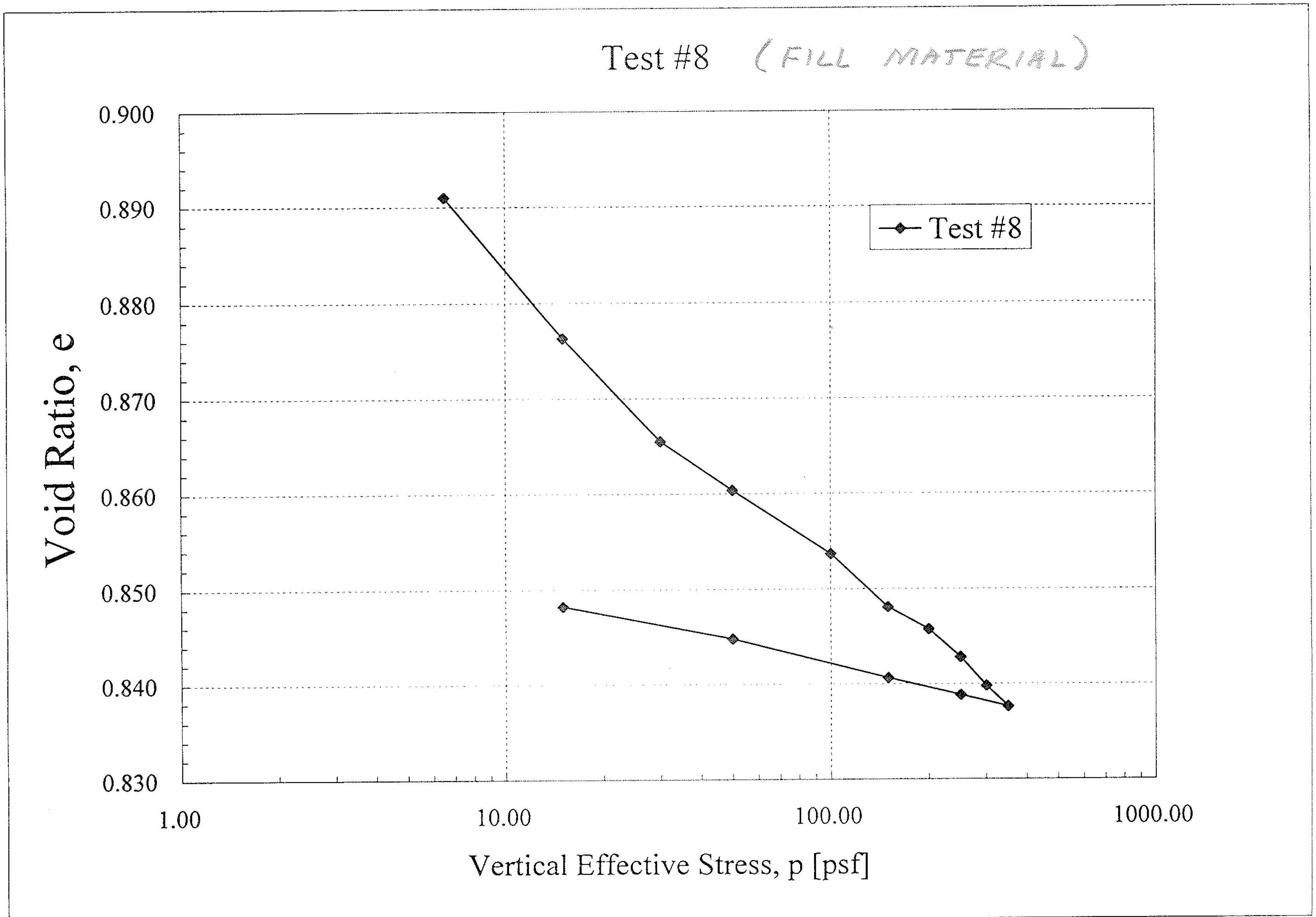
6



K+E SEMI-LOGARITHMIC • 3 CYCLES X 70 DIVISIONS
KEUFFEL & ESSER CO. MADE IN U.S.A.
46 5492
Pressure in PSF
10
100
1000
10000
2.0
1.8
1.6
1.4
1.2
1.0
VOID RATIO, e
CONSTRUCTED CONSOLIDATION CURVE
FOUNDATION LAYER 5, DEPTH: 40-60'
LJC#A FILE:07-110 11 JUL 07
SHEET 1#



LJC&A FILE: 07-110
SHEET 15



LJC & A FILE: 07-110
SHEET 16

LJC&A FILE: 07-110
SHEET 17

****

Consolidation and desiccation of soft layers---dredged fill

****

Problem Bayou Dupont Reclaimation 2503-05-44

******************Soil data for compressible foundation*****************

Material Type	Layer Thickness	Numbers of Sub-layers	Ca/Cc	Cr/Cc
2	4.00	9	.242	.166
3	2.00	6	.009	.323
4	2.00	6	.025	.700
5	32.00	10	.015	.125
6	20.00	10	.018	.206

Material type : 2 Specific Gravity of Solids: 2.72

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	1.550	.000E+00	.290E-02	.114E-02	.211E-02	-.100E+04	-.114E+01
2	1.540	.100E+02	.283E-02	.112E-02	.262E-02	-.102E+04	-.113E+01
3	1.430	.122E+03	.200E-02	.823E-03	.432E-03	-.384E+04	-.316E+01
4	.500	.400E+04	.100E-02	.667E-03	.168E-03	-.417E+04	-.278E+01

Material type : 3 Specific Gravity of Solids: 2.50

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	5.900	.000E+00	.284E-01	.412E-02	.768E-02	-.167E+03	-.686E+00
2	5.840	.100E+02	.250E-01	.365E-02	.220E-03	-.214E+03	-.782E+00
3	4.310	.340E+03	.200E-01	.377E-02	-.794E-03	-.824E+03	-.311E+01
4	1.000	.400E+04	.150E-01	.750E-02	-.113E-02	-.111E+04	-.829E+01

Material type : 4 Specific Gravity of Solids: 2.70

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	1.420	.000E+00	.284E-01	.117E-01	.659E-01	-.500E+03	-.587E+01
2	1.400	.100E+02	.250E-01	.104E-01	.176E-01	-.222E+04	-.231E+02
3	1.150	.600E+03	.150E-01	.698E-02	.198E-01	-.998E+04	-.696E+02
4	1.000	.400E+04	.500E-02	.250E-02	.298E-01	-.227E+05	-.567E+02

Material type : 5 Specific Gravity of Solids: 2.70

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	2.000	.000E+00	.283E-02	.945E-03	.104E-02	-.909E+02	-.859E-01
2	1.890	.100E+02	.240E-02	.830E-03	.544E-03	-.714E+04	-.593E+01
3	1.650	.250E+04	.200E-02	.755E-03	.110E-02	-.102E+05	-.772E+01

LJC&A FILE: 07-110
SHEET 18

4	1.500	.400E+04	.100E-02	.400E-03	.236E-02	-.100E+05	-.400E+01

Material type : 6 Specific Gravity of Solids: 2.70

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	2.000	.000E+00	.283E-02	.945E-03	.104E-02	-.909E+02	-.859E-01
2	1.890	.100E+02	.240E-02	.830E-03	.439E-03	-.315E+04	-.262E+01
3	1.600	.126E+04	.200E-02	.769E-03	.671E-03	-.676E+04	-.520E+01
4	1.300	.400E+04	.100E-02	.435E-03	.111E-02	-.913E+04	-.397E+01

**********************Soil data for dredged fill**********************

Material Type	Specific Gravity	Ca/Cc	Cr/Cc	Saturation Limit	Disication Limit
1	2.700	.020	.276	3.880	1.490

Material type : 1

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	.980	.000E+00	.200E+00	.101E+00	.122E+01	-.324E+03	-.327E+02
2	.960	.648E+01	.150E+00	.765E-01	.124E+01	-.375E+03	-.287E+02
3	.940	.150E+02	.100E+00	.515E-01	.139E+01	-.784E+03	-.404E+02
4	.930	.300E+02	.672E-01	.348E-01	.156E+01	-.175E+04	-.609E+02
5	.920	.500E+02	.390E-01	.203E-01	.142E+01	-.350E+04	-.711E+02
6	.910	.100E+03	.123E-01	.644E-02	.855E+00	-.750E+04	-.483E+02
7	.900	.200E+03	.610E-02	.321E-02	.274E+00	-.125E+05	-.401E+02
8	.890	.350E+03	.180E-02	.952E-03	.226E+00	-.150E+05	-.143E+02

Summary of lifts and print detail

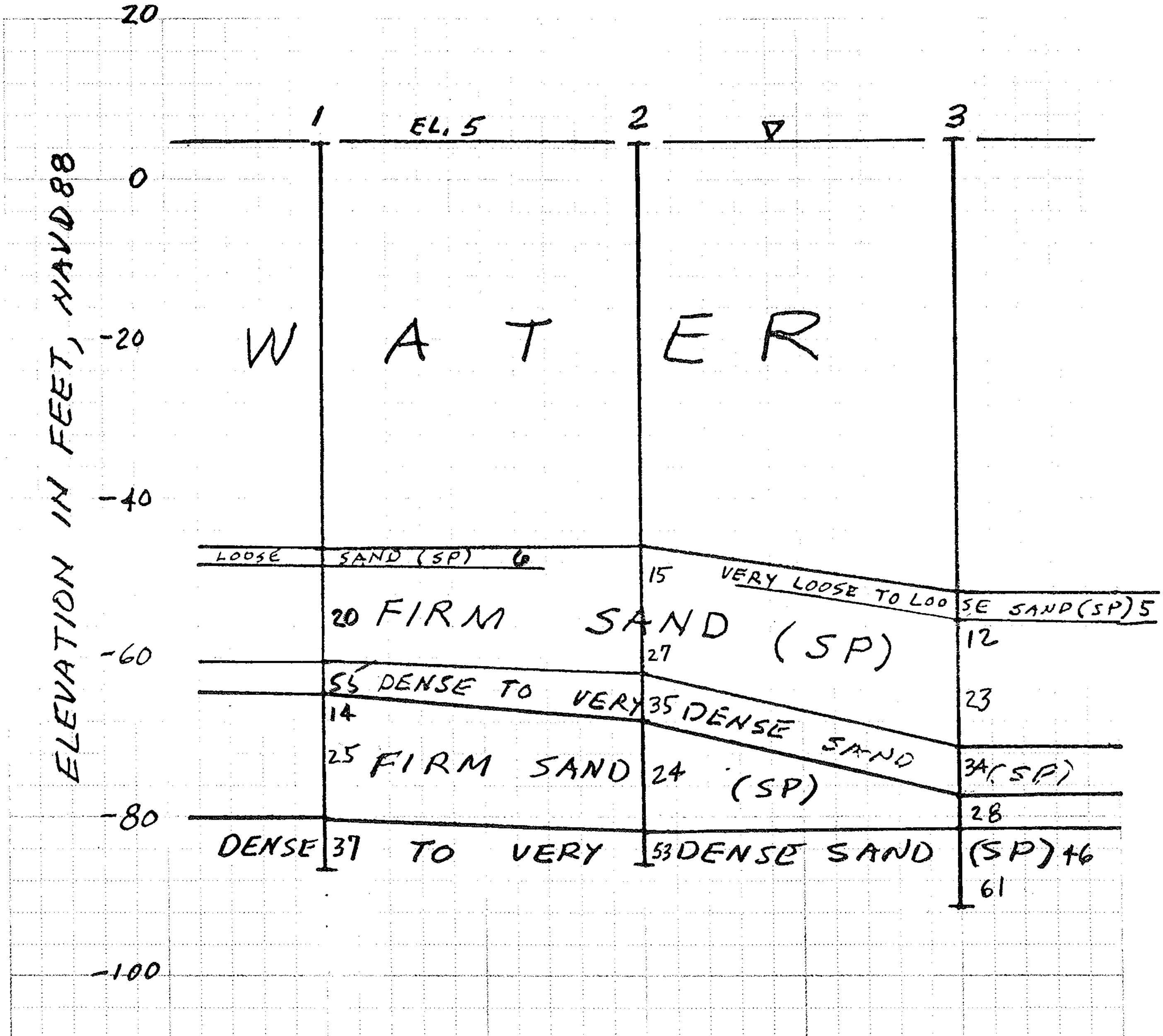
Time days	Material Type	Fill Height	# Sub-layers	Void ratio	Start Day	Dessic. Month	Print detai
0.	1	3.5	10	.98	10000.	6	1
120.					10000.	6	1
240.					10000.	6	1
365.					10000.	6	1
485.					10000.	6	1
605.					10000.	6	1
730.					10000.	6	1
850.					10000.	6	1
970.					10000.	6	1

Subject SUBSURFACE PROFILE — Sheet 18 — Of

BAYOU DUPONT — Made by CLE — Date 11 JUL 07

File No. 07-110 — Checked by — Date

LOUIS J. CAPOZZOLI & ASSOCIATES, INC. Geotechnical Engineers



NOTES: 1. SOIL CONDITIONS MAY VARY OUTSIDE OF THE BORINGS.

3. FOR DETAILS REFER TO THE INDIVIDUAL BORING LOGS.

3. ( ) GIVES UNIFIED SOIL CLASSIFICATION SYMBOL PER ASTM 2487.

4. NUMBERS TO THE RIGHT REFER TO AVERAGE SPT BLOW COUNT (N).

5. ▽ DENOTES FREE WATER LEVEL IN BOREHOLE AT TIME OF EXPLORATION.

▼ DENOTES WATER LEVEL AFTER 10 MINUTES.

Subject SUBSURFACE PROFILE — BAYOU DUPONT

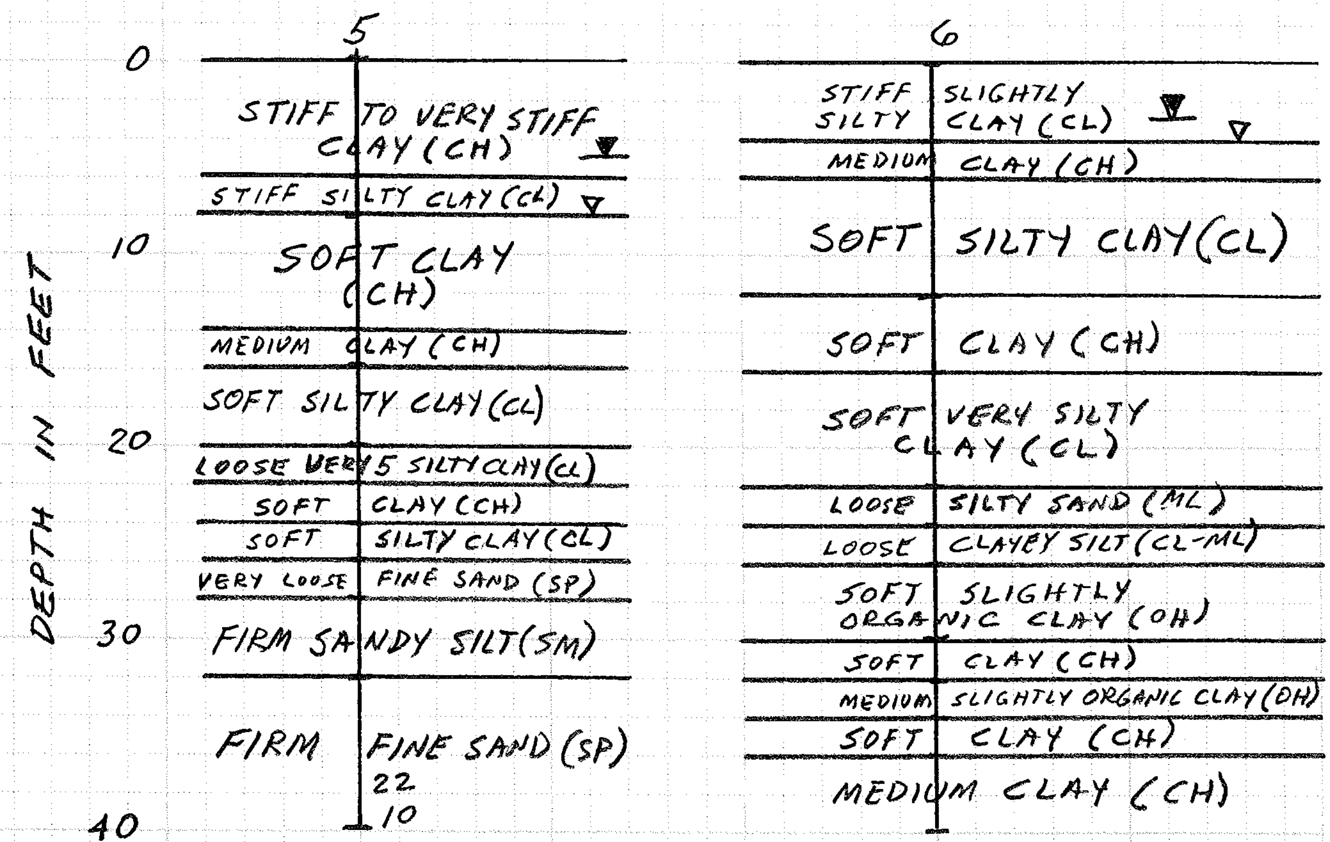
Sheet 19 Of

Made by CSE Date 11JUL07

File No. 07-110

Checked by Date

LOUIS J. CAPOZZOLI & ASSOCIATES, INC. Geotechnical Engineers



NOTES:
1. SOIL CONDITIONS MAY VARY OUTSIDE OF THE BORINGS.
2. FOR DETAILS REFER TO THE INDIVIDUAL BORING LOGS.
3. ( ) GIVES UNIFIED SOIL CLASSIFICATION SYMBOL PER ASTM 2487.
4. NUMBERS TO THE RIGHT REFER TO SPT BLOW COUNT (N).
5. ▽ DENOTES FREE WATER LEVEL IN BOREHOLE AT TIME OF EXPLORATION.
▼ DENOTES WATER LEVEL AFTER 10 MINUTES.

APPENDIX A

FIELD AND LABORATORY ANALYSES

Borings. Five borings - ranging in depth from 40 to 104 feet and designated 1 through 3 plus 5 and 6 - were taken between 16 and 24 May 2007 at the locations shown on sheet 1. Inriver boring (1 through 3) depths are from the top of casing, which was set above the marine platform surface. The top of casing is zero depth. Drilling for these borings was performed with our Failing 1500 skid-mounted rotary type drill rig mounted on a barge/tow vessel system. Borings 5 and 6 were drilled on land utilizing drill rig after unloading from barge. The ground surface is zero depth. The sixth proposed boring (number 4) was eliminated from the scope of work. This boring was to be in the river at a borrow area downstream of the area for borings 1, 2, and 3. All borings were advanced with a wet drilling process. The top 40 feet below mud line or from the ground surface of each boring was sampled continuously. Undisturbed samples of cohesive soils, suitable for laboratory analyses, were obtained by hydraulically pushing a 30 inch long, 3 inch O.D. thinwall Shelby tube sampler into the ground a distance of 24 inches at a time. Classification samples of cohesionless soils (sand and gravel) were extracted via the Standard Penetration Test - i.e. SPT blow counts. The SPT technique entailed driving a 24 inch long, 2 inch O.D., steel spoon into the ground with blows from a 140 pound hammer falling 30 inches. The resulting penetration resistance was the number of blows required to advance the penetrometer spoon 12 inches after first seating it for 6 inches.

The samples were classified in the field by our technician and then the Shelby tubes were prepared for transport. The tubes were sealed and stored/transported in a vertical position with the bottom of the sample (tube) downward. Immediately upon completion of sampling, the boreholes were backfilled as per statutory requirements. The detailed boring logs are attached at the back of this report. Summarized boring data is as follows:

Inriver Boring Number	Total Depth (Feet)	Continuous Sampling (Feet)	Sampling at Depths >50 Feet (Feet)	Sampling at Depths >100 Feet (Feet)
1	98	40	48	--
2	98	40	48	--
3	104	40	50	4
Totals	300	120	146	4

Onland Boring Number	Total Depth (Feet)	Continuous Sampling (Feet)
5	40	40
6	40	40
Totals	80	80

Expenses incurred included: mobilization/demobilization of our drilling equipment between our Baton Rouge facility and your site, 16 hours rig-up/rig-down, 24 hours standby, 5 days rental of barge/towboat, plus 5 days crew living expenses. Maritime insurance coverage charges apply to the water borings.

Laboratory. All soil samples in the Shelby tubes from the above field operations were taken to our soil mechanics laboratory where additional analyses were performed. The samples were each tested with a mini vane instrument and extruded. Laboratory testing included 6 unconfined and 29 unconsolidated, undrained triaxial compression tests with moisture content and density determinations, 29 miniature vane shear tests, 5 dry sieve analyses, 1 separate moisture content determination, plus 10 Atterberg limit determinations.

In addition 36 sample extrusions from Shelby tubes and 36 preserving extruded samples were done at our office.

The miniature vane shear and compression tests provided the strength properties of the soil. The moisture content and density, dry sieve analyses, plus Atterberg limit determination data furnished additional soil classifications to supplement field methods and provided other design parameters. The results of these analyses and 35 stress versus strain curves are on the tables and figures in an appendix.

Field and laboratory analyses provide the bases, along with our expertise and experience, for evaluating this site for the proposed project. All field and laboratory testing conformed to appropriate ASTM standards.

LOG OF BORING LEGEND

LJC&A: 07-110

1.	SPT	=	Standard Penetration Test (4/6/9) where 4 is the blows to seat and 15 is blows (N) for 12 inch penetration.
2.	QU (TSF)	=	Unconsolidated undrained triaxial, one point test 0.05 @ 0.12 is the compressive strength in tsf which is twice the cohesion and @ means the confining pressure at tsf. Note: tests without @ values following are for unconfined Compression shear tests.
3.	WC (%)	=	In situ water content
4.	Dry Wt. (PCF)	=	The dry unit weight of soil
5.	LL	=	Liquid Limit (%)
6.	PI	=	Plasticity Index (%)
7.	MV(KSF)	=	Miniature vane strength test done in end of sample in the Shelby tube and value is the cohesion in KSF.

LOG OF BORING

Project:	Bayou Dupont Plaquemines Parish, Louisiana Louisiana Department of Natural Resources (2503-05-44)	Boring:	1
		File:	07-110
For:	Sigma Consulting Group Baton Rouge, Louisiana	Date:	24-May-07
		Technician:	CAL

Undisturbed Sample
Standard Penetration Test
Classification Sample
(SLS) Slickensided

N 29° 42.099'
W 89° 58.799'

Boring Depth: 98 Feet

Depth Feet	SAMPLES	Description	(SPT) Recovery %	UU(TSF)	WC(%)	Dry Wt. (PCF)	LL	PI	MV(KSF)
0		Zero = top of casing, set 62 feet of 8 inch casing 74 feet of 4 inch casing; top of casing to water is 7 feet							
		Water surface El. 5.0 feet NAVD 88 (Estimated)							
		Water depth = 51.5 feet							
		Mudline El. -46.5 feet, NAVD 88							
		Loose brown sand (SP) 6 blows per foot (2/3/3)	66						
60		Firm brown sand (SP) 20 blows per foot (10/11/9)	61						
		Firm brown sand (SP) 23 blows per foot (9/11/12)	100						
65		Firm brown sand (SP) 20 blows per foot (10/10/10)	70						
		Firm gray sand (SP) 18 blows per foot (7/9/9)	72						
		Firm gray sand (SP) 20 blows per foot (5/9/11)	72						
70		Firm gray sand with clay layer (SP) 20 blows per foot (6/8/12)	100						
		Very dense gray sand with clay layer, shells, wood, and organic (SP) 58 blows per foot (15/32/26)	70						
75		Very dense gray sand with organic (SP) 51 blows per foot (19/27/24)	80						
		Firm gray sand (SP) 14 blows per foot (5/6/8)	53						
		Firm gray sand (SP) 14 blows per foot (5/6/8)	60						
80		Firm gray sand (SP) 25 blows per foot (13/12/13)	100						
		Firm gray sand (SP) 27 blows per foot (13/13/14)	100						
85		Firm gray sand (SP) 34 blows per foot (15/14/20)	100						
		Firm gray sand (SP) 15 blows per foot (5/6/9)	90						
		Firm gray sand (SP) 22 blows per foot (10/12/10)	100						
90		Firm gray sand (SP) 25 blows per foot (9/11/14)	100						
		Dense gray sand with organic (SP) 27 blows per foot (10/13/14)	100						
95		Dense gray sand with organic (SP) 37 blows per foot (11/14/23)	90						
		Dense gray sand with organic (SP) 46 blows per foot (11/22/24)	100						
100									

LOG OF BORING

Project: Bayou Dupont
Plaquemines Parish, Louisiana
Louisiana Department of Natural Resources (2503-05-44)
For: Sigma Consulting Group
Baton Rouge, Louisiana

Boring: 2
File: 07-110
Date: 16-May-07
Technician: CAL

Undisturbed Sample
Standard Penetration Test
Classification Sample
(SLS) Slickensided

N 29° 42.490'
W 89° 58.992'

Boring Depth: 98 Feet

Depth Feet	SAMPLES	Description	(SPT) Recovery %	UU(TSF)	WC(%)	Dry Wt. (PCF)	LL	PI	MV(KSF)
0		Zero = top of casing, set 62 feet of 8 inch casing; top of casing to water is 7 feet							
		Water surface El. 5.0 feet NAVD 88 (Estimated)							
		Water depth = 51.0 feet							
		Mudline El. -46.0 feet, NAVD 88							
	SPT	Firm brown sand (SP) 14 blows per foot (5/6/8)	44						
60	SPT	Firm brown sand with organic matter (SP) 17 blows per foot (8/9/8)	90						
	SPT	Firm brown sand (SP) 15 blows per foot (4/6/9)	20						
65	SPT	Firm gray sand with organic matter (SP) 23 blows per foot (8/8/15)	80						
	SPT	Firm gray sand (SP) 20 blows per foot (9/10/10)	55						
	SPT	Firm gray sand (SP) 23 blows per foot (9/11/12)	60						
70	SPT	Firm gray sand (SP) 27 blows per foot (13/13/14)	70						
	SPT	Firm gray sand (SP) 27 blows per foot (14/14/13)	80						
75	SPT	Dense gray sand 32 blows per foot (14/15/17)	80						
	SPT	Dense gray sand (SP) 37 blows per foot (17/18/19)	100						
	SPT	Dense gray sand (SP) 33 blows per foot (17/17/16)	100						
80	SPT	Firm gray sand (SP) 25 blows per foot (9/13/12)	70						
	SPT	Firm gray sand (SP) 27 blows per foot (14/14/13)	70						
85	SPT	Firm gray sand (SP) 29 blows per foot (12/14/15)	70						
	SPT	Firm gray sand (SP) 20 blows per foot (9/9/11)	55						
	SPT	Firm gray sand (SP) 13 blows per foot (7/8/5)	55						
90	SPT	Firm gray sand with organic matter (SP) 22 blows per foot (11/11/11)	80						
	SPT	Firm gray sand with organic matter (SP) 29 blows per foot (13/16/13)	100						
95	SPT	Very dense gray sand (SP) 50 blows per foot (18/24/26)	90						
	SPT	Very dense gray sand (SP) 56 blows per foot (19/27/29)	60						
100									

LOG OF BORING

Project:	Bayou Dupont Plaquemines Parish, Louisiana Louisiana Department of Natural Resources (2503-05-44)	Boring:	3
		File:	07-110
For:	Sigma Consulting Group Baton Rouge, Louisiana	Date:	17-May-07
		Technician:	CAL

■ Undisturbed Sample
☒ Standard Penetration Test
▯ Classification Sample
(SLS) Slickensided

N 29° 42.950'
W 89° 59.203'

Boring Depth: 104 Feet

Depth Feet	SAMPLES	Description	(SPT) Recovery %	UU(TSF)	WC(%)	Dry Wt. (PCF)	LL	PI	MV(KSF)
0		Zero = top of casing, set 64 feet of 8 inch casing; top of casing to water is 7 feet							
		Water surface El. 5.0 feet NAVD 88 (Estimated)							
		Water depth = 57.0 feet							
		Mudline El. -52.0 feet, NAVD 88							
65	☒	Loose brown sand with wood (SP) 7 blows per foot (3/3/4)	60						
	☒	Very loose brown sand (SP) 3 blows per foot (2/2/1)	75						
	☒	Firm gray sand with organic matter (SP) 10 blows per foot (5/5/5)	80						
70	☒	Firm gray sand (SP) 10 blows per foot (3/4/6)	90						
	☒	Firm gray sand (SP) 15 blows per foot (8/7/8)	90						
75	☒	Firm gray sand (SP) 18 blows per foot (6/7/11)	90						
	☒	Firm gray sand (SP) 26 blows per foot (12/12/14)	100						
	☒	Firm gray sand (SP) 22 blows per foot (9/11/11)	60						
80	☒	Firm gray sand (SP) 22 blows per foot (10/11/11)	90						
	☒	Firm gray sand (SP) 25 blows per foot (13/12/13)	80						
85	☒	Dense gray sand (SP) 32 blows per foot (16/15/17)	75						
	☒	Dense gray sand (SP) 37 blows per foot (17/17/20)	60						
	☒	Dense gray sand (SP) 32 blows per foot (16/15/17)	50						
90	☒	Firm gray sand (SP) 29 blows per foot (9/13/16)	80						
	☒	Firm gray sand (SP) 27 blows per foot (15/14/13)	80						
95	☒	Dense gray sand (SP) 42 blows per foot (18/20/22)	60						
	☒	Dense gray sand (SP) 49 blows per foot (22/23/26)	50						
	☒	Very dense gray sand (SP) 56 blows per foot (20/27/29)	50						
100	☒	Very dense gray sand (SP) 63 blows per foot (20/31/32)	55						
	☒	Very dense gray sand (SP) 63 blows per foot (30/32/31)	80						
105									

LOUIS J. CAPOZZOLI & ASSOCIATES, INC.
Geotechnical Engineers

LOG OF BORING

Project: Bayou Dupont
Plaquemines Parish, Louisiana
Louisiana Department of Natural Resources (2503-05-44)
For: Sigma Consulting Group
Baton Rouge, Louisiana

Boring: 5
File: 07-110
Date: 24-May-07
Technician: CAL

Undisturbed Sample
Standard Penetration Test
Classification Sample
(SLS) Slickensided

N 29° 40.120'
W 89° 58.606'

Boring Depth: 40 Feet

Depth Feet	Description	(SPT) Recovery %	UU(TSF)	WC(%)	Dry Wt. (PCF)	LL	PI	MV(KSF)
0	Very stiff tan and gray clay with grass roots and glass (CH)	95	2.22	33	87.5	72	45	--
	Stiff tan and gray clay with silt streaks and pockets (CH)	95	1.42	37	84.5	--	--	1.15
5	Stiff tan and gray clay with ferrous nodules (CH)	90	1.00	37	83.9	--	--	.63
	Medium gray silty clay with silt streaks (CL)	90	0.5	43	80.1	40	19	.49
	Free water encountered at 8 feet; rose to 5 feet in 10 minutes							
10	Soft gray clay (CH)	1.00	0.38 @.52	44	80.4	--	--	.46
	Soft gray clay (CH)	95	0.47@.65	37	81.4	--	--	.35
	Soft gray clay with silt streaks (CH)(SLS)	95	0.32@.75	38	82.0	--	--	.28
15	Medium gray clay with silt lenses (CH)	100	.60@.86	38	84.6	77	49	.22
	Soft gray silty clay (CH)	100	.34@.99	37	80.9	--	--	.06
	No sample recovered							
20	Loose gray very silty clay (CL) 5 blows per foot (1/2/3)	80	--	--	--	--	--	--
	Soft gray clay (CH)	100	.25@1.34	39	79.6	--	--	0.33
25	Soft gray silty clay (CL) with 3" silt layer	50	.27@1.45	46	82.1	35	16	.03
	Very loose gray fine sand (SP)	50	1.15@1.56	28	89.6	--	--	--
	Firm gray sandy silt (SM) with clay traces	40	1.24@1.68	28	93.3	--	--	0.05
30	Firm gray sandy silt (SM) with ½" clay layer	100	.55@1.80	27	94.4	--	--	--
	Firm gray fine sand (SP)	100	.64@1.92	31	85.8	--	--	--
35	Firm gray fine sand (SP) with ½" clay layer	65	.50@2.03	27	86.0	--	--	--
	Firm gray fine sand with silt traces (SP) 22 blows per foot (5/8/14)	75	--	--	--	--	--	--
	Firm gray fine sand with silt traces (SP) 10 blows per foot (6/7/3)	75	--	--	--	--	--	--
40	Clay encountered at 40 feet							

LOUIS J. CAPOZZOLI & ASSOCIATES, INC.
Geotechnical Engineers

LOG OF BORING

Project:	Bayou Dupont Plaquemines Parish, Louisiana Louisiana Department of Natural Resources (2503-05-44)	Boring: File:	6 07-110
For:	Sigma Consulting Group Baton Rouge, Louisiana	Date: Technician:	24-May-07 CAL

■ Undisturbed Sample
⊠ Standard Penetration Test
▯ Classification Sample
(SLS) Slickensided

N 29° 42.019'
W 89° 59.218'
Free water encountered at 4 feet; rose to 3 feet in 10 minutes
Boring Depth: 40 Feet

Depth Feet	SAMPLES	Description	(SPT) Recovery %	UU(TSF)	WC(%)	Dry Wt. (PCF)	LL	PI	MV(KSF)
0		Stiff brown, tan, and gray slightly silty clay with roots, shells, and sand pockets(CL)	95	1.13	23	99.1	--	--	2.1
		Stiff brown, tan, and gray slightly silty clay (CL)	90	1.06	26	96.1	47	26	.82
5		Medium tan and gray clay (CH) with silt streaks and pockets	90	.54@.32	32	91.5	--	--	.44
		Soft gray slightly silty clay (CL)	50	.35@.40	37	86.9	48	26	--
		Soft tan and gray silty clay with shells(CL) 2 blows per foot (2/1/1)	100	--	31	--	41	22	--
10		Soft gray silty clay (CL)	90	.42@.65	35	92.3	--	--	.23
		Soft gray clay with sand streaks and pockets (CH)	90	.32@.75	46	77.8	--		.16
15		Soft gray clay with organic matter traces (CH)	90	.45@.86	41	77.4	52	29	.25
		Soft gray very silty clay (CL)	100	.28 @ .99	38	81.6	--	--	.15
		Soft gray very silty clay with 3" sandy silt layer (CL)	90	.48@1.09	35	83.7	--	--	.08
20		Soft gray very silty clay with 2½" clayey silt layer (CL)	90	.26@1.22	38	84.1	34	14	.29
		Loose gray silty sand with clay traces (ML)	90	.69@1.34	33	82.0	--	--	--
25		Loose gray clayey silt with 1" clay layer (CL-ML)	95	.49@1.45	36	79.3	--	--	.09
		Soft gray slightly organic clay with silt pockets (OH)	100	.37@1.56	73	57.8	--	--	1.75
		Medium gray slightly organic clay with shells (OH)	100	.60@1.68	71	56.1	--	--	.32
30		Soft gray clay (CH)	90	.30@1.80	50	66.9	--	--	.25
		Medium gray slightly organic clay (OH)	100	.58@1.92	72	57.9	--	--	.45
35		Soft gray clay (CH) (SLS)	100	.46@2.03	64	61.3	93	69	.17
		Medium gray clay (CH)	100	.50@2.15	58	63.6	--	--	.38
		Medium gray clay with silt lenses (CH)	100	.57@2.26	54	68.0	--	--	.49
40									

LOUIS J. CAPOZZOLI & ASSOCIATES, INC.
Geotechnical Engineers

BAYOU DUPONT, PLAQUEMINES PARISH

LABORATORY DATA

TABLE 1

FILE NO: 07-110

BORING NUMBER	DEPTH FEET	MOISTURE %	UNIT WEIGHT		ATTERBERG LIMITS			COMPRESSION TEST				TEST TYPE
			WET PCF	DRY PCF	LL	PL	PI	TSF	% STRAIN	START PRESSURE KSF	TYPE FAILURE	
1	0.0 - 0.0											Dry Sieve
2	0.0 - 0.0											Dry Sieve
3	0.0 - 0.0											Dry Sieve
5	0.0 - 2.0	33	116.8	87.5	72	27	45	2.22	8		Multiple Shear	U
5	2.0 - 4.0	37	115.9	84.5				1.42	10		Multiple Shear	U
5	4.0 - 6.0	37	114.8	83.9				1.00	15		Yield	U
5	6.0 - 8.0	43	114.5	80.1	40	21	19	0.50	12		Multiple Shear	U
5	8.0 - 10.0	44	115.5	80.4				0.38	15	.52	Yield	QU
5	10.0 - 12.0	37	111.7	81.4				0.47	15	.65	Yield	QU
5	12.0 - 14.0	38	113.3	82.0				0.32	9	.75	SLS (60 Degrees)	QU
5	14.0 - 16.0	38	116.5	84.6	77	28	49	0.60	15	.86	Yield	QU
5	16.0 - 18.0	37	110.9	80.9				0.34	15	.99	Yield	QU
5	22.0 - 24.0	39	110.4	79.6				0.25	15	1.34	Yield	QU
5	24.0 - 26.0	46	119.6	82.1	35	19	16	0.27	15	1.45	Yield	QU
5	26.0 - 28.0	28	114.3	89.6				1.15	15	1.56	Yield	QU
5	28.0 - 30.0	28	119.1	93.3				1.24	13	1.68	Bulge	QU
5	30.0 - 32.0	27	119.7	94.4				0.55	8	1.80	Bulge	QU
5	32.0 - 34.0	31	112.5	85.8				0.64	2	1.92	Bulge	QU
5	34.0 - 36.0	27	109.1	86.0				0.50	3	2.03	Bulge	QU
5	36.5 - 38.0											Dry Sieve
5	38.5 - 40.0											Dry Sieve
6	0.0 - 2.0	23	121.9	99.1				1.13	13		Multiple Shear	U
6	2.0 - 4.0	26	120.9	96.1	47	21	26	1.06	15		Yield	U
6	4.0 - 6.0	32	121.2	91.5				0.54	14	.30	Multiple Shear	QU
6	6.0 - 8.0	37	118.6	86.9	48	22	26	0.35	15	.40	Yield	QU
6	8.5 - 10.0	31			41	19	22					MC
6	10.0 - 12.0	35	124.8	92.3				0.42	14	.65	Multiple Shear	QU
6	12.0 - 14.0	46	113.5	77.8				0.32	13	.75	Multiple Shear	QU
6	14.0 - 16.0	41	108.9	77.4	52	23	29	0.45	12	.86	Multiple Shear	QU
6	16.0 - 18.0	38	112.9	81.6				0.28	10	.99	Multiple Shear	QU
6	18.0 - 20.0	35	112.8	83.7				0.48	10	1.09	Multiple Shear	QU
6	20.0 - 22.0	38	116.3	84.1	34	20	14	0.26	15	1.22	Yield	QU
6	22.0 - 24.0	33	108.8	82.0				0.69	15	1.34	Yield	QU
6	24.0 - 26.0	36	107.5	79.3				0.49	15	1.45	Yield	QU
6	26.0 - 28.0	73	99.9	57.8				0.37	13	1.56	Multiple Shear	QU

BAYOU DUPONT, PLAQUEMINES PARISH

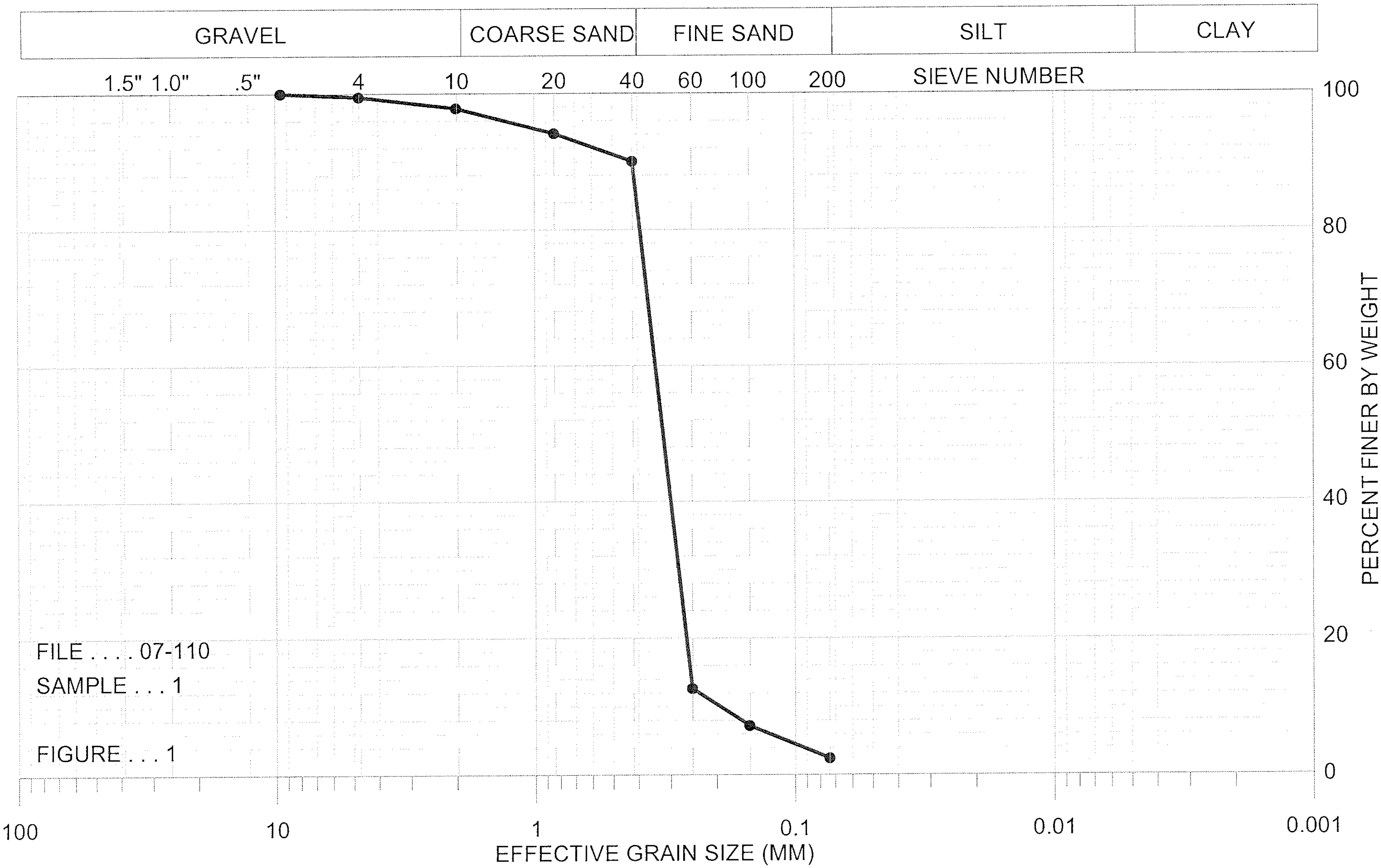
LABORATORY DATA

TABLE 2

FILE NO: 07-110

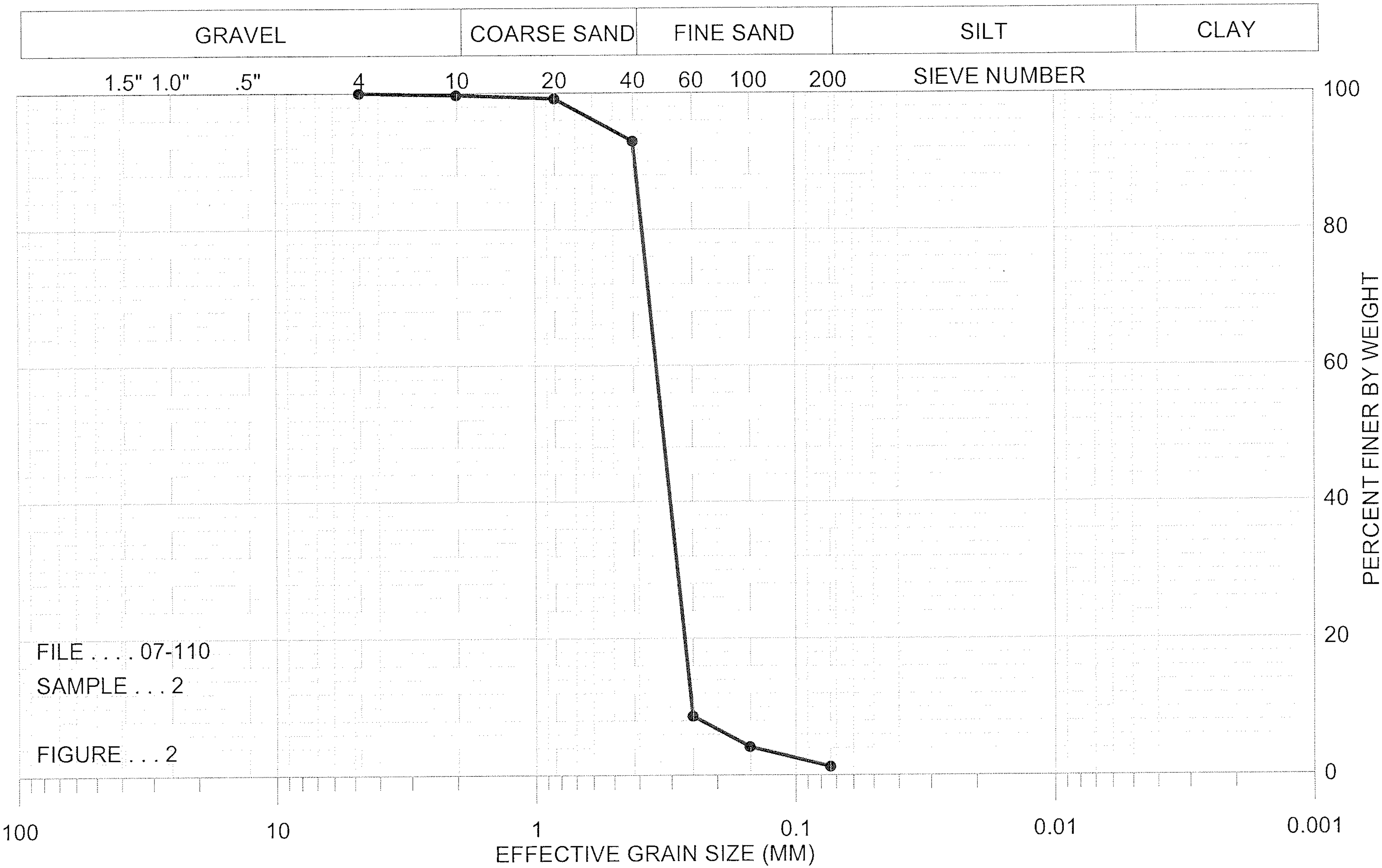
BORING NUMBER	DEPTH FEET	MOISTURE %	UNIT WEIGHT		ATTERBERG LIMITS			COMPRESSION TEST				TEST TYPE
			WET PCF	DRY PCF	LL	PL	PI	TSF	% STRAIN	START PRESSURE KSF	TYPE FAILURE	
6	28.0 - 30.0	71	95.9	56.1				0.60	10	1.68	Multiple Shear	QU
6	30.0 - 32.0	50	100.2	66.9				0.30	15	1.80	Yield	QU
6	32.0 - 34.0	72	99.6	57.9				0.58	10	1.92	Multiple Shear	QU
6	34.0 - 36.0	64	100.6	61.3	93	24	69	0.46	9	2.03	SLS (60 Degrees)	QU
6	36.0 - 38.0	58	100.3	63.6				0.50	15	2.15	Yield	QU
6	38.0 - 40.0	54	104.4	68.0				0.57	11	2.26	Multiple Shear	QU

GRAIN SIZE CURVE



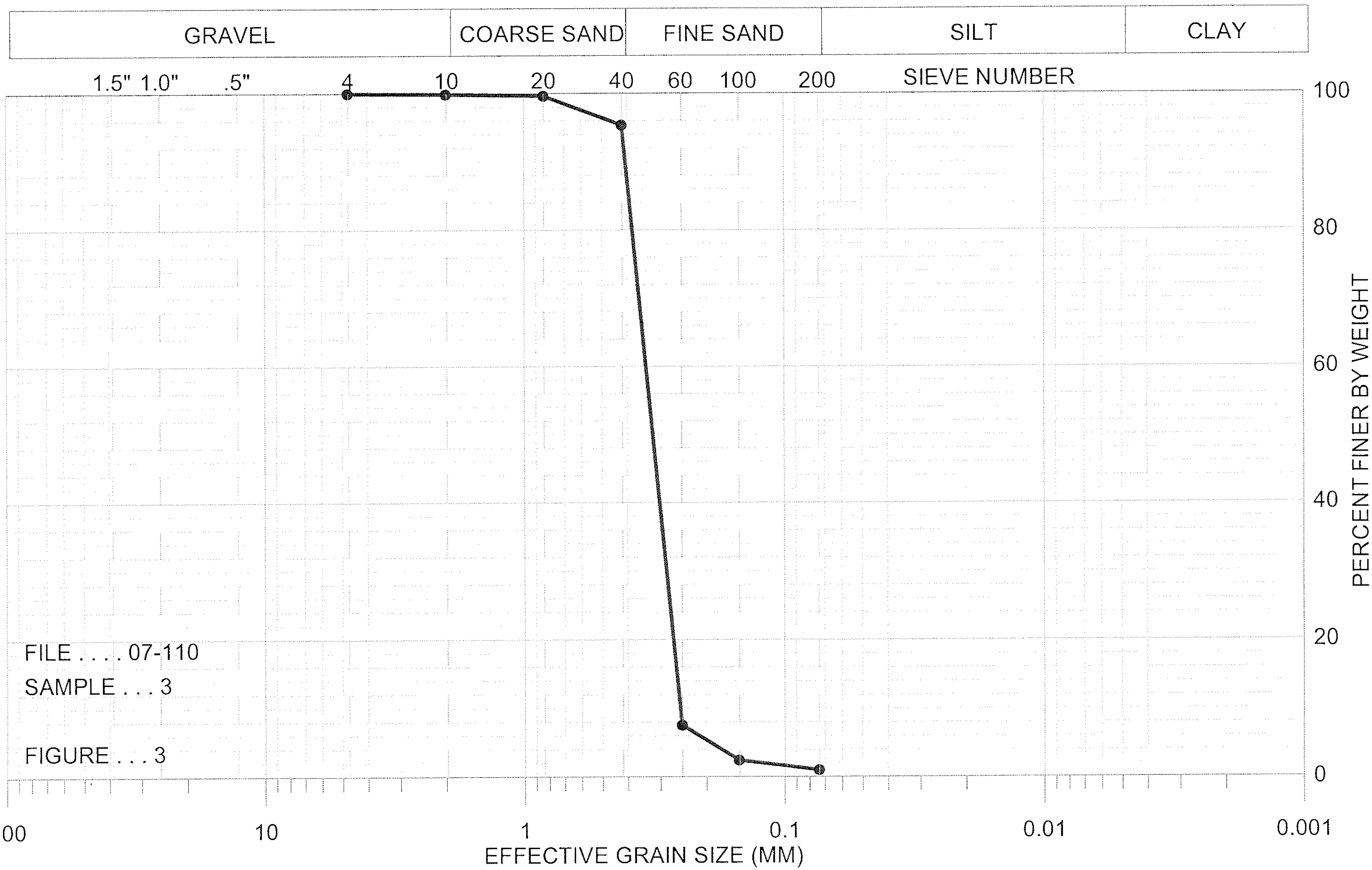
Louis J. Capozzoli & Associates, Inc.
Geotechnical Engineers

GRAIN SIZE CURVE



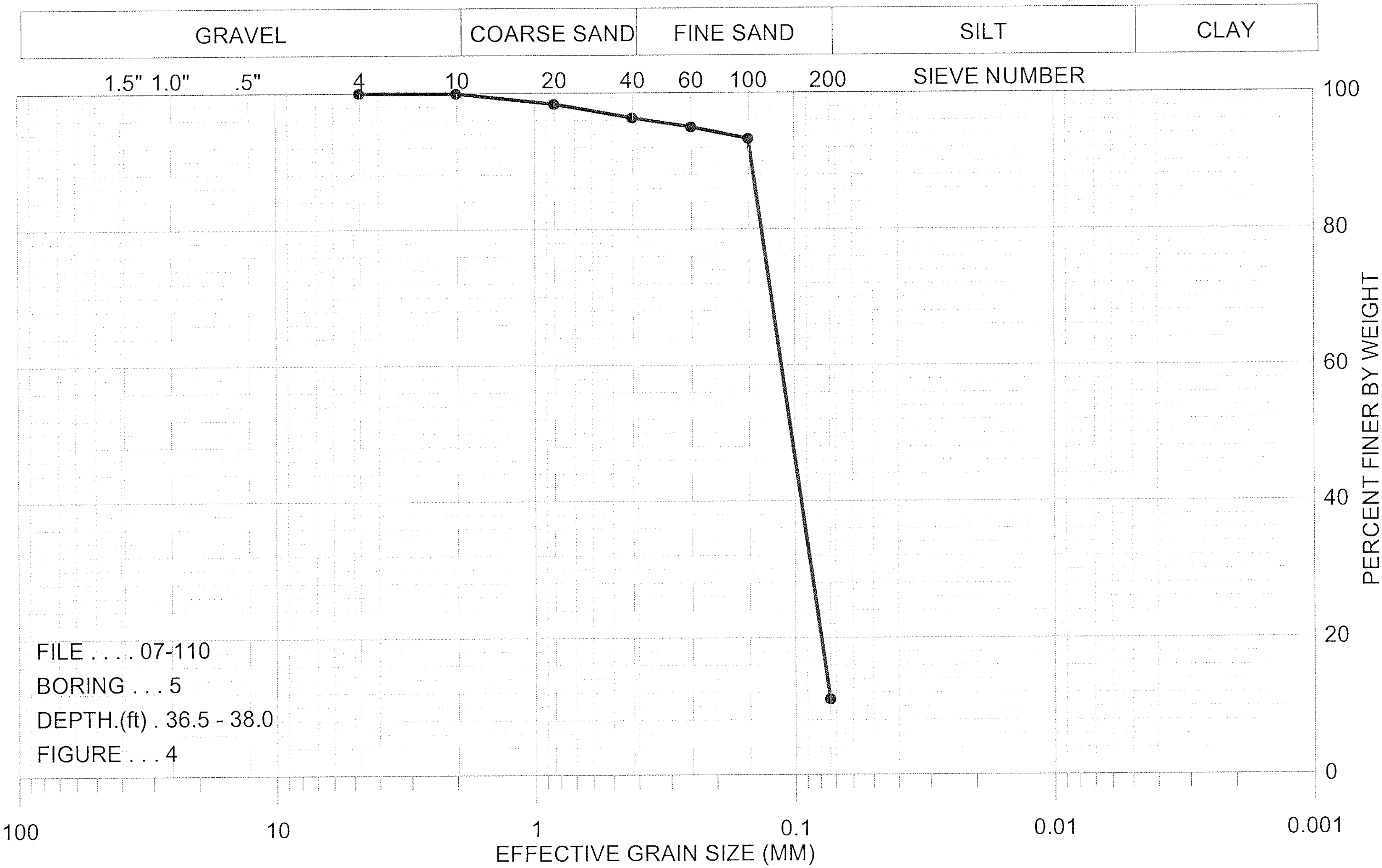
Louis J. Capozzoli & Associates, Inc.
Geotechnical Engineers

GRAIN SIZE CURVE



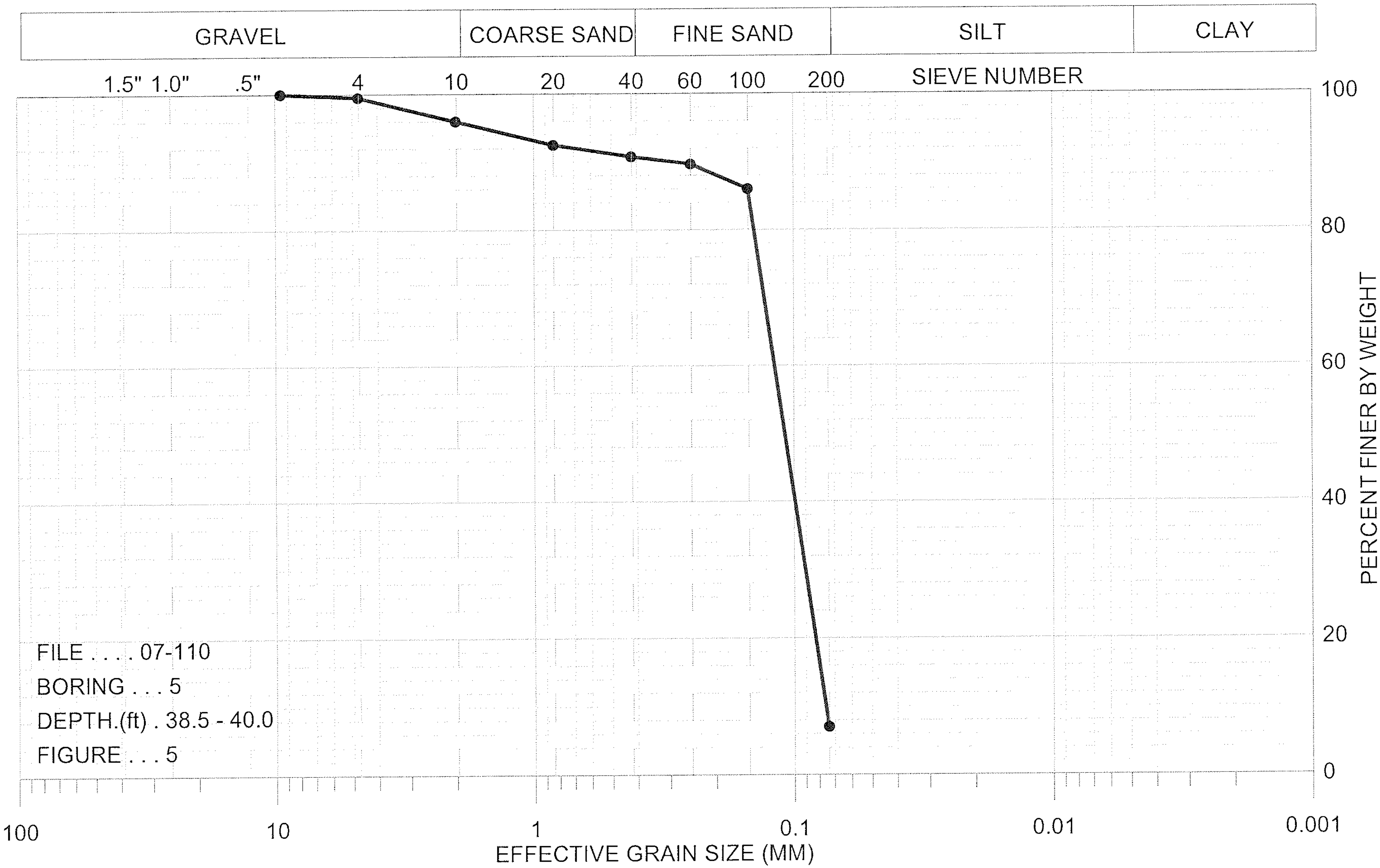
Louis J. Capozzoli & Associates, Inc.
Geotechnical Engineers

GRAIN SIZE CURVE



Louis J. Capozzoli & Associates, Inc.
Geotechnical Engineers

GRAIN SIZE CURVE

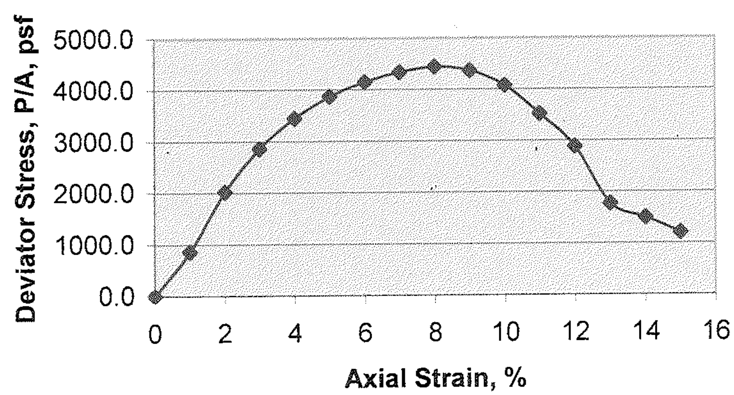


Louis J. Capozzoli & Associates, Inc.
Geotechnical Engineers

Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	5		
Depth, ft	0-2		
Load Ring No.	7	**Factor, lb/div**	1.0
Sample Diameter (in.)	2.83		
Sample Area, (sqft)	0.0437		
Sample Length (in.)	5		
Sample Weight (gm)	963.8		
Can No. 520	**Tare Wt (gm)**	22.5	
Moisture WW	105.5		
Moisture DW	84.7		
Moisture,%	33.4		
Wdensity,lb/cuft	116.7		
Ddensity,lb/cuft	87.4		
Chamber Pressure (psi)			
Back Pressure (psi)			
Start Press (ksf)			
Max Comp Dial Rdg	40		
Max Load Ring Rdg	211		
Type Failure	multiple shear		

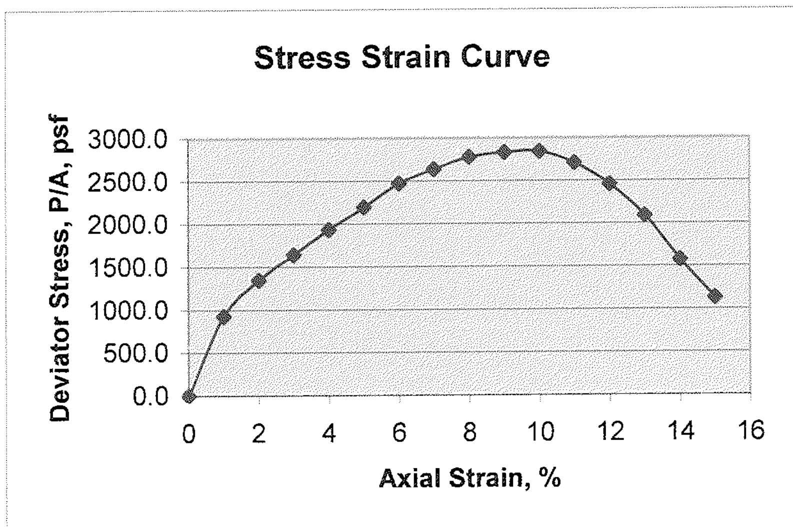
CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0437	0.00	0.0	0	0.0	0.0	0.00	#DIV/0!
5	38.0	1.0	0.044141	38.00	860.9	0	860.9	860.9	0.43	45
10	90.0	2.0	0.044592	90.00	2018.3	0	2018.3	2018.3	1.01	45
15	129.0	3.0	0.045052	129.00	2863.4	0	2863.4	2863.4	1.43	45
20	157.0	4.0	0.045521	157.00	3449.0	0	3449.0	3449.0	1.72	45
25	178.0	5.0	0.046	178.00	3869.6	0	3869.6	3869.6	1.93	45
30	193.0	6.0	0.046489	193.00	4151.5	0	4151.5	4151.5	2.08	45
35	204.0	7.0	0.046989	204.00	4341.4	0	4341.4	4341.4	2.17	45
40	211.0	8.0	0.0475	211.00	4442.1	0	4442.1	4442.1	2.22	45
45	210.0	9.0	0.048022	210.00	4373.0	0	4373.0	4373.0	2.19	45
50	198.0	10.0	0.048556	198.00	4077.8	0	4077.8	4077.8	2.04	45
55	173.0	11.0	0.049101	173.00	3523.3	0	3523.3	3523.3	1.76	45
60	143.0	12.0	0.049659	143.00	2879.6	0	2879.6	2879.6	1.44	45
65	89.0	13.0	0.05023	89.00	1771.9	0	1771.9	1771.9	0.89	45
70	76.0	14.0	0.050814	76.00	1495.7	0	1495.7	1495.7	0.75	45
75	62.0	15.0	0.051412	62.00	1205.9	0	1205.9	1205.9	0.60	45



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	5
Depth, ft	2-4
Load Ring No.	7
Factor, lb/div	1.0
Sample Diameter (in.)	2.83
Sample Area, (sqft)	0.0437
Sample Length (in.)	6
Sample Weight (gm)	1147.9
Can No.	555
Tare Wt (gm)	22.5
Moisture WW	102.8
Moisture DW	81.0
Moisture,%	37.3
Wdensity,lb/cuft	115.8
Ddensity,lb/cuft	84.4
Chamber Pressure (psi)	
Back Pressure (psi)	
Start Press (ksf)	
Max Comp Dial Rdg	60
Max Load Ring Rdg	138
Type Failure	multiple shear

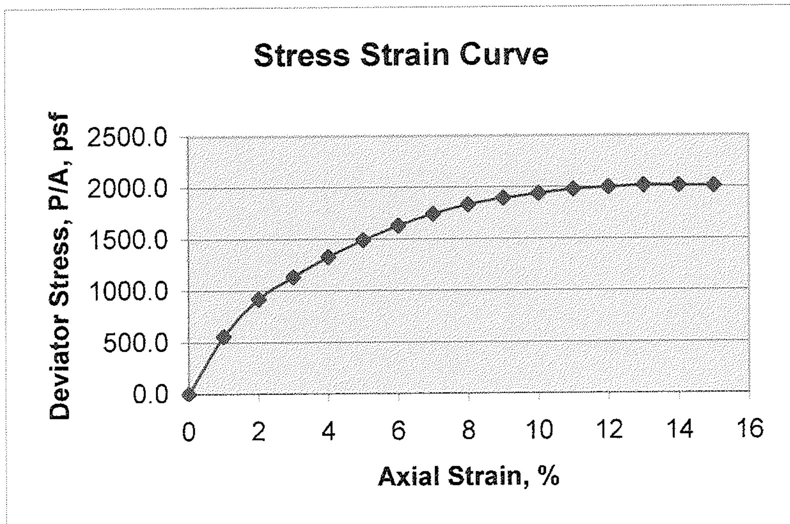
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0437	0.00	0.0	0	0.0	0.0	0.00	#DIV/0!
6	41.0	1.0	0.044141	41.00	928.8	0	928.8	928.8	0.46	45
12	60.0	2.0	0.044592	60.00	1345.5	0	1345.5	1345.5	0.67	45
18	74.0	3.0	0.045052	74.00	1642.6	0	1642.6	1642.6	0.82	45
24	88.0	4.0	0.045521	88.00	1933.2	0	1933.2	1933.2	0.97	45
30	101.0	5.0	0.046	101.00	2195.7	0	2195.7	2195.7	1.10	45
36	115.0	6.0	0.046489	115.00	2473.7	0	2473.7	2473.7	1.24	45
42	124.0	7.0	0.046989	124.00	2638.9	0	2638.9	2638.9	1.32	45
48	132.0	8.0	0.0475	132.00	2778.9	0	2778.9	2778.9	1.39	45
54	136.0	9.0	0.048022	136.00	2832.0	0	2832.0	2832.0	1.42	45
60	138.0	10.0	0.048556	138.00	2842.1	0	2842.1	2842.1	1.42	45
66	133.0	11.0	0.049101	133.00	2708.7	0	2708.7	2708.7	1.35	45
72	122.0	12.0	0.049659	122.00	2456.8	0	2456.8	2456.8	1.23	45
78	105.0	13.0	0.05023	105.00	2090.4	0	2090.4	2090.4	1.05	45
84	80.0	14.0	0.050814	80.00	1574.4	0	1574.4	1574.4	0.79	45
90	58.0	15.0	0.051412	58.00	1128.1	0	1128.1	1128.1	0.56	45



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	5
Depth, ft	4-6
Load Ring No.	7
Factor, lb/div	1.0
Sample Diameter (in.)	2.80
Sample Area, (sqft)	0.0427
Sample Length (in.)	6
Sample Weight (gm)	1113.0
Can No.	532
Tare Wt (gm)	22.5
Moisture WW	101.7
Moisture DW	80.4
Moisture,%	36.8
Wdensity,lb/cuft	114.9
Ddensity,lb/cuft	84.0
Chamber Pressure (psi)	
Back Pressure (psi)	
Start Press (ksf)	
Max Comp Dial Rdg	90
Max Load Ring Rdg	101
Type Failure	multiple shear

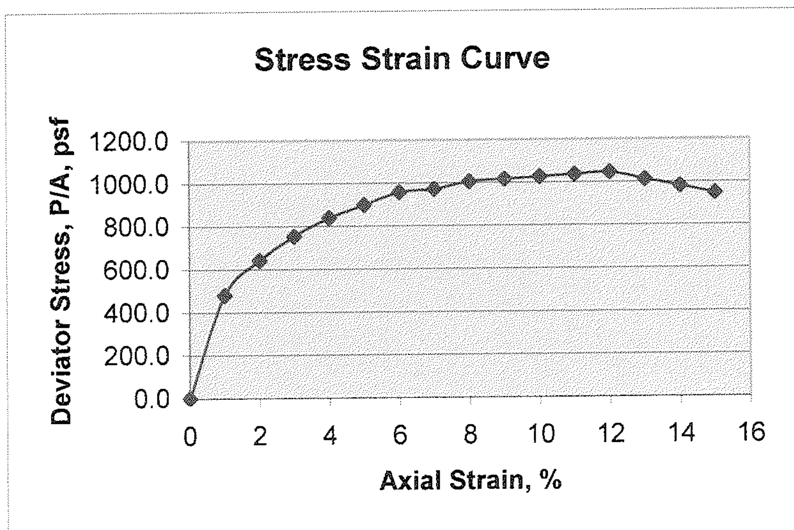
CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0427	0.00	0.0	0	0.0	0.0	0.00	#DIV/0!
6	24.0	1.0	0.043131	24.00	556.4	0	556.4	556.4	0.28	45
12	40.0	2.0	0.043571	40.00	918.0	0	918.0	918.0	0.46	45
18	50.0	3.0	0.044021	50.00	1135.8	0	1135.8	1135.8	0.57	45
24	59.0	4.0	0.044479	59.00	1326.5	0	1326.5	1326.5	0.66	45
30	67.0	5.0	0.044947	67.00	1490.6	0	1490.6	1490.6	0.75	45
36	74.0	6.0	0.045426	74.00	1629.0	0	1629.0	1629.0	0.81	45
42	80.0	7.0	0.045914	80.00	1742.4	0	1742.4	1742.4	0.87	45
48	85.0	8.0	0.046413	85.00	1831.4	0	1831.4	1831.4	0.92	45
54	89.0	9.0	0.046923	89.00	1896.7	0	1896.7	1896.7	0.95	45
60	92.0	10.0	0.047444	92.00	1939.1	0	1939.1	1939.1	0.97	45
66	95.0	11.0	0.047978	95.00	1980.1	0	1980.1	1980.1	0.99	45
72	97.0	12.0	0.048523	97.00	1999.1	0	1999.1	1999.1	1.00	45
78	99.0	13.0	0.04908	99.00	2017.1	0	2017.1	2017.1	1.01	45
84	100.0	14.0	0.049651	100.00	2014.1	0	2014.1	2014.1	1.01	45
90	101.0	15.0	0.050235	101.00	2010.5	0	2010.5	2010.5	1.01	45



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	5		
Depth, ft	6-8		
Load Ring No.	7	**Factor, lb/div**	1.0
Sample Diameter (in.)	2.75		
Sample Area, (sqft)	0.0412		
Sample Length (in.)	4		
Sample Weight (gm)	713.5		
Can No. 403	**Tare Wt (gm)**	27.3	
Moisture WW	147.5		
Moisture DW	111.4		
Moisture,%	42.9		
Wdensity,lb/cuft	114.5		
Ddensity,lb/cuft	80.1		
Chamber Pressure (psi)			
Back Pressure (psi)			
Start Press (ksf)			
Max Comp Dial Rdg	48		
Max Load Ring Rdg	49		
Type Failure	multiple shear		

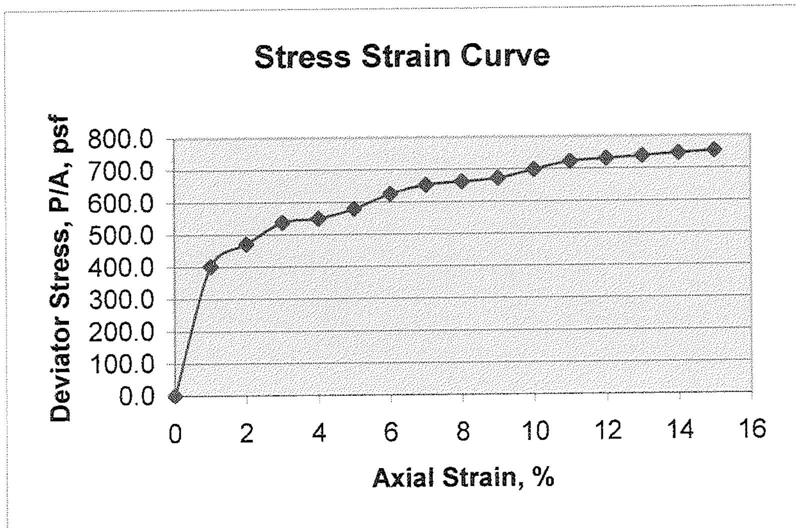
CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0412	0.00	0.0	0	0.0	0.0	0.00	#DIV/0!
4	20.0	1.0	0.041616	20.00	480.6	0	480.6	480.6	0.24	45
8	27.0	2.0	0.042041	27.00	642.2	0	642.2	642.2	0.32	45
12	32.0	3.0	0.042474	32.00	753.4	0	753.4	753.4	0.38	45
16	36.0	4.0	0.042917	36.00	838.8	0	838.8	838.8	0.42	45
20	39.0	5.0	0.043368	39.00	899.3	0	899.3	899.3	0.45	45
24	42.0	6.0	0.04383	42.00	958.3	0	958.3	958.3	0.48	45
28	43.0	7.0	0.044301	43.00	970.6	0	970.6	970.6	0.49	45
32	45.0	8.0	0.044783	45.00	1004.9	0	1004.9	1004.9	0.50	45
36	46.0	9.0	0.045275	46.00	1016.0	0	1016.0	1016.0	0.51	45
40	47.0	10.0	0.045778	47.00	1026.7	0	1026.7	1026.7	0.51	45
44	48.0	11.0	0.046292	48.00	1036.9	0	1036.9	1036.9	0.52	45
48	49.0	12.0	0.046818	49.00	1046.6	0	1046.6	1046.6	0.52	45
52	48.0	13.0	0.047356	48.00	1013.6	0	1013.6	1013.6	0.51	45
56	47.0	14.0	0.047907	47.00	981.1	0	981.1	981.1	0.49	45
60	46.0	15.0	0.048471	46.00	949.0	0	949.0	949.0	0.47	45



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	5		
Depth, ft	8-10		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.73		
Sample Area, (sqft)	0.0406		
Sample Length (in.)	5		
Sample Weight (gm)	886.8		
Can No.	538	**Tare Wt (gm)**	22.5
Moisture WW	153.9		
Moisture DW	114.0		
Moisture,%	43.6		
Wdensity,lb/cuft	115.6		
Ddensity,lb/cuft	80.5		
Chamber Pressure (psi)	7.5		
Back Pressure (psi)	3.9		
Start Press (ksf)			
Max Comp Dial Rdg	75		
Max Load Ring Rdg	48		
Type Failure	yield		

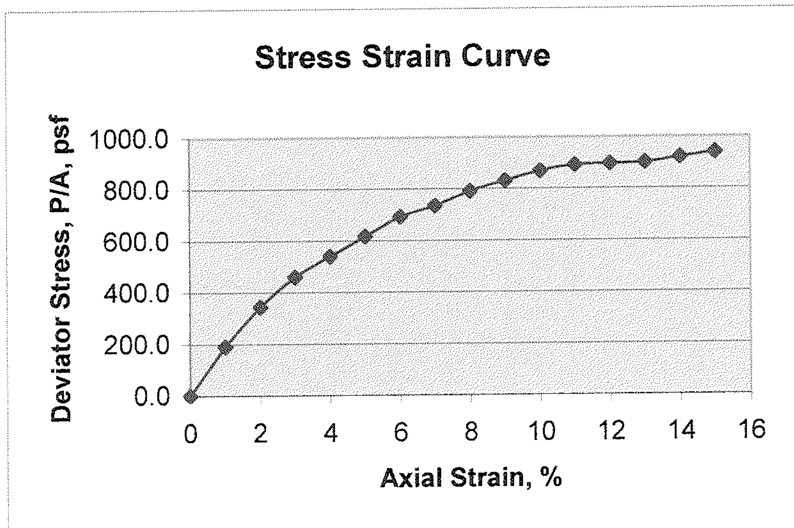
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0406	0.00	0.0	518.4	518.4	0.0	0.00	0
5	22.0	1.0	0.04101	16.50	402.3	518.4	920.7	402.3	0.20	16
10	26.0	2.0	0.041429	19.50	470.7	518.4	989.1	470.7	0.24	17
15	30.0	3.0	0.041856	22.50	537.6	518.4	1056.0	537.6	0.27	19
20	31.0	4.0	0.042292	23.25	549.8	518.4	1068.2	549.8	0.27	19
25	33.0	5.0	0.042737	24.75	579.1	518.4	1097.5	579.1	0.29	20
30	36.0	6.0	0.043191	27.00	625.1	518.4	1143.5	625.1	0.31	21
35	38.0	7.0	0.043656	28.50	652.8	518.4	1171.2	652.8	0.33	21
40	39.0	8.0	0.04413	29.25	662.8	518.4	1181.2	662.8	0.33	21
45	40.0	9.0	0.044615	30.00	672.4	518.4	1190.8	672.4	0.34	21
50	42.0	10.0	0.045111	31.50	698.3	518.4	1216.7	698.3	0.35	22
55	44.0	11.0	0.045618	33.00	723.4	518.4	1241.8	723.4	0.36	22
60	45.0	12.0	0.046136	33.75	731.5	518.4	1249.9	731.5	0.37	22
65	46.0	13.0	0.046667	34.50	739.3	518.4	1257.7	739.3	0.37	23
70	47.0	14.0	0.047209	35.25	746.7	518.4	1265.1	746.7	0.37	23
75	48.0	15.0	0.047765	36.00	753.7	518.4	1272.1	753.7	0.38	23



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	5
Depth, ft	10-12
Load Ring No.	3
Factor, lb/div	0.75
Sample Diameter (in.)	2.8
Sample Area, (sqft)	0.0427
Sample Length (in.)	5
Sample Weight (gm)	902.6
Can No.	417
Tare Wt (gm)	27.3
Moisture WW	132.1
Moisture DW	103.6
Moisture,%	37.4
Wdensity,lb/cuft	111.8
Ddensity,lb/cuft	81.4
Chamber Pressure (psi)	9.2
Back Pressure (psi)	4.7
Start Press (ksf)	
Max Comp Dial Rdg	75
Max Load Ring Rdg	63
Type Failure	yield

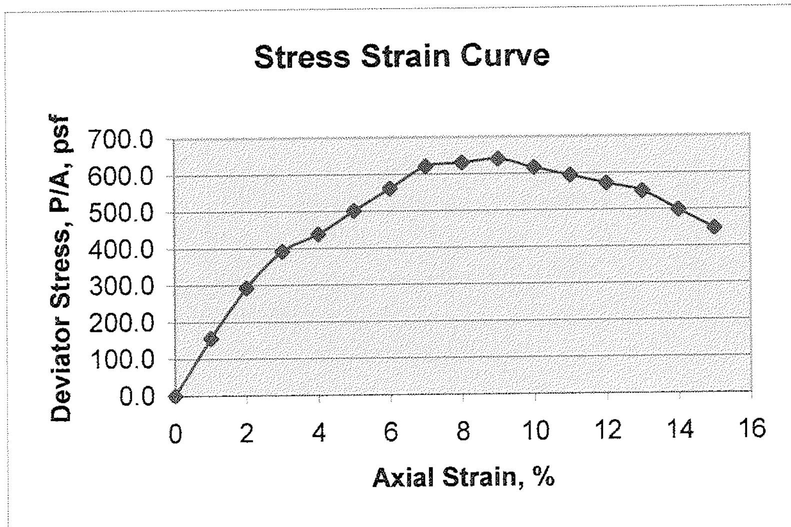
CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0427	0.00	0.0	648	648.0	0.0	0.00	0
5	11.0	1.0	0.043131	8.25	191.3	648	839.3	191.3	0.10	7
10	20.0	2.0	0.043571	15.00	344.3	648	992.3	344.3	0.17	12
15	27.0	3.0	0.044021	20.25	460.0	648	1108.0	460.0	0.23	15
20	32.0	4.0	0.044479	24.00	539.6	648	1187.6	539.6	0.27	16
25	37.0	5.0	0.044947	27.75	617.4	648	1265.4	617.4	0.31	18
30	42.0	6.0	0.045426	31.50	693.4	648	1341.4	693.4	0.35	19
35	45.0	7.0	0.045914	33.75	735.1	648	1383.1	735.1	0.37	20
40	49.0	8.0	0.046413	36.75	791.8	648	1439.8	791.8	0.40	21
45	52.0	9.0	0.046923	39.00	831.1	648	1479.1	831.1	0.42	21
50	55.0	10.0	0.047444	41.25	869.4	648	1517.4	869.4	0.43	22
55	57.0	11.0	0.047978	42.75	891.0	648	1539.0	891.0	0.45	22
60	58.0	12.0	0.048523	43.50	896.5	648	1544.5	896.5	0.45	22
65	59.0	13.0	0.04908	44.25	901.6	648	1549.6	901.6	0.45	22
70	61.0	14.0	0.049651	45.75	921.4	648	1569.4	921.4	0.46	23
75	63.0	15.0	0.050235	47.25	940.6	648	1588.6	940.6	0.47	23



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	5		
Depth, ft	12-14		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.8		
Sample Area, (sqft)	0.0427		
Sample Length (in.)	5		
Sample Weight (gm)	914.8		
Can No. 530	**Tare Wt (gm)**	22.5	
Moisture WW	122.1		
Moisture DW	94.6		
Moisture,%	38.1		
Wdensity,lb/cuft	113.4		
Ddensity,lb/cuft	82.1		
Chamber Pressure (psi)	10.8		
Back Pressure (psi)	5.6		
Start Press (ksf)			
Max Comp Dial Rdg	45		
Max Load Ring Rdg	40		
Type Failure	SLS		

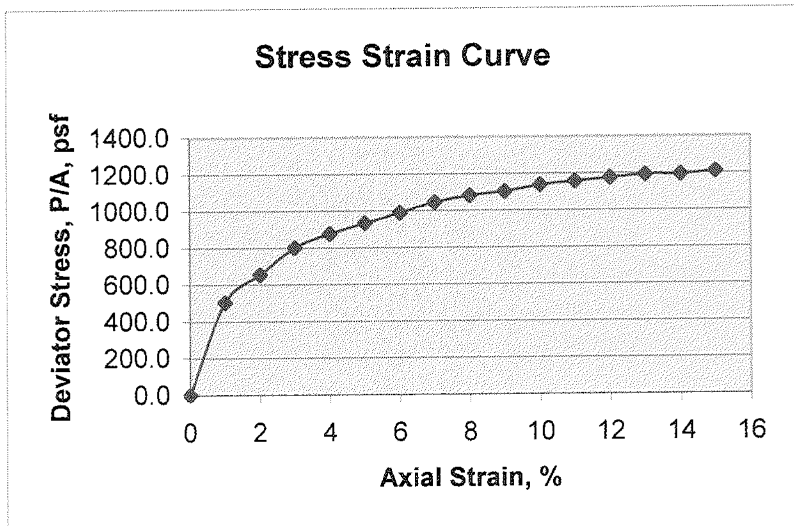
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0427	0.00	0.0	748.8	748.8	0.0	0.00	0
5	9.0	1.0	0.043131	6.75	156.5	748.8	905.3	156.5	0.08	5
10	17.0	2.0	0.043571	12.75	292.6	748.8	1041.4	292.6	0.15	9
15	23.0	3.0	0.044021	17.25	391.9	748.8	1140.7	391.9	0.20	12
20	26.0	4.0	0.044479	19.50	438.4	748.8	1187.2	438.4	0.22	13
25	30.0	5.0	0.044947	22.50	500.6	748.8	1249.4	500.6	0.25	14
30	34.0	6.0	0.045426	25.50	561.4	748.8	1310.2	561.4	0.28	15
35	38.0	7.0	0.045914	28.50	620.7	748.8	1369.5	620.7	0.31	16
40	39.0	8.0	0.046413	29.25	630.2	748.8	1379.0	630.2	0.32	16
45	40.0	9.0	0.046923	30.00	639.3	748.8	1388.1	639.3	0.32	17
50	39.0	10.0	0.047444	29.25	616.5	748.8	1365.3	616.5	0.31	16
55	38.0	11.0	0.047978	28.50	594.0	748.8	1342.8	594.0	0.30	16
60	37.0	12.0	0.048523	27.75	571.9	748.8	1320.7	571.9	0.29	15
65	36.0	13.0	0.04908	27.00	550.1	748.8	1298.9	550.1	0.28	15
70	33.0	14.0	0.049651	24.75	498.5	748.8	1247.3	498.5	0.25	14
75	30.0	15.0	0.050235	22.50	447.9	748.8	1196.7	447.9	0.22	13



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	5
Depth, ft	14-16
Load Ring No.	3
Factor, lb/div	0.75
Sample Diameter (in.)	2.8
Sample Area, (sqft)	0.0427
Sample Length (in.)	5
Sample Weight (gm)	941.0
Can No.	402
Tare Wt (gm)	27.3
Moisture WW	109.6
Moisture DW	87.1
Moisture,%	37.6
Wdensity,lb/cuft	116.6
Ddensity,lb/cuft	84.7
Chamber Pressure (psi)	12.5
Back Pressure (psi)	6.5
Start Press (ksf)	
Max Comp Dial Rdg	75
Max Load Ring Rdg	81
Type Failure	yield

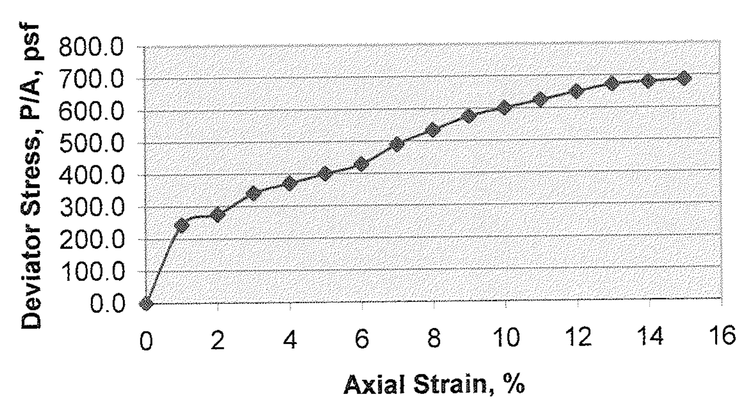
CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0427	0.00	0.0	864	864.0	0.0	0.00	0
5	29.0	1.0	0.043131	21.75	504.3	864	1368.3	504.3	0.25	13
10	38.0	2.0	0.043571	28.50	654.1	864	1518.1	654.1	0.33	15
15	47.0	3.0	0.044021	35.25	800.8	864	1664.8	800.8	0.40	18
20	52.0	4.0	0.044479	39.00	876.8	864	1740.8	876.8	0.44	19
25	56.0	5.0	0.044947	42.00	934.4	864	1798.4	934.4	0.47	19
30	60.0	6.0	0.045426	45.00	990.6	864	1854.6	990.6	0.50	20
35	64.0	7.0	0.045914	48.00	1045.4	864	1909.4	1045.4	0.52	21
40	67.0	8.0	0.046413	50.25	1082.7	864	1946.7	1082.7	0.54	21
45	69.0	9.0	0.046923	51.75	1102.9	864	1966.9	1102.9	0.55	21
50	72.0	10.0	0.047444	54.00	1138.2	864	2002.2	1138.2	0.57	22
55	74.0	11.0	0.047978	55.50	1156.8	864	2020.8	1156.8	0.58	22
60	76.0	12.0	0.048523	57.00	1174.7	864	2038.7	1174.7	0.59	22
65	78.0	13.0	0.04908	58.50	1191.9	864	2055.9	1191.9	0.60	22
70	79.0	14.0	0.049651	59.25	1193.3	864	2057.3	1193.3	0.60	22
75	81.0	15.0	0.050235	60.75	1209.3	864	2073.3	1209.3	0.60	22



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	5
Depth, ft	16-18
Load Ring No.	3
Factor, lb/div	0.75
Sample Diameter (in.)	2.8
Sample Area, (sqft)	0.0427
Sample Length (in.)	5
Sample Weight (gm)	896.0
Can No.	131
Tare Wt (gm)	19.3
Moisture WW	91.4
Moisture DW	71.9
Moisture,%	37.1
Wdensity,lb/cuft	111.0
Ddensity,lb/cuft	81.0
Chamber Pressure (psi)	14.2
Back Pressure (psi)	7.3
Start Press (ksf)	
Max Comp Dial Rdg	75
Max Load Ring Rdg	46
Type Failure	yield

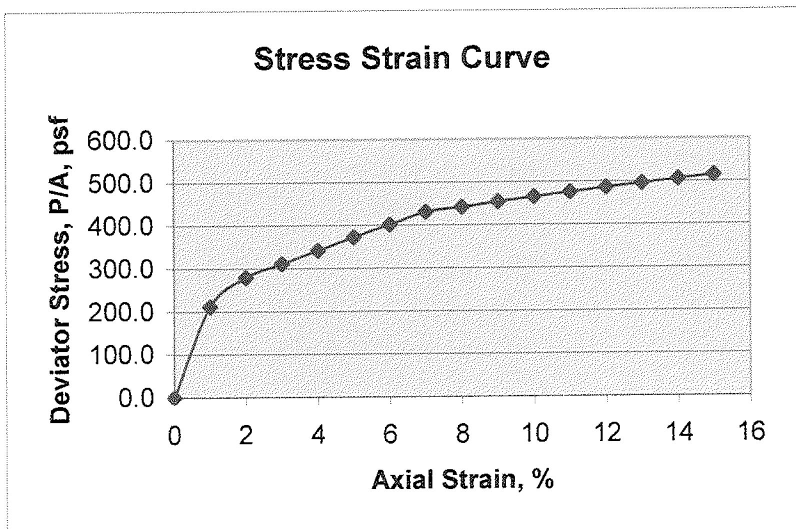
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0427	0.00	0.0	993.6	993.6	0.0	0.00	0
5	14.0	1.0	0.043131	10.50	243.4	993.6	1237.0	243.4	0.12	6
10	16.0	2.0	0.043571	12.00	275.4	993.6	1269.0	275.4	0.14	7
15	20.0	3.0	0.044021	15.00	340.7	993.6	1334.3	340.7	0.17	8
20	22.0	4.0	0.044479	16.50	371.0	993.6	1364.6	371.0	0.19	9
25	24.0	5.0	0.044947	18.00	400.5	993.6	1394.1	400.5	0.20	10
30	26.0	6.0	0.045426	19.50	429.3	993.6	1422.9	429.3	0.21	10
35	30.0	7.0	0.045914	22.50	490.0	993.6	1483.6	490.0	0.25	11
40	33.0	8.0	0.046413	24.75	533.3	993.6	1526.9	533.3	0.27	12
45	36.0	9.0	0.046923	27.00	575.4	993.6	1569.0	575.4	0.29	13
50	38.0	10.0	0.047444	28.50	600.7	993.6	1594.3	600.7	0.30	13
55	40.0	11.0	0.047978	30.00	625.3	993.6	1618.9	625.3	0.31	13
60	42.0	12.0	0.048523	31.50	649.2	993.6	1642.8	649.2	0.32	14
65	44.0	13.0	0.04908	33.00	672.4	993.6	1666.0	672.4	0.34	14
70	45.0	14.0	0.049651	33.75	679.7	993.6	1673.3	679.7	0.34	14
75	46.0	15.0	0.050235	34.50	686.8	993.6	1680.4	686.8	0.34	14



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	5		
Depth, ft	22-24		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.78		
Sample Area, (sqft)	0.0421		
Sample Length (in.)	4		
Sample Weight (gm)	703.0		
Can No.	580	**Tare Wt (gm)**	22.5
Moisture WW	76.4		
Moisture DW	61.4		
Moisture,%	38.6		
Wdensity,lb/cuft	110.4		
Ddensity,lb/cuft	79.7		
Chamber Pressure (psi)	19.2		
Back Pressure (psi)	9.9		
Start Press (ksf)			
Max Comp Dial Rdg	60		
Max Load Ring Rdg	34		
Type Failure	yield		

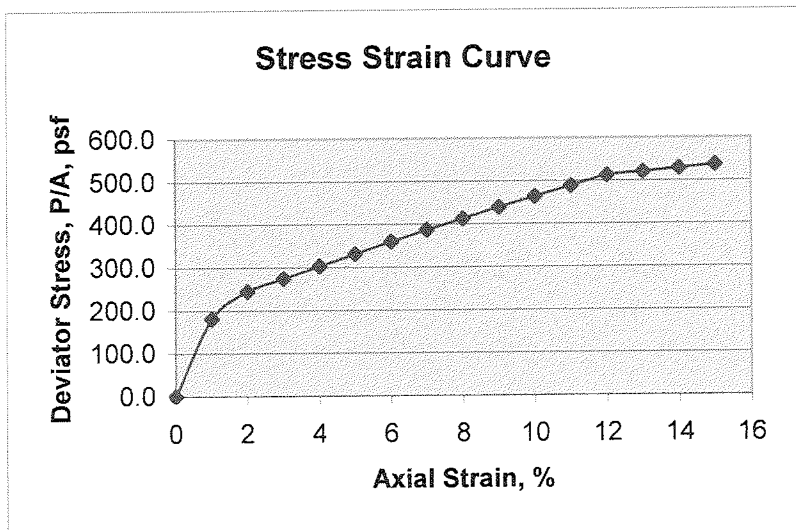
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0421	0.00	0.0	1339.2	1339.2	0.0	0.00	0
4	12.0	1.0	0.042525	9.00	211.6	1339.2	1550.8	211.6	0.11	4
8	16.0	2.0	0.042959	12.00	279.3	1339.2	1618.5	279.3	0.14	5
12	18.0	3.0	0.043402	13.50	311.0	1339.2	1650.2	311.0	0.16	6
16	20.0	4.0	0.043854	15.00	342.0	1339.2	1681.2	342.0	0.17	6
20	22.0	5.0	0.044316	16.50	372.3	1339.2	1711.5	372.3	0.19	7
24	24.0	6.0	0.044787	18.00	401.9	1339.2	1741.1	401.9	0.20	7
28	26.0	7.0	0.045269	19.50	430.8	1339.2	1770.0	430.8	0.22	8
32	27.0	8.0	0.045761	20.25	442.5	1339.2	1781.7	442.5	0.22	8
36	28.0	9.0	0.046264	21.00	453.9	1339.2	1793.1	453.9	0.23	8
40	29.0	10.0	0.046778	21.75	465.0	1339.2	1804.2	465.0	0.23	8
44	30.0	11.0	0.047303	22.50	475.7	1339.2	1814.9	475.7	0.24	9
48	31.0	12.0	0.047841	23.25	486.0	1339.2	1825.2	486.0	0.24	9
52	32.0	13.0	0.048391	24.00	496.0	1339.2	1835.2	496.0	0.25	9
56	33.0	14.0	0.048953	24.75	505.6	1339.2	1844.8	505.6	0.25	9
60	34.0	15.0	0.049529	25.50	514.8	1339.2	1854.0	514.8	0.26	9



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	5		
Depth, ft	24-26		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.875		
Sample Area, (sqft)	0.0451		
Sample Length (in.)	5		
Sample Weight (gm)	1018.8		
Can No. 582	**Tare Wt (gm)**	22.5	
Moisture WW	104.7		
Moisture DW	78.9		
Moisture,%	45.7		
Wdensity,lb/cuft	119.5		
Ddensity,lb/cuft	82.0		
Chamber Pressure (psi)	20.9		
Back Pressure (psi)	10.8		
Start Press (ksf)			
Max Comp Dial Rdg	75		
Max Load Ring Rdg	38		
Type Failure	yield		

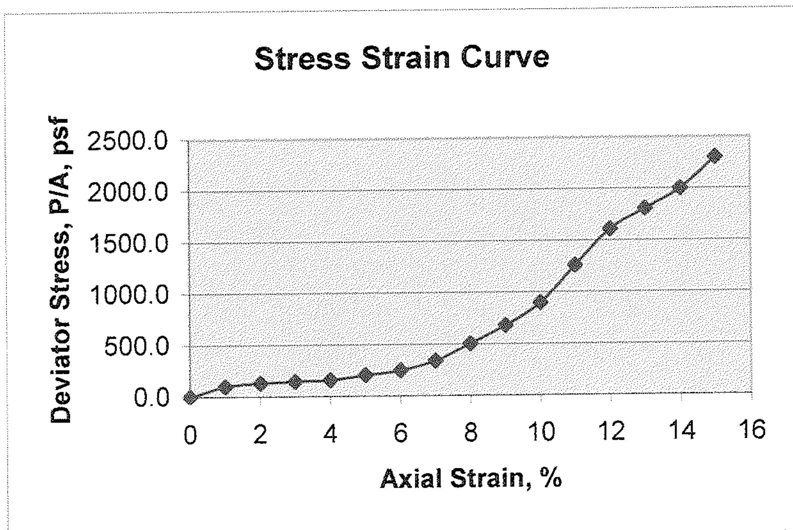
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	1454.4	1454.4	0.0	0.00	0
5	11.0	1.0	0.045556	8.25	181.1	1454.4	1635.5	181.1	0.09	3
10	15.0	2.0	0.04602	11.25	244.5	1454.4	1698.9	244.5	0.12	4
15	17.0	3.0	0.046495	12.75	274.2	1454.4	1728.6	274.2	0.14	5
20	19.0	4.0	0.046979	14.25	303.3	1454.4	1757.7	303.3	0.15	5
25	21.0	5.0	0.047474	15.75	331.8	1454.4	1786.2	331.8	0.17	6
30	23.0	6.0	0.047979	17.25	359.5	1454.4	1813.9	359.5	0.18	6
35	25.0	7.0	0.048495	18.75	386.6	1454.4	1841.0	386.6	0.19	7
40	27.0	8.0	0.049022	20.25	413.1	1454.4	1867.5	413.1	0.21	7
45	29.0	9.0	0.04956	21.75	438.9	1454.4	1893.3	438.9	0.22	7
50	31.0	10.0	0.050111	23.25	464.0	1454.4	1918.4	464.0	0.23	8
55	33.0	11.0	0.050674	24.75	488.4	1454.4	1942.8	488.4	0.24	8
60	35.0	12.0	0.05125	26.25	512.2	1454.4	1966.6	512.2	0.26	9
65	36.0	13.0	0.051839	27.00	520.8	1454.4	1975.2	520.8	0.26	9
70	37.0	14.0	0.052442	27.75	529.2	1454.4	1983.6	529.2	0.26	9
75	38.0	15.0	0.053059	28.50	537.1	1454.4	1991.5	537.1	0.27	9



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	5
Depth, ft	26-28
Load Ring No.	3
Factor, lb/div	0.75
Sample Diameter (in.)	2.875
Sample Area, (sqft)	0.0451
Sample Length (in.)	5
Sample Weight (gm)	973.2
Can No.	538
Tare Wt (gm)	22.5
Moisture WW	102.0
Moisture DW	84.8
Moisture,%	27.6
Wdensity,lb/cuft	114.2
Ddensity,lb/cuft	89.5
Chamber Pressure (psi)	22.4
Back Pressure (psi)	11.6
Start Press (ksf)	
Max Comp Dial Rdg	75
Max Load Ring Rdg	163
Type Failure	yield

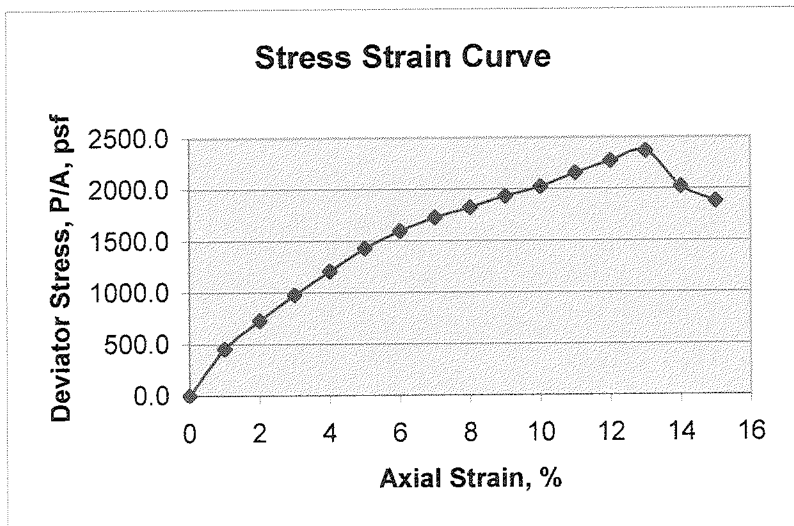
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	1555.2	1555.2	0.0	0.00	0
5	6.0	1.0	0.045556	4.50	98.8	1555.2	1654.0	98.8	0.05	2
10	8.0	2.0	0.04602	6.00	130.4	1555.2	1685.6	130.4	0.07	2
15	9.0	3.0	0.046495	6.75	145.2	1555.2	1700.4	145.2	0.07	3
20	10.0	4.0	0.046979	7.50	159.6	1555.2	1714.8	159.6	0.08	3
25	13.0	5.0	0.047474	9.75	205.4	1555.2	1760.6	205.4	0.10	4
30	16.0	6.0	0.047979	12.00	250.1	1555.2	1805.3	250.1	0.13	4
35	22.0	7.0	0.048495	16.50	340.2	1555.2	1895.4	340.2	0.17	6
40	33.0	8.0	0.049022	24.75	504.9	1555.2	2060.1	504.9	0.25	8
45	45.0	9.0	0.04956	33.75	681.0	1555.2	2236.2	681.0	0.34	10
50	60.0	10.0	0.050111	45.00	898.0	1555.2	2453.2	898.0	0.45	13
55	85.0	11.0	0.050674	63.75	1258.0	1555.2	2813.2	1258.0	0.63	16
60	110.0	12.0	0.05125	82.50	1609.8	1555.2	3165.0	1609.8	0.80	19
65	125.0	13.0	0.051839	93.75	1808.5	1555.2	3363.7	1808.5	0.90	20
70	140.0	14.0	0.052442	105.00	2002.2	1555.2	3557.4	2002.2	1.00	21
75	163.0	15.0	0.053059	122.25	2304.0	1555.2	3859.2	2304.0	1.15	23



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	5		
Depth, ft	28-30		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.79		
Sample Area, (sqft)	0.0424		
Sample Length (in.)	4		
Sample Weight (gm)	764.3		
Can No. 213		**Tare Wt (gm)**	18.3
Moisture WW	102.7		
Moisture DW	84.4		
Moisture,%	27.7		
Wdensity,lb/cuft	119.2		
Ddensity,lb/cuft	93.4		
Chamber Pressure (psi)	24.2		
Back Pressure (psi)	12.5		
Start Press (ksf)			
Max Comp Dial Rdg	52		
Max Load Ring Rdg	154		
Type Failure	bulge		

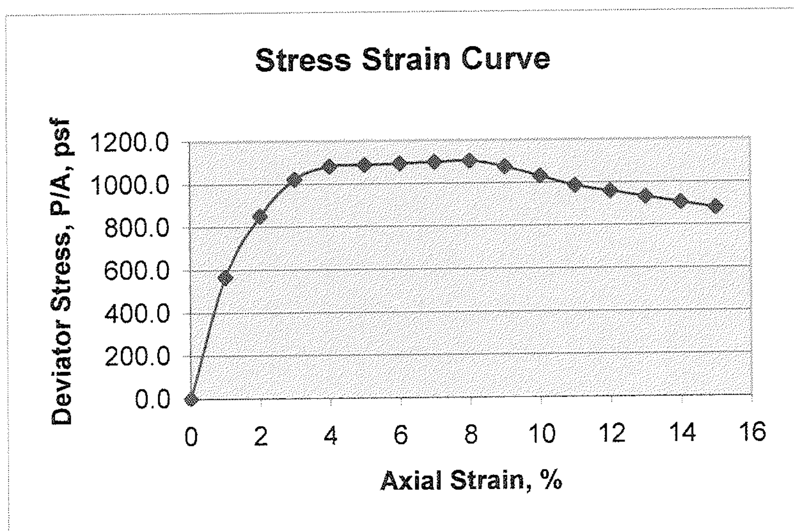
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0424	0.00	0.0	1684.8	1684.8	0.0	0.00	0
4	26.0	1.0	0.042828	19.50	455.3	1684.8	2140.1	455.3	0.23	7
8	42.0	2.0	0.043265	31.50	728.1	1684.8	2412.9	728.1	0.36	10
12	57.0	3.0	0.043711	42.75	978.0	1684.8	2662.8	978.0	0.49	13
16	71.0	4.0	0.044167	53.25	1205.7	1684.8	2890.5	1205.7	0.60	15
20	85.0	5.0	0.044632	63.75	1428.4	1684.8	3113.2	1428.4	0.71	17
24	96.0	6.0	0.045106	72.00	1596.2	1684.8	3281.0	1596.2	0.80	18
28	105.0	7.0	0.045591	78.75	1727.3	1684.8	3412.1	1727.3	0.86	19
32	112.0	8.0	0.046087	84.00	1822.6	1684.8	3507.4	1822.6	0.91	19
36	120.0	9.0	0.046593	90.00	1931.6	1684.8	3616.4	1931.6	0.97	20
40	127.0	10.0	0.047111	95.25	2021.8	1684.8	3706.6	2021.8	1.01	21
44	137.0	11.0	0.04764	102.75	2156.8	1684.8	3841.6	2156.8	1.08	21
48	146.0	12.0	0.048182	109.50	2272.6	1684.8	3957.4	2272.6	1.14	22
52	154.0	13.0	0.048736	115.50	2369.9	1684.8	4054.7	2369.9	1.18	22
56	133.0	14.0	0.049302	99.75	2023.2	1684.8	3708.0	2023.2	1.01	21
60	125.0	15.0	0.049882	93.75	1879.4	1684.8	3564.2	1879.4	0.94	20



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	5		
Depth, ft	30-32		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.73		
Sample Area, (sqft)	0.0406		
Sample Length (in.)	4		
Sample Weight (gm)	735.5		
Can No.	128	**Tare Wt (gm)**	19.3
Moisture WW	102.7		
Moisture DW	84.4		
Moisture,%	28.1		
Wdensity,lb/cuft	119.8		
Ddensity,lb/cuft	93.5		
Chamber Pressure (psi)	25.8		
Back Pressure (psi)	13.3		
Start Press (ksf)			
Max Comp Dial Rdg	32		
Max Load Ring Rdg	65		
Type Failure	bulge		

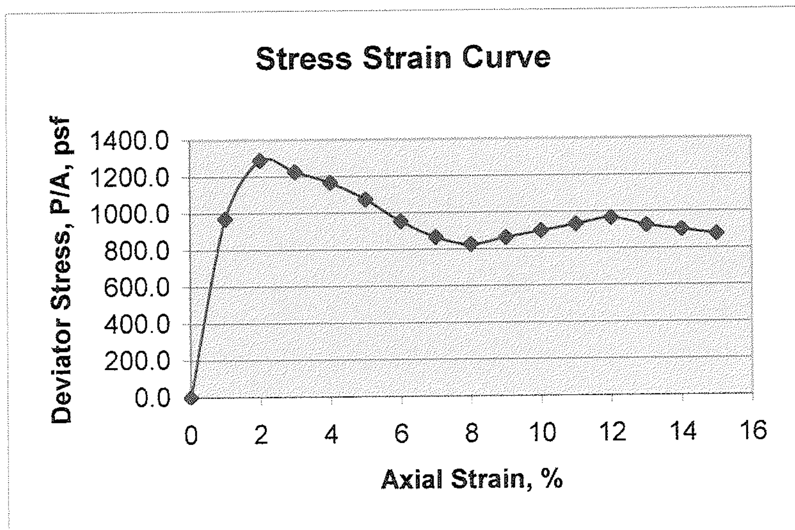
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0406	0.00	0.0	1800	1800.0	0.0	0.00	0
4	31.0	1.0	0.04101	23.25	566.9	1800	2366.9	566.9	0.28	8
8	47.0	2.0	0.041429	35.25	850.9	1800	2650.9	850.9	0.43	11
12	57.0	3.0	0.041856	42.75	1021.4	1800	2821.4	1021.4	0.51	12
16	61.0	4.0	0.042292	45.75	1081.8	1800	2881.8	1081.8	0.54	13
20	62.0	5.0	0.042737	46.50	1088.1	1800	2888.1	1088.1	0.54	13
24	63.0	6.0	0.043191	47.25	1094.0	1800	2894.0	1094.0	0.55	13
28	64.0	7.0	0.043656	48.00	1099.5	1800	2899.5	1099.5	0.55	13
32	65.0	8.0	0.04413	48.75	1104.7	1800	2904.7	1104.7	0.55	13
36	64.0	9.0	0.044615	48.00	1075.9	1800	2875.9	1075.9	0.54	13
40	62.0	10.0	0.045111	46.50	1030.8	1800	2830.8	1030.8	0.52	13
44	60.0	11.0	0.045618	45.00	986.5	1800	2786.5	986.5	0.49	12
48	59.0	12.0	0.046136	44.25	959.1	1800	2759.1	959.1	0.48	12
52	58.0	13.0	0.046667	43.50	932.1	1800	2732.1	932.1	0.47	12
56	57.0	14.0	0.047209	42.75	905.5	1800	2705.5	905.5	0.45	11
60	56.0	15.0	0.047765	42.00	879.3	1800	2679.3	879.3	0.44	11



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	5		
Depth, ft	32-34		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)		2.875	
Sample Area, (sqft)		0.0451	
Sample Length (in.)		4	
Sample Weight (gm)		766.3	
Can No.	498	**Tare Wt (gm)**	27.3
Moisture WW		111.7	
Moisture DW		91.7	
Moisture,%		31.1	
Wdensity,lb/cuft		112.4	
Ddensity,lb/cuft		85.7	
Chamber Pressure (psi)		27.5	
Back Pressure (psi)		14.2	
Start Press (ksf)			
Max Comp Dial Rdg		8	
Max Load Ring Rdg		79	
Type Failure		bulge	

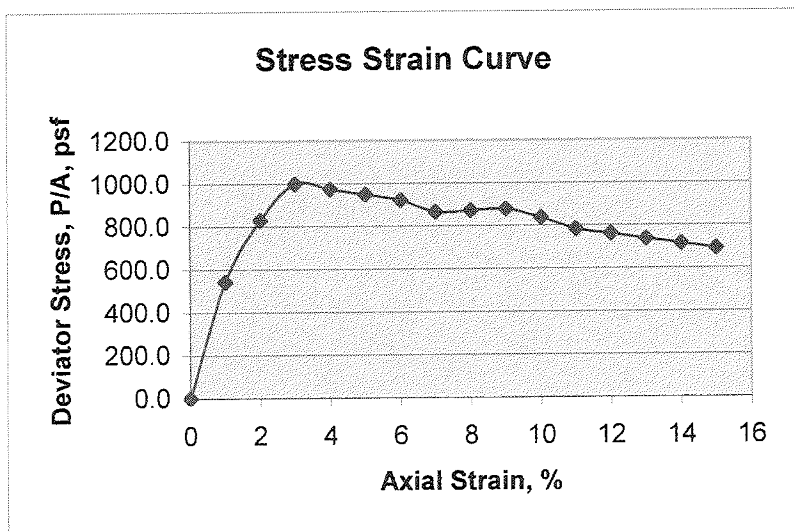
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	1915.2	1915.2	0.0	0.00	0
4	59.0	1.0	0.045556	44.25	971.3	1915.2	2886.5	971.3	0.49	11
8	79.0	2.0	0.04602	59.25	1287.5	1915.2	3202.7	1287.5	0.64	14
12	76.0	3.0	0.046495	57.00	1225.9	1915.2	3141.1	1225.9	0.61	14
16	73.0	4.0	0.046979	54.75	1165.4	1915.2	3080.6	1165.4	0.58	13
20	68.0	5.0	0.047474	51.00	1074.3	1915.2	2989.5	1074.3	0.54	12
24	61.0	6.0	0.047979	45.75	953.5	1915.2	2868.7	953.5	0.48	11
28	56.0	7.0	0.048495	42.00	866.1	1915.2	2781.3	866.1	0.43	10
32	54.0	8.0	0.049022	40.50	826.2	1915.2	2741.4	826.2	0.41	10
36	57.0	9.0	0.04956	42.75	862.6	1915.2	2777.8	862.6	0.43	10
40	60.0	10.0	0.050111	45.00	898.0	1915.2	2813.2	898.0	0.45	11
44	63.0	11.0	0.050674	47.25	932.4	1915.2	2847.6	932.4	0.47	11
48	66.0	12.0	0.05125	49.50	965.9	1915.2	2881.1	965.9	0.48	11
52	64.0	13.0	0.051839	48.00	925.9	1915.2	2841.1	925.9	0.46	11
56	63.0	14.0	0.052442	47.25	901.0	1915.2	2816.2	901.0	0.45	11
60	62.0	15.0	0.053059	46.50	876.4	1915.2	2791.6	876.4	0.44	11



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	5		
Depth, ft	34-36		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.875		
Sample Area, (sqft)	0.0451		
Sample Length (in.)	4		
Sample Weight (gm)	743.0		
Can No. 121	**Tare Wt (gm)**	19.3	
Moisture WW	104.0		
Moisture DW	86.1		
Moisture,%	26.8		
Wdensity,lb/cuft	109.0		
Ddensity,lb/cuft	85.9		
Chamber Pressure (psi)	29.2		
Back Pressure (psi)	15.1		
Start Press (ksf)			
Max Comp Dial Rdg	12		
Max Load Ring Rdg	62		
Type Failure	bulge		

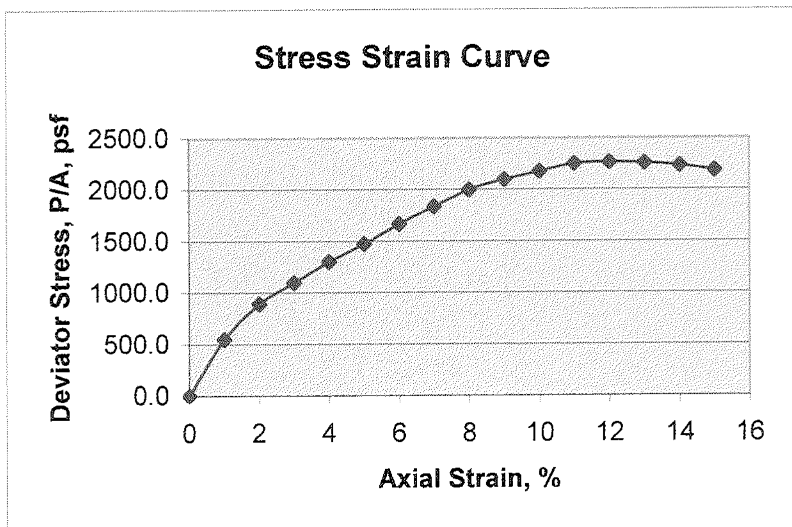
CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	2030.4	2030.4	0.0	0.00	0
4	33.0	1.0	0.045556	24.75	543.3	2030.4	2573.7	543.3	0.27	7
8	51.0	2.0	0.04602	38.25	831.2	2030.4	2861.6	831.2	0.42	10
12	62.0	3.0	0.046495	46.50	1000.1	2030.4	3030.5	1000.1	0.50	11
16	61.0	4.0	0.046979	45.75	973.8	2030.4	3004.2	973.8	0.49	11
20	60.0	5.0	0.047474	45.00	947.9	2030.4	2978.3	947.9	0.47	11
24	59.0	6.0	0.047979	44.25	922.3	2030.4	2952.7	922.3	0.46	10
28	56.0	7.0	0.048495	42.00	866.1	2030.4	2896.5	866.1	0.43	10
32	57.0	8.0	0.049022	42.75	872.1	2030.4	2902.5	872.1	0.44	10
36	58.0	9.0	0.04956	43.50	877.7	2030.4	2908.1	877.7	0.44	10
40	56.0	10.0	0.050111	42.00	838.1	2030.4	2868.5	838.1	0.42	10
44	53.0	11.0	0.050674	39.75	784.4	2030.4	2814.8	784.4	0.39	9
48	52.0	12.0	0.05125	39.00	761.0	2030.4	2791.4	761.0	0.38	9
52	51.0	13.0	0.051839	38.25	737.9	2030.4	2768.3	737.9	0.37	9
56	50.0	14.0	0.052442	37.50	715.1	2030.4	2745.5	715.1	0.36	9
60	49.0	15.0	0.053059	36.75	692.6	2030.4	2723.0	692.6	0.35	8



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	0-2		
Load Ring No.	7	**Factor, lb/div**	1.0
Sample Diameter (in.)	2.875		
Sample Area, (sqft)	0.0451		
Sample Length (in.)	4		
Sample Weight (gm)	830.4		
Can No. 116		**Tare Wt (gm)**	19.3
Moisture WW	101.1		
Moisture DW	85.8		
Moisture,%	23.0		
Wdensity,lb/cuft	121.8		
Ddensity,lb/cuft	99.0		
Chamber Pressure (psi)			
Back Pressure (psi)			
Start Press (ksf)			
Max Comp Dial Rdg	52		
Max Load Ring Rdg	117		
Type Failure	multiple shear		

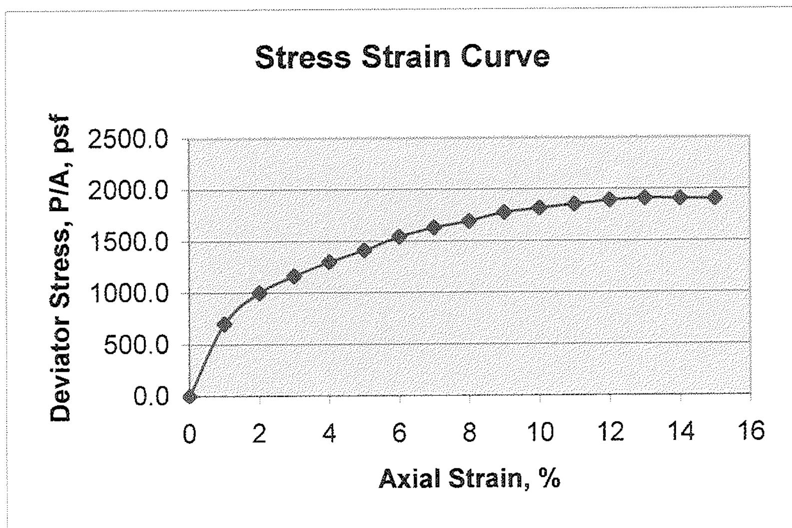
CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	0	0.0	0.0	0.00	#DIV/0!
4	25.0	1.0	0.045556	25.00	548.8	0	548.8	548.8	0.27	45
8	41.0	2.0	0.04602	41.00	890.9	0	890.9	890.9	0.45	45
12	51.0	3.0	0.046495	51.00	1096.9	0	1096.9	1096.9	0.55	45
16	61.0	4.0	0.046979	61.00	1298.4	0	1298.4	1298.4	0.65	45
20	70.0	5.0	0.047474	70.00	1474.5	0	1474.5	1474.5	0.74	45
24	80.0	6.0	0.047979	80.00	1667.4	0	1667.4	1667.4	0.83	45
28	89.0	7.0	0.048495	89.00	1835.3	0	1835.3	1835.3	0.92	45
32	98.0	8.0	0.049022	98.00	1999.1	0	1999.1	1999.1	1.00	45
36	104.0	9.0	0.04956	104.00	2098.4	0	2098.4	2098.4	1.05	45
40	109.0	10.0	0.050111	109.00	2175.2	0	2175.2	2175.2	1.09	45
44	114.0	11.0	0.050674	114.00	2249.7	0	2249.7	2249.7	1.12	45
48	116.0	12.0	0.05125	116.00	2263.4	0	2263.4	2263.4	1.13	45
52	117.0	13.0	0.051839	117.00	2257.0	0	2257.0	2257.0	1.13	45
56	117.0	14.0	0.052442	117.00	2231.0	0	2231.0	2231.0	1.12	45
60	116.0	15.0	0.053059	116.00	2186.3	0	2186.3	2186.3	1.09	45



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	6
Depth, ft	2-4
Load Ring No.	7
Factor, lb/div	1.0
Sample Diameter (in.)	2.875
Sample Area, (sqft)	0.0451
Sample Length (in.)	5
Sample Weight (gm)	1029.9
Can No.	539
Tare Wt (gm)	22.5
Moisture WW	111.7
Moisture DW	93.4
Moisture,%	25.8
Wdensity,lb/cuft	120.8
Ddensity,lb/cuft	96.0
Chamber Pressure (psi)	
Back Pressure (psi)	
Start Press (ksf)	
Max Comp Dial Rdg	75
Max Load Ring Rdg	101
Type Failure	yield

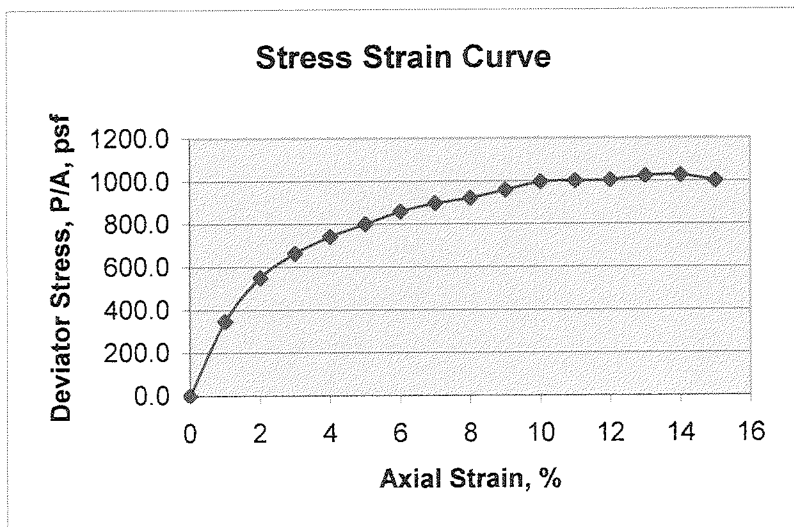
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	0	0.0	0.0	0.00	#DIV/0!
5	32.0	1.0	0.045556	32.00	702.4	0	702.4	702.4	0.35	45
10	46.0	2.0	0.04602	46.00	999.6	0	999.6	999.6	0.50	45
15	54.0	3.0	0.046495	54.00	1161.4	0	1161.4	1161.4	0.58	45
20	61.0	4.0	0.046979	61.00	1298.4	0	1298.4	1298.4	0.65	45
25	67.0	5.0	0.047474	67.00	1411.3	0	1411.3	1411.3	0.71	45
30	74.0	6.0	0.047979	74.00	1542.4	0	1542.4	1542.4	0.77	45
35	79.0	7.0	0.048495	79.00	1629.0	0	1629.0	1629.0	0.81	45
40	83.0	8.0	0.049022	83.00	1693.1	0	1693.1	1693.1	0.85	45
45	88.0	9.0	0.04956	88.00	1775.6	0	1775.6	1775.6	0.89	45
50	91.0	10.0	0.050111	91.00	1816.0	0	1816.0	1816.0	0.91	45
55	94.0	11.0	0.050674	94.00	1855.0	0	1855.0	1855.0	0.93	45
60	97.0	12.0	0.05125	97.00	1892.7	0	1892.7	1892.7	0.95	45
65	99.0	13.0	0.051839	99.00	1909.8	0	1909.8	1909.8	0.95	45
70	100.0	14.0	0.052442	100.00	1906.9	0	1906.9	1906.9	0.95	45
75	101.0	15.0	0.053059	101.00	1903.5	0	1903.5	1903.5	0.95	45



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	4-6		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.8		
Sample Area, (sqft)	0.0427		
Sample Length (in.)	4		
Sample Weight (gm)	782.9		
Can No.	425	**Tare Wt (gm)**	27.3
Moisture WW	107.4		
Moisture DW	87.8		
Moisture,%	32.4		
Wdensity,lb/cuft	121.3		
Ddensity,lb/cuft	91.6		
Chamber Pressure (psi)	4.2		
Back Pressure (psi)	2.1		
Start Press (ksf)			
Max Comp Dial Rdg	56		
Max Load Ring Rdg	68		
Type Failure	multiple shear		

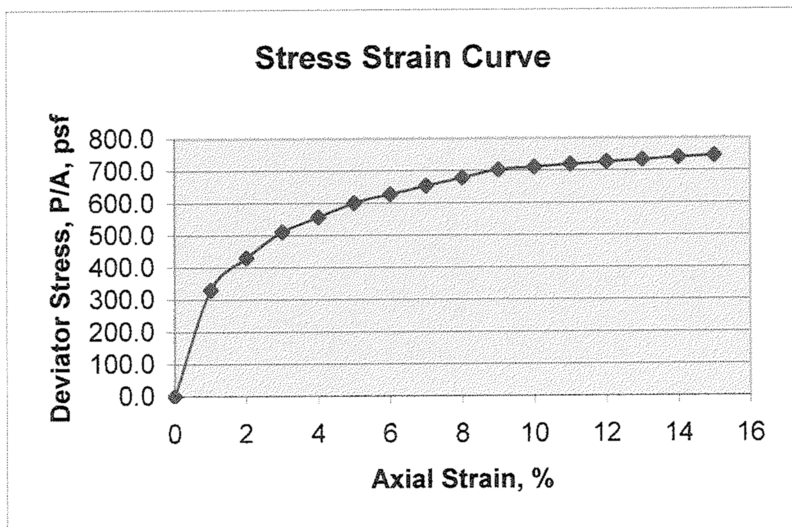
CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0427	0.00	0.0	302.4	302.4	0.0	0.00	0
4	20.0	1.0	0.043131	15.00	347.8	302.4	650.2	347.8	0.17	20
8	32.0	2.0	0.043571	24.00	550.8	302.4	853.2	550.8	0.28	25
12	39.0	3.0	0.044021	29.25	664.5	302.4	966.9	664.5	0.33	28
16	44.0	4.0	0.044479	33.00	741.9	302.4	1044.3	741.9	0.37	29
20	48.0	5.0	0.044947	36.00	800.9	302.4	1103.3	800.9	0.40	30
24	52.0	6.0	0.045426	39.00	858.5	302.4	1160.9	858.5	0.43	30
28	55.0	7.0	0.045914	41.25	898.4	302.4	1200.8	898.4	0.45	31
32	57.0	8.0	0.046413	42.75	921.1	302.4	1223.5	921.1	0.46	31
36	60.0	9.0	0.046923	45.00	959.0	302.4	1261.4	959.0	0.48	32
40	63.0	10.0	0.047444	47.25	995.9	302.4	1298.3	995.9	0.50	32
44	64.0	11.0	0.047978	48.00	1000.5	302.4	1302.9	1000.5	0.50	32
48	65.0	12.0	0.048523	48.75	1004.7	302.4	1307.1	1004.7	0.50	32
52	67.0	13.0	0.04908	50.25	1023.8	302.4	1326.2	1023.8	0.51	32
56	68.0	14.0	0.049651	51.00	1027.2	302.4	1329.6	1027.2	0.51	32
60	67.0	15.0	0.050235	50.25	1000.3	302.4	1302.7	1000.3	0.50	32



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	6-8		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.8		
Sample Area, (sqft)	0.0427		
Sample Length (in.)	5.0		
Sample Weight (gm)	958.2		
Can No. 408		**Tare Wt (gm)**	27.3
Moisture WW	115.8		
Moisture DW	92.1		
Moisture,%	36.6		
Wdensity,lb/cuft	118.7		
Ddensity,lb/cuft	86.9		
Chamber Pressure (psi)	5.8		
Back Pressure (psi)	3		
Start Press (ksf)			
Max Comp Dial Rdg	75		
Max Load Ring Rdg	50		
Type Failure	yield		

CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0427	0.00	0.0	403.2	403.2	0.0	0.00	0
5	19.0	1.0	0.043131	14.25	330.4	403.2	733.6	330.4	0.17	16
10	25.0	2.0	0.043571	18.75	430.3	403.2	833.5	430.3	0.22	19
15	30.0	3.0	0.044021	22.50	511.1	403.2	914.3	511.1	0.26	21
20	33.0	4.0	0.044479	24.75	556.4	403.2	959.6	556.4	0.28	22
25	36.0	5.0	0.044947	27.00	600.7	403.2	1003.9	600.7	0.30	23
30	38.0	6.0	0.045426	28.50	627.4	403.2	1030.6	627.4	0.31	24
35	40.0	7.0	0.045914	30.00	653.4	403.2	1056.6	653.4	0.33	24
40	42.0	8.0	0.046413	31.50	678.7	403.2	1081.9	678.7	0.34	25
45	44.0	9.0	0.046923	33.00	703.3	403.2	1106.5	703.3	0.35	25
50	45.0	10.0	0.047444	33.75	711.4	403.2	1114.6	711.4	0.36	25
55	46.0	11.0	0.047978	34.50	719.1	403.2	1122.3	719.1	0.36	25
60	47.0	12.0	0.048523	35.25	726.5	403.2	1129.7	726.5	0.36	25
65	48.0	13.0	0.04908	36.00	733.5	403.2	1136.7	733.5	0.37	25
70	49.0	14.0	0.049651	36.75	740.2	403.2	1143.4	740.2	0.37	26
75	50.0	15.0	0.050235	37.50	746.5	403.2	1149.7	746.5	0.37	26

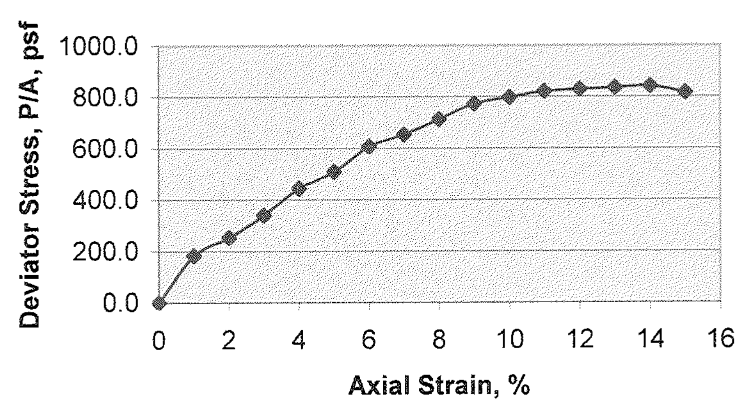


Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	10-12		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.73		
Sample Area, (sqft)	0.0406		
Sample Length (in.)	5.0		
Sample Weight (gm)	958.2		
Can No. 400		**Tare Wt (gm)**	27.3
Moisture WW	121.3		
Moisture DW	96.8		
Moisture,%	35.3		
Wdensity,lb/cuft	124.9		
Ddensity,lb/cuft	92.3		
Chamber Pressure (psi)	9.2		
Back Pressure (psi)	4.7		
Start Press (ksf)			
Max Comp Dial Rdg	70		
Max Load Ring Rdg	53		
Type Failure	multiple shear		

CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0406	0.00	0.0	648	648.0	0.0	0.00	0
5	10.0	1.0	0.04101	7.50	182.9	648	830.9	182.9	0.09	7
10	14.0	2.0	0.041429	10.50	253.4	648	901.4	253.4	0.13	9
15	19.0	3.0	0.041856	14.25	340.5	648	988.5	340.5	0.17	12
20	25.0	4.0	0.042292	18.75	443.3	648	1091.3	443.3	0.22	14
25	29.0	5.0	0.042737	21.75	508.9	648	1156.9	508.9	0.25	16
30	35.0	6.0	0.043191	26.25	607.8	648	1255.8	607.8	0.30	18
35	38.0	7.0	0.043656	28.50	652.8	648	1300.8	652.8	0.33	19
40	42.0	8.0	0.04413	31.50	713.8	648	1361.8	713.8	0.36	20
45	46.0	9.0	0.044615	34.50	773.3	648	1421.3	773.3	0.39	20
50	48.0	10.0	0.045111	36.00	798.0	648	1446.0	798.0	0.40	21
55	50.0	11.0	0.045618	37.50	822.0	648	1470.0	822.0	0.41	21
60	51.0	12.0	0.046136	38.25	829.1	648	1477.1	829.1	0.41	21
65	52.0	13.0	0.046667	39.00	835.7	648	1483.7	835.7	0.42	21
70	53.0	14.0	0.047209	39.75	842.0	648	1490.0	842.0	0.42	21
75	52.0	15.0	0.047765	39.00	816.5	648	1464.5	816.5	0.41	21

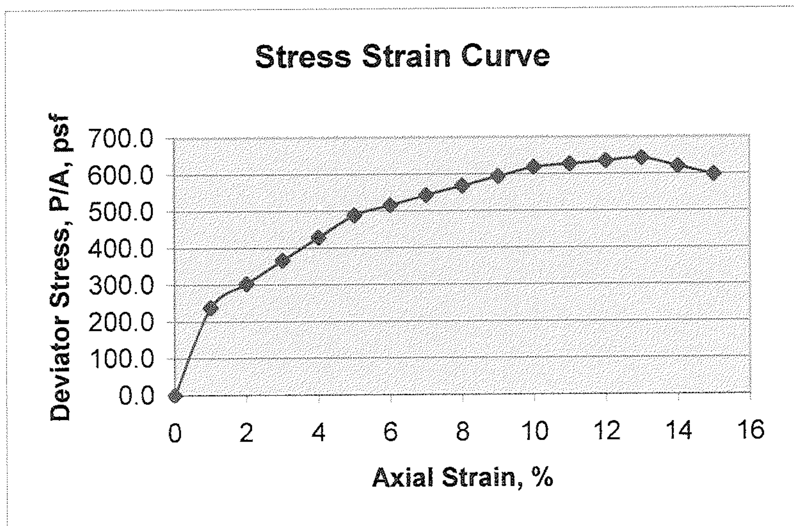
Stress Strain Curve



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	12-14		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)		2.83	
Sample Area, (sqft)		0.0437	
Sample Length (in.)		5.0	
Sample Weight (gm)		936.9	
Can No.	163	**Tare Wt (gm)**	19.3
Moisture WW		101.9	
Moisture DW		75.9	
Moisture,%		45.9	
Wdensity,lb/cuft		113.4	
Ddensity,lb/cuft		77.7	
Chamber Pressure (psi)		10.8	
Back Pressure (psi)		5.6	
Start Press (ksf)			
Max Comp Dial Rdg		65	
Max Load Ring Rdg		43	
Type Failure		multiple shear	

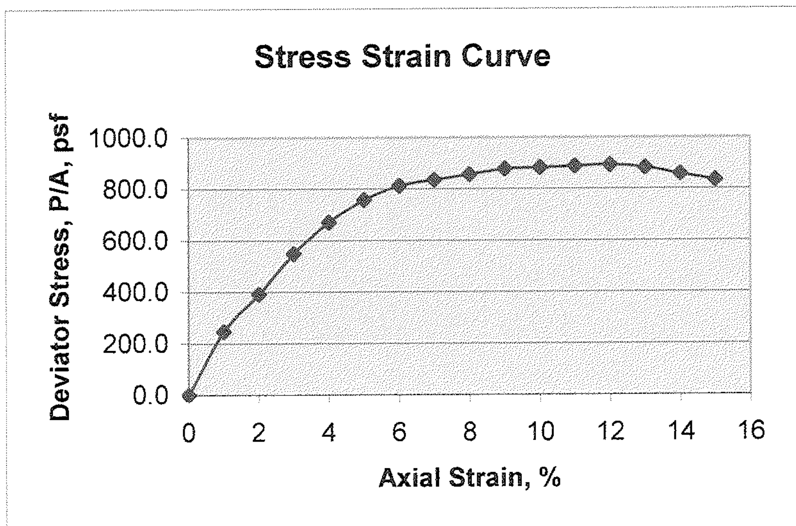
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0437	0.00	0.0	748.8	748.8	0.0	0.00	0
5	14.0	1.0	0.044141	10.50	237.9	748.8	986.7	237.9	0.12	8
10	18.0	2.0	0.044592	13.50	302.7	748.8	1051.5	302.7	0.15	10
15	22.0	3.0	0.045052	16.50	366.2	748.8	1115.0	366.2	0.18	11
20	26.0	4.0	0.045521	19.50	428.4	748.8	1177.2	428.4	0.21	13
25	30.0	5.0	0.046	22.50	489.1	748.8	1237.9	489.1	0.24	14
30	32.0	6.0	0.046489	24.00	516.2	748.8	1265.0	516.2	0.26	14
35	34.0	7.0	0.046989	25.50	542.7	748.8	1291.5	542.7	0.27	15
40	36.0	8.0	0.0475	27.00	568.4	748.8	1317.2	568.4	0.28	15
45	38.0	9.0	0.048022	28.50	593.5	748.8	1342.3	593.5	0.30	16
50	40.0	10.0	0.048556	30.00	617.8	748.8	1366.6	617.8	0.31	16
55	41.0	11.0	0.049101	30.75	626.3	748.8	1375.1	626.3	0.31	16
60	42.0	12.0	0.049659	31.50	634.3	748.8	1383.1	634.3	0.32	17
65	43.0	13.0	0.05023	32.25	642.0	748.8	1390.8	642.0	0.32	17
70	42.0	14.0	0.050814	31.50	619.9	748.8	1368.7	619.9	0.31	16
75	41.0	15.0	0.051412	30.75	598.1	748.8	1346.9	598.1	0.30	16



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	6
Depth, ft	14-16
Load Ring No.	3
Factor, lb/div	0.75
Sample Diameter (in.)	2.875
Sample Area, (sqft)	0.0451
Sample Length (in.)	5.0
Sample Weight (gm)	927.7
Can No.	413
Tare Wt (gm)	27.3
Moisture WW	133.1
Moisture DW	102.5
Moisture,%	40.7
Wdensity,lb/cuft	108.8
Ddensity,lb/cuft	77.4
Chamber Pressure (psi)	12.5
Back Pressure (psi)	6.5
Start Press (ksf)	
Max Comp Dial Rdg	60
Max Load Ring Rdg	61
Type Failure	multiple shear

CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	864	864.0	0.0	0.00	0
5	15.0	1.0	0.045556	11.25	247.0	864	1111.0	247.0	0.12	7
10	24.0	2.0	0.04602	18.00	391.1	864	1255.1	391.1	0.20	10
15	34.0	3.0	0.046495	25.50	548.4	864	1412.4	548.4	0.27	14
20	42.0	4.0	0.046979	31.50	670.5	864	1534.5	670.5	0.34	16
25	48.0	5.0	0.047474	36.00	758.3	864	1622.3	758.3	0.38	17
30	52.0	6.0	0.047979	39.00	812.9	864	1676.9	812.9	0.41	18
35	54.0	7.0	0.048495	40.50	835.1	864	1699.1	835.1	0.42	18
40	56.0	8.0	0.049022	42.00	856.8	864	1720.8	856.8	0.43	18
45	58.0	9.0	0.04956	43.50	877.7	864	1741.7	877.7	0.44	19
50	59.0	10.0	0.050111	44.25	883.0	864	1747.0	883.0	0.44	19
55	60.0	11.0	0.050674	45.00	888.0	864	1752.0	888.0	0.44	19
60	61.0	12.0	0.05125	45.75	892.7	864	1756.7	892.7	0.45	19
65	61.0	13.0	0.051839	45.75	882.5	864	1746.5	882.5	0.44	19
70	60.0	14.0	0.052442	45.00	858.1	864	1722.1	858.1	0.43	18
75	59.0	15.0	0.053059	44.25	834.0	864	1698.0	834.0	0.42	18

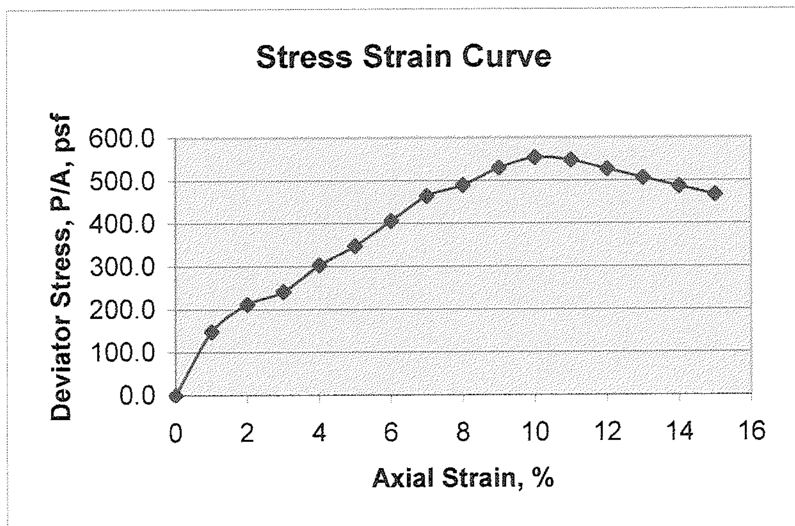


Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	6
Depth, ft	16-18

Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)		2.875	
Sample Area, (sqft)		0.0451	
Sample Length (in.)		5.0	
Sample Weight (gm)		961.5	
Can No.	42	**Tare Wt (gm)**	13.5
Moisture WW		121.8	
Moisture DW		91.8	
Moisture,%		38.3	
Wdensity,lb/cuft		112.8	
Ddensity,lb/cuft		81.6	
Chamber Pressure (psi)		14.2	
Back Pressure (psi)		7.3	
Start Press (ksf)			
Max Comp Dial Rdg		50	
Max Load Ring Rdg		37	
Type Failure		multiple shear	

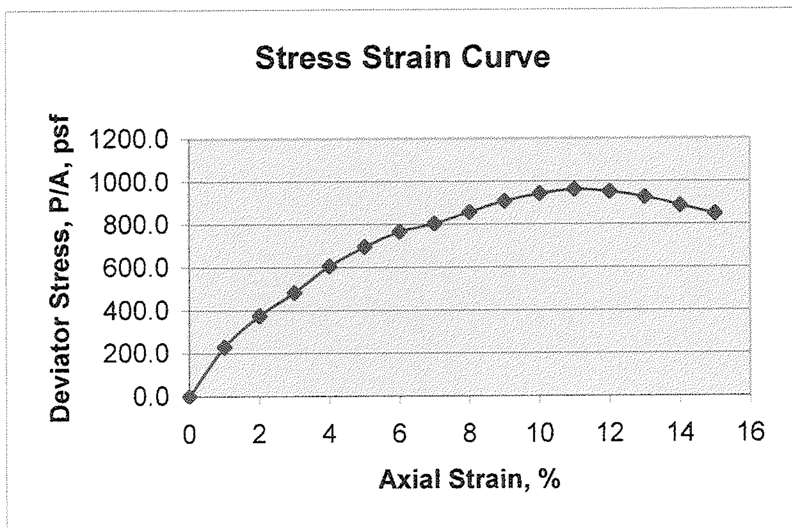
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	993.6	993.6	0.0	0.00	0
5	9.0	1.0	0.045556	6.75	148.2	993.6	1141.8	148.2	0.07	4
10	13.0	2.0	0.04602	9.75	211.9	993.6	1205.5	211.9	0.11	6
15	15.0	3.0	0.046495	11.25	242.0	993.6	1235.6	242.0	0.12	6
20	19.0	4.0	0.046979	14.25	303.3	993.6	1296.9	303.3	0.15	8
25	22.0	5.0	0.047474	16.50	347.6	993.6	1341.2	347.6	0.17	8
30	26.0	6.0	0.047979	19.50	406.4	993.6	1400.0	406.4	0.20	10
35	30.0	7.0	0.048495	22.50	464.0	993.6	1457.6	464.0	0.23	11
40	32.0	8.0	0.049022	24.00	489.6	993.6	1483.2	489.6	0.24	11
45	35.0	9.0	0.04956	26.25	529.7	993.6	1523.3	529.7	0.26	12
50	37.0	10.0	0.050111	27.75	553.8	993.6	1547.4	553.8	0.28	12
55	37.0	11.0	0.050674	27.75	547.6	993.6	1541.2	547.6	0.27	12
60	36.0	12.0	0.05125	27.00	526.8	993.6	1520.4	526.8	0.26	12
65	35.0	13.0	0.051839	26.25	506.4	993.6	1500.0	506.4	0.25	11
70	34.0	14.0	0.052442	25.50	486.3	993.6	1479.9	486.3	0.24	11
75	33.0	15.0	0.053059	24.75	466.5	993.6	1460.1	466.5	0.23	11



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	6
Depth, ft	18-20
Load Ring No.	3
Factor, lb/div	0.75
Sample Diameter (in.)	2.875
Sample Area, (sqft)	0.0451
Sample Length (in.)	4
Sample Weight (gm)	768.8
Can No.	111
Tare Wt (gm)	19.3
Moisture WW	123.3
Moisture DW	96.4
Moisture,%	34.9
Wdensity,lb/cuft	112.7
Ddensity,lb/cuft	83.6
Chamber Pressure (psi)	15.8
Back Pressure (psi)	8.2
Start Press (ksf)	
Max Comp Dial Rdg	44
Max Load Ring Rdg	65
Type Failure	multiple shear

CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	1094.4	1094.4	0.0	0.00	0
4	14.0	1.0	0.045556	10.50	230.5	1094.4	1324.9	230.5	0.12	5
8	23.0	2.0	0.04602	17.25	374.8	1094.4	1469.2	374.8	0.19	8
12	30.0	3.0	0.046495	22.50	483.9	1094.4	1578.3	483.9	0.24	10
16	38.0	4.0	0.046979	28.50	606.7	1094.4	1701.1	606.7	0.30	12
20	44.0	5.0	0.047474	33.00	695.1	1094.4	1789.5	695.1	0.35	14
24	49.0	6.0	0.047979	36.75	766.0	1094.4	1860.4	766.0	0.38	15
28	52.0	7.0	0.048495	39.00	804.2	1094.4	1898.6	804.2	0.40	15
32	56.0	8.0	0.049022	42.00	856.8	1094.4	1951.2	856.8	0.43	16
36	60.0	9.0	0.04956	45.00	908.0	1094.4	2002.4	908.0	0.45	16
40	63.0	10.0	0.050111	47.25	942.9	1094.4	2037.3	942.9	0.47	17
44	65.0	11.0	0.050674	48.75	962.0	1094.4	2056.4	962.0	0.48	17
48	65.0	12.0	0.05125	48.75	951.2	1094.4	2045.6	951.2	0.48	17
52	64.0	13.0	0.051839	48.00	925.9	1094.4	2020.3	925.9	0.46	17
56	62.0	14.0	0.052442	46.50	886.7	1094.4	1981.1	886.7	0.44	16
60	60.0	15.0	0.053059	45.00	848.1	1094.4	1942.5	848.1	0.42	16

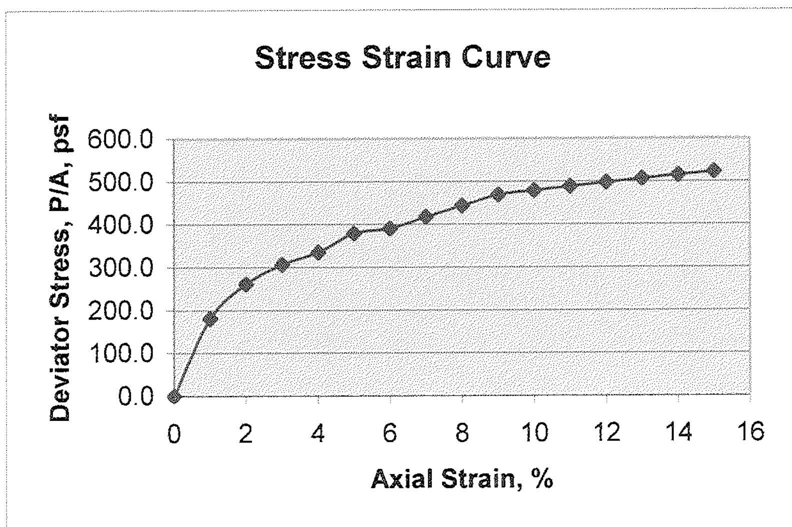


Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	6
Depth, ft	20-22

Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)		2.875	
Sample Area, (sqft)		0.0451	
Sample Length (in.)		5	
Sample Weight (gm)		990.1	
Can No.	137	**Tare Wt (gm)**	19.3
Moisture WW		117.0	
Moisture DW		96.4	
Moisture,%		90.0	
Wdensity,lb/cuft		116.2	
Ddensity,lb/cuft		61.1	
Chamber Pressure (psi)		17.5	
Back Pressure (psi)		9	
Start Press (ksf)			
Max Comp Dial Rdg		75	
Max Load Ring Rdg		37	
Type Failure		yield	

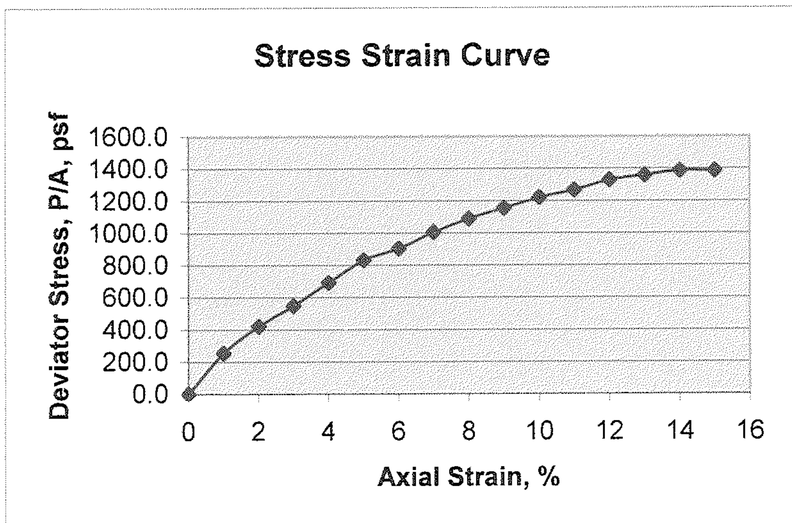
CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	1224	1224.0	0.0	0.00	0
5	11.0	1.0	0.045556	8.25	181.1	1224	1405.1	181.1	0.09	4
10	16.0	2.0	0.04602	12.00	260.8	1224	1484.8	260.8	0.13	5
15	19.0	3.0	0.046495	14.25	306.5	1224	1530.5	306.5	0.15	6
20	21.0	4.0	0.046979	15.75	335.3	1224	1559.3	335.3	0.17	7
25	24.0	5.0	0.047474	18.00	379.2	1224	1603.2	379.2	0.19	8
30	25.0	6.0	0.047979	18.75	390.8	1224	1614.8	390.8	0.20	8
35	27.0	7.0	0.048495	20.25	417.6	1224	1641.6	417.6	0.21	8
40	29.0	8.0	0.049022	21.75	443.7	1224	1667.7	443.7	0.22	9
45	31.0	9.0	0.04956	23.25	469.1	1224	1693.1	469.1	0.23	9
50	32.0	10.0	0.050111	24.00	478.9	1224	1702.9	478.9	0.24	9
55	33.0	11.0	0.050674	24.75	488.4	1224	1712.4	488.4	0.24	9
60	34.0	12.0	0.05125	25.50	497.6	1224	1721.6	497.6	0.25	10
65	35.0	13.0	0.051839	26.25	506.4	1224	1730.4	506.4	0.25	10
70	36.0	14.0	0.052442	27.00	514.9	1224	1738.9	514.9	0.26	10
75	37.0	15.0	0.053059	27.75	523.0	1224	1747.0	523.0	0.26	10



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	6
Depth, ft	22-24
Load Ring No.	3
Factor, lb/div	0.75
Sample Diameter (in.)	2.83
Sample Area, (sqft)	0.0437
Sample Length (in.)	4
Sample Weight (gm)	717.9
Can No.	531
Tare Wt (gm)	22.5
Moisture WW	110.3
Moisture DW	88.7
Moisture,%	90.0
Wdensity,lb/cuft	108.7
Ddensity,lb/cuft	57.2
Chamber Pressure (psi)	19.2
Back Pressure (psi)	9.9
Start Press (ksf)	
Max Comp Dial Rdg	60
Max Load Ring Rdg	95
Type Failure	yield

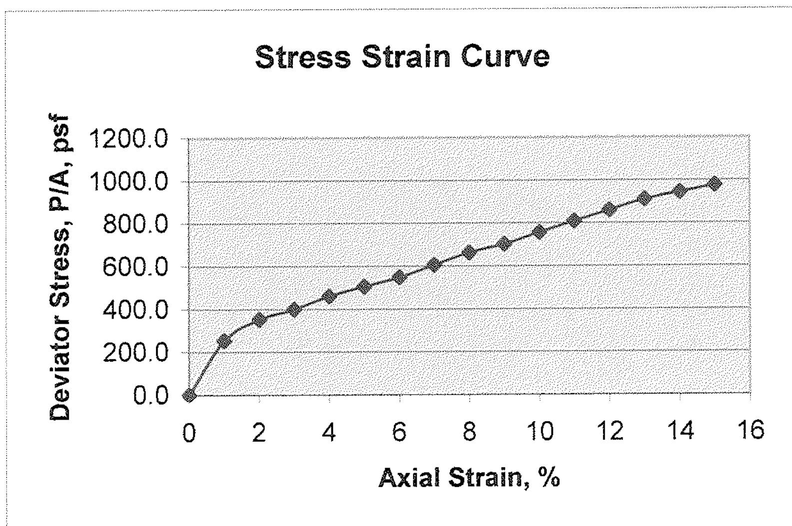
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0437	0.00	0.0	1339.2	1339.2	0.0	0.00	0
4	15.0	1.0	0.044141	11.25	254.9	1339.2	1594.1	254.9	0.13	5
8	25.0	2.0	0.044592	18.75	420.5	1339.2	1759.7	420.5	0.21	8
12	33.0	3.0	0.045052	24.75	549.4	1339.2	1888.6	549.4	0.27	10
16	42.0	4.0	0.045521	31.50	692.0	1339.2	2031.2	692.0	0.35	12
20	51.0	5.0	0.046	38.25	831.5	1339.2	2170.7	831.5	0.42	13
24	56.0	6.0	0.046489	42.00	903.4	1339.2	2242.6	903.4	0.45	14
28	63.0	7.0	0.046989	47.25	1005.5	1339.2	2344.7	1005.5	0.50	15
32	69.0	8.0	0.0475	51.75	1089.5	1339.2	2428.7	1089.5	0.54	16
36	74.0	9.0	0.048022	55.50	1155.7	1339.2	2494.9	1155.7	0.58	17
40	79.0	10.0	0.048556	59.25	1220.3	1339.2	2559.5	1220.3	0.61	17
44	83.0	11.0	0.049101	62.25	1267.8	1339.2	2607.0	1267.8	0.63	18
48	88.0	12.0	0.049659	66.00	1329.1	1339.2	2668.3	1329.1	0.66	18
52	91.0	13.0	0.05023	68.25	1358.8	1339.2	2698.0	1358.8	0.68	19
56	94.0	14.0	0.050814	70.50	1387.4	1339.2	2726.6	1387.4	0.69	19
60	95.0	15.0	0.051412	71.25	1385.9	1339.2	2725.1	1385.9	0.69	19



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	24-26		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.83		
Sample Area, (sqft)	0.0437		
Sample Length (in.)	5		
Sample Weight (gm)	887.4		
Can No. 331	**Tare Wt (gm)**	18.3	
Moisture WW	110.9		
Moisture DW	86.6		
Moisture,%	90.0		
Wdensity,lb/cuft	107.4		
Ddensity,lb/cuft	56.5		
Chamber Pressure (psi)	20.9		
Back Pressure (psi)	10.8		
Start Press (ksf)			
Max Comp Dial Rdg	75		
Max Load Ring Rdg	67		
Type Failure	yield		

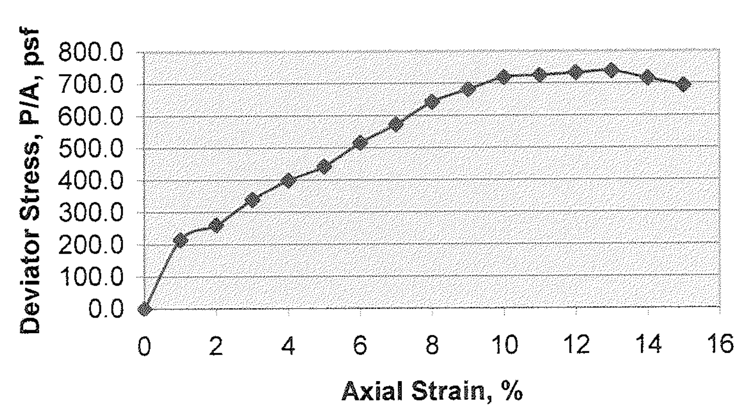
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0437	0.00	0.0	1454.4	1454.4	0.0	0.00	0
5	15.0	1.0	0.044141	11.25	254.9	1454.4	1709.3	254.9	0.13	5
10	21.0	2.0	0.044592	15.75	353.2	1454.4	1807.6	353.2	0.18	6
15	24.0	3.0	0.045052	18.00	399.5	1454.4	1853.9	399.5	0.20	7
20	28.0	4.0	0.045521	21.00	461.3	1454.4	1915.7	461.3	0.23	8
25	31.0	5.0	0.046	23.25	505.4	1454.4	1959.8	505.4	0.25	8
30	34.0	6.0	0.046489	25.50	548.5	1454.4	2002.9	548.5	0.27	9
35	38.0	7.0	0.046989	28.50	606.5	1454.4	2060.9	606.5	0.30	10
40	42.0	8.0	0.0475	31.50	663.2	1454.4	2117.6	663.2	0.33	11
45	45.0	9.0	0.048022	33.75	702.8	1454.4	2157.2	702.8	0.35	11
50	49.0	10.0	0.048556	36.75	756.9	1454.4	2211.3	756.9	0.38	12
55	53.0	11.0	0.049101	39.75	809.6	1454.4	2264.0	809.6	0.40	12
60	57.0	12.0	0.049659	42.75	860.9	1454.4	2315.3	860.9	0.43	13
65	61.0	13.0	0.05023	45.75	910.8	1454.4	2365.2	910.8	0.46	13
70	64.0	14.0	0.050814	48.00	944.6	1454.4	2399.0	944.6	0.47	14
75	67.0	15.0	0.051412	50.25	977.4	1454.4	2431.8	977.4	0.49	14



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	26-28		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.875		
Sample Area, (sqft)	0.0451		
Sample Length (in.)	5		
Sample Weight (gm)	851.1		
Can No. 199	**Tare Wt (gm)**	19.3	
Moisture WW	103.3		
Moisture DW	67.9		
Moisture,%	90.0		
Wdensity,lb/cuft	99.8		
Ddensity,lb/cuft	52.6		
Chamber Pressure (psi)	22.4		
Back Pressure (psi)	11.6		
Start Press (ksf)			
Max Comp Dial Rdg	65		
Max Load Ring Rdg	51		
Type Failure	multiple shear		

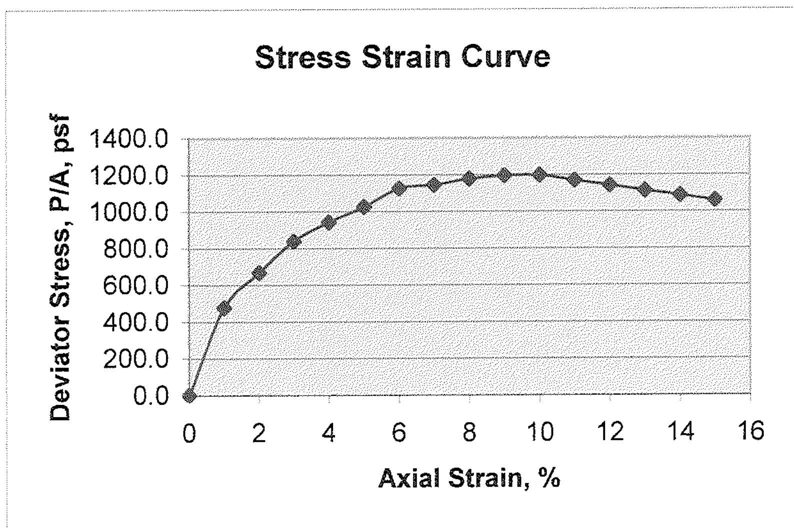
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	1555.2	1555.2	0.0	0.00	0
5	13.0	1.0	0.045556	9.75	214.0	1555.2	1769.2	214.0	0.11	4
10	16.0	2.0	0.04602	12.00	260.8	1555.2	1816.0	260.8	0.13	4
15	21.0	3.0	0.046495	15.75	338.7	1555.2	1893.9	338.7	0.17	6
20	25.0	4.0	0.046979	18.75	399.1	1555.2	1954.3	399.1	0.20	6
25	28.0	5.0	0.047474	21.00	442.4	1555.2	1997.6	442.4	0.22	7
30	33.0	6.0	0.047979	24.75	515.9	1555.2	2071.1	515.9	0.26	8
35	37.0	7.0	0.048495	27.75	572.2	1555.2	2127.4	572.2	0.29	9
40	42.0	8.0	0.049022	31.50	642.6	1555.2	2197.8	642.6	0.32	10
45	45.0	9.0	0.04956	33.75	681.0	1555.2	2236.2	681.0	0.34	10
50	48.0	10.0	0.050111	36.00	718.4	1555.2	2273.6	718.4	0.36	11
55	49.0	11.0	0.050674	36.75	725.2	1555.2	2280.4	725.2	0.36	11
60	50.0	12.0	0.05125	37.50	731.7	1555.2	2286.9	731.7	0.37	11
65	51.0	13.0	0.051839	38.25	737.9	1555.2	2293.1	737.9	0.37	11
70	50.0	14.0	0.052442	37.50	715.1	1555.2	2270.3	715.1	0.36	11
75	49.0	15.0	0.053059	36.75	692.6	1555.2	2247.8	692.6	0.35	10



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	28-30		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)		2.875	
Sample Area, (sqft)		0.0451	
Sample Length (in.)		4	
Sample Weight (gm)		653.1	
Can No.	405	**Tare Wt (gm)**	27.3
Moisture WW		115.7	
Moisture DW		79.0	
Moisture,%		90.0	
Wdensity,lb/cuft		95.8	
Ddensity,lb/cuft		50.4	
Chamber Pressure (psi)		24.2	
Back Pressure (psi)		12.5	
Start Press (ksf)			
Max Comp Dial Rdg		40	
Max Load Ring Rdg		80	
Type Failure		multiple shear	

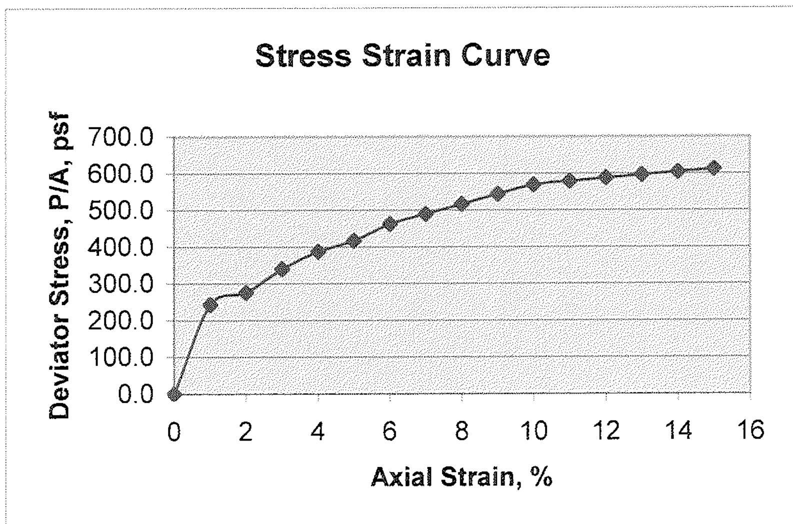
CMP Dial (in.) 0.001	Load Reading	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	1684.8	1684.8	0.0	0.00	0
4	29.0	1.0	0.045556	21.75	477.4	1684.8	2162.2	477.4	0.24	7
8	41.0	2.0	0.04602	30.75	668.2	1684.8	2353.0	668.2	0.33	9
12	52.0	3.0	0.046495	39.00	838.8	1684.8	2523.6	838.8	0.42	11
16	59.0	4.0	0.046979	44.25	941.9	1684.8	2626.7	941.9	0.47	12
20	65.0	5.0	0.047474	48.75	1026.9	1684.8	2711.7	1026.9	0.51	13
24	72.0	6.0	0.047979	54.00	1125.5	1684.8	2810.3	1125.5	0.56	14
28	74.0	7.0	0.048495	55.50	1144.5	1684.8	2829.3	1144.5	0.57	14
32	77.0	8.0	0.049022	57.75	1178.0	1684.8	2862.8	1178.0	0.59	15
36	79.0	9.0	0.04956	59.25	1195.5	1684.8	2880.3	1195.5	0.60	15
40	80.0	10.0	0.050111	60.00	1197.3	1684.8	2882.1	1197.3	0.60	15
44	79.0	11.0	0.050674	59.25	1169.2	1684.8	2854.0	1169.2	0.58	14
48	78.0	12.0	0.05125	58.50	1141.5	1684.8	2826.3	1141.5	0.57	14
52	77.0	13.0	0.051839	57.75	1114.0	1684.8	2798.8	1114.0	0.56	14
56	76.0	14.0	0.052442	57.00	1086.9	1684.8	2771.7	1086.9	0.54	14
60	75.0	15.0	0.053059	56.25	1060.1	1684.8	2744.9	1060.1	0.53	13



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	30-32		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.8		
Sample Area, (sqft)	0.0427		
Sample Length (in.)	5		
Sample Weight (gm)	809.5		
Can No.	318	**Tare Wt (gm)**	18.3
Moisture WW	103.4		
Moisture DW	75.1		
Moisture,%	90.0		
Wdensity,lb/cuft	100.3		
Ddensity,lb/cuft	52.8		
Chamber Pressure (psi)	25.8		
Back Pressure (psi)	13.3		
Start Press (ksf)			
Max Comp Dial Rdg	75		
Max Load Ring Rdg	41		
Type Failure	yield		

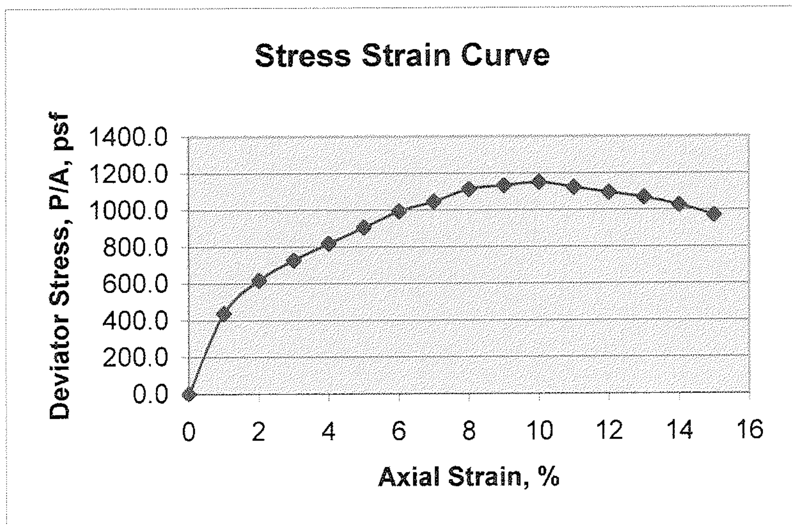
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0427	0.00	0.0	1800	1800.0	0.0	0.00	0
5	14.0	1.0	0.043131	10.50	243.4	1800	2043.4	243.4	0.12	4
10	16.0	2.0	0.043571	12.00	275.4	1800	2075.4	275.4	0.14	4
15	20.0	3.0	0.044021	15.00	340.7	1800	2140.7	340.7	0.17	5
20	23.0	4.0	0.044479	17.25	387.8	1800	2187.8	387.8	0.19	6
25	25.0	5.0	0.044947	18.75	417.2	1800	2217.2	417.2	0.21	6
30	28.0	6.0	0.045426	21.00	462.3	1800	2262.3	462.3	0.23	6
35	30.0	7.0	0.045914	22.50	490.0	1800	2290.0	490.0	0.25	7
40	32.0	8.0	0.046413	24.00	517.1	1800	2317.1	517.1	0.26	7
45	34.0	9.0	0.046923	25.50	543.4	1800	2343.4	543.4	0.27	7
50	36.0	10.0	0.047444	27.00	569.1	1800	2369.1	569.1	0.28	8
55	37.0	11.0	0.047978	27.75	578.4	1800	2378.4	578.4	0.29	8
60	38.0	12.0	0.048523	28.50	587.4	1800	2387.4	587.4	0.29	8
65	39.0	13.0	0.04908	29.25	596.0	1800	2396.0	596.0	0.30	8
70	40.0	14.0	0.049651	30.00	604.2	1800	2404.2	604.2	0.30	8
75	41.0	15.0	0.050235	30.75	612.1	1800	2412.1	612.1	0.31	8



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	32-34		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.84		
Sample Area, (sqft)	0.0440		
Sample Length (in.)	5		
Sample Weight (gm)	828.1		
Can No.	212	**Tare Wt (gm)**	18.3
Moisture WW	111.6		
Moisture DW	72.5		
Moisture,%	90.0		
Wdensity,lb/cuft	99.6		
Ddensity,lb/cuft	52.4		
Chamber Pressure (psi)	27.5		
Back Pressure (psi)	14.2		
Start Press (ksf)			
Max Comp Dial Rdg	50		
Max Load Ring Rdg	75		
Type Failure	multiple shear		

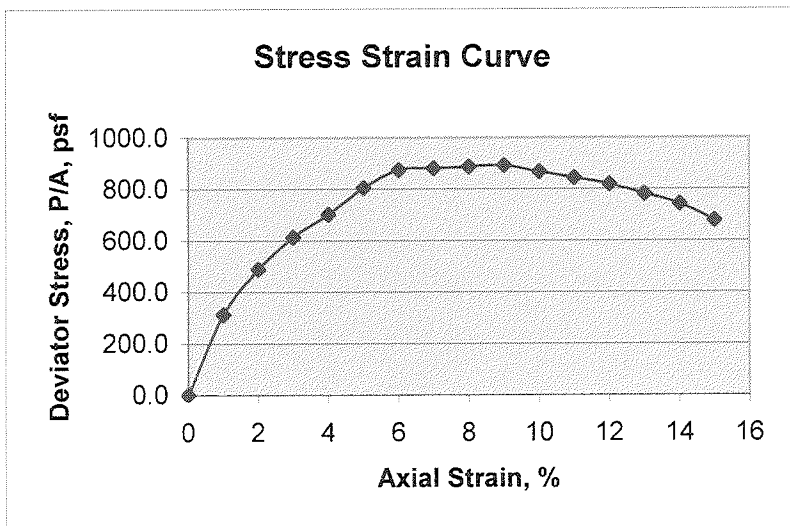
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.044	0.00	0.0	1915.2	1915.2	0.0	0.00	0
5	26.0	1.0	0.044444	19.50	438.8	1915.2	2354.0	438.8	0.22	6
10	37.0	2.0	0.044898	27.75	618.1	1915.2	2533.3	618.1	0.31	8
15	44.0	3.0	0.045361	33.00	727.5	1915.2	2642.7	727.5	0.36	9
20	50.0	4.0	0.045833	37.50	818.2	1915.2	2733.4	818.2	0.41	10
25	56.0	5.0	0.046316	42.00	906.8	1915.2	2822.0	906.8	0.45	11
30	62.0	6.0	0.046809	46.50	993.4	1915.2	2908.6	993.4	0.50	12
35	66.0	7.0	0.047312	49.50	1046.3	1915.2	2961.5	1046.3	0.52	12
40	71.0	8.0	0.047826	53.25	1113.4	1915.2	3028.6	1113.4	0.56	13
45	73.0	9.0	0.048352	54.75	1132.3	1915.2	3047.5	1132.3	0.57	13
50	75.0	10.0	0.048889	56.25	1150.6	1915.2	3065.8	1150.6	0.58	13
55	74.0	11.0	0.049438	55.50	1122.6	1915.2	3037.8	1122.6	0.56	13
60	73.0	12.0	0.05	54.75	1095.0	1915.2	3010.2	1095.0	0.55	13
65	72.0	13.0	0.050575	54.00	1067.7	1915.2	2982.9	1067.7	0.53	12
70	70.0	14.0	0.051163	52.50	1026.1	1915.2	2941.3	1026.1	0.51	12
75	67.0	15.0	0.051765	50.25	970.7	1915.2	2885.9	970.7	0.49	11



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110
Boring No.	6
Depth, ft	34-36
Load Ring No.	3
Factor, lb/div	0.75
Sample Diameter (in.)	2.875
Sample Area, (sqft)	0.0451
Sample Length (in.)	5
Sample Weight (gm)	857.0
Can No.	558
Tare Wt (gm)	22.5
Moisture WW	102.0
Moisture DW	70.9
Moisture,%	90.0
Wdensity,lb/cuft	100.5
Ddensity,lb/cuft	52.9
Chamber Pressure (psi)	29.2
Back Pressure (psi)	15.1
Start Press (ksf)	
Max Comp Dial Rdg	45
Max Load Ring Rdg	59
Type Failure	SLS

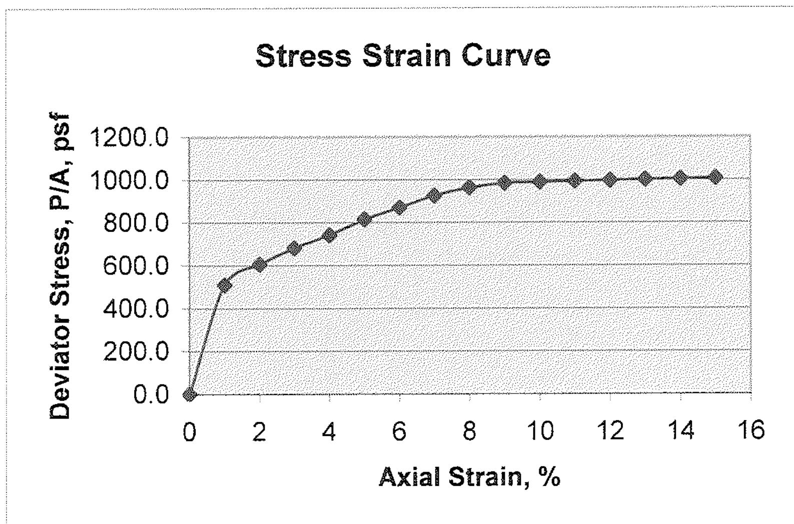
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0451	0.00	0.0	2030.4	2030.4	0.0	0.00	0
5	19.0	1.0	0.045556	14.25	312.8	2030.4	2343.2	312.8	0.16	4
10	30.0	2.0	0.04602	22.50	488.9	2030.4	2519.3	488.9	0.24	6
15	38.0	3.0	0.046495	28.50	613.0	2030.4	2643.4	613.0	0.31	7
20	44.0	4.0	0.046979	33.00	702.4	2030.4	2732.8	702.4	0.35	8
25	51.0	5.0	0.047474	38.25	805.7	2030.4	2836.1	805.7	0.40	9
30	56.0	6.0	0.047979	42.00	875.4	2030.4	2905.8	875.4	0.44	10
35	57.0	7.0	0.048495	42.75	881.5	2030.4	2911.9	881.5	0.44	10
40	58.0	8.0	0.049022	43.50	887.4	2030.4	2917.8	887.4	0.44	10
45	59.0	9.0	0.04956	44.25	892.8	2030.4	2923.2	892.8	0.45	10
50	58.0	10.0	0.050111	43.50	868.1	2030.4	2898.5	868.1	0.43	10
55	57.0	11.0	0.050674	42.75	843.6	2030.4	2874.0	843.6	0.42	10
60	56.0	12.0	0.05125	42.00	819.5	2030.4	2849.9	819.5	0.41	10
65	54.0	13.0	0.051839	40.50	781.3	2030.4	2811.7	781.3	0.39	9
70	52.0	14.0	0.052442	39.00	743.7	2030.4	2774.1	743.7	0.37	9
75	48.0	15.0	0.053059	36.00	678.5	2030.4	2708.9	678.5	0.34	8



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	36-38		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.83		
Sample Area, (sqft)	0.0437		
Sample Length (in.)	5		
Sample Weight (gm)	827.9		
Can No.	509	**Tare Wt (gm)**	22.5
Moisture WW	121.7		
Moisture DW	85.4		
Moisture,%	90.0		
Wdensity,lb/cuft	100.2		
Ddensity,lb/cuft	52.8		
Chamber Pressure (psi)	30.8		
Back Pressure (psi)	15.9		
Start Press (ksf)			
Max Comp Dial Rdg	75		
Max Load Ring Rdg	69		
Type Failure	yield		

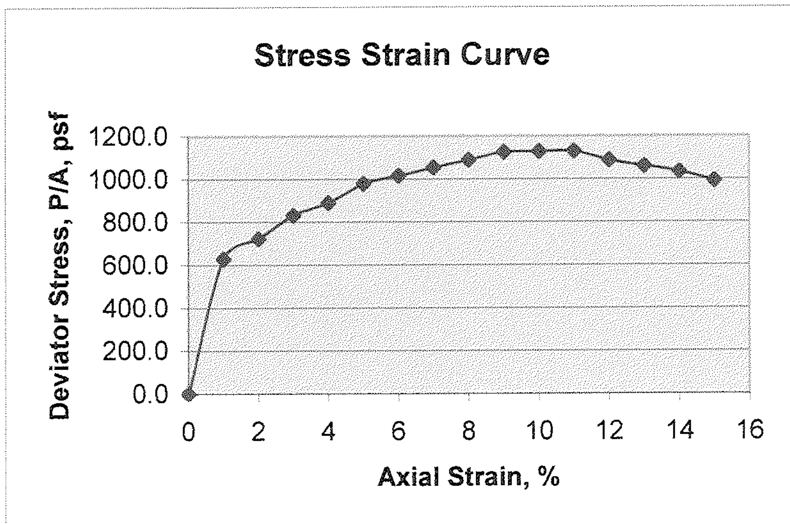
CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0437	0.00	0.0	2145.6	2145.6	0.0	0.00	0
5	30.0	1.0	0.044141	22.50	509.7	2145.6	2655.3	509.7	0.25	6
10	36.0	2.0	0.044592	27.00	605.5	2145.6	2751.1	605.5	0.30	7
15	41.0	3.0	0.045052	30.75	682.6	2145.6	2828.2	682.6	0.34	8
20	45.0	4.0	0.045521	33.75	741.4	2145.6	2887.0	741.4	0.37	8
25	50.0	5.0	0.046	37.50	815.2	2145.6	2960.8	815.2	0.41	9
30	54.0	6.0	0.046489	40.50	871.2	2145.6	3016.8	871.2	0.44	10
35	58.0	7.0	0.046989	43.50	925.7	2145.6	3071.3	925.7	0.46	10
40	61.0	8.0	0.0475	45.75	963.2	2145.6	3108.8	963.2	0.48	10
45	63.0	9.0	0.048022	47.25	983.9	2145.6	3129.5	983.9	0.49	11
50	64.0	10.0	0.048556	48.00	988.6	2145.6	3134.2	988.6	0.49	11
55	65.0	11.0	0.049101	48.75	992.8	2145.6	3138.4	992.8	0.50	11
60	66.0	12.0	0.049659	49.50	996.8	2145.6	3142.4	996.8	0.50	11
65	67.0	13.0	0.05023	50.25	1000.4	2145.6	3146.0	1000.4	0.50	11
70	68.0	14.0	0.050814	51.00	1003.7	2145.6	3149.3	1003.7	0.50	11
75	69.0	15.0	0.051412	51.75	1006.6	2145.6	3152.2	1006.6	0.50	11



Louis J. Capozzoli & Associates, Inc.

Job No.	07-110		
Boring No.	6		
Depth, ft	38-40		
Load Ring No.	3	**Factor, lb/div**	0.75
Sample Diameter (in.)	2.83		
Sample Area, (sqft)	0.0437		
Sample Length (in.)	5		
Sample Weight (gm)	861.7		
Can No.	415	**Tare Wt (gm)**	27.3
Moisture WW	108.2		
Moisture DW	80.0		
Moisture,%	90.0		
Wdensity,lb/cuft	104.3		
Ddensity,lb/cuft	54.9		
Chamber Pressure (psi)	32.5		
Back Pressure (psi)	16.8		
Start Press (ksf)			
Max Comp Dial Rdg	55		
Max Load Ring Rdg	74		
Type Failure	multiple shear		

CMP Dial (in.) 0.001	**Load Reading**	Strain	Area,sqft	Load,lb	P/A, psf	Sigma3 psf	Sigma1 psf	Q Sig1-Sig3 psf	Su Q/2 ksf	Alpha
0	0.0	0.0	0.0437	0.00	0.0	2260.8	2260.8	0.0	0.00	0
5	37.0	1.0	0.044141	27.75	628.7	2260.8	2889.5	628.7	0.31	7
10	43.0	2.0	0.044592	32.25	723.2	2260.8	2984.0	723.2	0.36	8
15	50.0	3.0	0.045052	37.50	832.4	2260.8	3093.2	832.4	0.42	9
20	54.0	4.0	0.045521	40.50	889.7	2260.8	3150.5	889.7	0.44	9
25	60.0	5.0	0.046	45.00	978.3	2260.8	3239.1	978.3	0.49	10
30	63.0	6.0	0.046489	47.25	1016.4	2260.8	3277.2	1016.4	0.51	10
35	66.0	7.0	0.046989	49.50	1053.4	2260.8	3314.2	1053.4	0.53	11
40	69.0	8.0	0.0475	51.75	1089.5	2260.8	3350.3	1089.5	0.54	11
45	72.0	9.0	0.048022	54.00	1124.5	2260.8	3385.3	1124.5	0.56	11
50	73.0	10.0	0.048556	54.75	1127.6	2260.8	3388.4	1127.6	0.56	11
55	74.0	11.0	0.049101	55.50	1130.3	2260.8	3391.1	1130.3	0.57	11
60	72.0	12.0	0.049659	54.00	1087.4	2260.8	3348.2	1087.4	0.54	11
65	71.0	13.0	0.05023	53.25	1060.1	2260.8	3320.9	1060.1	0.53	11
70	70.0	14.0	0.050814	52.50	1033.2	2260.8	3294.0	1033.2	0.52	11
75	68.0	15.0	0.051412	51.00	992.0	2260.8	3252.8	992.0	0.50	10



Subject DESIGN STRENGTH
JACK & BORE SITES, B-5&6

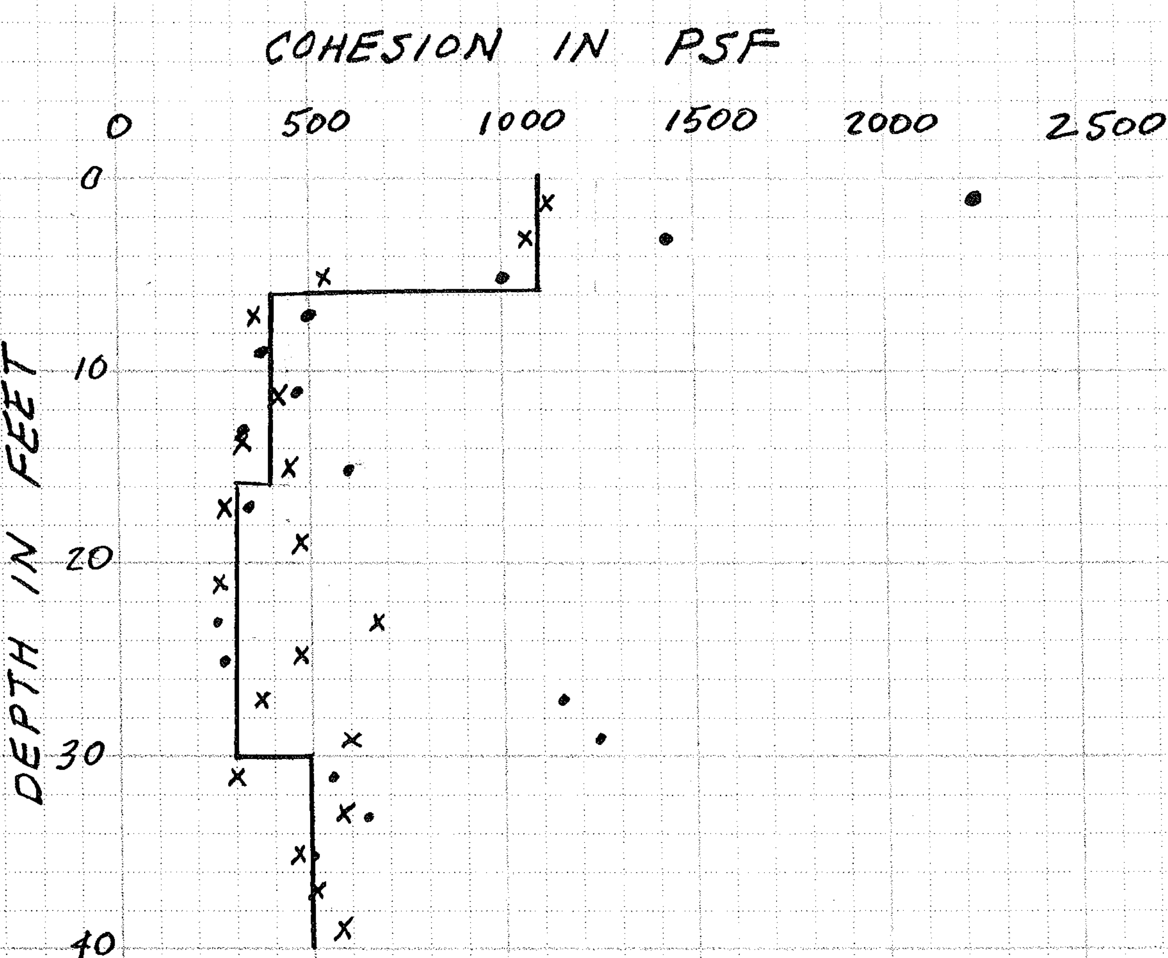
Sheet ______ Of ______

Made by LJC Date 1JUN07

File No. 07-110

Checked by ______ Date ______

LOUIS J. CAPOZZOLI & ASSOCIATES, INC. Geotechnical Engineers



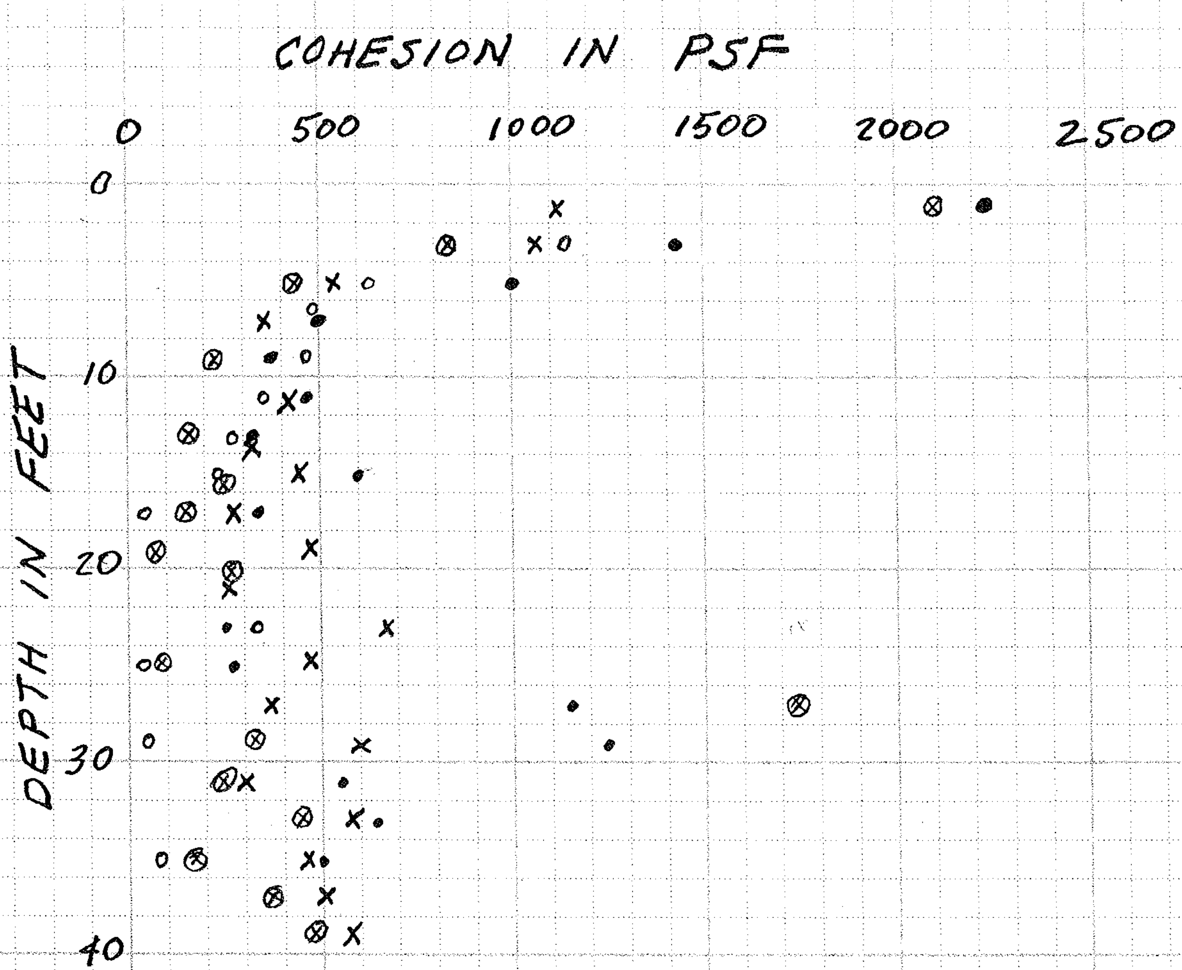
LEGEND:

• = BORING 5 BY LJC&A
X = BORING 6 BY LJC&A

Subject DESIGN STRENGTH
JACK & BORE SITES, B-5&6
File No. 07-110

Sheet ______ Of ______
Made by LJC Date 1JUN07
Checked by ______ Date ______

LOUIS J. CAPOZZOLI & ASSOCIATES, INC. Geotechnical Engineers



LEGEND:

• = BORING 5 Shear Strengths
o = BORING 5 Mini-Vane Strength

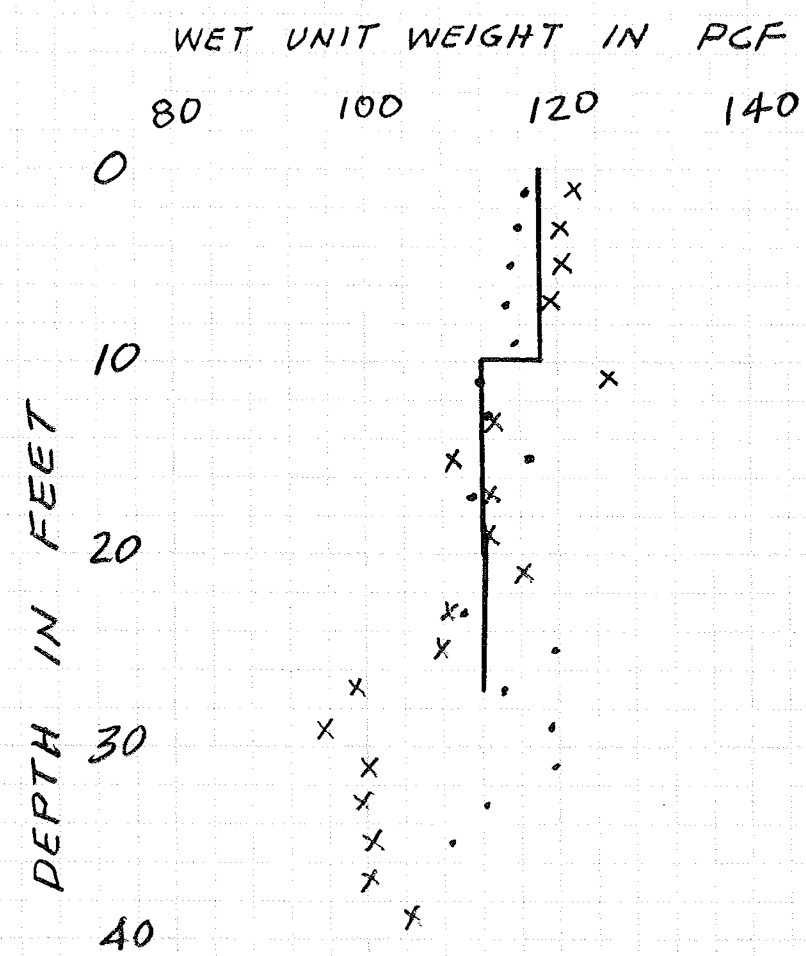
x = BORING 6 Shear Strengths
⊗ = BORING 6 Mini-Vane Strength

NOTE: BORINGS 5 & 6 AND TESTS BY LJC&A.

Subject SOIL DATA
BORINGS 5 & 6
File No. 07-110

Sheet ____ Of ____
Made by [illegible] Date 1JUN07
Checked by ____ Date ____

LOUIS J. CAPOZZOLI & ASSOCIATES, INC. Geotechnical Engineers



APPENDIX B

Charles L. Eustis, PE
Louis J. Capozzoli & Associates
A subsidiary of GeoEngineers
10555 Airline Highway
Baton Rouge, LA 70816

July 9, 2007

Dear Charlie,

Please find enclosed the lab testing results for the Bayou Dupont Reclamation project, including:

- Two self-weight consolidation tests (Test #7 and Test #8)
- Two combined hydrometer and wet sieving tests (Tests # 1 & 2)
- Organic matter content measurements (3 parallel measurements)
- Surface settlement analysis based on PSDDF (a summary table, settlement – time and settlement – log time plots, and example calculations for the 3.5 ft thick dredged fill

I thank you very much for offering the opportunity to providing service to you, and it has been my pleasure to collaborate with you.

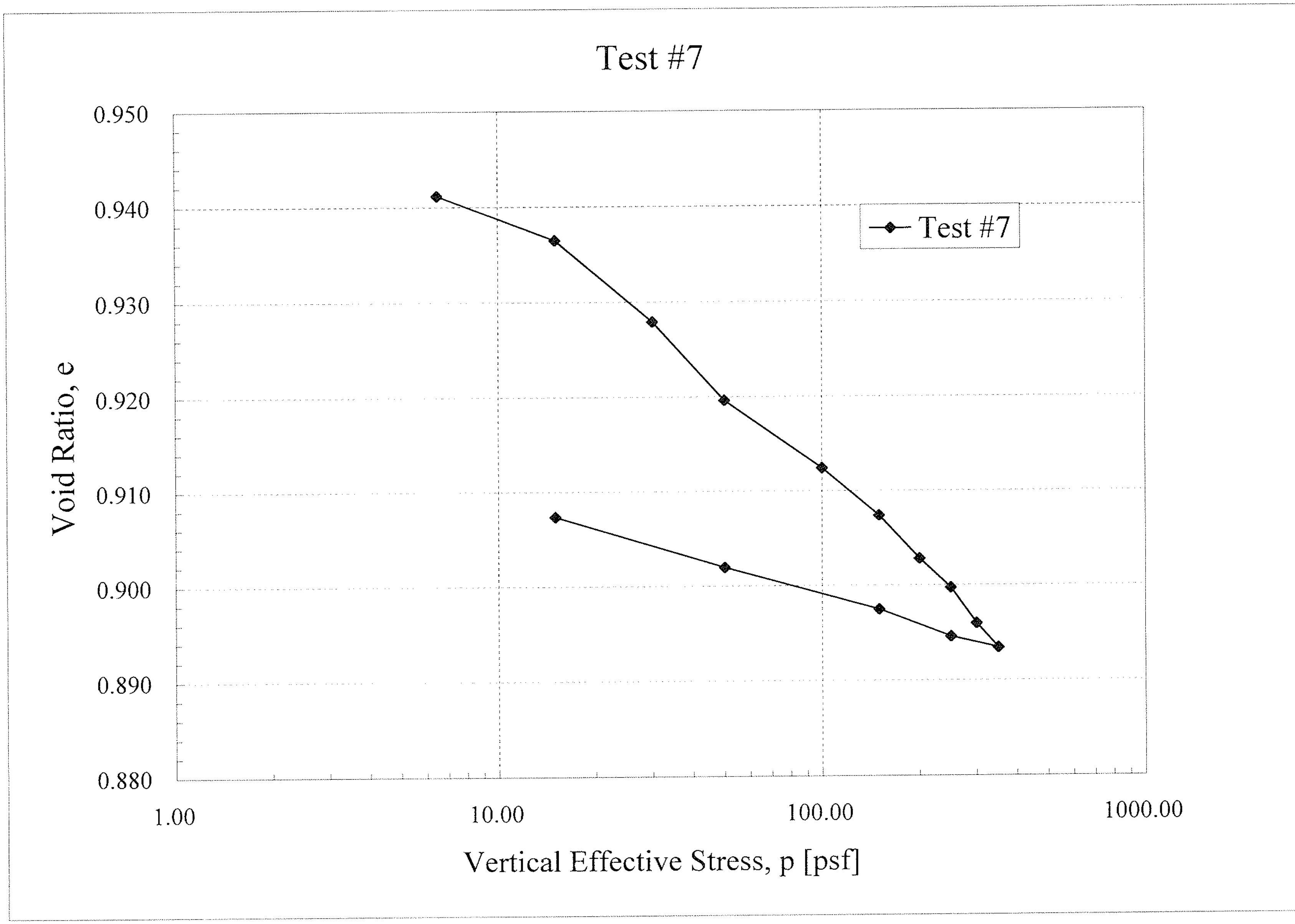
Yours sincerely,

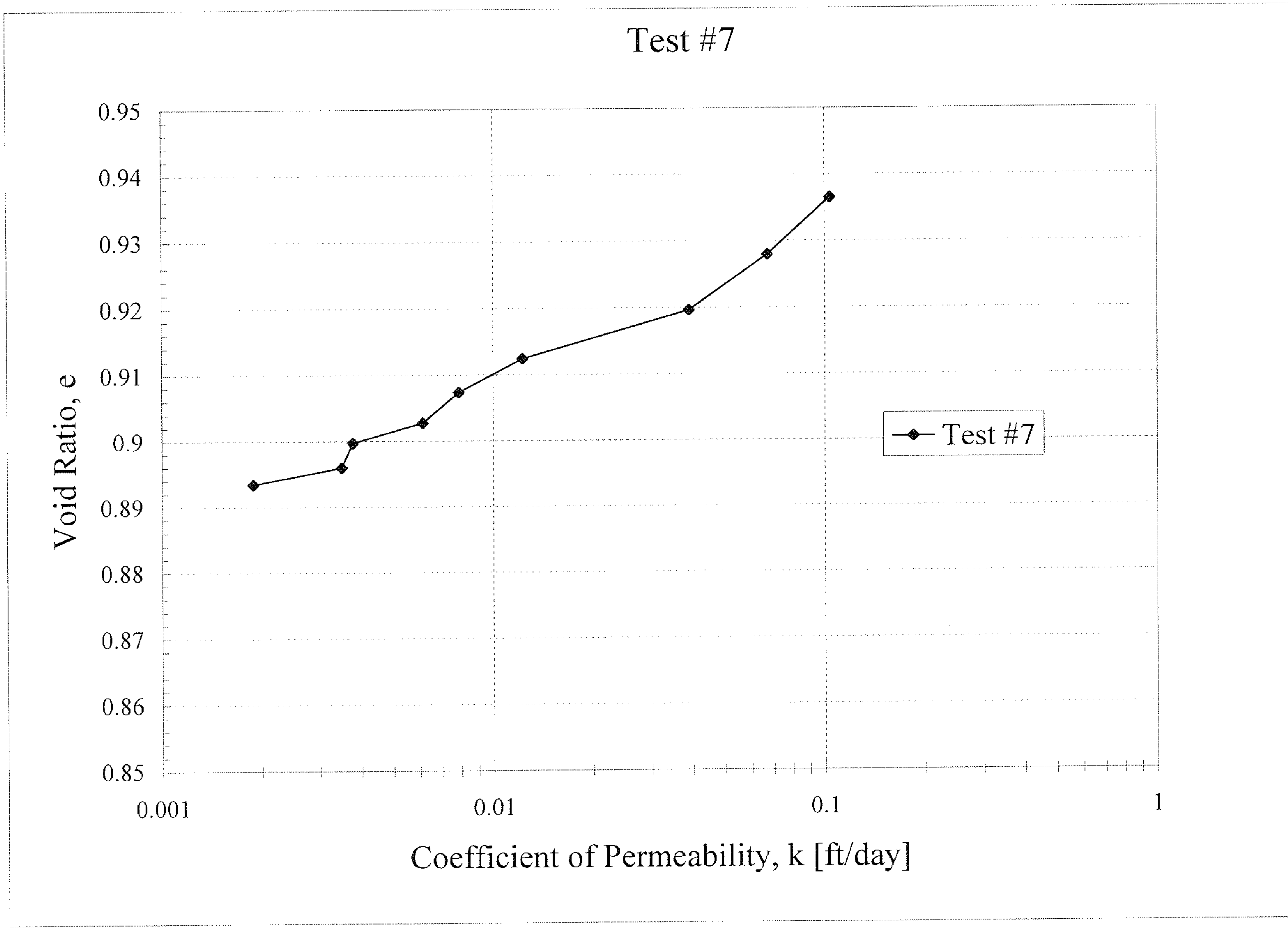
Guoping (Gregg) Zhang

Results of Self-weight Consolidation (Contract No.2503-05-44) Test #7

Loading Steps	Load Increment(psf)	Subtotal Deformation Δ H(inch)	Accumulative Deformation $\Sigma \Delta$ H(inch)	ε_v %	H (inch)	$e = \frac{H - H_s}{H_s}$	m_v (ft^2/lb)	k (ft/day)
1	6.48	0.00063	0.00063	0.06	0.99937	0.9411		
2	15.00	0.00240	0.00303	0.30	0.99697	0.9365	2.8199E-04	1.0371E-01
3	30.00	0.00440	0.00743	0.74	0.99257	0.9279	2.9434E-04	6.7153E-02
4	50.00	0.00427	0.01170	1.17	0.98830	0.9196	2.1515E-04	3.8958E-02
5	100.00	0.00371	0.01541	1.54	0.98459	0.9124	7.5055E-05	1.2256E-02
6	150.00	0.00261	0.01802	1.80	0.98198	0.9074	5.2931E-05	7.8715E-03
7	200.00	0.00237	0.02039	2.04	0.97961	0.9028	4.8256E-05	6.1219E-03
8	250.00	0.00157	0.02195	2.20	0.97805	0.8997	3.1988E-05	3.7708E-03
9	300.00	0.00193	0.02389	2.39	0.97611	0.8960	3.9534E-05	3.4837E-03
10	350.00	0.00131	0.02520	2.52	0.97480	0.8934	2.6841E-05	1.8851E-03
11	250.00	-0.00060	0.02460	2.46	0.97540	0.8946	6.1475E-06	
12	150.00	-0.00148	0.02311	2.31	0.97689	0.8975	1.5219E-05	
13	50.00	-0.00231	0.02081	2.08	0.97919	0.9019	2.3609E-05	
14	15.00	-0.00277	0.01803	1.80	0.98197	0.9073	8.0901E-05	

Cc=0.034 Cr=0.010 $C_\alpha = 0.0008$

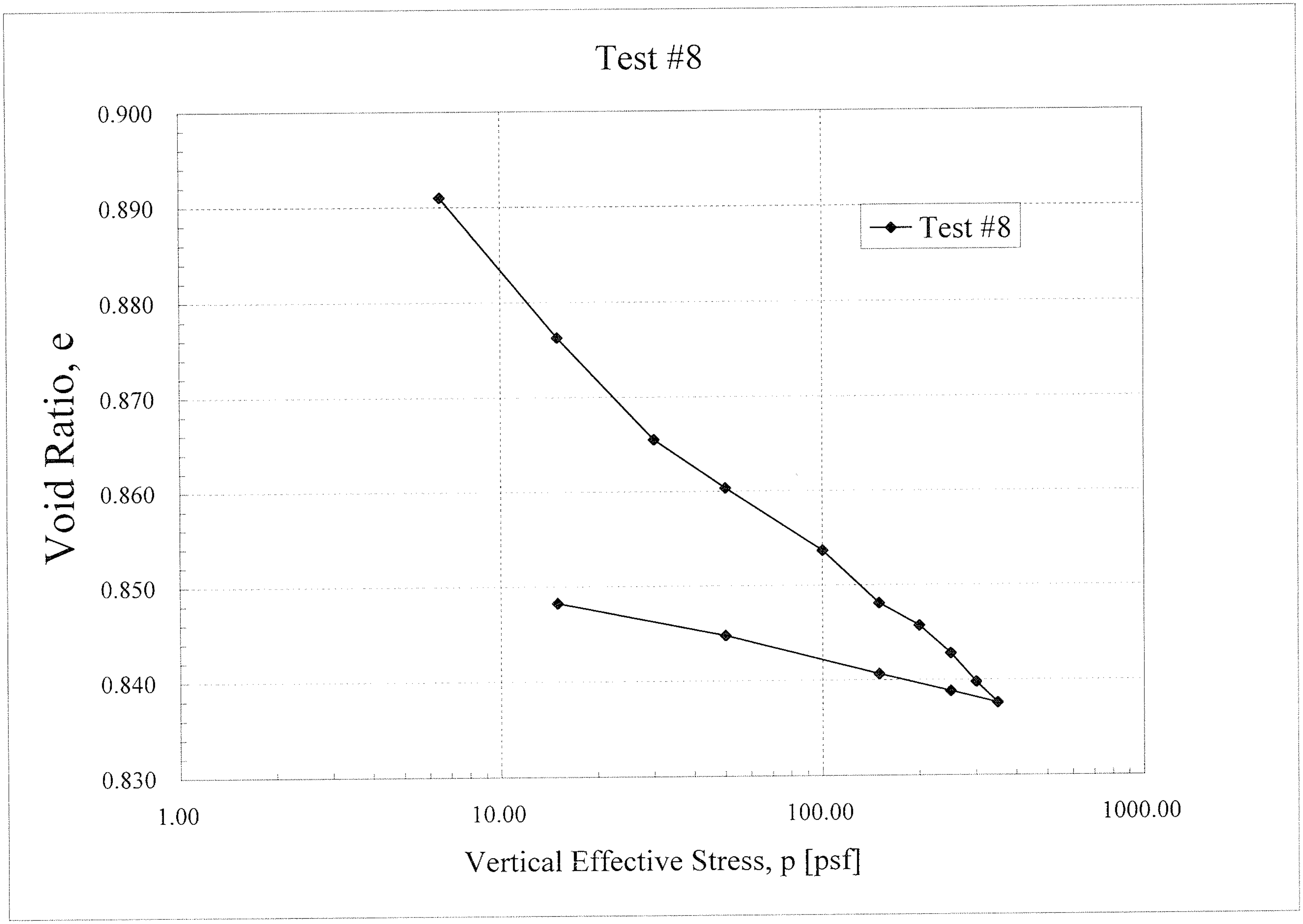

Test #7
0.950
0.940
0.930
0.920
0.910
0.900
0.890
0.880
Void Ratio, e
Test #7
1.00
10.00
100.00
1000.00
Vertical Effective Stress, p [psf]

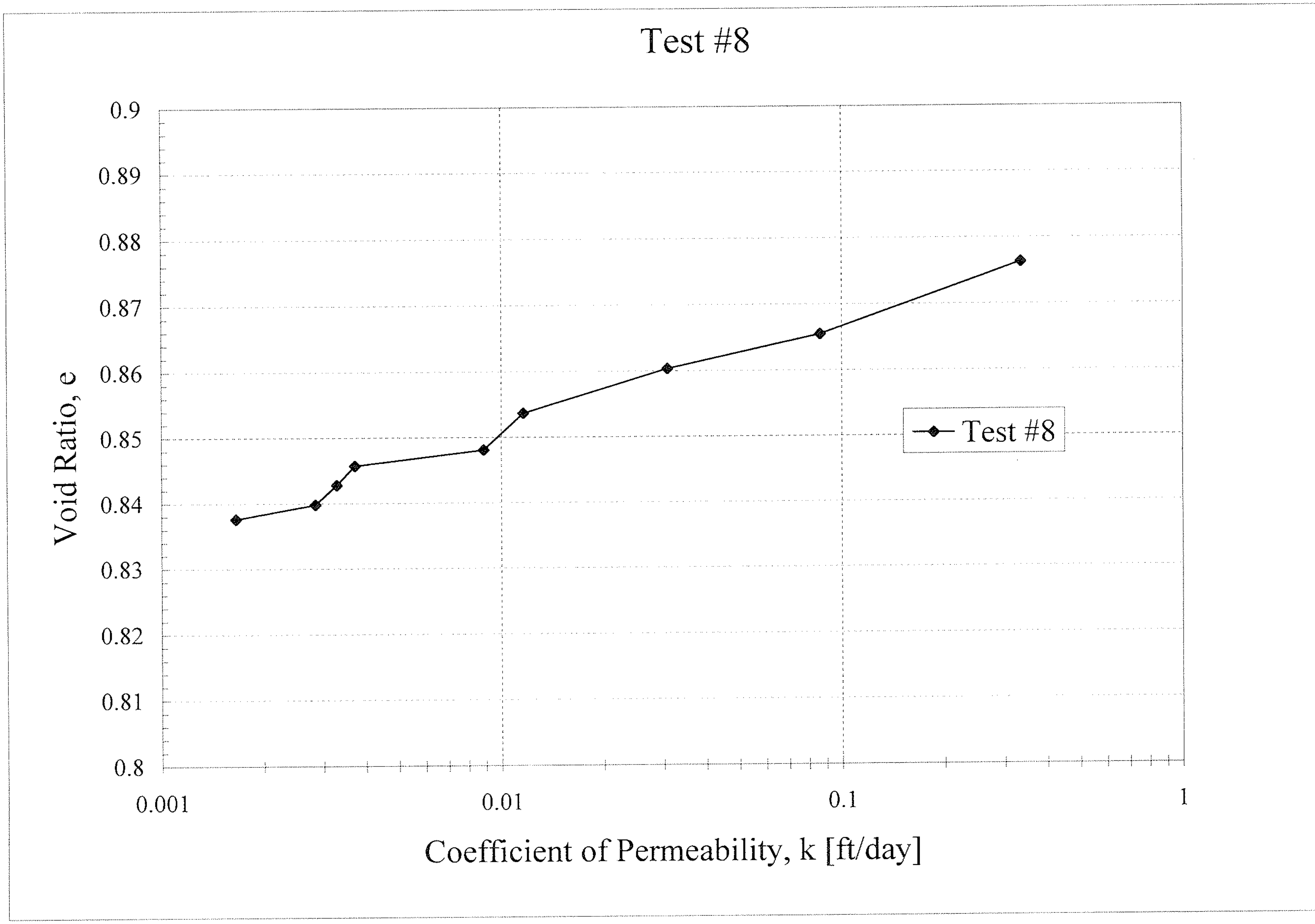

Test #7
0.95
0.94
0.93
0.92
0.91
0.9
0.89
0.88
0.87
0.86
0.85
Void Ratio, e
0.001
0.01
0.1
1
Coefficient of Permeability, k [ft/day]
Test #7

Results of Self-weight Consolidation (Contract No. 2503-05-44) Test #8

Loading Steps	Load Increment(psf)	Subtotal Deformation Δ H(inch)	Accumulative Deformation $\Sigma \Delta$ H(inch)	ε_v %	H (inch)	$e = \frac{H - H_s}{H_s}$	m_v (ft^2/lb)	k (ft/sec)
1	6.48	0.00146	0.00146	0.15	0.99854	0.8910		
2	15.00	0.00776	0.00922	0.92	0.99078	0.8763	9.1240E-04	3.3525E-01
3	30.00	0.00570	0.01492	1.49	0.98508	0.8655	3.8347E-04	8.6570E-02
4	50.00	0.00272	0.01764	1.76	0.98236	0.8603	1.3783E-04	3.0799E-02
5	100.00	0.00353	0.02117	2.12	0.97883	0.8537	7.1874E-05	1.1599E-02
6	150.00	0.00295	0.02412	2.41	0.97588	0.8481	6.0343E-05	8.8691E-03
7	200.00	0.00124	0.02536	2.54	0.97464	0.8457	2.5384E-05	3.7128E-03
8	250.00	0.00155	0.02691	2.69	0.97309	0.8428	3.1882E-05	3.2827E-03
9	300.00	0.00158	0.02850	2.85	0.97150	0.8398	3.2525E-05	2.8376E-03
10	350.00	0.00116	0.02965	2.97	0.97035	0.8376	2.3788E-05	1.6555E-03
11	250.00	-0.00065	0.02901	2.90	0.97099	0.8388	6.6525E-06	
12	150.00	-0.00096	0.02805	2.80	0.97195	0.8406	9.8895E-06	
13	50.00	-0.00218	0.02586	2.59	0.97414	0.8448	2.2447E-05	
14	15.00	-0.00183	0.02403	2.40	0.97597	0.8482	5.3818E-05	

Cc=0.031 Cr=0.008 $C_\alpha = 0.0005$

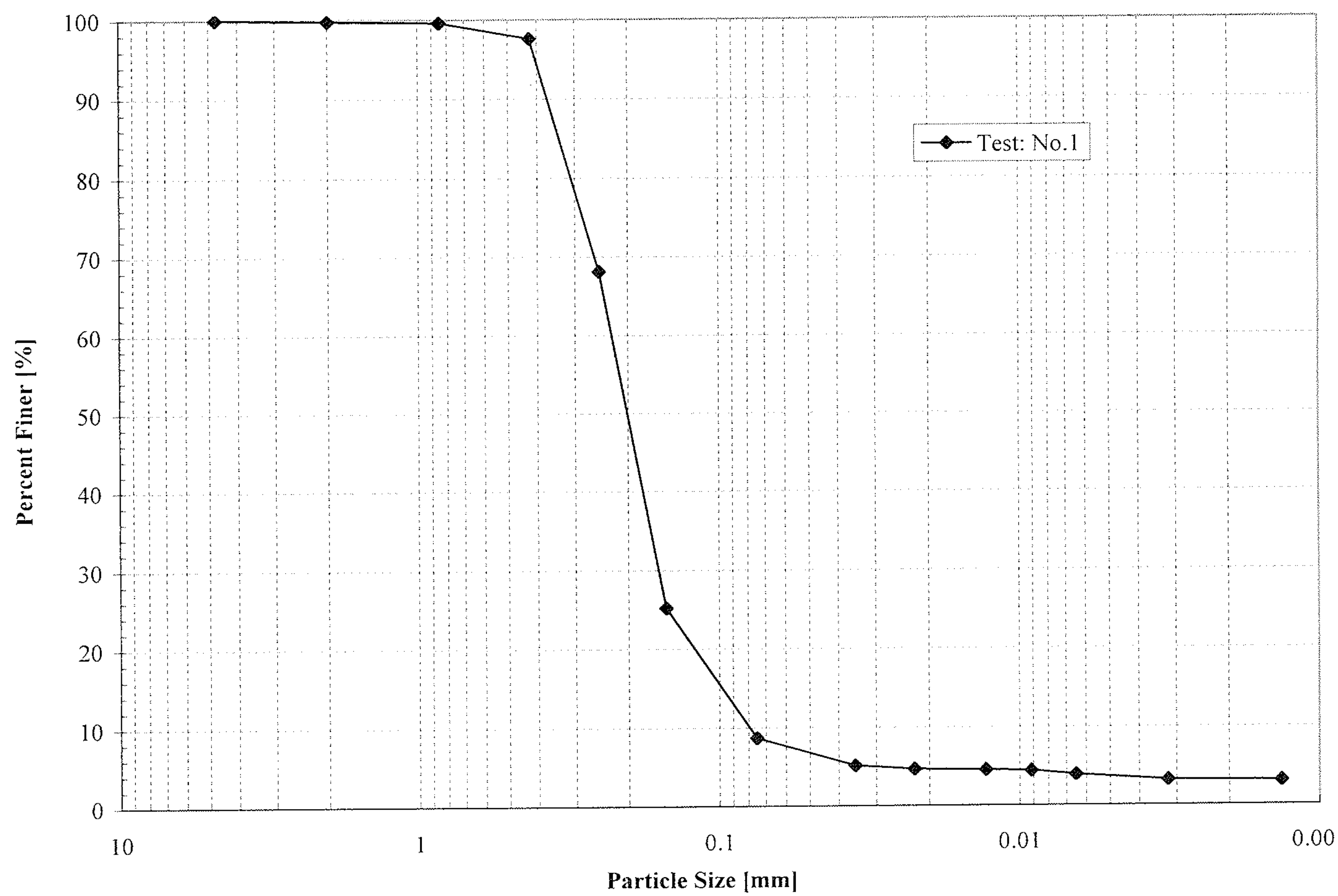

Test #8
Void Ratio, e
0.900
0.890
0.880
0.870
0.860
0.850
0.840
0.830
1.00
10.00
100.00
1000.00
Vertical Effective Stress, p [psf]
Test #8


Test #8
Void Ratio, e
0.9
0.89
0.88
0.87
0.86
0.85
0.84
0.83
0.82
0.81
0.8
0.001
0.01
0.1
1
Coefficient of Permeability, k [ft/day]
Test #8

Bayou Dupont Reclamation (Contract No. 2503-05-44)

Standard: ASTM D422-63

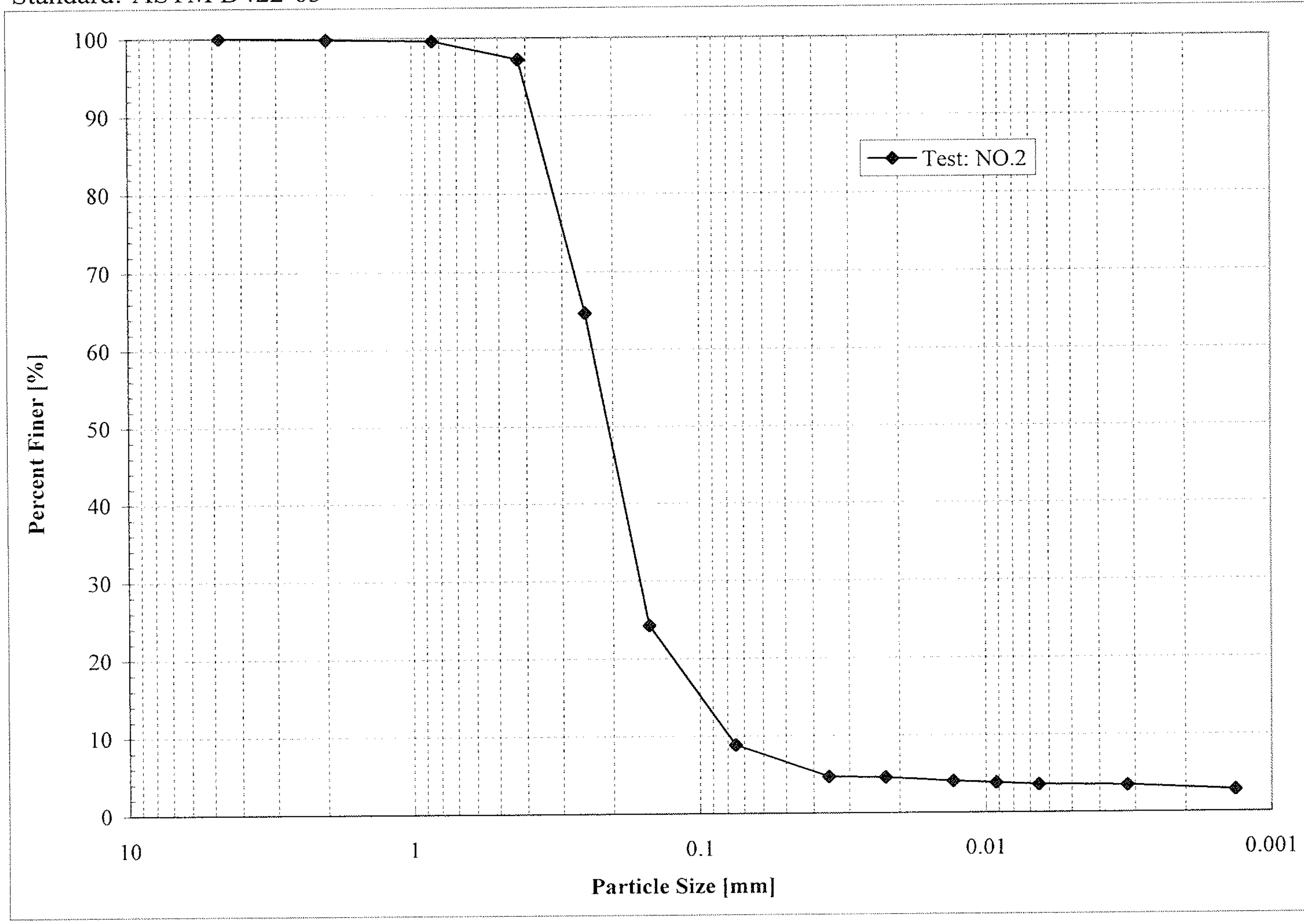
Date: 06/19/2007



Bayou Dupont Reclamation (Contract No. 2503-05-44)

Standard: ASTM D422-63

Date: 06/19/2007



Bayou Dupont Relamation (Contract No. 2503-05-44)

Results of Organic Matter Test

Standard: ASTM D2974-00

Date : 06/22/2007

Sample	Organic Matter, %	Average Organic Matter, %
1	0.81	0.84
2	1.00	
3	0.72	

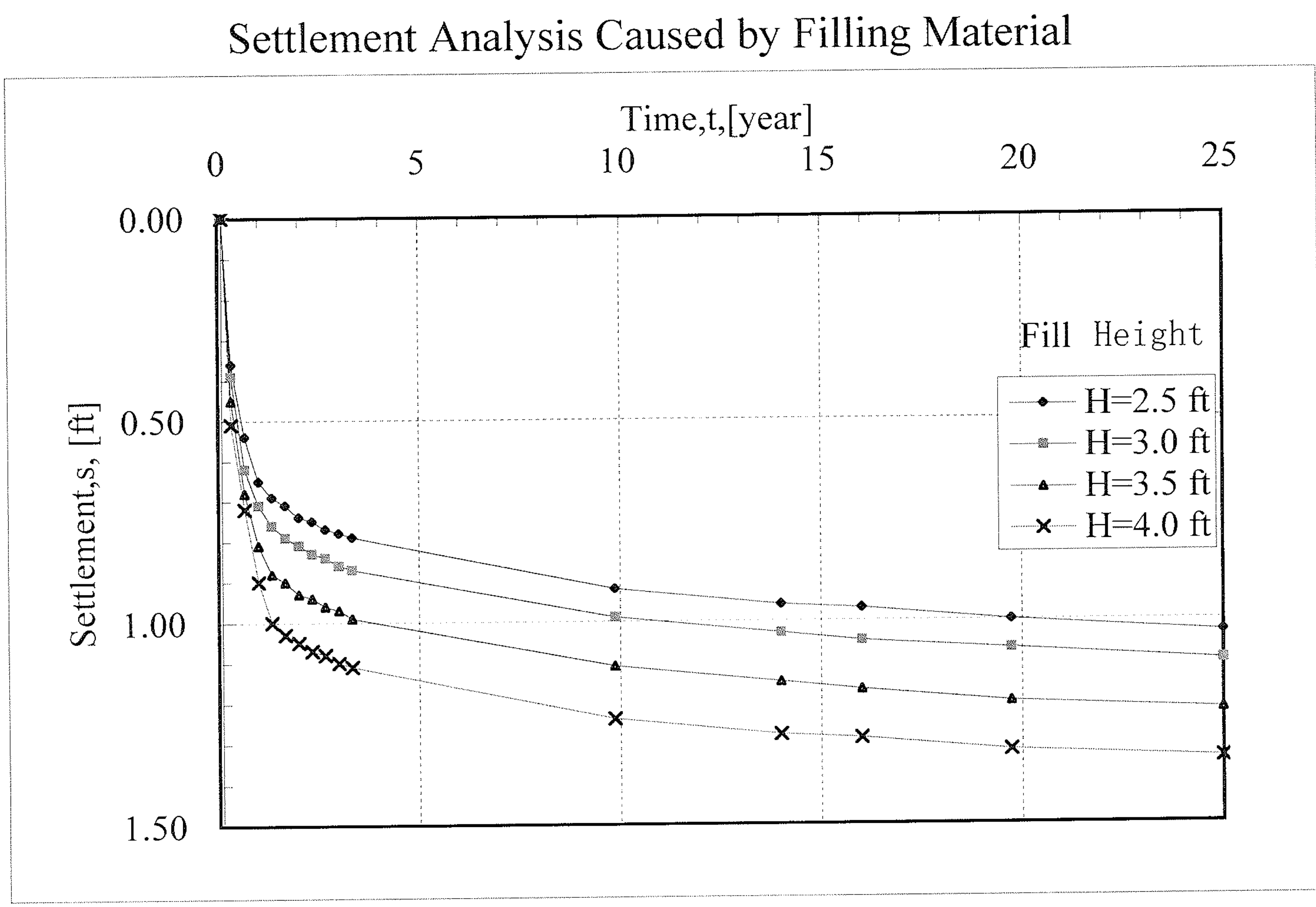
Bayou Dupont Reclamation (Contract No. 2503-05-44)

Settlement Analysis Caused by Filling Material

Day	Year	Top Surface Elevation (ft)				Settlement (ft)			
		H=2.5	H=3.0	H=3.5	H=4.0	H=2.5	H=3.0	H=3.5	H=4.0
0	0.00	0.50	1.00	1.50	2.00	0.00	0.00	0.00	0.00
120	0.33	0.14	0.61	1.05	1.49	0.36	0.39	0.45	0.51
240	0.66	-0.04	0.38	0.82	1.28	0.54	0.62	0.68	0.72
365	1.00	-0.15	0.29	0.69	1.10	0.65	0.71	0.81	0.90
485	1.33	-0.19	0.24	0.62	1.00	0.69	0.76	0.88	1.00
605	1.66	-0.21	0.21	0.60	0.97	0.71	0.79	0.90	1.03
730	2.00	-0.24	0.19	0.57	0.95	0.74	0.81	0.93	1.05
850	2.33	-0.25	0.17	0.56	0.93	0.75	0.83	0.94	1.07
970	2.66	-0.27	0.16	0.54	0.92	0.77	0.84	0.96	1.08
1095	3.00	-0.28	0.14	0.53	0.90	0.78	0.86	0.97	1.10
1215	3.33	-0.29	0.13	0.51	0.89	0.79	0.87	0.99	1.11
3600	9.86	-0.42	0.01	0.39	0.76	0.92	0.99	1.11	1.24
5110	14.00	-0.46	-0.03	0.35	0.72	0.96	1.03	1.15	1.28
5840	16.00	-0.47	-0.05	0.33	0.71	0.97	1.05	1.17	1.29
7200	19.73	-0.50	-0.07	0.30	0.68	1.00	1.07	1.20	1.32
9125	25.00	-0.53	-0.10	0.28	0.66	1.03	1.10	1.22	1.34

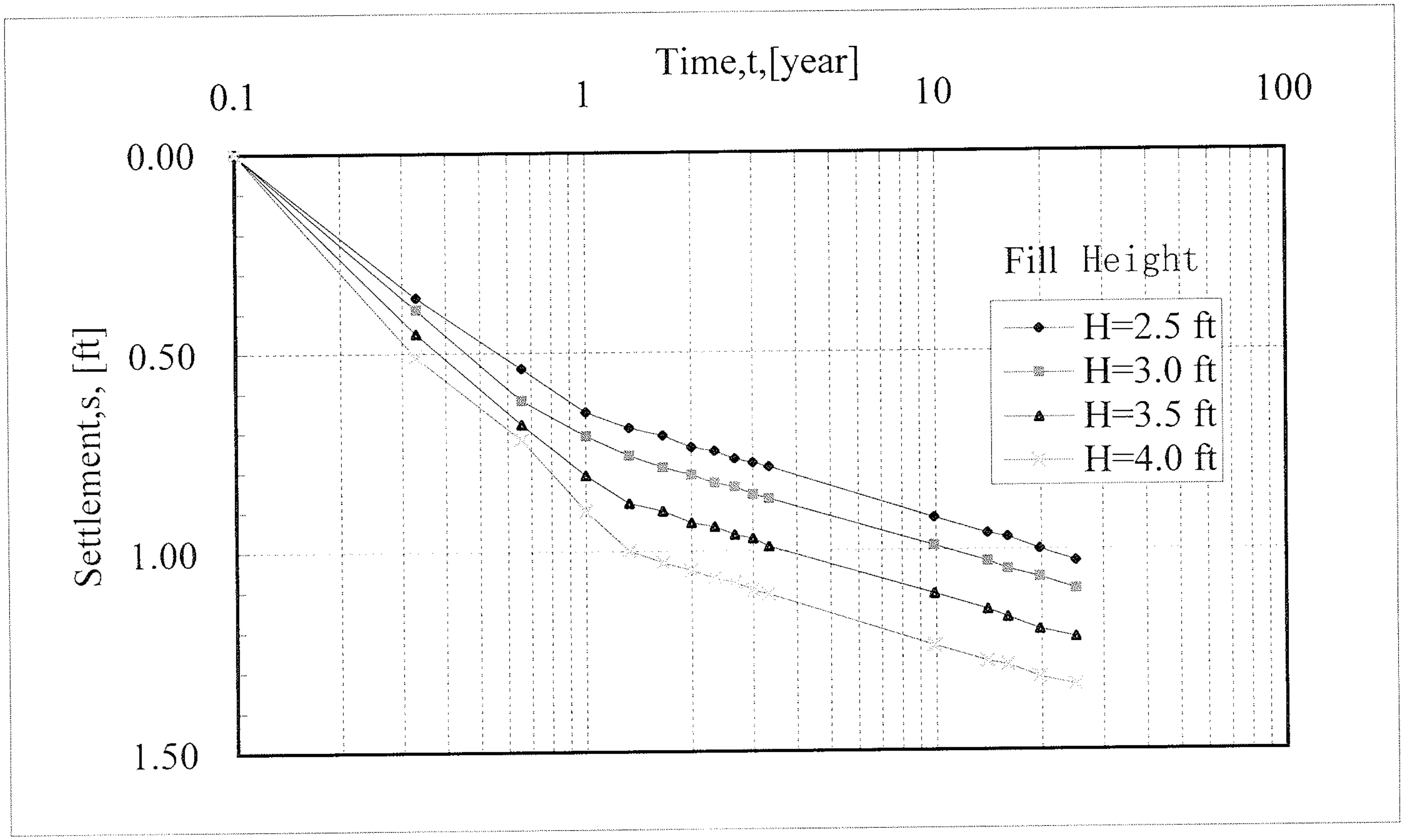
Note: The above results are obtained by using PSDDF.H is the thickness of filling material.

Bayou Dupont Reclamation (Contract No. 2503-05-44)



Bayou Dupont Reclamation (Contract No. 2503-05-44)

Settlement Analysis Caused by Filling Material



$$V_s = \frac{M_d}{G\ \rho_w}$$

$$H_s = \frac{V_s}{A}$$

$$e_0 = \frac{H_0 - H_s}{H_s}$$

$$e_f = \frac{H_f - H_s}{H_s}$$

$$c_v = \frac{0.197 H_{50}^2}{t_{50}}$$

$$c_v = \frac{0.848 H_{90}^2}{t_{90}}$$

$$k = c_v \gamma_w m_v$$

$$m_v = \frac{\varepsilon_{v1} - \varepsilon_{v2}}{\sigma_2' - \sigma_1'} = \frac{e_1 - e_2}{1 + e_1} \frac{1}{\sigma_2' - \sigma_1'}$$

****

Consolidation and desiccation of soft layers---dredged fill

****

Problem Bayou Dupont Reclaimation 2503-05-44

******************Soil data for compressible foundation*****************

Material Type	Layer Thickness	Numbers of Sub-layers	Ca/Cc	Cr/Cc
2	4.00	9	.242	.166
3	2.00	6	.009	.323
4	2.00	6	.025	.700
5	32.00	10	.015	.125
6	20.00	10	.018	.206

Material type : 2 Specific Gravity of Solids: 2.72

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	1.550	.000E+00	.290E-02	.114E-02	.211E-02	-.100E+04	-.114E+01
2	1.540	.100E+02	.283E-02	.112E-02	.262E-02	-.102E+04	-.113E+01
3	1.430	.122E+03	.200E-02	.823E-03	.432E-03	-.384E+04	-.316E+01
4	.500	.400E+04	.100E-02	.667E-03	.168E-03	-.417E+04	-.278E+01

Material type : 3 Specific Gravity of Solids: 2.50

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	5.900	.000E+00	.284E-01	.412E-02	.768E-02	-.167E+03	-.686E+00
2	5.840	.100E+02	.250E-01	.365E-02	.220E-03	-.214E+03	-.782E+00
3	4.310	.340E+03	.200E-01	.377E-02	-.794E-03	-.824E+03	-.311E+01
4	1.000	.400E+04	.150E-01	.750E-02	-.113E-02	-.111E+04	-.829E+01

Material type : 4 Specific Gravity of Solids: 2.70

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	1.420	.000E+00	.284E-01	.117E-01	.659E-01	-.500E+03	-.587E+01
2	1.400	.100E+02	.250E-01	.104E-01	.176E-01	-.222E+04	-.231E+02
3	1.150	.600E+03	.150E-01	.698E-02	.198E-01	-.998E+04	-.696E+02
4	1.000	.400E+04	.500E-02	.250E-02	.298E-01	-.227E+05	-.567E+02

Material type : 5 Specific Gravity of Solids: 2.70

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	2.000	.000E+00	.283E-02	.945E-03	.104E-02	-.909E+02	-.859E-01
2	1.890	.100E+02	.240E-02	.830E-03	.544E-03	-.714E+04	-.593E+01
3	1.650	.250E+04	.200E-02	.755E-03	.110E-02	-.102E+05	-.772E+01

4	1.500	.400E+04	.100E-02	.400E-03	.236E-02	-.100E+05	-.400E+01

Material type : 6 Specific Gravity of Solids: 2.70

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	2.000	.000E+00	.283E-02	.945E-03	.104E-02	-.909E+02	-.859E-01
2	1.890	.100E+02	.240E-02	.830E-03	.439E-03	-.315E+04	-.262E+01
3	1.600	.126E+04	.200E-02	.769E-03	.671E-03	-.676E+04	-.520E+01
4	1.300	.400E+04	.100E-02	.435E-03	.111E-02	-.913E+04	-.397E+01

**********************Soil data for dredged fill**********************

Material Type	Specific Gravity	Ca/Cc	Cr/Cc	Saturation Limit	Disication Limit
1	2.700	.020	.276	3.880	1.490

Material type : 1

I	Void Ratio	Effective Stress	Perm-eability	k/1+e PK	Beta	Dsde	Alpha
1	.980	.000E+00	.200E+00	.101E+00	.122E+01	-.324E+03	-.327E+02
2	.960	.648E+01	.150E+00	.765E-01	.124E+01	-.375E+03	-.287E+02
3	.940	.150E+02	.100E+00	.515E-01	.139E+01	-.784E+03	-.404E+02
4	.930	.300E+02	.672E-01	.348E-01	.156E+01	-.175E+04	-.609E+02
5	.920	.500E+02	.390E-01	.203E-01	.142E+01	-.350E+04	-.711E+02
6	.910	.100E+03	.123E-01	.644E-02	.855E+00	-.750E+04	-.483E+02
7	.900	.200E+03	.610E-02	.321E-02	.274E+00	-.125E+05	-.401E+02
8	.890	.350E+03	.180E-02	.952E-03	.226E+00	-.150E+05	-.143E+02

Summary of lifts and print detail

Time days	Material Type	Fill Height	# Sub-layers	Void ratio	Start Day	Dessic. Month	Print detai
0.	1	3.5	10	.98	10000.	6	1
120.					10000.	6	1
240.					10000.	6	1
365.					10000.	6	1
485.					10000.	6	1
605.					10000.	6	1
730.					10000.	6	1
850.					10000.	6	1
970.					10000.	6	1

1095.	10000.	6	1
1215.	10000.	6	1
3600.	10000.	6	1
5110.	10000.	6	1
5840.	10000.	6	1
7200.	10000.	6	1
9125.	10000.	6	1

==

Summary of monthly rainfall and evaporation potential

Month	Rainfall	Evaporation
1	.180	.300
2	.240	.300
3	.290	.460
4	.210	.570
5	.230	.660
6	.570	.620
7	.580	.570
8	.570	.570
9	.600	.490
10	.400	.410
11	.160	.330
12	.160	.250

***************************Calculation data***************************

tau	Lower layer Void ratio	Lower layer Permeability	drainage path Length
.113E-01	.570	.10000E-01	z = 16.56

Summary of desiccation parameters

Parameter	Value
Surface Drainage Efficiency	.20
maximum evaporation efficiency	.50
saturation at desiccation limit	.75
maximum crust thickness	-1.73
time to desic. after initial fill	10000.00
month of initial desiccation	6
elevation of fixed water table	.00
elevation of top of incompres. found.	-62.00

**************Initial Conditions in Compressible Foundation*************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
60.00	60.00	22.30	2.00	2.00	1.85	6
57.92	57.92	21.59	1.87	1.87	1.83	6
55.88	55.88	20.88	1.86	1.86	1.81	6
53.85	53.85	20.17	1.84	1.84	1.80	6
51.83	51.83	19.45	1.82	1.82	1.78	6
49.83	49.83	18.74	1.80	1.80	1.76	6
47.84	47.84	18.03	1.79	1.79	1.74	6
45.86	45.86	17.32	1.77	1.77	1.73	6
43.89	43.89	16.60	1.75	1.75	1.71	6
41.93	41.93	15.89	1.73	1.73	1.69	6
39.99	39.99	15.18	1.72	1.72	1.67	6
39.99	39.99	15.18	1.82	1.82	1.80	5
36.73	36.73	14.02	1.81	1.81	1.79	5
33.48	33.48	12.86	1.79	1.79	1.78	5
30.25	30.25	11.70	1.78	1.78	1.76	5
27.03	27.03	10.54	1.77	1.77	1.75	5
23.82	23.82	9.38	1.76	1.76	1.74	5
20.63	20.63	8.22	1.75	1.75	1.73	5
17.45	17.45	7.06	1.74	1.74	1.72	5
14.29	14.29	5.90	1.72	1.72	1.71	5
11.14	11.14	4.74	1.71	1.71	1.69	5
8.00	8.00	3.58	1.70	1.70	1.68	5
8.00	8.00	3.58	1.09	1.09	1.08	4
7.67	7.67	3.42	1.09	1.09	1.08	4
7.33	7.33	3.26	1.09	1.09	1.08	4
7.00	7.00	3.10	1.09	1.09	1.08	4
6.67	6.67	2.94	1.09	1.09	1.08	4
6.33	6.33	2.78	1.09	1.09	1.08	4
6.00	6.00	2.62	1.08	1.08	1.08	4
6.00	6.00	2.62	2.73	2.73	2.56	3
5.66	5.66	2.53	2.72	2.72	2.55	3
5.33	5.33	2.44	2.71	2.71	2.54	3
5.00	5.00	2.35	2.71	2.71	2.54	3
4.66	4.66	2.26	2.70	2.70	2.53	3
4.33	4.33	2.17	2.69	2.69	2.52	3
4.00	4.00	2.08	2.68	2.68	2.51	3
4.00	4.00	2.08	.95	.95	.90	2
3.55	3.55	1.85	.94	.94	.90	2
3.10	3.10	1.62	.93	.93	.89	2
2.65	2.65	1.39	.93	.93	.88	2

2.21	2.21	1.16	.92	.92	.88	2
1.76	1.76	.93	.92	.92	.87	2
1.32	1.32	.69	.91	.91	.87	2
.88	.88	.46	.90	.90	.86	2
.44	.44	.23	.90	.90	.85	2
.00	.00	.00	.89	.89	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
60.00	405.92	.00	405.92	218.40	187.52	6
57.92	611.31	75.59	535.72	348.21	187.52	6
55.88	814.33	151.17	663.15	475.64	187.52	6
53.85	1016.57	226.76	789.81	602.29	187.52	6
51.83	1218.03	302.35	915.68	728.16	187.52	6
49.83	1418.71	377.94	1040.77	853.26	187.52	6
47.84	1618.61	453.52	1165.08	977.57	187.52	6
45.86	1817.73	529.11	1288.62	1101.10	187.52	6
43.89	2016.07	604.70	1411.37	1223.86	187.52	6
41.93	2213.63	680.28	1533.34	1345.83	187.52	6
39.99	2410.41	755.87	1654.54	1467.02	187.52	6
39.99	2410.41	755.87	1654.54	1467.02	187.52	5
36.73	2736.93	878.89	1858.04	1670.52	187.51	5
33.48	3062.58	1001.91	2060.68	1873.16	187.52	5
30.25	3387.38	1124.92	2262.46	2074.94	187.51	5
27.03	3711.32	1247.94	2463.38	2275.87	187.51	5
23.82	4034.41	1370.96	2663.45	2475.93	187.51	5
20.63	4356.63	1493.97	2862.66	2675.14	187.51	5
17.45	4678.00	1616.99	3061.01	2873.49	187.51	5
14.29	4998.51	1740.01	3258.50	3070.98	187.51	5
11.14	5318.16	1863.03	3455.13	3267.62	187.51	5
8.00	5636.95	1986.04	3650.91	3463.39	187.51	5
8.00	5636.95	1986.04	3650.91	3463.39	187.51	4
7.67	5674.72	2002.99	3671.73	3484.21	187.51	4
7.33	5712.47	2019.94	3692.54	3505.02	187.51	4
7.00	5750.22	2036.88	3713.34	3525.83	187.51	4
6.67	5787.97	2053.83	3734.14	3546.62	187.51	4
6.33	5825.70	2070.77	3754.93	3567.41	187.52	4
6.00	5863.43	2087.72	3775.71	3588.19	187.51	4
6.00	5863.43	2087.72	3775.71	3588.19	187.51	3
5.66	5892.75	2096.14	3796.61	3609.10	187.51	3
5.33	5922.03	2104.55	3817.47	3629.96	187.51	3
5.00	5951.26	2112.97	3838.29	3650.78	187.51	3
4.66	5980.46	2121.39	3859.07	3671.55	187.51	3

4.33	6009.61	2129.80	3879.80	3692.29	187.51	3
4.00	6038.71	2138.22	3900.49	3712.98	187.51	3
4.00	6038.71	2138.22	3900.49	3712.98	187.52	2
3.55	6091.63	2163.06	3928.57	3741.05	187.51	2
3.10	6144.46	2187.91	3956.55	3769.04	187.51	2
2.65	6197.21	2212.75	3984.46	3796.94	187.51	2
2.21	6249.87	2237.60	4012.27	3824.76	187.51	2
1.76	6302.44	2262.44	4040.00	3852.49	187.51	2
1.32	6354.93	2287.29	4067.64	3880.13	187.51	2
.88	6407.33	2312.13	4095.20	3907.69	187.51	2
.44	6459.65	2336.98	4122.67	3935.16	187.51	2
.00	6511.88	2361.82	4150.06	3962.54	187.51	2

Time = 0. Degree of Consolidation = 0.%

Total Settlement = .000

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 0. = .000

Settlement caused by Secondary Compression at time 0. = .000

******************Initial Conditions in Dredged Fill******************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
3.50	3.50	1.77	.98	.98	.98	1
3.15	3.15	1.59	.98	.98	.94	1
2.80	2.80	1.41	.98	.98	.93	1
2.45	2.45	1.24	.98	.98	.92	1
2.10	2.10	1.06	.98	.98	.91	1
1.75	1.75	.88	.98	.98	.91	1
1.40	1.40	.71	.98	.98	.91	1
1.05	1.05	.53	.98	.98	.91	1
.70	.70	.35	.98	.98	.90	1
.35	.35	.18	.98	.98	.90	1
.00	.00	.00	.98	.98	.90	1

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
3.50	.00	.00	.00	.00	.00	1
3.15	40.59	.00	40.59	21.84	18.75	1
2.80	81.18	.00	81.18	43.68	37.50	1
2.45	121.77	.00	121.77	65.52	56.25	1
2.10	162.37	.00	162.37	87.36	75.01	1
1.75	202.96	.00	202.96	109.20	93.76	1
1.40	243.55	.00	243.55	131.04	112.51	1

1.05	284.14	.00	284.14	152.88	131.26	1
.70	324.73	.00	324.73	174.72	150.01	1
.35	365.32	.00	365.32	196.56	168.76	1
.00	405.92	.00	405.92	218.40	187.52	1

Time = 0. Degree of Consolidation = 0.%

Total Settlement = .000

Settlement at End of Primary Consolidation = .112

Settlement caused by Primary Consolidation at time 0. = .000

Settlement caused by Secondary Compression at time 0. = .000

**************Current Conditions in Compressible Foundation************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
60.00	59.66	22.30	2.00	1.85	1.85	6
57.92	57.64	21.59	1.87	1.84	1.83	6
55.88	55.62	20.88	1.86	1.82	1.81	6
53.85	53.61	20.17	1.84	1.81	1.80	6
51.83	51.62	19.45	1.82	1.80	1.78	6
49.83	49.63	18.74	1.80	1.78	1.76	6
47.84	47.65	18.03	1.79	1.77	1.74	6
45.86	45.68	17.32	1.77	1.75	1.73	6
43.89	43.73	16.60	1.75	1.74	1.71	6
41.93	41.78	15.89	1.73	1.72	1.69	6
39.99	39.85	15.18	1.72	1.70	1.67	6
39.99	39.85	15.18	1.82	1.81	1.80	5
36.73	36.59	14.02	1.81	1.80	1.79	5
33.48	33.35	12.86	1.79	1.79	1.78	5
30.25	30.12	11.70	1.78	1.78	1.76	5
27.03	26.91	10.54	1.77	1.77	1.75	5
23.82	23.71	9.38	1.76	1.75	1.74	5
20.63	20.52	8.22	1.75	1.74	1.73	5
17.45	17.35	7.06	1.74	1.73	1.72	5
14.29	14.19	5.90	1.72	1.72	1.71	5
11.14	11.04	4.74	1.71	1.71	1.69	5
8.00	7.91	3.58	1.70	1.69	1.68	5
8.00	7.91	3.58	1.09	1.09	1.08	4
7.67	7.58	3.42	1.09	1.08	1.08	4
7.33	7.25	3.26	1.09	1.08	1.08	4
7.00	6.91	3.10	1.09	1.08	1.08	4
6.67	6.58	2.94	1.09	1.08	1.08	4
6.33	6.25	2.78	1.09	1.08	1.08	4
6.00	5.91	2.62	1.08	1.08	1.08	4

6.00	5.91	2.62	2.73	2.66	2.56	3
5.66	5.59	2.53	2.72	2.66	2.55	3
5.33	5.26	2.44	2.71	2.65	2.54	3
5.00	4.93	2.35	2.71	2.64	2.54	3
4.66	4.60	2.26	2.70	2.63	2.53	3
4.33	4.28	2.17	2.69	2.63	2.52	3
4.00	3.95	2.08	2.68	2.62	2.51	3
4.00	3.95	2.08	.95	.93	.90	2
3.55	3.50	1.85	.94	.92	.90	2
3.10	3.06	1.62	.93	.91	.89	2
2.65	2.62	1.39	.93	.91	.88	2
2.21	2.18	1.16	.92	.90	.88	2
1.76	1.74	.93	.92	.89	.87	2
1.32	1.30	.69	.91	.89	.87	2
.88	.87	.46	.90	.88	.86	2
.44	.43	.23	.90	.87	.85	2
.00	.00	.00	.89	.86	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
59.66	399.01	185.77	213.25	211.50	1.75	6
57.64	601.00	240.61	360.39	337.90	22.49	6
55.62	802.43	295.26	507.16	463.74	43.42	6
53.61	1003.28	351.31	651.97	589.01	62.97	6
51.62	1203.55	409.68	793.87	713.68	80.18	6
49.63	1403.20	470.87	932.33	837.75	94.58	6
47.65	1602.20	534.84	1067.36	961.16	106.20	6
45.68	1800.53	601.34	1199.19	1083.91	115.28	6
43.73	1998.16	669.74	1328.42	1205.95	122.47	6
41.78	2195.08	739.79	1455.30	1327.29	128.01	6
39.85	2391.28	810.94	1580.33	1447.89	132.44	6
39.85	2391.28	810.94	1580.33	1447.89	132.44	5
36.59	2717.43	927.04	1790.39	1651.03	139.36	5
33.35	3042.77	1046.24	1996.53	1853.35	143.18	5
30.12	3367.27	1167.49	2199.77	2054.83	144.94	5
26.91	3690.91	1290.13	2400.78	2255.46	145.32	5
23.71	4013.70	1414.03	2599.67	2455.23	144.45	5
20.52	4335.62	1539.05	2796.57	2654.13	142.44	5
17.35	4656.66	1665.90	2990.76	2852.15	138.60	5
14.19	4976.81	1794.87	3181.93	3049.28	132.65	5
11.04	5296.05	1925.78	3370.27	3245.51	124.76	5
7.91	5614.37	2059.69	3554.68	3440.81	113.87	5
7.91	5614.37	2059.69	3554.68	3440.81	113.87	4
7.58	5652.10	2076.72	3575.38	3461.60	113.78	4

7.25	5689.83	2093.68	3596.15	3482.38	113.77	4
6.91	5727.54	2110.56	3616.99	3503.15	113.84	4
6.58	5765.25	2127.36	3637.89	3523.91	113.98	4
6.25	5802.95	2144.09	3658.87	3544.67	114.20	4
5.91	5840.65	2160.74	3679.91	3565.42	114.50	4
5.91	5840.65	2160.74	3679.91	3565.42	114.50	3
5.59	5869.60	2169.04	3700.56	3585.95	114.61	3
5.26	5898.51	2177.40	3721.11	3606.44	114.67	3
4.93	5927.38	2185.80	3741.57	3626.89	114.68	3
4.60	5956.20	2194.26	3761.94	3647.30	114.65	3
4.28	5984.98	2202.76	3782.22	3667.66	114.56	3
3.95	6013.72	2211.31	3802.41	3687.98	114.43	3
3.95	6013.72	2211.31	3802.41	3687.98	114.43	2
3.50	6066.37	2240.24	3826.14	3715.80	110.34	2
3.06	6118.93	2269.79	3849.14	3743.51	105.63	2
2.62	6171.38	2299.94	3871.44	3771.12	100.33	2
2.18	6223.73	2330.63	3893.10	3798.62	94.48	2
1.74	6275.97	2361.84	3914.13	3826.02	88.12	2
1.30	6328.10	2393.52	3934.58	3853.30	81.28	2
.87	6380.13	2425.64	3954.49	3880.48	74.01	2
.43	6432.04	2458.15	3973.89	3907.54	66.34	2
.00	6483.83	2491.01	3992.82	3934.49	58.33	2

Time = 120. Degree of Consolidation = 46.%

Total Settlement = .339

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 120. = .339

Settlement caused by Secondary Compression at time 120. = .000

******************Current Conditions in Dredged Fill******************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	185.77	213.25	211.50	1.75	1

Time = 120. Degree of Consolidation = 99.%
Total Settlement = .111
Settlement at End of Primary Consolidation = .112
Settlement caused by Primary Consolidation at time 120. = .111
Settlement caused by Secondary Compression at time 120. = .000
Surface Elevation = 1.05

**************Current Conditions in Compressible Foundation*************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
60.00	59.44	22.30	2.00	1.85	1.85	6
57.92	57.41	21.59	1.87	1.83	1.83	6
55.88	55.40	20.88	1.86	1.82	1.81	6
53.85	53.40	20.17	1.84	1.80	1.80	6
51.83	51.41	19.45	1.82	1.78	1.78	6
49.83	49.43	18.74	1.80	1.77	1.76	6
47.84	47.46	18.03	1.79	1.75	1.74	6
45.86	45.51	17.32	1.77	1.74	1.73	6
43.89	43.56	16.60	1.75	1.72	1.71	6
41.93	41.63	15.89	1.73	1.71	1.69	6
39.99	39.70	15.18	1.72	1.69	1.67	6
39.99	39.70	15.18	1.82	1.81	1.80	5
36.73	36.46	14.02	1.81	1.80	1.79	5
33.48	33.22	12.86	1.79	1.78	1.78	5
30.25	30.00	11.70	1.78	1.77	1.76	5
27.03	26.79	10.54	1.77	1.76	1.75	5
23.82	23.59	9.38	1.76	1.75	1.74	5
20.63	20.41	8.22	1.75	1.74	1.73	5

17.45	17.25	7.06	1.74	1.72	1.72	5
14.29	14.10	5.90	1.72	1.71	1.71	5
11.14	10.96	4.74	1.71	1.70	1.69	5
8.00	7.84	3.58	1.70	1.68	1.68	5
8.00	7.84	3.58	1.09	1.08	1.08	4
7.67	7.51	3.42	1.09	1.08	1.08	4
7.33	7.17	3.26	1.09	1.08	1.08	4
7.00	6.84	3.10	1.09	1.08	1.08	4
6.67	6.51	2.94	1.09	1.08	1.08	4
6.33	6.18	2.78	1.09	1.08	1.08	4
6.00	5.84	2.62	1.08	1.08	1.08	4
6.00	5.84	2.62	2.73	2.59	2.56	3
5.66	5.52	2.53	2.72	2.58	2.55	3
5.33	5.20	2.44	2.71	2.57	2.54	3
5.00	4.88	2.35	2.71	2.57	2.54	3
4.66	4.56	2.26	2.70	2.56	2.53	3
4.33	4.24	2.17	2.69	2.55	2.52	3
4.00	3.92	2.08	2.68	2.54	2.51	3
4.00	3.92	2.08	.95	.91	.90	2
3.55	3.48	1.85	.94	.90	.90	2
3.10	3.04	1.62	.93	.90	.89	2
2.65	2.60	1.39	.93	.89	.88	2
2.21	2.16	1.16	.92	.89	.88	2
1.76	1.73	.93	.92	.88	.87	2
1.32	1.29	.69	.91	.87	.87	2
.88	.86	.46	.90	.87	.86	2
.44	.43	.23	.90	.86	.85	2
.00	.00	.00	.89	.85	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
59.44	399.01	187.26	211.75	211.50	.25	6
57.41	600.89	259.71	341.18	337.79	3.39	6
55.40	802.04	329.15	472.89	463.35	9.54	6
53.40	1002.48	396.92	605.57	588.21	17.36	6
51.41	1202.23	463.91	738.33	712.37	25.96	6
49.43	1401.29	530.81	870.48	835.84	34.64	6
47.46	1599.66	598.15	1001.51	958.62	42.89	6
45.51	1797.33	665.91	1131.42	1080.70	50.72	6
43.56	1994.29	734.32	1259.97	1202.08	57.89	6
41.63	2190.55	803.33	1387.22	1322.75	64.47	6
39.70	2386.09	873.16	1512.93	1442.71	70.23	6
39.70	2386.09	873.16	1512.93	1442.71	70.23	5
36.46	2711.82	987.18	1724.65	1645.42	79.22	5

33.22	3036.75	1104.33	1932.42	1847.33	85.09	5
30.00	3360.85	1224.40	2136.45	2048.41	88.04	5
26.79	3684.10	1347.18	2336.92	2248.64	88.28	5
23.59	4006.48	1472.47	2534.01	2448.01	86.00	5
20.41	4327.98	1600.07	2727.91	2646.49	81.42	5
17.25	4648.59	1730.87	2917.72	2844.08	73.64	5
14.10	4968.26	1864.82	3103.44	3040.74	62.70	5
10.96	5287.00	2001.69	3285.30	3236.46	48.85	5
7.84	5604.77	2141.27	3463.49	3431.21	32.29	5
7.84	5604.77	2141.27	3463.49	3431.21	32.29	4
7.51	5642.46	2158.44	3484.03	3451.96	32.07	4
7.17	5680.15	2175.52	3504.63	3472.70	31.93	4
6.84	5717.83	2192.53	3525.30	3493.44	31.87	4
6.51	5755.51	2209.46	3546.05	3514.16	31.89	4
6.18	5793.17	2226.30	3566.87	3534.88	31.98	4
5.84	5830.83	2243.08	3587.75	3555.60	32.16	4
5.84	5830.83	2243.08	3587.75	3555.60	32.16	3
5.52	5859.36	2251.42	3607.95	3575.71	32.23	3
5.20	5887.85	2259.81	3628.04	3595.79	32.26	3
4.88	5916.30	2268.26	3648.04	3615.82	32.23	3
4.56	5944.71	2276.76	3667.95	3635.81	32.14	3
4.24	5973.07	2285.32	3687.75	3655.75	32.00	3
3.92	6001.39	2293.92	3707.46	3675.65	31.81	3
3.92	6001.39	2293.92	3707.46	3675.65	31.81	2
3.48	6053.77	2317.69	3736.07	3703.19	32.88	2
3.04	6106.06	2342.16	3763.90	3730.64	33.26	2
2.60	6158.28	2367.28	3791.00	3758.01	32.99	2
2.16	6210.40	2392.99	3817.41	3785.29	32.12	2
1.73	6262.43	2419.25	3843.18	3812.48	30.71	2
1.29	6314.37	2446.01	3868.36	3839.57	28.79	2
.86	6366.22	2473.25	3892.97	3866.57	26.40	2
.43	6417.98	2500.91	3917.06	3893.48	23.58	2
.00	6469.63	2528.95	3940.68	3920.29	20.39	2

Time = 240. Degree of Consolidation = 77.%

Total Settlement = .566

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 240. = .566

Settlement caused by Secondary Compression at time 240. = .000

******************Current Conditions in Dredged Fill******************

***** Coordinates *****   ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

***** Stresses *****   ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.26	211.75	211.50	.25	1

Time = 240. Degree of Consolidation = 99.%
Total Settlement = .111
Settlement at End of Primary Consolidation = .112
Settlement caused by Primary Consolidation at time 240. = .111
Settlement caused by Secondary Compression at time 240. = .000
Surface Elevation = .82

**************Current Conditions in Compressible Foundation************

***** Coordinates *****   ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
60.00	59.31	22.30	2.00	1.85	1.85	6
57.92	57.28	21.59	1.87	1.83	1.83	6
55.88	55.27	20.88	1.86	1.81	1.81	6
53.85	53.27	20.17	1.84	1.80	1.80	6

51.83	51.29	19.45	1.82	1.78	1.78	6
49.83	49.31	18.74	1.80	1.76	1.76	6
47.84	47.35	18.03	1.79	1.74	1.74	6
45.86	45.40	17.32	1.77	1.73	1.73	6
43.89	43.47	16.60	1.75	1.71	1.71	6
41.93	41.54	15.89	1.73	1.69	1.69	6
39.99	39.63	15.18	1.72	1.68	1.67	6
39.99	39.63	15.18	1.82	1.80	1.80	5
36.73	36.38	14.02	1.81	1.79	1.79	5
33.48	33.16	12.86	1.79	1.78	1.78	5
30.25	29.94	11.70	1.78	1.77	1.76	5
27.03	26.74	10.54	1.77	1.76	1.75	5
23.82	23.55	9.38	1.76	1.74	1.74	5
20.63	20.37	8.22	1.75	1.73	1.73	5
17.45	17.21	7.06	1.74	1.72	1.72	5
14.29	14.07	5.90	1.72	1.71	1.71	5
11.14	10.94	4.74	1.71	1.69	1.69	5
8.00	7.82	3.58	1.70	1.68	1.68	5
8.00	7.82	3.58	1.09	1.08	1.08	4
7.67	7.49	3.42	1.09	1.08	1.08	4
7.33	7.15	3.26	1.09	1.08	1.08	4
7.00	6.82	3.10	1.09	1.08	1.08	4
6.67	6.49	2.94	1.09	1.08	1.08	4
6.33	6.16	2.78	1.09	1.08	1.08	4
6.00	5.83	2.62	1.08	1.08	1.08	4
6.00	5.83	2.62	2.73	2.57	2.56	3
5.66	5.50	2.53	2.72	2.56	2.55	3
5.33	5.18	2.44	2.71	2.56	2.54	3
5.00	4.86	2.35	2.71	2.55	2.54	3
4.66	4.55	2.26	2.70	2.54	2.53	3
4.33	4.23	2.17	2.69	2.53	2.52	3
4.00	3.91	2.08	2.68	2.53	2.51	3
4.00	3.91	2.08	.95	.90	.90	2
3.55	3.47	1.85	.94	.90	.90	2
3.10	3.03	1.62	.93	.89	.89	2
2.65	2.59	1.39	.93	.89	.88	2
2.21	2.16	1.16	.92	.88	.88	2
1.76	1.72	.93	.92	.87	.87	2
1.32	1.29	.69	.91	.87	.87	2
.88	.86	.46	.90	.86	.86	2
.44	.43	.23	.90	.86	.85	2
.00	.00	.00	.89	.85	.85	2

***** Stresses *****   ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
59.31	399.01	187.51	211.50	211.50	.01	6
57.28	600.88	263.10	337.77	337.77	.00	6
55.27	801.96	338.69	463.27	463.27	.00	6
53.27	1002.27	414.28	587.99	587.99	.00	6
51.29	1201.79	489.86	711.93	711.93	.00	6
49.31	1400.54	565.45	835.09	835.09	.00	6
47.35	1598.50	641.04	957.46	957.46	.00	6
45.40	1795.69	715.35	1080.34	1079.07	1.27	6
43.47	1992.13	787.67	1204.45	1199.92	4.54	6
41.54	2187.82	858.97	1328.86	1320.02	8.83	6
39.63	2382.79	929.82	1452.97	1439.40	13.57	6
39.63	2382.79	929.82	1452.97	1439.40	13.57	5
36.38	2708.11	1045.25	1662.86	1641.71	21.15	5
33.16	3032.63	1162.66	1869.97	1843.21	26.76	5
29.94	3356.32	1282.94	2073.39	2043.88	29.50	5
26.74	3679.16	1405.87	2273.30	2243.71	29.59	5
23.55	4001.14	1530.35	2470.79	2442.67	28.12	5
20.37	4322.25	1655.87	2666.38	2640.76	25.62	5
17.21	4642.47	1782.30	2860.18	2837.97	22.21	5
14.07	4961.82	1908.92	3052.90	3034.29	18.61	5
10.94	5280.28	2035.04	3245.24	3229.74	15.50	5
7.82	5597.87	2159.85	3438.01	3424.31	13.70	5
7.82	5597.87	2159.86	3438.01	3424.31	13.70	4
7.49	5635.55	2177.13	3458.43	3445.05	13.38	4
7.15	5673.23	2194.31	3478.93	3465.78	13.14	4
6.82	5710.91	2211.40	3499.51	3486.51	13.00	4
6.49	5748.57	2228.40	3520.17	3507.23	12.94	4
6.16	5786.23	2245.32	3540.91	3527.94	12.97	4
5.83	5823.88	2262.14	3561.74	3548.64	13.09	4
5.83	5823.88	2262.14	3561.74	3548.64	13.09	3
5.50	5852.31	2270.50	3581.81	3568.66	13.15	3
5.18	5880.71	2278.87	3601.84	3588.64	13.20	3
4.86	5909.06	2287.25	3621.81	3608.58	13.24	3
4.55	5937.37	2295.63	3641.74	3628.47	13.27	3
4.23	5965.63	2304.02	3661.61	3648.32	13.30	3
3.91	5993.86	2312.42	3681.44	3668.12	13.31	3
3.91	5993.86	2312.42	3681.44	3668.12	13.31	2
3.47	6046.17	2337.85	3708.32	3695.59	12.73	2
3.03	6098.40	2363.40	3735.00	3722.97	12.02	2
2.59	6150.53	2389.07	3761.46	3750.27	11.19	2
2.16	6202.58	2414.85	3787.74	3777.47	10.27	2
1.72	6254.54	2440.72	3813.82	3804.58	9.24	2

1.29	6306.41	2466.66	3839.74	3831.61	8.14	2
.86	6358.19	2492.68	3865.50	3858.54	6.96	2
.43	6409.87	2518.78	3891.10	3885.38	5.71	2
.00	6461.47	2544.93	3916.55	3912.13	4.41	2

Time = 365. Degree of Consolidation = 94.%

Total Settlement = .697

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 365. = .697

Settlement caused by Secondary Compression at time 365. = .000

******************Current Conditions in Dredged Fill******************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.51	211.50	211.50	.01	1

Time = 365. Degree of Consolidation = 99.%

Total Settlement = .111

Settlement at End of Primary Consolidation = .112

Settlement caused by Primary Consolidation at time 365. = .111

Settlement caused by Secondary Compression at time 365. = .000

Surface Elevation = .69

**************Current Conditions in Compressible Foundation************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
60.00	59.24	22.30	2.00	1.85	1.85	6
57.92	57.21	21.59	1.87	1.83	1.83	6
55.88	55.20	20.88	1.86	1.81	1.81	6
53.85	53.20	20.17	1.84	1.80	1.80	6
51.83	51.22	19.45	1.82	1.78	1.78	6
49.83	49.24	18.74	1.80	1.76	1.76	6
47.84	47.28	18.03	1.79	1.74	1.74	6
45.86	45.33	17.32	1.77	1.73	1.73	6
43.89	43.40	16.60	1.75	1.71	1.71	6
41.93	41.47	15.89	1.73	1.69	1.69	6
39.99	39.56	15.18	1.72	1.67	1.67	6
39.99	39.56	15.18	1.82	1.80	1.80	5
36.73	36.32	14.02	1.81	1.79	1.79	5
33.48	33.10	12.86	1.79	1.78	1.78	5
30.25	29.88	11.70	1.78	1.76	1.76	5
27.03	26.68	10.54	1.77	1.75	1.75	5
23.82	23.50	9.38	1.76	1.74	1.74	5
20.63	20.33	8.22	1.75	1.73	1.73	5
17.45	17.17	7.06	1.74	1.72	1.72	5
14.29	14.03	5.90	1.72	1.71	1.71	5
11.14	10.90	4.74	1.71	1.69	1.69	5
8.00	7.78	3.58	1.70	1.68	1.68	5
8.00	7.78	3.58	1.09	1.08	1.08	4
7.67	7.45	3.42	1.09	1.08	1.08	4
7.33	7.11	3.26	1.09	1.08	1.08	4
7.00	6.78	3.10	1.09	1.08	1.08	4
6.67	6.45	2.94	1.09	1.08	1.08	4
6.33	6.12	2.78	1.09	1.08	1.08	4
6.00	5.79	2.62	1.08	1.08	1.08	4
6.00	5.79	2.62	2.73	2.56	2.56	3
5.66	5.47	2.53	2.72	2.55	2.55	3
5.33	5.15	2.44	2.71	2.54	2.54	3

5.00	4.83	2.35	2.71	2.53	2.54	3
4.66	4.51	2.26	2.70	2.53	2.53	3
4.33	4.20	2.17	2.69	2.52	2.52	3
4.00	3.88	2.08	2.68	2.51	2.51	3
4.00	3.88	2.08	.95	.89	.90	2
3.55	3.44	1.85	.94	.88	.90	2
3.10	3.01	1.62	.93	.88	.89	2
2.65	2.57	1.39	.93	.87	.88	2
2.21	2.14	1.16	.92	.87	.88	2
1.76	1.71	.93	.92	.86	.87	2
1.32	1.28	.69	.91	.85	.87	2
.88	.85	.46	.90	.85	.86	2
.44	.43	.23	.90	.84	.85	2
.00	.00	.00	.89	.84	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
59.24	399.01	187.51	211.50	211.50	.00	6
57.21	600.88	263.10	337.77	337.77	.00	6
55.20	801.96	338.69	463.27	463.27	.00	6
53.20	1002.27	414.28	587.99	587.99	.00	6
51.22	1201.79	489.86	711.93	711.93	.00	6
49.24	1400.54	565.45	835.09	835.09	.00	6
47.28	1598.50	641.04	957.46	957.46	.00	6
45.33	1795.69	716.63	1079.06	1079.06	.00	6
43.40	1992.09	792.21	1199.88	1199.88	.00	6
41.47	2187.72	867.80	1319.92	1319.92	.00	6
39.56	2382.57	943.39	1439.18	1439.18	.00	6
39.56	2382.57	943.39	1439.18	1439.18	.00	5
36.32	2707.78	1066.40	1641.37	1641.37	.00	5
33.10	3032.13	1189.42	1842.70	1842.70	.00	5
29.88	3355.62	1312.44	2043.18	2043.18	.00	5
26.68	3678.25	1435.46	2242.79	2242.80	.00	5
23.50	4000.03	1558.47	2441.55	2441.55	.00	5
20.33	4320.94	1681.49	2639.45	2639.45	.00	5
17.17	4641.00	1804.51	2836.50	2836.50	.00	5
14.03	4960.21	1927.53	3032.68	3032.68	.00	5
10.90	5278.55	2050.54	3228.01	3228.01	.00	5
7.78	5596.03	2173.56	3422.47	3422.47	.00	5
7.78	5596.03	2173.56	3422.47	3422.47	.00	4
7.45	5633.72	2190.51	3443.21	3443.21	.00	4
7.11	5671.39	2207.45	3463.94	3463.94	.00	4
6.78	5709.06	2224.40	3484.66	3484.66	.00	4

6.45	5746.72	2241.34	3505.37	3505.37	.00	4
6.12	5784.37	2258.29	3526.08	3526.08	.00	4
5.79	5822.01	2275.23	3546.78	3546.78	.00	4
5.79	5822.01	2275.24	3546.78	3546.78	.00	3
5.47	5850.37	2283.65	3566.72	3566.72	.00	3
5.15	5878.68	2292.07	3586.62	3586.62	.00	3
4.83	5906.96	2300.48	3606.47	3606.47	.00	3
4.51	5935.19	2308.90	3626.29	3626.29	.00	3
4.20	5963.37	2317.32	3646.06	3646.06	.00	3
3.88	5991.52	2325.73	3665.78	3665.78	.00	3
3.88	5991.52	2325.73	3665.78	3665.78	.00	2
3.44	6043.61	2350.58	3693.03	3693.03	.00	2
3.01	6095.62	2375.42	3720.19	3720.19	.00	2
2.57	6147.54	2400.27	3747.27	3747.27	.00	2
2.14	6199.37	2425.11	3774.25	3774.26	.00	2
1.71	6251.12	2449.96	3801.16	3801.16	.00	2
1.28	6302.78	2474.80	3827.97	3827.97	.00	2
.85	6354.35	2499.65	3854.70	3854.70	.00	2
.43	6405.84	2524.49	3881.34	3881.34	.00	2
.00	6457.24	2549.34	3907.90	3907.90	.00	2

Time = 485. Degree of Consolidation = 104.%

Total Settlement = .765

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 485. = .738

Settlement caused by Secondary Compression at time 485. = .027

******************Current Conditions in Dredged Fill******************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 485. Degree of Consolidation = 99.%

Total Settlement = .111

Settlement at End of Primary Consolidation = .112

Settlement caused by Primary Consolidation at time 485. = .111

Settlement caused by Secondary Compression at time 485. = .000

Surface Elevation = .62

**************Current Conditions in Compressible Foundation************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
60.00	59.21	22.30	2.00	1.85	1.85	6
57.92	57.19	21.59	1.87	1.83	1.83	6
55.88	55.18	20.88	1.86	1.81	1.81	6
53.85	53.18	20.17	1.84	1.80	1.80	6
51.83	51.19	19.45	1.82	1.78	1.78	6
49.83	49.22	18.74	1.80	1.76	1.76	6
47.84	47.26	18.03	1.79	1.74	1.74	6
45.86	45.31	17.32	1.77	1.73	1.73	6
43.89	43.37	16.60	1.75	1.71	1.71	6
41.93	41.45	15.89	1.73	1.69	1.69	6
39.99	39.54	15.18	1.72	1.67	1.67	6
39.99	39.54	15.18	1.82	1.80	1.80	5
36.73	36.30	14.02	1.81	1.79	1.79	5
33.48	33.07	12.86	1.79	1.78	1.78	5
30.25	29.86	11.70	1.78	1.76	1.76	5
27.03	26.66	10.54	1.77	1.75	1.75	5
23.82	23.47	9.38	1.76	1.74	1.74	5

20.63	20.30	8.22	1.75	1.73	1.73	5
17.45	17.14	7.06	1.74	1.72	1.72	5
14.29	14.00	5.90	1.72	1.71	1.71	5
11.14	10.87	4.74	1.71	1.69	1.69	5
8.00	7.75	3.58	1.70	1.68	1.68	5
8.00	7.75	3.58	1.09	1.08	1.08	4
7.67	7.42	3.42	1.09	1.08	1.08	4
7.33	7.09	3.26	1.09	1.08	1.08	4
7.00	6.76	3.10	1.09	1.08	1.08	4
6.67	6.42	2.94	1.09	1.08	1.08	4
6.33	6.09	2.78	1.09	1.08	1.08	4
6.00	5.76	2.62	1.08	1.08	1.08	4
6.00	5.76	2.62	2.73	2.56	2.56	3
5.66	5.44	2.53	2.72	2.55	2.55	3
5.33	5.12	2.44	2.71	2.54	2.54	3
5.00	4.80	2.35	2.71	2.53	2.54	3
4.66	4.49	2.26	2.70	2.52	2.53	3
4.33	4.17	2.17	2.69	2.52	2.52	3
4.00	3.85	2.08	2.68	2.51	2.51	3
4.00	3.85	2.08	.95	.88	.90	2
3.55	3.42	1.85	.94	.87	.90	2
3.10	2.99	1.62	.93	.87	.89	2
2.65	2.56	1.39	.93	.86	.88	2
2.21	2.13	1.16	.92	.85	.88	2
1.76	1.70	.93	.92	.85	.87	2
1.32	1.27	.69	.91	.84	.87	2
.88	.85	.46	.90	.84	.86	2
.44	.42	.23	.90	.83	.85	2
.00	.00	.00	.89	.82	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
59.21	399.01	187.51	211.50	211.50	.00	6
57.19	600.88	263.10	337.77	337.77	.00	6
55.18	801.96	338.69	463.27	463.27	.00	6
53.18	1002.27	414.28	587.99	587.99	.00	6
51.19	1201.79	489.86	711.93	711.93	.00	6
49.22	1400.54	565.45	835.09	835.09	.00	6
47.26	1598.50	641.04	957.46	957.46	.00	6
45.31	1795.69	716.63	1079.06	1079.06	.00	6
43.37	1992.09	792.21	1199.88	1199.88	.00	6
41.45	2187.72	867.80	1319.92	1319.92	.00	6
39.54	2382.57	943.39	1439.18	1439.18	.00	6

39.54	2382.57	943.39	1439.18	1439.18	.00	5
36.30	2707.78	1066.40	1641.37	1641.37	.00	5
33.07	3032.13	1189.42	1842.70	1842.70	.00	5
29.86	3355.62	1312.44	2043.18	2043.18	.00	5
26.66	3678.25	1435.46	2242.79	2242.80	.00	5
23.47	4000.03	1558.47	2441.55	2441.55	.00	5
20.30	4320.94	1681.49	2639.45	2639.45	.00	5
17.14	4641.00	1804.51	2836.50	2836.50	.00	5
14.00	4960.20	1927.53	3032.68	3032.68	.00	5
10.87	5278.55	2050.54	3228.01	3228.01	.00	5
7.75	5596.03	2173.56	3422.47	3422.47	.00	5
7.75	5596.03	2173.56	3422.47	3422.47	.00	4
7.42	5633.72	2190.51	3443.21	3443.21	.00	4
7.09	5671.39	2207.45	3463.94	3463.94	.00	4
6.76	5709.06	2224.40	3484.66	3484.66	.00	4
6.42	5746.72	2241.34	3505.37	3505.37	.00	4
6.09	5784.37	2258.29	3526.08	3526.08	.00	4
5.76	5822.01	2275.23	3546.78	3546.78	.00	4
5.76	5822.01	2275.24	3546.78	3546.78	.00	3
5.44	5850.36	2283.65	3566.70	3566.70	.00	3
5.12	5878.66	2292.07	3586.59	3586.59	.00	3
4.80	5906.91	2300.48	3606.43	3606.43	.00	3
4.49	5935.13	2308.90	3626.23	3626.23	.00	3
4.17	5963.30	2317.32	3645.98	3645.99	.00	3
3.85	5991.43	2325.73	3665.70	3665.70	.00	3
3.85	5991.43	2325.73	3665.70	3665.70	.00	2
3.42	6043.35	2350.58	3692.77	3692.77	.00	2
2.99	6095.18	2375.42	3719.76	3719.76	.00	2
2.56	6146.93	2400.27	3746.66	3746.66	.00	2
2.13	6198.58	2425.11	3773.47	3773.47	.00	2
1.70	6250.15	2449.96	3800.19	3800.19	.00	2
1.27	6301.64	2474.80	3826.83	3826.83	.00	2
.85	6353.03	2499.65	3853.38	3853.38	.00	2
.42	6404.34	2524.49	3879.85	3879.85	.00	2
.00	6455.56	2549.34	3906.22	3906.23	.00	2

Time = 605. Degree of Consolidation = 107.%

Total Settlement = .792

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 605. = .738

Settlement caused by Secondary Compression at time 605. = .054

******************Current Conditions in Dredged Fill******************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

***** Stresses ***** ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 605. Degree of Consolidation = 99.%
Total Settlement = .111
Settlement at End of Primary Consolidation = .112
Settlement caused by Primary Consolidation at time 605. = .111
Settlement caused by Secondary Compression at time 605. = .000
Surface Elevation = .60

**************Current Conditions in Compressible Foundation************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
60.00	59.19	22.30	2.00	1.85	1.85	6
57.92	57.16	21.59	1.87	1.83	1.83	6

55.88	55.15	20.88	1.86	1.81	1.81	6
53.85	53.15	20.17	1.84	1.80	1.80	6
51.83	51.17	19.45	1.82	1.78	1.78	6
49.83	49.19	18.74	1.80	1.76	1.76	6
47.84	47.23	18.03	1.79	1.74	1.74	6
45.86	45.28	17.32	1.77	1.73	1.73	6
43.89	43.35	16.60	1.75	1.71	1.71	6
41.93	41.42	15.89	1.73	1.69	1.69	6
39.99	39.51	15.18	1.72	1.67	1.67	6
39.99	39.51	15.18	1.82	1.80	1.80	5
36.73	36.27	14.02	1.81	1.79	1.79	5
33.48	33.05	12.86	1.79	1.78	1.78	5
30.25	29.83	11.70	1.78	1.76	1.76	5
27.03	26.63	10.54	1.77	1.75	1.75	5
23.82	23.45	9.38	1.76	1.74	1.74	5
20.63	20.28	8.22	1.75	1.73	1.73	5
17.45	17.12	7.06	1.74	1.72	1.72	5
14.29	13.98	5.90	1.72	1.71	1.71	5
11.14	10.85	4.74	1.71	1.69	1.69	5
8.00	7.73	3.58	1.70	1.68	1.68	5
8.00	7.73	3.58	1.09	1.08	1.08	4
7.67	7.40	3.42	1.09	1.08	1.08	4
7.33	7.06	3.26	1.09	1.08	1.08	4
7.00	6.73	3.10	1.09	1.08	1.08	4
6.67	6.40	2.94	1.09	1.08	1.08	4
6.33	6.07	2.78	1.09	1.08	1.08	4
6.00	5.74	2.62	1.08	1.08	1.08	4
6.00	5.74	2.62	2.73	2.55	2.56	3
5.66	5.42	2.53	2.72	2.54	2.55	3
5.33	5.10	2.44	2.71	2.54	2.54	3
5.00	4.78	2.35	2.71	2.53	2.54	3
4.66	4.47	2.26	2.70	2.52	2.53	3
4.33	4.15	2.17	2.69	2.51	2.52	3
4.00	3.83	2.08	2.68	2.51	2.51	3
4.00	3.83	2.08	.95	.87	.90	2
3.55	3.40	1.85	.94	.86	.90	2
3.10	2.97	1.62	.93	.85	.89	2
2.65	2.54	1.39	.93	.85	.88	2
2.21	2.12	1.16	.92	.84	.88	2
1.76	1.69	.93	.92	.84	.87	2
1.32	1.27	.69	.91	.83	.87	2
.88	.84	.46	.90	.82	.86	2
.44	.42	.23	.90	.82	.85	2
.00	.00	.00	.89	.81	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
59.19	399.01	187.51	211.50	211.50	.00	6
57.16	600.88	263.10	337.77	337.77	.00	6
55.15	801.96	338.69	463.27	463.27	.00	6
53.15	1002.27	414.28	587.99	587.99	.00	6
51.17	1201.79	489.86	711.93	711.93	.00	6
49.19	1400.54	565.45	835.09	835.09	.00	6
47.23	1598.50	641.04	957.46	957.46	.00	6
45.28	1795.69	716.63	1079.06	1079.06	.00	6
43.35	1992.09	792.21	1199.88	1199.88	.00	6
41.42	2187.72	867.80	1319.92	1319.92	.00	6
39.51	2382.57	943.39	1439.18	1439.18	.00	6
39.51	2382.57	943.39	1439.18	1439.18	.00	5
36.27	2707.78	1066.40	1641.37	1641.37	.00	5
33.05	3032.13	1189.42	1842.70	1842.70	.00	5
29.83	3355.62	1312.44	2043.18	2043.18	.00	5
26.63	3678.25	1435.46	2242.79	2242.80	.00	5
23.45	4000.03	1558.47	2441.55	2441.55	.00	5
20.28	4320.94	1681.49	2639.45	2639.45	.00	5
17.12	4641.00	1804.51	2836.50	2836.50	.00	5
13.98	4960.20	1927.53	3032.68	3032.68	.00	5
10.85	5278.55	2050.54	3228.01	3228.01	.00	5
7.73	5596.03	2173.56	3422.47	3422.47	.00	5
7.73	5596.03	2173.56	3422.47	3422.47	.00	4
7.40	5633.72	2190.51	3443.21	3443.21	.00	4
7.06	5671.39	2207.45	3463.94	3463.94	.00	4
6.73	5709.06	2224.40	3484.66	3484.66	.00	4
6.40	5746.72	2241.34	3505.37	3505.37	.00	4
6.07	5784.37	2258.29	3526.08	3526.08	.00	4
5.74	5822.01	2275.23	3546.78	3546.78	.00	4
5.74	5822.01	2275.24	3546.78	3546.78	.00	3
5.42	5850.34	2283.65	3566.69	3566.69	.00	3
5.10	5878.63	2292.07	3586.56	3586.56	.00	3
4.78	5906.87	2300.48	3606.39	3606.39	.00	3
4.47	5935.07	2308.90	3626.17	3626.17	.00	3
4.15	5963.23	2317.32	3645.91	3645.91	.00	3
3.83	5991.34	2325.73	3665.61	3665.61	.00	3
3.83	5991.34	2325.73	3665.61	3665.61	.00	2
3.40	6043.11	2350.58	3692.53	3692.54	.00	2
2.97	6094.80	2375.42	3719.37	3719.37	.00	2
2.54	6146.39	2400.27	3746.12	3746.12	.00	2
2.12	6197.90	2425.11	3772.78	3772.78	.00	2

1.69	6249.32	2449.96	3799.36	3799.36	.00	2
1.27	6300.65	2474.80	3825.85	3825.85	.00	2
.84	6351.90	2499.65	3852.25	3852.25	.00	2
.42	6403.05	2524.49	3878.56	3878.56	.00	2
.00	6454.12	2549.34	3904.78	3904.79	.00	2

Time = 730. Degree of Consolidation = 110.%

Total Settlement = .815

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 730. = .738

Settlement caused by Secondary Compression at time 730. = .077

******************Current Conditions in Dredged Fill******************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

***** Stresses ***** ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 730. Degree of Consolidation = 99.%

Total Settlement = .111

Settlement at End of Primary Consolidation = .112

Settlement caused by Primary Consolidation at time 730. = .111

Settlement caused by Secondary Compression at time 730. = .000

Surface Elevation = .57

**************Current Conditions in Compressible Foundation*************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
60.00	59.17	22.30	2.00	1.85	1.85	6
57.92	57.14	21.59	1.87	1.83	1.83	6
55.88	55.13	20.88	1.86	1.81	1.81	6
53.85	53.13	20.17	1.84	1.80	1.80	6
51.83	51.15	19.45	1.82	1.78	1.78	6
49.83	49.18	18.74	1.80	1.76	1.76	6
47.84	47.21	18.03	1.79	1.74	1.74	6
45.86	45.27	17.32	1.77	1.73	1.73	6
43.89	43.33	16.60	1.75	1.71	1.71	6
41.93	41.41	15.89	1.73	1.69	1.69	6
39.99	39.49	15.18	1.72	1.67	1.67	6
39.99	39.49	15.18	1.82	1.80	1.80	5
36.73	36.25	14.02	1.81	1.79	1.79	5
33.48	33.03	12.86	1.79	1.78	1.78	5
30.25	29.81	11.70	1.78	1.76	1.76	5
27.03	26.62	10.54	1.77	1.75	1.75	5
23.82	23.43	9.38	1.76	1.74	1.74	5
20.63	20.26	8.22	1.75	1.73	1.73	5
17.45	17.10	7.06	1.74	1.72	1.72	5
14.29	13.96	5.90	1.72	1.71	1.71	5
11.14	10.83	4.74	1.71	1.69	1.69	5
8.00	7.71	3.58	1.70	1.68	1.68	5
8.00	7.71	3.58	1.09	1.08	1.08	4
7.67	7.38	3.42	1.09	1.08	1.08	4
7.33	7.05	3.26	1.09	1.08	1.08	4
7.00	6.71	3.10	1.09	1.08	1.08	4
6.67	6.38	2.94	1.09	1.08	1.08	4
6.33	6.05	2.78	1.09	1.08	1.08	4
6.00	5.72	2.62	1.08	1.08	1.08	4
6.00	5.72	2.62	2.73	2.55	2.56	3
5.66	5.40	2.53	2.72	2.54	2.55	3
5.33	5.08	2.44	2.71	2.53	2.54	3

5.00	4.76	2.35	2.71	2.53	2.54	3
4.66	4.45	2.26	2.70	2.52	2.53	3
4.33	4.13	2.17	2.69	2.51	2.52	3
4.00	3.82	2.08	2.68	2.50	2.51	3
4.00	3.82	2.08	.95	.86	.90	2
3.55	3.39	1.85	.94	.85	.90	2
3.10	2.96	1.62	.93	.85	.89	2
2.65	2.53	1.39	.93	.84	.88	2
2.21	2.11	1.16	.92	.83	.88	2
1.76	1.68	.93	.92	.83	.87	2
1.32	1.26	.69	.91	.82	.87	2
.88	.84	.46	.90	.82	.86	2
.44	.42	.23	.90	.81	.85	2
.00	.00	.00	.89	.80	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
59.17	399.01	187.51	211.50	211.50	.00	6
57.14	600.88	263.10	337.77	337.77	.00	6
55.13	801.96	338.69	463.27	463.27	.00	6
53.13	1002.27	414.28	587.99	587.99	.00	6
51.15	1201.79	489.86	711.93	711.93	.00	6
49.18	1400.54	565.45	835.09	835.09	.00	6
47.21	1598.50	641.04	957.46	957.46	.00	6
45.27	1795.69	716.63	1079.06	1079.06	.00	6
43.33	1992.09	792.21	1199.88	1199.88	.00	6
41.41	2187.72	867.80	1319.92	1319.92	.00	6
39.49	2382.57	943.39	1439.18	1439.18	.00	6
39.49	2382.57	943.39	1439.18	1439.18	.00	5
36.25	2707.78	1066.40	1641.37	1641.37	.00	5
33.03	3032.13	1189.42	1842.70	1842.70	.00	5
29.81	3355.62	1312.44	2043.18	2043.18	.00	5
26.62	3678.25	1435.46	2242.79	2242.80	.00	5
23.43	4000.03	1558.47	2441.55	2441.55	.00	5
20.26	4320.94	1681.49	2639.45	2639.45	.00	5
17.10	4641.00	1804.51	2836.50	2836.50	.00	5
13.96	4960.20	1927.53	3032.68	3032.68	.00	5
10.83	5278.55	2050.54	3228.01	3228.01	.00	5
7.71	5596.03	2173.56	3422.47	3422.47	.00	5
7.71	5596.03	2173.56	3422.47	3422.47	.00	4
7.38	5633.72	2190.51	3443.21	3443.21	.00	4
7.05	5671.39	2207.45	3463.94	3463.94	.00	4
6.71	5709.06	2224.40	3484.66	3484.66	.00	4

6.38	5746.72	2241.34	3505.37	3505.37	.00	4
6.05	5784.37	2258.29	3526.08	3526.08	.00	4
5.72	5822.01	2275.23	3546.78	3546.78	.00	4
5.72	5822.01	2275.24	3546.78	3546.78	.00	3
5.40	5850.33	2283.65	3566.68	3566.68	.00	3
5.08	5878.60	2292.07	3586.53	3586.53	.00	3
4.76	5906.83	2300.48	3606.34	3606.34	.00	3
4.45	5935.01	2308.90	3626.11	3626.11	.00	3
4.13	5963.16	2317.32	3645.84	3645.84	.00	3
3.82	5991.26	2325.73	3665.52	3665.53	.00	3
3.82	5991.26	2325.73	3665.52	3665.53	.00	2
3.39	6042.91	2350.58	3692.33	3692.33	.00	2
2.96	6094.47	2375.42	3719.05	3719.05	.00	2
2.53	6145.94	2400.27	3745.67	3745.68	.00	2
2.11	6197.33	2425.11	3772.22	3772.22	.00	2
1.68	6248.63	2449.96	3798.67	3798.67	.00	2
1.26	6299.84	2474.80	3825.03	3825.04	.00	2
.84	6350.96	2499.65	3851.31	3851.31	.00	2
.42	6402.00	2524.49	3877.50	3877.51	.00	2
.00	6452.94	2549.34	3903.61	3903.61	.00	2

Time = 850. Degree of Consolidation = 113.%

Total Settlement = .834

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 850. = .738

Settlement caused by Secondary Compression at time 850. = .096

******************Current Conditions in Dredged Fill******************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

***** Stresses ***** ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 850. Degree of Consolidation = 99.%
Total Settlement = .111
Settlement at End of Primary Consolidation = .112
Settlement caused by Primary Consolidation at time 850. = .111
Settlement caused by Secondary Compression at time 850. = .000
Surface Elevation = .56

**************Current Conditions in Compressible Foundation*************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
60.00	59.15	22.30	2.00	1.85	1.85	6
57.92	57.13	21.59	1.87	1.83	1.83	6
55.88	55.12	20.88	1.86	1.81	1.81	6
53.85	53.12	20.17	1.84	1.80	1.80	6
51.83	51.13	19.45	1.82	1.78	1.78	6
49.83	49.16	18.74	1.80	1.76	1.76	6
47.84	47.20	18.03	1.79	1.74	1.74	6
45.86	45.25	17.32	1.77	1.73	1.73	6
43.89	43.31	16.60	1.75	1.71	1.71	6
41.93	41.39	15.89	1.73	1.69	1.69	6
39.99	39.48	15.18	1.72	1.67	1.67	6
39.99	39.48	15.18	1.82	1.80	1.80	5
36.73	36.24	14.02	1.81	1.79	1.79	5
33.48	33.01	12.86	1.79	1.78	1.78	5
30.25	29.80	11.70	1.78	1.76	1.76	5
27.03	26.60	10.54	1.77	1.75	1.75	5

23.82	23.41	9.38	1.76	1.74	1.74	5
20.63	20.24	8.22	1.75	1.73	1.73	5
17.45	17.09	7.06	1.74	1.72	1.72	5
14.29	13.94	5.90	1.72	1.71	1.71	5
11.14	10.81	4.74	1.71	1.69	1.69	5
8.00	7.69	3.58	1.70	1.68	1.68	5
8.00	7.69	3.58	1.09	1.08	1.08	4
7.67	7.36	3.42	1.09	1.08	1.08	4
7.33	7.03	3.26	1.09	1.08	1.08	4
7.00	6.70	3.10	1.09	1.08	1.08	4
6.67	6.37	2.94	1.09	1.08	1.08	4
6.33	6.03	2.78	1.09	1.08	1.08	4
6.00	5.70	2.62	1.08	1.08	1.08	4
6.00	5.70	2.62	2.73	2.55	2.56	3
5.66	5.38	2.53	2.72	2.54	2.55	3
5.33	5.07	2.44	2.71	2.53	2.54	3
5.00	4.75	2.35	2.71	2.53	2.54	3
4.66	4.43	2.26	2.70	2.52	2.53	3
4.33	4.12	2.17	2.69	2.51	2.52	3
4.00	3.80	2.08	2.68	2.50	2.51	3
4.00	3.80	2.08	.95	.85	.90	2
3.55	3.37	1.85	.94	.85	.90	2
3.10	2.95	1.62	.93	.84	.89	2
2.65	2.52	1.39	.93	.83	.88	2
2.21	2.10	1.16	.92	.83	.88	2
1.76	1.67	.93	.92	.82	.87	2
1.32	1.25	.69	.91	.81	.87	2
.88	.83	.46	.90	.81	.86	2
.44	.42	.23	.90	.80	.85	2
.00	.00	.00	.89	.80	.85	2

***** Stresses *****   ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
59.15	399.01	187.51	211.50	211.50	.00	6
57.13	600.88	263.10	337.77	337.77	.00	6
55.12	801.96	338.69	463.27	463.27	.00	6
53.12	1002.27	414.28	587.99	587.99	.00	6
51.13	1201.79	489.86	711.93	711.93	.00	6
49.16	1400.54	565.45	835.09	835.09	.00	6
47.20	1598.50	641.04	957.46	957.47	.00	6
45.25	1795.69	716.63	1079.06	1079.06	.00	6
43.31	1992.09	792.21	1199.88	1199.88	.00	6
41.39	2187.72	867.80	1319.92	1319.92	.00	6

39.48	2382.57	943.39	1439.18	1439.18	.00	6
39.48	2382.57	943.39	1439.18	1439.18	.00	5
36.24	2707.78	1066.40	1641.37	1641.37	.00	5
33.01	3032.13	1189.42	1842.70	1842.70	.00	5
29.80	3355.62	1312.44	2043.18	2043.18	.00	5
26.60	3678.25	1435.46	2242.79	2242.80	.00	5
23.41	4000.03	1558.47	2441.55	2441.55	.00	5
20.24	4320.94	1681.49	2639.45	2639.45	.00	5
17.09	4641.00	1804.51	2836.50	2836.50	.00	5
13.94	4960.21	1927.53	3032.68	3032.68	.00	5
10.81	5278.55	2050.54	3228.01	3228.01	.00	5
7.69	5596.03	2173.56	3422.47	3422.47	.00	5
7.69	5596.03	2173.56	3422.47	3422.47	.00	4
7.36	5633.72	2190.51	3443.21	3443.21	.00	4
7.03	5671.39	2207.45	3463.94	3463.94	.00	4
6.70	5709.06	2224.40	3484.66	3484.66	.00	4
6.37	5746.72	2241.34	3505.37	3505.37	.00	4
6.03	5784.37	2258.29	3526.08	3526.08	.00	4
5.70	5822.01	2275.23	3546.78	3546.78	.00	4
5.70	5822.01	2275.24	3546.78	3546.78	.00	3
5.38	5850.32	2283.65	3566.67	3566.67	.00	3
5.07	5878.59	2292.07	3586.52	3586.52	.00	3
4.75	5906.81	2300.48	3606.32	3606.33	.00	3
4.43	5934.99	2308.90	3626.09	3626.09	.00	3
4.12	5963.13	2317.32	3645.81	3645.81	.00	3
3.80	5991.22	2325.73	3665.49	3665.49	.00	3
3.80	5991.22	2325.73	3665.49	3665.49	.00	2
3.37	6042.77	2350.58	3692.19	3692.19	.00	2
2.95	6094.22	2375.42	3718.80	3718.80	.00	2
2.52	6145.59	2400.27	3745.32	3745.32	.00	2
2.10	6196.87	2425.11	3771.76	3771.76	.00	2
1.67	6248.07	2449.96	3798.11	3798.11	.00	2
1.25	6299.17	2474.80	3824.37	3824.37	.00	2
.83	6350.19	2499.65	3850.54	3850.54	.00	2
.42	6401.12	2524.49	3876.62	3876.63	.00	2
.00	6451.96	2549.34	3902.62	3902.62	.00	2

Time = 970. Degree of Consolidation = 115.%

Total Settlement = .850

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 970. = .738

Settlement caused by Secondary Compression at time 970. = .111

******************Current Conditions in Dredged Fill******************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

***** Stresses ***** ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 970. Degree of Consolidation = 99.%
Total Settlement = .111
Settlement at End of Primary Consolidation = .112
Settlement caused by Primary Consolidation at time 970. = .111
Settlement caused by Secondary Compression at time 970. = .000
Surface Elevation = .54

*************Current Conditions in Compressible Foundation************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material

60.00	59.14	22.30	2.00	1.85	1.85	6
57.92	57.11	21.59	1.87	1.83	1.83	6
55.88	55.10	20.88	1.86	1.81	1.81	6
53.85	53.11	20.17	1.84	1.80	1.80	6
51.83	51.12	19.45	1.82	1.78	1.78	6
49.83	49.15	18.74	1.80	1.76	1.76	6
47.84	47.18	18.03	1.79	1.74	1.74	6
45.86	45.24	17.32	1.77	1.73	1.73	6
43.89	43.30	16.60	1.75	1.71	1.71	6
41.93	41.38	15.89	1.73	1.69	1.69	6
39.99	39.46	15.18	1.72	1.67	1.67	6
39.99	39.46	15.18	1.82	1.80	1.80	5
36.73	36.22	14.02	1.81	1.79	1.79	5
33.48	33.00	12.86	1.79	1.78	1.78	5
30.25	29.78	11.70	1.78	1.76	1.76	5
27.03	26.59	10.54	1.77	1.75	1.75	5
23.82	23.40	9.38	1.76	1.74	1.74	5
20.63	20.23	8.22	1.75	1.73	1.73	5
17.45	17.07	7.06	1.74	1.72	1.72	5
14.29	13.93	5.90	1.72	1.71	1.71	5
11.14	10.80	4.74	1.71	1.69	1.69	5
8.00	7.68	3.58	1.70	1.68	1.68	5
8.00	7.68	3.58	1.09	1.08	1.08	4
7.67	7.35	3.42	1.09	1.08	1.08	4
7.33	7.02	3.26	1.09	1.08	1.08	4
7.00	6.68	3.10	1.09	1.08	1.08	4
6.67	6.35	2.94	1.09	1.08	1.08	4
6.33	6.02	2.78	1.09	1.08	1.08	4
6.00	5.69	2.62	1.08	1.08	1.08	4
6.00	5.69	2.62	2.73	2.55	2.56	3
5.66	5.37	2.53	2.72	2.54	2.55	3
5.33	5.05	2.44	2.71	2.53	2.54	3
5.00	4.73	2.35	2.71	2.53	2.54	3
4.66	4.42	2.26	2.70	2.52	2.53	3
4.33	4.10	2.17	2.69	2.51	2.52	3
4.00	3.79	2.08	2.68	2.50	2.51	3
4.00	3.79	2.08	.95	.84	.90	2
3.55	3.36	1.85	.94	.84	.90	2
3.10	2.93	1.62	.93	.83	.89	2
2.65	2.51	1.39	.93	.83	.88	2
2.21	2.09	1.16	.92	.82	.88	2
1.76	1.67	.93	.92	.81	.87	2
1.32	1.25	.69	.91	.81	.87	2
.88	.83	.46	.90	.80	.86	2

.44	.42	.23	.90	.80	.85	2
.00	.00	.00	.89	.79	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
59.14	399.01	187.51	211.50	211.50	.00	6
57.11	600.88	263.10	337.77	337.77	.00	6
55.10	801.96	338.69	463.27	463.27	.00	6
53.11	1002.27	414.28	587.99	587.99	.00	6
51.12	1201.79	489.86	711.93	711.93	.00	6
49.15	1400.54	565.45	835.09	835.09	.00	6
47.18	1598.50	641.04	957.46	957.47	.00	6
45.24	1795.69	716.63	1079.06	1079.06	.00	6
43.30	1992.09	792.21	1199.88	1199.88	.00	6
41.38	2187.72	867.80	1319.92	1319.92	.00	6
39.46	2382.57	943.39	1439.18	1439.18	.00	6
39.46	2382.57	943.39	1439.18	1439.18	.00	5
36.22	2707.78	1066.40	1641.37	1641.37	.00	5
33.00	3032.13	1189.42	1842.70	1842.70	.00	5
29.78	3355.62	1312.44	2043.18	2043.18	.00	5
26.59	3678.25	1435.46	2242.79	2242.80	.00	5
23.40	4000.03	1558.47	2441.55	2441.55	.00	5
20.23	4320.94	1681.49	2639.45	2639.45	.00	5
17.07	4641.00	1804.51	2836.50	2836.50	.00	5
13.93	4960.21	1927.53	3032.68	3032.68	.00	5
10.80	5278.55	2050.54	3228.01	3228.01	.00	5
7.68	5596.03	2173.56	3422.47	3422.47	.00	5
7.68	5596.03	2173.56	3422.47	3422.47	.00	4
7.35	5633.72	2190.51	3443.21	3443.21	.00	4
7.02	5671.39	2207.45	3463.94	3463.94	.00	4
6.68	5709.06	2224.40	3484.66	3484.66	.00	4
6.35	5746.72	2241.34	3505.37	3505.37	.00	4
6.02	5784.37	2258.29	3526.08	3526.08	.00	4
5.69	5822.01	2275.23	3546.78	3546.78	.00	4
5.69	5822.01	2275.24	3546.78	3546.78	.00	3
5.37	5850.32	2283.65	3566.67	3566.67	.00	3
5.05	5878.59	2292.07	3586.52	3586.52	.00	3
4.73	5906.81	2300.48	3606.32	3606.33	.00	3
4.42	5934.99	2308.90	3626.09	3626.09	.00	3
4.10	5963.13	2317.32	3645.81	3645.81	.00	3
3.79	5991.22	2325.73	3665.49	3665.49	.00	3
3.79	5991.22	2325.73	3665.49	3665.49	.00	2

3.36	6042.67	2350.58	3692.09	3692.09	.00	2
2.93	6094.03	2375.42	3718.60	3718.61	.00	2
2.51	6145.30	2400.27	3745.03	3745.03	.00	2
2.09	6196.49	2425.11	3771.37	3771.37	.00	2
1.67	6247.58	2449.96	3797.62	3797.62	.00	2
1.25	6298.59	2474.80	3823.79	3823.79	.00	2
.83	6349.51	2499.65	3849.86	3849.86	.00	2
.42	6400.34	2524.49	3875.85	3875.85	.00	2
.00	6451.09	2549.34	3901.75	3901.75	.00	2

Time = 1095. Degree of Consolidation = 117.%

Total Settlement = .864

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 1095. = .738

Settlement caused by Secondary Compression at time 1095. = .125

******************Current Conditions in Dredged Fill******************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

***** Stresses ***** ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1

.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 1095. Degree of Consolidation = 99.%

Total Settlement = .111

Settlement at End of Primary Consolidation = .112

Settlement caused by Primary Consolidation at time 1095. = .111

Settlement caused by Secondary Compression at time 1095. = .000

Surface Elevation = .53

**************Current Conditions in Compressible Foundation************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
60.00	59.13	22.30	2.00	1.85	1.85	6
57.92	57.10	21.59	1.87	1.83	1.83	6
55.88	55.09	20.88	1.86	1.81	1.81	6
53.85	53.09	20.17	1.84	1.80	1.80	6
51.83	51.11	19.45	1.82	1.78	1.78	6
49.83	49.13	18.74	1.80	1.76	1.76	6
47.84	47.17	18.03	1.79	1.74	1.74	6
45.86	45.22	17.32	1.77	1.73	1.73	6
43.89	43.29	16.60	1.75	1.71	1.71	6
41.93	41.36	15.89	1.73	1.69	1.69	6
39.99	39.45	15.18	1.72	1.67	1.67	6
39.99	39.45	15.18	1.82	1.80	1.80	5
36.73	36.21	14.02	1.81	1.79	1.79	5
33.48	32.99	12.86	1.79	1.78	1.78	5
30.25	29.77	11.70	1.78	1.76	1.76	5
27.03	26.57	10.54	1.77	1.75	1.75	5
23.82	23.39	9.38	1.76	1.74	1.74	5
20.63	20.22	8.22	1.75	1.73	1.73	5
17.45	17.06	7.06	1.74	1.72	1.72	5
14.29	13.92	5.90	1.72	1.71	1.71	5
11.14	10.79	4.74	1.71	1.69	1.69	5
8.00	7.67	3.58	1.70	1.68	1.68	5
8.00	7.67	3.58	1.09	1.08	1.08	4
7.67	7.34	3.42	1.09	1.08	1.08	4
7.33	7.00	3.26	1.09	1.08	1.08	4
7.00	6.67	3.10	1.09	1.08	1.08	4
6.67	6.34	2.94	1.09	1.08	1.08	4
6.33	6.01	2.78	1.09	1.08	1.08	4

6.00	5.68	2.62	1.08	1.08	1.08	4
6.00	5.68	2.62	2.73	2.55	2.56	3
5.66	5.36	2.53	2.72	2.54	2.55	3
5.33	5.04	2.44	2.71	2.53	2.54	3
5.00	4.72	2.35	2.71	2.53	2.54	3
4.66	4.41	2.26	2.70	2.52	2.53	3
4.33	4.09	2.17	2.69	2.51	2.52	3
4.00	3.77	2.08	2.68	2.50	2.51	3
4.00	3.77	2.08	.95	.84	.90	2
3.55	3.35	1.85	.94	.83	.90	2
3.10	2.93	1.62	.93	.83	.89	2
2.65	2.50	1.39	.93	.82	.88	2
2.21	2.08	1.16	.92	.81	.88	2
1.76	1.66	.93	.92	.81	.87	2
1.32	1.25	.69	.91	.80	.87	2
.88	.83	.46	.90	.80	.86	2
.44	.41	.23	.90	.79	.85	2
.00	.00	.00	.89	.78	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
59.13	399.01	187.51	211.50	211.50	.00	6
57.10	600.88	263.10	337.77	337.77	.00	6
55.09	801.96	338.69	463.27	463.27	.00	6
53.09	1002.27	414.28	587.99	587.99	.00	6
51.11	1201.79	489.86	711.93	711.93	.00	6
49.13	1400.54	565.45	835.09	835.09	.00	6
47.17	1598.50	641.04	957.46	957.46	.00	6
45.22	1795.69	716.63	1079.06	1079.06	.00	6
43.29	1992.09	792.21	1199.88	1199.88	.00	6
41.36	2187.72	867.80	1319.92	1319.92	.00	6
39.45	2382.57	943.39	1439.18	1439.18	.00	6
39.45	2382.57	943.39	1439.18	1439.18	.00	5
36.21	2707.78	1066.40	1641.37	1641.37	.00	5
32.99	3032.13	1189.42	1842.70	1842.70	.00	5
29.77	3355.62	1312.44	2043.18	2043.18	.00	5
26.57	3678.25	1435.46	2242.79	2242.80	.00	5
23.39	4000.03	1558.47	2441.55	2441.55	.00	5
20.22	4320.94	1681.49	2639.45	2639.45	.00	5
17.06	4641.00	1804.51	2836.50	2836.50	.00	5
13.92	4960.21	1927.53	3032.68	3032.68	.00	5
10.79	5278.55	2050.54	3228.01	3228.01	.00	5
7.67	5596.03	2173.56	3422.47	3422.47	.00	5

7.67	5596.03	2173.56	3422.47	3422.47	.00	4
7.34	5633.72	2190.51	3443.21	3443.21	.00	4
7.00	5671.39	2207.45	3463.94	3463.94	.00	4
6.67	5709.06	2224.40	3484.66	3484.66	.00	4
6.34	5746.72	2241.34	3505.37	3505.37	.00	4
6.01	5784.37	2258.29	3526.08	3526.08	.00	4
5.68	5822.01	2275.23	3546.78	3546.78	.00	4
5.68	5822.01	2275.24	3546.78	3546.78	.00	3
5.36	5850.32	2283.65	3566.67	3566.67	.00	3
5.04	5878.59	2292.07	3586.52	3586.52	.00	3
4.72	5906.81	2300.48	3606.32	3606.33	.00	3
4.41	5934.99	2308.90	3626.09	3626.09	.00	3
4.09	5963.13	2317.32	3645.81	3645.81	.00	3
3.77	5991.22	2325.73	3665.49	3665.49	.00	3
3.77	5991.22	2325.73	3665.49	3665.49	.00	2
3.35	6042.59	2350.58	3692.01	3692.01	.00	2
2.93	6093.87	2375.42	3718.44	3718.44	.00	2
2.50	6145.06	2400.27	3744.79	3744.79	.00	2
2.08	6196.16	2425.11	3771.04	3771.04	.00	2
1.66	6247.17	2449.96	3797.21	3797.21	.00	2
1.25	6298.09	2474.80	3823.29	3823.29	.00	2
.83	6348.93	2499.65	3849.28	3849.28	.00	2
.41	6399.68	2524.49	3875.18	3875.19	.00	2
.00	6450.34	2549.34	3901.00	3901.00	.00	2

Time = 1215. Degree of Consolidation = 119.%
Total Settlement = .876
Settlement at End of Primary Consolidation = .738
Settlement caused by Primary Consolidation at time 1215. = .738
Settlement caused by Secondary Compression at time 1215. = .137

******************Current Conditions in Dredged Fill******************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1

.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 1215. Degree of Consolidation = 99.%

Total Settlement = .111

Settlement at End of Primary Consolidation = .112

Settlement caused by Primary Consolidation at time 1215. = .111

Settlement caused by Secondary Compression at time 1215. = .000

Surface Elevation = .51

**************Current Conditions in Compressible Foundation*************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
60.00	59.00	22.30	2.00	1.85	1.85	6
57.92	56.98	21.59	1.87	1.83	1.83	6
55.88	54.97	20.88	1.86	1.81	1.81	6
53.85	52.97	20.17	1.84	1.80	1.80	6
51.83	50.98	19.45	1.82	1.78	1.78	6
49.83	49.01	18.74	1.80	1.76	1.76	6
47.84	47.05	18.03	1.79	1.74	1.74	6
45.86	45.10	17.32	1.77	1.73	1.73	6
43.89	43.16	16.60	1.75	1.71	1.71	6
41.93	41.24	15.89	1.73	1.69	1.69	6
39.99	39.33	15.18	1.72	1.67	1.67	6
39.99	39.33	15.18	1.82	1.80	1.80	5
36.73	36.09	14.02	1.81	1.79	1.79	5

33.48	32.86	12.86	1.79	1.78	1.78	5
30.25	29.65	11.70	1.78	1.76	1.76	5
27.03	26.45	10.54	1.77	1.75	1.75	5
23.82	23.26	9.38	1.76	1.74	1.74	5
20.63	20.09	8.22	1.75	1.73	1.73	5
17.45	16.93	7.06	1.74	1.72	1.72	5
14.29	13.79	5.90	1.72	1.71	1.71	5
11.14	10.66	4.74	1.71	1.69	1.69	5
8.00	7.54	3.58	1.70	1.68	1.68	5
8.00	7.54	3.58	1.09	1.08	1.08	4
7.67	7.21	3.42	1.09	1.08	1.08	4
7.33	6.88	3.26	1.09	1.08	1.08	4
7.00	6.55	3.10	1.09	1.08	1.08	4
6.67	6.21	2.94	1.09	1.08	1.08	4
6.33	5.88	2.78	1.09	1.08	1.08	4
6.00	5.55	2.62	1.08	1.08	1.08	4
6.00	5.55	2.62	2.73	2.55	2.56	3
5.66	5.23	2.53	2.72	2.54	2.55	3
5.33	4.91	2.44	2.71	2.53	2.54	3
5.00	4.60	2.35	2.71	2.53	2.54	3
4.66	4.28	2.26	2.70	2.52	2.53	3
4.33	3.96	2.17	2.69	2.51	2.52	3
4.00	3.65	2.08	2.68	2.50	2.51	3
4.00	3.65	2.08	.95	.78	.90	2
3.55	3.24	1.85	.94	.77	.90	2
3.10	2.83	1.62	.93	.77	.89	2
2.65	2.42	1.39	.93	.76	.88	2
2.21	2.01	1.16	.92	.75	.88	2
1.76	1.61	.93	.92	.75	.87	2
1.32	1.20	.69	.91	.74	.87	2
.88	.80	.46	.90	.74	.86	2
.44	.40	.23	.90	.73	.85	2
.00	.00	.00	.89	.72	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
59.00	399.01	187.51	211.50	211.50	.00	6
56.98	600.88	263.10	337.77	337.77	.00	6
54.97	801.96	338.69	463.27	463.27	.00	6
52.97	1002.27	414.28	587.99	587.99	.00	6
50.98	1201.79	489.86	711.93	711.93	.00	6
49.01	1400.54	565.45	835.09	835.09	.00	6
47.05	1598.50	641.04	957.46	957.46	.00	6

45.10	1795.69	716.63	1079.06	1079.06	.00	6
43.16	1992.09	792.21	1199.88	1199.88	.00	6
41.24	2187.72	867.80	1319.92	1319.92	.00	6
39.33	2382.57	943.39	1439.18	1439.18	.00	6
39.33	2382.57	943.39	1439.18	1439.18	.00	5
36.09	2707.78	1066.40	1641.37	1641.37	.00	5
32.86	3032.13	1189.42	1842.70	1842.70	.00	5
29.65	3355.62	1312.44	2043.18	2043.18	.00	5
26.45	3678.25	1435.46	2242.79	2242.80	.00	5
23.26	4000.03	1558.47	2441.55	2441.55	.00	5
20.09	4320.94	1681.49	2639.45	2639.45	.00	5
16.93	4641.00	1804.51	2836.50	2836.50	.00	5
13.79	4960.21	1927.53	3032.68	3032.68	.00	5
10.66	5278.55	2050.54	3228.01	3228.01	.00	5
7.54	5596.03	2173.56	3422.47	3422.47	.00	5
7.54	5596.03	2173.56	3422.47	3422.47	.00	4
7.21	5633.72	2190.51	3443.21	3443.21	.00	4
6.88	5671.39	2207.45	3463.94	3463.94	.00	4
6.55	5709.06	2224.40	3484.66	3484.66	.00	4
6.21	5746.72	2241.34	3505.37	3505.37	.00	4
5.88	5784.37	2258.29	3526.08	3526.08	.00	4
5.55	5822.01	2275.23	3546.78	3546.78	.00	4
5.55	5822.01	2275.24	3546.78	3546.78	.00	3
5.23	5850.32	2283.65	3566.67	3566.67	.00	3
4.91	5878.59	2292.07	3586.52	3586.52	.00	3
4.60	5906.81	2300.48	3606.32	3606.33	.00	3
4.28	5934.99	2308.90	3626.09	3626.09	.00	3
3.96	5963.13	2317.32	3645.81	3645.81	.00	3
3.65	5991.22	2325.73	3665.49	3665.49	.00	3
3.65	5991.22	2325.73	3665.49	3665.49	.00	2
3.24	6041.73	2350.58	3691.15	3691.15	.00	2
2.83	6092.14	2375.42	3716.72	3716.72	.00	2
2.42	6142.47	2400.27	3742.20	3742.20	.00	2
2.01	6192.71	2425.11	3767.59	3767.59	.00	2
1.61	6242.85	2449.96	3792.89	3792.89	.00	2
1.20	6292.90	2474.80	3818.10	3818.10	.00	2
.80	6342.87	2499.65	3843.22	3843.22	.00	2
.40	6392.74	2524.49	3868.24	3868.25	.00	2
.00	6442.52	2549.34	3893.18	3893.18	.00	2

Time = 3600. Degree of Consolidation = 136.%

Total Settlement = 1.001

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 3600. = .738

Settlement caused by Secondary Compression at time 3600. = .263

******************Current Conditions in Dredged Fill******************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

***** Stresses ***** ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 3600. Degree of Consolidation = 99.%
Total Settlement = .111
Settlement at End of Primary Consolidation = .112
Settlement caused by Primary Consolidation at time 3600. = .111
Settlement caused by Secondary Compression at time 3600. = .000
Surface Elevation = .39

*************Current Conditions in Compressible Foundation*************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Ecop	Material
60.00	58.96	22.30	2.00	1.85	1.85	6
57.92	56.94	21.59	1.87	1.83	1.83	6
55.88	54.93	20.88	1.86	1.81	1.81	6
53.85	52.93	20.17	1.84	1.80	1.80	6
51.83	50.94	19.45	1.82	1.78	1.78	6
49.83	48.97	18.74	1.80	1.76	1.76	6
47.84	47.01	18.03	1.79	1.74	1.74	6
45.86	45.06	17.32	1.77	1.73	1.73	6
43.89	43.12	16.60	1.75	1.71	1.71	6
41.93	41.20	15.89	1.73	1.69	1.69	6
39.99	39.29	15.18	1.72	1.67	1.67	6
39.99	39.29	15.18	1.82	1.80	1.80	5
36.73	36.05	14.02	1.81	1.79	1.79	5
33.48	32.82	12.86	1.79	1.78	1.78	5
30.25	29.61	11.70	1.78	1.76	1.76	5
27.03	26.41	10.54	1.77	1.75	1.75	5
23.82	23.22	9.38	1.76	1.74	1.74	5
20.63	20.05	8.22	1.75	1.73	1.73	5
17.45	16.89	7.06	1.74	1.72	1.72	5
14.29	13.75	5.90	1.72	1.71	1.71	5
11.14	10.62	4.74	1.71	1.69	1.69	5
8.00	7.50	3.58	1.70	1.68	1.68	5
8.00	7.50	3.58	1.09	1.08	1.08	4
7.67	7.17	3.42	1.09	1.08	1.08	4
7.33	6.84	3.26	1.09	1.08	1.08	4
7.00	6.51	3.10	1.09	1.08	1.08	4
6.67	6.17	2.94	1.09	1.08	1.08	4
6.33	5.84	2.78	1.09	1.08	1.08	4
6.00	5.51	2.62	1.08	1.08	1.08	4
6.00	5.51	2.62	2.73	2.55	2.56	3
5.66	5.19	2.53	2.72	2.54	2.55	3
5.33	4.87	2.44	2.71	2.53	2.54	3
5.00	4.56	2.35	2.71	2.53	2.54	3
4.66	4.24	2.26	2.70	2.52	2.53	3
4.33	3.92	2.17	2.69	2.51	2.52	3
4.00	3.61	2.08	2.68	2.50	2.51	3
4.00	3.61	2.08	.95	.76	.90	2
3.55	3.20	1.85	.94	.75	.90	2
3.10	2.80	1.62	.93	.75	.89	2
2.65	2.39	1.39	.93	.74	.88	2
2.21	1.99	1.16	.92	.74	.88	2
1.76	1.59	.93	.92	.73	.87	2

1.32	1.19	.69	.91	.72	.87	2
.88	.79	.46	.90	.72	.86	2
.44	.40	.23	.90	.71	.85	2
.00	.00	.00	.89	.70	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
58.96	399.01	187.51	211.50	211.50	.00	6
56.94	600.88	263.10	337.77	337.77	.00	6
54.93	801.96	338.69	463.27	463.27	.00	6
52.93	1002.27	414.28	587.99	587.99	.00	6
50.94	1201.79	489.86	711.93	711.93	.00	6
48.97	1400.54	565.45	835.09	835.09	.00	6
47.01	1598.50	641.04	957.46	957.46	.00	6
45.06	1795.69	716.63	1079.06	1079.06	.00	6
43.12	1992.09	792.21	1199.88	1199.88	.00	6
41.20	2187.72	867.80	1319.92	1319.92	.00	6
39.29	2382.57	943.39	1439.18	1439.18	.00	6
39.29	2382.57	943.39	1439.18	1439.18	.00	5
36.05	2707.78	1066.40	1641.37	1641.37	.00	5
32.82	3032.13	1189.42	1842.70	1842.70	.00	5
29.61	3355.62	1312.44	2043.18	2043.18	.00	5
26.41	3678.25	1435.46	2242.79	2242.80	.00	5
23.22	4000.03	1558.47	2441.55	2441.55	.00	5
20.05	4320.94	1681.49	2639.45	2639.45	.00	5
16.89	4641.00	1804.51	2836.50	2836.50	.00	5
13.75	4960.21	1927.53	3032.68	3032.68	.00	5
10.62	5278.55	2050.54	3228.01	3228.01	.00	5
7.50	5596.03	2173.56	3422.47	3422.47	.00	5
7.50	5596.03	2173.56	3422.47	3422.47	.00	4
7.17	5633.72	2190.51	3443.21	3443.21	.00	4
6.84	5671.39	2207.45	3463.94	3463.94	.00	4
6.51	5709.06	2224.40	3484.66	3484.66	.00	4
6.17	5746.72	2241.34	3505.37	3505.37	.00	4
5.84	5784.37	2258.29	3526.08	3526.08	.00	4
5.51	5822.01	2275.23	3546.78	3546.78	.00	4
5.51	5822.01	2275.24	3546.78	3546.78	.00	3
5.19	5850.32	2283.65	3566.67	3566.67	.00	3
4.87	5878.59	2292.07	3586.52	3586.52	.00	3
4.56	5906.81	2300.48	3606.32	3606.33	.00	3
4.24	5934.99	2308.90	3626.09	3626.09	.00	3
3.92	5963.13	2317.32	3645.81	3645.81	.00	3
3.61	5991.22	2325.73	3665.49	3665.49	.00	3

3.61	5991.22	2325.73	3665.49	3665.49	.00	2
3.20	6041.45	2350.58	3690.87	3690.87	.00	2
2.80	6091.59	2375.42	3716.17	3716.17	.00	2
2.39	6141.64	2400.27	3741.37	3741.37	.00	2
1.99	6191.60	2425.11	3766.48	3766.49	.00	2
1.59	6241.47	2449.96	3791.51	3791.51	.00	2
1.19	6291.24	2474.80	3816.44	3816.44	.00	2
.79	6340.92	2499.65	3841.27	3841.28	.00	2
.40	6390.52	2524.49	3866.02	3866.02	.00	2
.00	6440.02	2549.34	3890.68	3890.68	.00	2

Time = 5110. Degree of Consolidation = 141.%

Total Settlement = 1.041

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 5110. = .738

Settlement caused by Secondary Compression at time 5110. = .303

******************Current Conditions in Dredged Fill******************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

***** Stresses ***** ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1

1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 5110. Degree of Consolidation = 99.%
Total Settlement = .111
Settlement at End of Primary Consolidation = .112
Settlement caused by Primary Consolidation at time 5110. = .111
Settlement caused by Secondary Compression at time 5110. = .000
Surface Elevation = .35

**************Current Conditions in Compressible Foundation*************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
60.00	58.95	22.30	2.00	1.85	1.85	6
57.92	56.92	21.59	1.87	1.83	1.83	6
55.88	54.91	20.88	1.86	1.81	1.81	6
53.85	52.91	20.17	1.84	1.80	1.80	6
51.83	50.93	19.45	1.82	1.78	1.78	6
49.83	48.95	18.74	1.80	1.76	1.76	6
47.84	46.99	18.03	1.79	1.74	1.74	6
45.86	45.04	17.32	1.77	1.73	1.73	6
43.89	43.11	16.60	1.75	1.71	1.71	6
41.93	41.18	15.89	1.73	1.69	1.69	6
39.99	39.27	15.18	1.72	1.67	1.67	6
39.99	39.27	15.18	1.82	1.80	1.80	5
36.73	36.03	14.02	1.81	1.79	1.79	5
33.48	32.80	12.86	1.79	1.78	1.78	5
30.25	29.59	11.70	1.78	1.76	1.76	5
27.03	26.39	10.54	1.77	1.75	1.75	5
23.82	23.21	9.38	1.76	1.74	1.74	5
20.63	20.04	8.22	1.75	1.73	1.73	5
17.45	16.88	7.06	1.74	1.72	1.72	5
14.29	13.73	5.90	1.72	1.71	1.71	5
11.14	10.60	4.74	1.71	1.69	1.69	5
8.00	7.49	3.58	1.70	1.68	1.68	5
8.00	7.49	3.58	1.09	1.08	1.08	4
7.67	7.15	3.42	1.09	1.08	1.08	4
7.33	6.82	3.26	1.09	1.08	1.08	4
7.00	6.49	3.10	1.09	1.08	1.08	4
6.67	6.16	2.94	1.09	1.08	1.08	4

6.33	5.83	2.78	1.09	1.08	1.08	4
6.00	5.50	2.62	1.08	1.08	1.08	4
6.00	5.50	2.62	2.73	2.55	2.56	3
5.66	5.18	2.53	2.72	2.54	2.55	3
5.33	4.86	2.44	2.71	2.53	2.54	3
5.00	4.54	2.35	2.71	2.53	2.54	3
4.66	4.22	2.26	2.70	2.52	2.53	3
4.33	3.91	2.17	2.69	2.51	2.52	3
4.00	3.59	2.08	2.68	2.50	2.51	3
4.00	3.59	2.08	.95	.75	.90	2
3.55	3.19	1.85	.94	.75	.90	2
3.10	2.78	1.62	.93	.74	.89	2
2.65	2.38	1.39	.93	.73	.88	2
2.21	1.98	1.16	.92	.73	.88	2
1.76	1.58	.93	.92	.72	.87	2
1.32	1.18	.69	.91	.71	.87	2
.88	.79	.46	.90	.71	.86	2
.44	.39	.23	.90	.70	.85	2
.00	.00	.00	.89	.70	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
58.95	399.01	187.51	211.50	211.50	.00	6
56.92	600.88	263.10	337.77	337.77	.00	6
54.91	801.96	338.69	463.27	463.27	.00	6
52.91	1002.27	414.28	587.99	587.99	.00	6
50.93	1201.79	489.86	711.93	711.93	.00	6
48.95	1400.54	565.45	835.09	835.09	.00	6
46.99	1598.50	641.04	957.46	957.46	.00	6
45.04	1795.69	716.63	1079.06	1079.06	.00	6
43.11	1992.09	792.21	1199.88	1199.88	.00	6
41.18	2187.72	867.80	1319.92	1319.92	.00	6
39.27	2382.57	943.39	1439.18	1439.18	.00	6
39.27	2382.57	943.39	1439.18	1439.18	.00	5
36.03	2707.78	1066.40	1641.37	1641.37	.00	5
32.80	3032.13	1189.42	1842.70	1842.70	.00	5
29.59	3355.62	1312.44	2043.18	2043.18	.00	5
26.39	3678.25	1435.46	2242.79	2242.80	.00	5
23.21	4000.03	1558.47	2441.55	2441.55	.00	5
20.04	4320.94	1681.49	2639.45	2639.45	.00	5
16.88	4641.00	1804.51	2836.50	2836.50	.00	5
13.73	4960.20	1927.53	3032.68	3032.68	.00	5
10.60	5278.55	2050.54	3228.01	3228.01	.00	5

7.49	5596.03	2173.56	3422.47	3422.47	.00	5
7.49	5596.03	2173.56	3422.47	3422.47	.00	4
7.15	5633.72	2190.51	3443.21	3443.21	.00	4
6.82	5671.39	2207.45	3463.94	3463.94	.00	4
6.49	5709.06	2224.40	3484.66	3484.66	.00	4
6.16	5746.72	2241.34	3505.37	3505.37	.00	4
5.83	5784.37	2258.29	3526.08	3526.08	.00	4
5.50	5822.01	2275.23	3546.78	3546.78	.00	4
5.50	5822.01	2275.24	3546.78	3546.78	.00	3
5.18	5850.32	2283.65	3566.67	3566.67	.00	3
4.86	5878.59	2292.07	3586.52	3586.52	.00	3
4.54	5906.81	2300.48	3606.32	3606.32	.00	3
4.22	5934.99	2308.90	3626.09	3626.09	.00	3
3.91	5963.13	2317.32	3645.81	3645.81	.00	3
3.59	5991.22	2325.73	3665.49	3665.49	.00	3
3.59	5991.22	2325.73	3665.49	3665.49	.00	2
3.19	6041.34	2350.58	3690.76	3690.76	.00	2
2.78	6091.37	2375.42	3715.94	3715.95	.00	2
2.38	6141.31	2400.27	3741.04	3741.04	.00	2
1.98	6191.15	2425.11	3766.04	3766.04	.00	2
1.58	6240.91	2449.96	3790.95	3790.95	.00	2
1.18	6290.57	2474.80	3815.76	3815.77	.00	2
.79	6340.14	2499.65	3840.49	3840.49	.00	2
.39	6389.62	2524.49	3865.13	3865.13	.00	2
.00	6439.01	2549.34	3889.67	3889.67	.00	2

Time = 5840. Degree of Consolidation = 143.%
Total Settlement = 1.057
Settlement at End of Primary Consolidation = .738
Settlement caused by Primary Consolidation at time 5840. = .738
Settlement caused by Secondary Compression at time 5840. = .319

******************Current Conditions in Dredged Fill******************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1

1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 5840. Degree of Consolidation = 99.%
Total Settlement = .111
Settlement at End of Primary Consolidation = .112
Settlement caused by Primary Consolidation at time 5840. = .111
Settlement caused by Secondary Compression at time 5840. = .000
Surface Elevation = .33

**************Current Conditions in Compressible Foundation*************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
60.00	58.92	22.30	2.00	1.85	1.85	6
57.92	56.89	21.59	1.87	1.83	1.83	6
55.88	54.88	20.88	1.86	1.81	1.81	6
53.85	52.88	20.17	1.84	1.80	1.80	6
51.83	50.90	19.45	1.82	1.78	1.78	6
49.83	48.92	18.74	1.80	1.76	1.76	6
47.84	46.96	18.03	1.79	1.74	1.74	6
45.86	45.01	17.32	1.77	1.73	1.73	6
43.89	43.08	16.60	1.75	1.71	1.71	6
41.93	41.15	15.89	1.73	1.69	1.69	6
39.99	39.24	15.18	1.72	1.67	1.67	6
39.99	39.24	15.18	1.82	1.80	1.80	5

36.73	36.00	14.02	1.81	1.79	1.79	5
33.48	32.78	12.86	1.79	1.78	1.78	5
30.25	29.56	11.70	1.78	1.76	1.76	5
27.03	26.36	10.54	1.77	1.75	1.75	5
23.82	23.18	9.38	1.76	1.74	1.74	5
20.63	20.01	8.22	1.75	1.73	1.73	5
17.45	16.85	7.06	1.74	1.72	1.72	5
14.29	13.71	5.90	1.72	1.71	1.71	5
11.14	10.58	4.74	1.71	1.69	1.69	5
8.00	7.46	3.58	1.70	1.68	1.68	5
8.00	7.46	3.58	1.09	1.08	1.08	4
7.67	7.13	3.42	1.09	1.08	1.08	4
7.33	6.79	3.26	1.09	1.08	1.08	4
7.00	6.46	3.10	1.09	1.08	1.08	4
6.67	6.13	2.94	1.09	1.08	1.08	4
6.33	5.80	2.78	1.09	1.08	1.08	4
6.00	5.47	2.62	1.08	1.08	1.08	4
6.00	5.47	2.62	2.73	2.55	2.56	3
5.66	5.15	2.53	2.72	2.54	2.55	3
5.33	4.83	2.44	2.71	2.53	2.54	3
5.00	4.51	2.35	2.71	2.53	2.54	3
4.66	4.20	2.26	2.70	2.52	2.53	3
4.33	3.88	2.17	2.69	2.51	2.52	3
4.00	3.56	2.08	2.68	2.50	2.51	3
4.00	3.56	2.08	.95	.74	.90	2
3.55	3.16	1.85	.94	.73	.90	2
3.10	2.76	1.62	.93	.73	.89	2
2.65	2.36	1.39	.93	.72	.88	2
2.21	1.97	1.16	.92	.71	.88	2
1.76	1.57	.93	.92	.71	.87	2
1.32	1.17	.69	.91	.70	.87	2
.88	.78	.46	.90	.69	.86	2
.44	.39	.23	.90	.69	.85	2
.00	.00	.00	.89	.68	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
58.92	399.01	187.51	211.50	211.50	.00	6
56.89	600.88	263.10	337.77	337.77	.00	6
54.88	801.96	338.69	463.27	463.27	.00	6
52.88	1002.27	414.28	587.99	587.99	.00	6
50.90	1201.79	489.86	711.93	711.93	.00	6
48.92	1400.54	565.45	835.09	835.09	.00	6

46.96	1598.50	641.04	957.46	957.47	.00	6
45.01	1795.69	716.63	1079.06	1079.06	.00	6
43.08	1992.09	792.21	1199.88	1199.88	.00	6
41.15	2187.72	867.80	1319.92	1319.92	.00	6
39.24	2382.57	943.39	1439.18	1439.18	.00	6
39.24	2382.57	943.39	1439.18	1439.18	.00	5
36.00	2707.78	1066.40	1641.37	1641.37	.00	5
32.78	3032.13	1189.42	1842.70	1842.70	.00	5
29.56	3355.62	1312.44	2043.18	2043.18	.00	5
26.36	3678.25	1435.46	2242.79	2242.80	.00	5
23.18	4000.03	1558.47	2441.55	2441.55	.00	5
20.01	4320.94	1681.49	2639.45	2639.45	.00	5
16.85	4641.00	1804.51	2836.50	2836.50	.00	5
13.71	4960.21	1927.53	3032.68	3032.68	.00	5
10.58	5278.55	2050.54	3228.01	3228.01	.00	5
7.46	5596.03	2173.56	3422.47	3422.47	.00	5
7.46	5596.03	2173.56	3422.47	3422.47	.00	4
7.13	5633.72	2190.51	3443.21	3443.21	.00	4
6.79	5671.39	2207.45	3463.94	3463.94	.00	4
6.46	5709.06	2224.40	3484.66	3484.66	.00	4
6.13	5746.72	2241.34	3505.37	3505.37	.00	4
5.80	5784.37	2258.29	3526.08	3526.08	.00	4
5.47	5822.01	2275.23	3546.78	3546.78	.00	4
5.47	5822.01	2275.24	3546.78	3546.78	.00	3
5.15	5850.32	2283.65	3566.67	3566.67	.00	3
4.83	5878.59	2292.07	3586.52	3586.52	.00	3
4.51	5906.81	2300.48	3606.32	3606.33	.00	3
4.20	5934.99	2308.90	3626.09	3626.09	.00	3
3.88	5963.13	2317.32	3645.81	3645.81	.00	3
3.56	5991.22	2325.73	3665.49	3665.49	.00	3
3.56	5991.22	2325.73	3665.49	3665.49	.00	2
3.16	6041.15	2350.58	3690.57	3690.57	.00	2
2.76	6090.99	2375.42	3715.56	3715.57	.00	2
2.36	6140.73	2400.27	3740.47	3740.47	.00	2
1.97	6190.39	2425.11	3765.27	3765.27	.00	2
1.57	6239.95	2449.96	3789.99	3789.99	.00	2
1.17	6289.41	2474.80	3814.61	3814.61	.00	2
.78	6338.79	2499.65	3839.14	3839.14	.00	2
.39	6388.07	2524.49	3863.57	3863.58	.00	2
.00	6437.26	2549.34	3887.92	3887.92	.00	2

Time = 7200. Degree of Consolidation = 147.%

Total Settlement = 1.085

Settlement at End of Primary Consolidation = .738

Settlement caused by Primary Consolidation at time 7200. = .738
Settlement caused by Secondary Compression at time 7200. = .347

******************Current Conditions in Dredged Fill******************

***** Coordinates *****			***** Void Ratios *****			
A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1
1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 7200. Degree of Consolidation = 99.%
Total Settlement = .111
Settlement at End of Primary Consolidation = .112
Settlement caused by Primary Consolidation at time 7200. = .111
Settlement caused by Secondary Compression at time 7200. = .000
Surface Elevation = .30

**************Current Conditions in Compressible Foundation*************

***** Coordinates *****   ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
60.00	58.90	22.30	2.00	1.85	1.85	6
57.92	56.87	21.59	1.87	1.83	1.83	6
55.88	54.86	20.88	1.86	1.81	1.81	6
53.85	52.86	20.17	1.84	1.80	1.80	6
51.83	50.88	19.45	1.82	1.78	1.78	6
49.83	48.90	18.74	1.80	1.76	1.76	6
47.84	46.94	18.03	1.79	1.74	1.74	6
45.86	44.99	17.32	1.77	1.73	1.73	6
43.89	43.06	16.60	1.75	1.71	1.71	6
41.93	41.13	15.89	1.73	1.69	1.69	6
39.99	39.22	15.18	1.72	1.67	1.67	6
39.99	39.22	15.18	1.82	1.80	1.80	5
36.73	35.98	14.02	1.81	1.79	1.79	5
33.48	32.76	12.86	1.79	1.78	1.78	5
30.25	29.54	11.70	1.78	1.76	1.76	5
27.03	26.34	10.54	1.77	1.75	1.75	5
23.82	23.16	9.38	1.76	1.74	1.74	5
20.63	19.99	8.22	1.75	1.73	1.73	5
17.45	16.83	7.06	1.74	1.72	1.72	5
14.29	13.68	5.90	1.72	1.71	1.71	5
11.14	10.55	4.74	1.71	1.69	1.69	5
8.00	7.44	3.58	1.70	1.68	1.68	5
8.00	7.44	3.58	1.09	1.08	1.08	4
7.67	7.11	3.42	1.09	1.08	1.08	4
7.33	6.77	3.26	1.09	1.08	1.08	4
7.00	6.44	3.10	1.09	1.08	1.08	4
6.67	6.11	2.94	1.09	1.08	1.08	4
6.33	5.78	2.78	1.09	1.08	1.08	4
6.00	5.45	2.62	1.08	1.08	1.08	4
6.00	5.45	2.62	2.73	2.55	2.56	3
5.66	5.13	2.53	2.72	2.54	2.55	3
5.33	4.81	2.44	2.71	2.53	2.54	3
5.00	4.49	2.35	2.71	2.53	2.54	3
4.66	4.18	2.26	2.70	2.52	2.53	3
4.33	3.86	2.17	2.69	2.51	2.52	3
4.00	3.54	2.08	2.68	2.50	2.51	3
4.00	3.54	2.08	.95	.73	.90	2
3.55	3.14	1.85	.94	.72	.90	2
3.10	2.75	1.62	.93	.72	.89	2
2.65	2.35	1.39	.93	.71	.88	2
2.21	1.95	1.16	.92	.70	.88	2

1.76	1.56	.93	.92	.70	.87	2
1.32	1.17	.69	.91	.69	.87	2
.88	.78	.46	.90	.68	.86	2
.44	.39	.23	.90	.68	.85	2
.00	.00	.00	.89	.67	.85	2

	***** Stresses *****		***** Pore Pressures *****			
XI	Total	Effective	Total	Static	Excess	Material
58.90	399.01	187.51	211.50	211.50	.00	6
56.87	600.88	263.10	337.77	337.77	.00	6
54.86	801.96	338.69	463.27	463.27	.00	6
52.86	1002.27	414.28	587.99	587.99	.00	6
50.88	1201.79	489.86	711.93	711.93	.00	6
48.90	1400.54	565.45	835.09	835.09	.00	6
46.94	1598.50	641.04	957.46	957.46	.00	6
44.99	1795.69	716.63	1079.06	1079.06	.00	6
43.06	1992.09	792.21	1199.88	1199.88	.00	6
41.13	2187.72	867.80	1319.92	1319.92	.00	6
39.22	2382.57	943.39	1439.18	1439.18	.00	6
39.22	2382.57	943.39	1439.18	1439.18	.00	5
35.98	2707.78	1066.40	1641.37	1641.37	.00	5
32.76	3032.13	1189.42	1842.70	1842.70	.00	5
29.54	3355.62	1312.44	2043.18	2043.18	.00	5
26.34	3678.25	1435.46	2242.79	2242.80	.00	5
23.16	4000.03	1558.47	2441.55	2441.55	.00	5
19.99	4320.94	1681.49	2639.45	2639.45	.00	5
16.83	4641.00	1804.51	2836.50	2836.50	.00	5
13.68	4960.20	1927.53	3032.68	3032.68	.00	5
10.55	5278.55	2050.54	3228.01	3228.01	.00	5
7.44	5596.03	2173.56	3422.47	3422.47	.00	5
7.44	5596.03	2173.56	3422.47	3422.47	.00	4
7.11	5633.72	2190.51	3443.21	3443.21	.00	4
6.77	5671.39	2207.45	3463.94	3463.94	.00	4
6.44	5709.06	2224.40	3484.66	3484.66	.00	4
6.11	5746.72	2241.34	3505.37	3505.37	.00	4
5.78	5784.37	2258.29	3526.08	3526.08	.00	4
5.45	5822.01	2275.23	3546.78	3546.78	.00	4
5.45	5822.01	2275.24	3546.78	3546.78	.00	3
5.13	5850.32	2283.65	3566.67	3566.67	.00	3
4.81	5878.59	2292.07	3586.52	3586.52	.00	3
4.49	5906.81	2300.48	3606.32	3606.32	.00	3
4.18	5934.99	2308.90	3626.09	3626.09	.00	3
3.86	5963.13	2317.32	3645.81	3645.81	.00	3

3.54	5991.22	2325.73	3665.49	3665.49	.00	3
3.54	5991.22	2325.73	3665.49	3665.49	.00	2
3.14	6041.01	2350.58	3690.43	3690.43	.00	2
2.75	6090.70	2375.42	3715.28	3715.28	.00	2
2.35	6140.30	2400.27	3740.03	3740.03	.00	2
1.95	6189.81	2425.11	3764.69	3764.70	.00	2
1.56	6239.22	2449.96	3789.26	3789.27	.00	2
1.17	6288.55	2474.80	3813.74	3813.74	.00	2
.78	6337.78	2499.65	3838.13	3838.13	.00	2
.39	6386.91	2524.49	3862.42	3862.42	.00	2
.00	6435.96	2549.34	3886.62	3886.62	.00	2

Time = 9125. Degree of Consolidation = 150.%
Total Settlement = 1.106
Settlement at End of Primary Consolidation = .738
Settlement caused by Primary Consolidation at time 9125. = .738
Settlement caused by Secondary Compression at time 9125. = .368

******************Current Conditions in Dredged Fill******************

***** Coordinates ***** ***** Void Ratios *****

A	XI	Z	Einitial	E	Eeop	Material
3.50	3.39	1.77	.98	.98	.98	1
3.15	3.04	1.59	.98	.94	.94	1
2.80	2.70	1.41	.98	.93	.93	1
2.45	2.36	1.24	.98	.92	.92	1
2.10	2.02	1.06	.98	.92	.91	1
1.75	1.68	.88	.98	.91	.91	1
1.40	1.35	.71	.98	.91	.91	1
1.05	1.01	.53	.98	.91	.91	1
.70	.67	.35	.98	.90	.90	1
.35	.34	.18	.98	.90	.90	1
.00	.00	.00	.98	.90	.90	1

***** Stresses ***** ***** Pore Pressures *****

XI	Total	Effective	Total	Static	Excess	Material
3.39	.00	.00	.00	.00	.00	1
3.04	40.33	15.43	24.90	21.58	3.32	1
2.70	80.39	36.61	43.78	42.89	.89	1
2.36	120.36	49.52	70.83	64.11	6.73	1
2.02	160.27	71.35	88.92	85.26	3.66	1
1.68	200.13	89.50	110.63	106.37	4.26	1

1.35	239.95	108.27	131.68	127.44	4.23	1
1.01	279.75	131.26	148.49	148.49	.00	1
.67	319.53	150.01	169.51	169.51	.00	1
.34	359.28	168.76	190.52	190.52	.00	1
.00	399.01	187.52	211.50	211.50	.00	1

Time = 9125. Degree of Consolidation = 99.%

Total Settlement = .111

Settlement at End of Primary Consolidation = .112

Settlement caused by Primary Consolidation at time 9125. = .111

Settlement caused by Secondary Compression at time 9125. = .000

Surface Elevation = .28

APPENDIX C

State of Louisiana

KATHLEEN BABINEAUX BLANCO
GOVERNOR

SCOTT A. ANGELLE
SECRETARY

DEPARTMENT OF NATURAL RESOURCES
OFFICE OF COASTAL RESTORATION AND MANAGEMENT

May 10, 2007

Mr. Charles L. Eustis, P.E.
Louis J. Capozzoli and Associates, Inc.
10555 Airline Highway
Baton Rouge, LA 70816

Re: LDNR Contract No. 2503-05-44, "Engineering Assistance for Coastal Restoration Projects"
Geotechnical Investigation and Analysis for Bayou Dupont
Background Geotechnical Data

Dear Mr. Eustis:

Enclosed is the background data for the five (5) existing borings near the marsh creation area for the Bayou Dupont project (Borings 4-8). Please note that Boring 8 is outside of the project boundaries and therefore may be disregarded. Please call me at 342-9419 if there are any questions.

Sincerely,

Whitney C. Thompson

Whitney C. Thompson, E.I.
Project Engineer

Enclosures

COASTAL ENGINEERING DIVISION
P. O. BOX 44027 • BATON ROUGE, LA 70804-4027 • 617 N. THIRD STREET • 10TH FLOOR • BATON ROUGE, LA 70802
PHONE (225) 342-7308 • FAX (225) 342-9417 • WEB http://www.dnr.state.la.us
AN EQUAL OPPORTUNITY EMPLOYER

LEGEND AND NOTES FOR LOG OF BORING AND TEST RESULTS

PP	Pocket penetrometer: Resistance in tons per square foot
SPT	Standard Penetration Test: Number of blows of a 140-lb hammer dropped 30 inches required to drive 2-in. O.D., 1.4-in. I.D. sampler a distance of 1 foot into the soil after first seating it 6 inches
SPLR	Type of Sampling — Shelby, SPT, Auger, No sample
SYMBOL	Clay, Silt, Sand, Peat/Humus, Shells, Stone/Gravel Predominant type shown heavy; Modifying type shown light
USC	Unified Soil Classification
DENSITY	Unit weight in pounds per cubic foot

SHEAR TESTS

TYPE

- UC Unconfined compression shear
- OB Unconsolidated undrained triaxial compression shear on one specimen confined at the approximate overburden pressure
- UU Unconsolidated undrained triaxial compression shear
- CU Consolidated undrained triaxial compression shear
- DS Direct shear

Ø Angle of internal friction in degrees

c Cohesion in pounds per square foot

ATTERBERG LIMITS

- LL Liquid Limit
- PL Plastic Limit
- PI Plasticity Index

OTHER TESTS

- CON Consolidation
- PD Particle size distribution (sieve and/or hydrometer)
- k Coefficient of permeability in centimeters per second
- SP Swelling pressure in pounds per square foot

Other laboratory test results reported on separate figures

GENERAL NOTES

(1) If a ground water depth is shown on the boring log, these observations were made at the time of drilling and were measured below the existing ground surface. These observations are shown on the boring logs. However, ground water levels may vary due to seasonal fluctuations and other factors. If important to construction, the depth to ground water should be determined by those persons responsible for construction immediately prior to beginning work.

(2) While the individual logs of borings are considered to be representative of subsurface conditions at their respective locations on the dates shown, it is not warranted that they are representative of subsurface conditions at other locations and times.

EUSTIS ENGINEERING COMPANY, INC.

LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM
BAYOU DUPONT, LOUISIANA

(Sheet 1 of 1)

Ground Elev.: -1.3 **Datum:** NAVD 88 **Gr. Water Depth:** N/A **Job No.:** 19183 **Date Drilled:** 4/03/06 **Boring:** 4

Refer to "Legends & Notes"

Scale In Feet	PP	SPT	SPLR	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density Dry	Density Wet	Shear Tests Type	Shear Tests ø	Shear Tests C	Atterberg Limits LL	Atterberg Limits PL	Atterberg Limits PI		Other Tests
0	0.00				Very soft gray clay	CH	1	1-2	87	50	93	OB	0	50	109	29	80		
	0.00						2	3-4											
	0.00				Very soft gray organic clay	OH	3	5-6	262	20	74	UC	–	125	267	88	179		
	0.50				Loose gray sandy silt	ML	4	7.0-8							32	25	7		CON
10	0.00				Very soft gray clay w/sandy silt lenses & pockets	CH	5	9-10	76	55	98	OB	0	58					
	0.00				w/sand pockets & shell fragments		6	11-12											
	0.50						7	13-14	63	63	102	UC	–	148					
	0.25						8	15-16											
	0.00						9	17-18											
20	0.25						10	19-20											
	0.50						11	24-25	79	53	95	UC	–	173					
30	0.75						12	29-30											
	0.50						13	34-35											
40	0.50						14	39-40											
50																			

Comments: Estimated water surface at el 0.2.
N 29° 39.078'; 90° 01.346'

EUSTIS ENGINEERING COMPANY, INC.

LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM
BAYOU DUPONT, LOUISIANA

Ground Elev.: -1.8 **Datum:** NAVD 88 **Gr. Water Depth:** N/A **Job No.:** 19183 **Date Drilled:** 3/30/06 **Boring:** 5

Refer to "Legends & Notes"

PP	SPT	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density Dry	Density Wet	Shear Tests Type	ø	C	Atterberg Limits LL	PL	PI		Other Tests
0.00		Very soft black humus	Pt	1	1-2											
0.00				2	3-4	664	9	66	OB	0	53.2	672	245	427		
0.00		Very soft gray clay w/silt lenses & pockets	CH	3	5-6											
0.25				4	7-8	71	56	96	UC	--	65	150	47	103		
0.00				5	9-10											
0.25		Loose to medium compact gray clayey silt w/ clay layers	ML	6	11-12	34	88	117	OB	0	355	48	21	27		
0.25				7	13-14											
0.75				8	15-16											
0.00		Medium compact gray sandy silt w/clay layers	ML	9	17-18	24	98	122	OB	0	1658	35	26	9		CON
0.25				10	19-20											
0.25		Very soft gray clay	CH	11	24-25	82	52	94	UC	--	160					
0.25		w/sandy silt lenses & layers		12	29-30											
0.25				13	34-35	63	61	99	UC	--	180					
0.50		Soft gray clay w/sand lenses	CH	14	39-40											
0.75				15	44-45	60	63	101	OB	0	392					
0.75				16	49-50											

Comments: Estimated water surface at el 0.2.
N 29° 38.887'; W 90° 00.743'

EUSTIS ENGINEERING COMPANY, INC.

LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM
BAYOU DUPONT, LOUISIANA

Ground Elev.: -1.8 **Datum:** NAVD 88 **Gr. Water Depth:** N/A **Job No.:** 19183 **Date Drilled:** 3/30/06 **Boring:** 5

Refer to "Legends & Notes"

Scale In Feet	PP	SPT	SPLR	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits				Other Tests
										Dry	Wet	Type	ø	C	LL	PL	PI		
50					Soft gray clay	CH													
	0.50						17	54-55	64	63	102	UC	--	395					
					Medium stiff gray clay	CH													
60	0.75						18	59-60											
70																			
80																			
90																			
100																			

Comments: Estimated water surface at el 0.2.
N 29° 38.887'; W 90° 00.743'

EUSTIS ENGINEERING COMPANY, INC.

LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM
BAYOU DUPONT, LOUISIANA

Ground Elev.: -0.3 **Datum:** NAVD 88 **Gr. Water Depth:** N/A **Job No.:** 19183 **Date Drilled:** 4/03/06 **Boring:** 6 **Refer to "Legends & Notes"**

Scale In Feet	PP	SPT	SPLR	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits				Other Tests
										Dry	Wet	Type	ø	C	LL	PL	PI		
0	0.00				Very soft dark brown humus	Pt	1	1-2											
	0.75				Very soft gray clay w/silt pockets	CH	2	3-4	63	61	100	UC	–	145	88	27	61		CON
	1.00						3	5-6											
	0.75						4	7-8	63	62	100	UC	–	215	81	23	58		
10	0.50						5	9-10											
	1.25				Medium compact gray clayey silt w/clay lenses	ML	6	11-12	38	83	115	OB	0	552					
	1.25						7	13-14											
	0.50				Very soft gray clay w/silt lenses & layers	CH	8	15-16	57	65	102	OB	0	150	68	28	40		
	0.50						9	17-18											
20	0.75						10	19-20											
	0.50				w/sandy silt pockets		11	24-25											
30	1.00						12	29-30	64	61	100	UC	--	233					
	0.75						13	34-35											
40	0.75						14	39-40											
50																			

Comments: Estimated water surface at el 0.2.
N 29° 39.418'; W 90° 00.719'

EUSTIS ENGINEERING COMPANY, INC.

LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM
BAYOU DUPONT, LOUISIANA

(Sheet 1 of 1)

Ground Elev.: -2.7 **Datum:** NAVD 88 **Gr. Water Depth:** N/A **Job No.:** 19183 **Date Drilled:** 4/03/06 **Boring:** 7

Refer to "Legends & Notes"

Scale In Feet	PP	SPT	SPLR	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits				Other Tests
										Dry	Wet	Type	ø	C	LL	PL	PI		
0	0.00				Very soft black humus	Pt	1	1-2	700	8	64	OB	0	64					
	0.00						2	3-4											
	1.50				Medium compact gray clayey silt	ML	3	5-6											
	0.75						4	7-8	29	93	120	OB	0	616	36	31	5		
10	0.50						5	9-10											
	0.00						6	11-12											
	0.00				Very soft gray clay w/sand lenses & pockets	CH	7	13-14	79	54	97	OB	0	100	80	25	55		
	0.50						8	15-16											
	0.00				w/sand pockets		9	17-18											
20	0.50						10	19-20	74	57	99	UC	–	95					
	0.25						11	24-25											
30	0.50				w/sand lenses & pockets		12	29-30											
	0.75						13	34-35	69	55	96	UC	–	98	74	22	52		CON
					Medium stiff gray clay	CH													
40	1.00						14	39-40											
50																			

Comments: Estimated water surface at el 0.2.
N 29° 38.889'; W 90° 00.290'

EUSTIS ENGINEERING COMPANY, INC.

LOG OF BORING AND TEST RESULTS

STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM
BAYOU DUPONT, LOUISIANA

Ground Elev.: -1.8 **Datum:** NAVD 88 **Gr. Water Depth:** N/A **Job No.:** 19183 **Date Drilled:** 3/31/06 **Boring:** 8

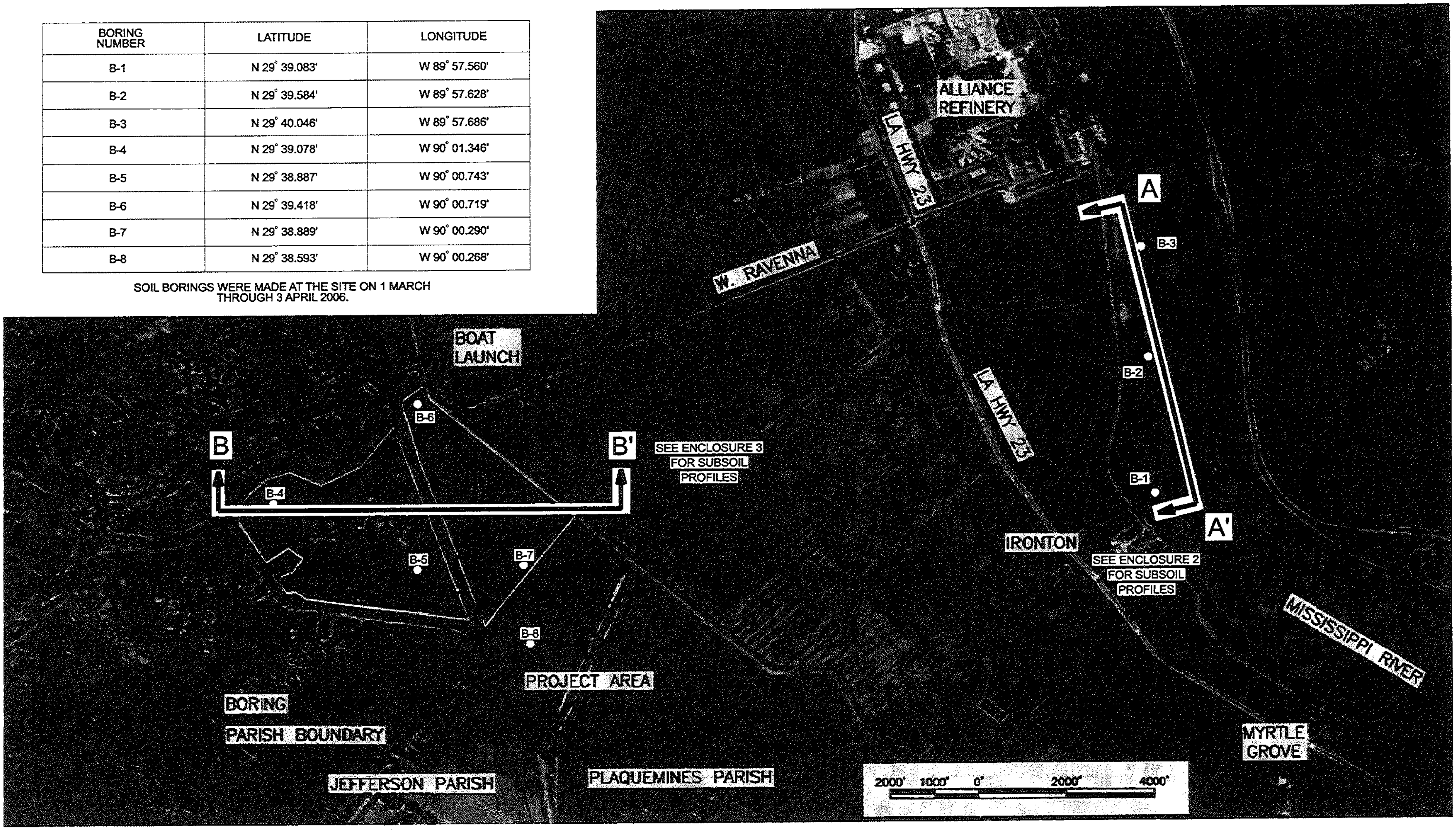
Refer to "Legends & Notes"

Scale In Feet	PP	SPT	SPLR	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits				Other Tests
										Dry	Wet	Type	ø	C	LL	PL	PI		
0	0.00				Very soft black humus	Pt	1	1-2	528	10	65	UC	--	13					
	0.00						2	3-4											
	0.00						3	5-6											
	0.00				Very soft dark gray humus w/roots and clay layers	Pt	4	7-8	208	25	76	OB	0	57	224	67	157		CON
	0.00						5	9-10											
10	0.50				Very loose gray clayey silt	ML	6	11-12											
	0.00						7	13-14	38	83	115	OB	0	221					
	0.00				Very soft gray clay	CH	8	15-16											
	0.00				w/sand lenses & pockets		9	17-18											
20	0.00						10	19-20	81	52	93	UC	--	155					
	0.00						11	24-25											
30	0.25						12	29-30	57	64	100	UC	--	130					
	0.50						13	34-35											
40	0.50						14	39-40											
50																			

Comments: Estimated water surface at el 0.2.
N 29° 38.593'; W 90° 00.268'

BORING NUMBER	LATITUDE	LONGITUDE
B-1	N 29° 39.083'	W 89° 57.560'
B-2	N 29° 39.584'	W 89° 57.628'
B-3	N 29° 40.046'	W 89° 57.686'
B-4	N 29° 39.078'	W 90° 01.346'
B-5	N 29° 38.887'	W 90° 00.743'
B-6	N 29° 39.418'	W 90° 00.719'
B-7	N 29° 38.889'	W 90° 00.290'
B-8	N 29° 38.593'	W 90° 00.268'

SOIL BORINGS WERE MADE AT THE SITE ON 1 MARCH THROUGH 3 APRIL 2006.

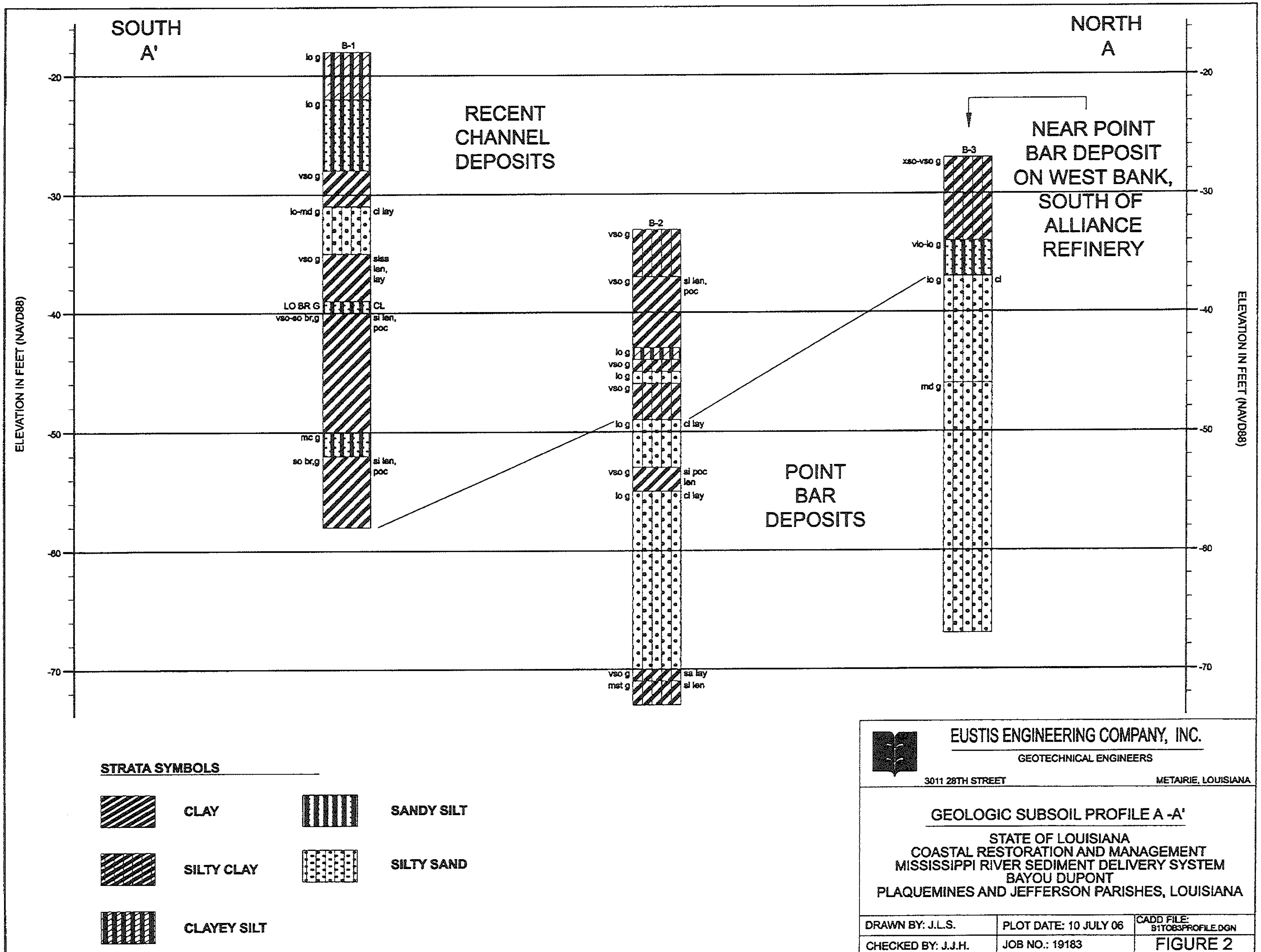


REV	DATE	DESCRIPTION	REV	DATE	DESCRIPTION

EUSTIS ENGINEERING COMPANY, INC.
GEOTECHNICAL ENGINEERING & CQC SERVICES
3011 28TH STREET METAIRIE, LOUISIANA

LOCATION OF BORINGS AND GEOLOGIC SUBSOIL PROFILES
STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM
BAYOU DUPONT
PLAQUEMINES AND JEFFERSON PARISHES, LOUISIANA

DRAWN BY: J.L.S.
CHECKED BY: J.J.H.
SCALE: AS SHOWN
FILE: BORINGS.DGN
DATE: 10 JULY 06
JOB NO. 19183
FIGURE 1

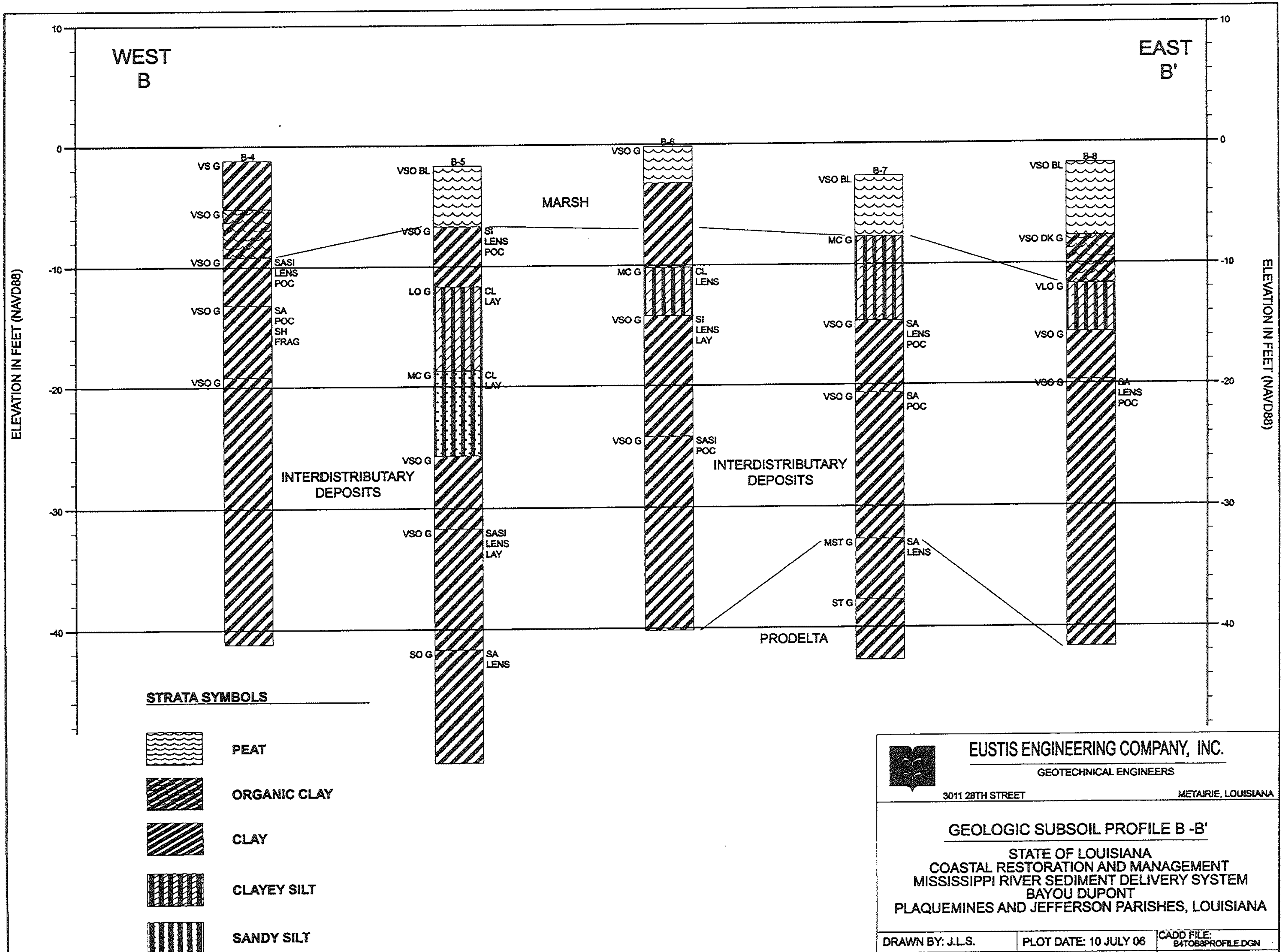


EUSTIS ENGINEERING COMPANY, INC.
GEOTECHNICAL ENGINEERS
3011 28TH STREET METAIRIE, LOUISIANA

GEOLOGIC SUBSOIL PROFILE A -A'

STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM
BAYOU DUPONT
PLAQUEMINES AND JEFFERSON PARISHES, LOUISIANA

DRAWN BY: J.L.S.	PLOT DATE: 10 JULY 06	CADD FILE: B1TOB3PROFILE.DGN
CHECKED BY: J.J.H.	JOB NO.: 19183	FIGURE 2

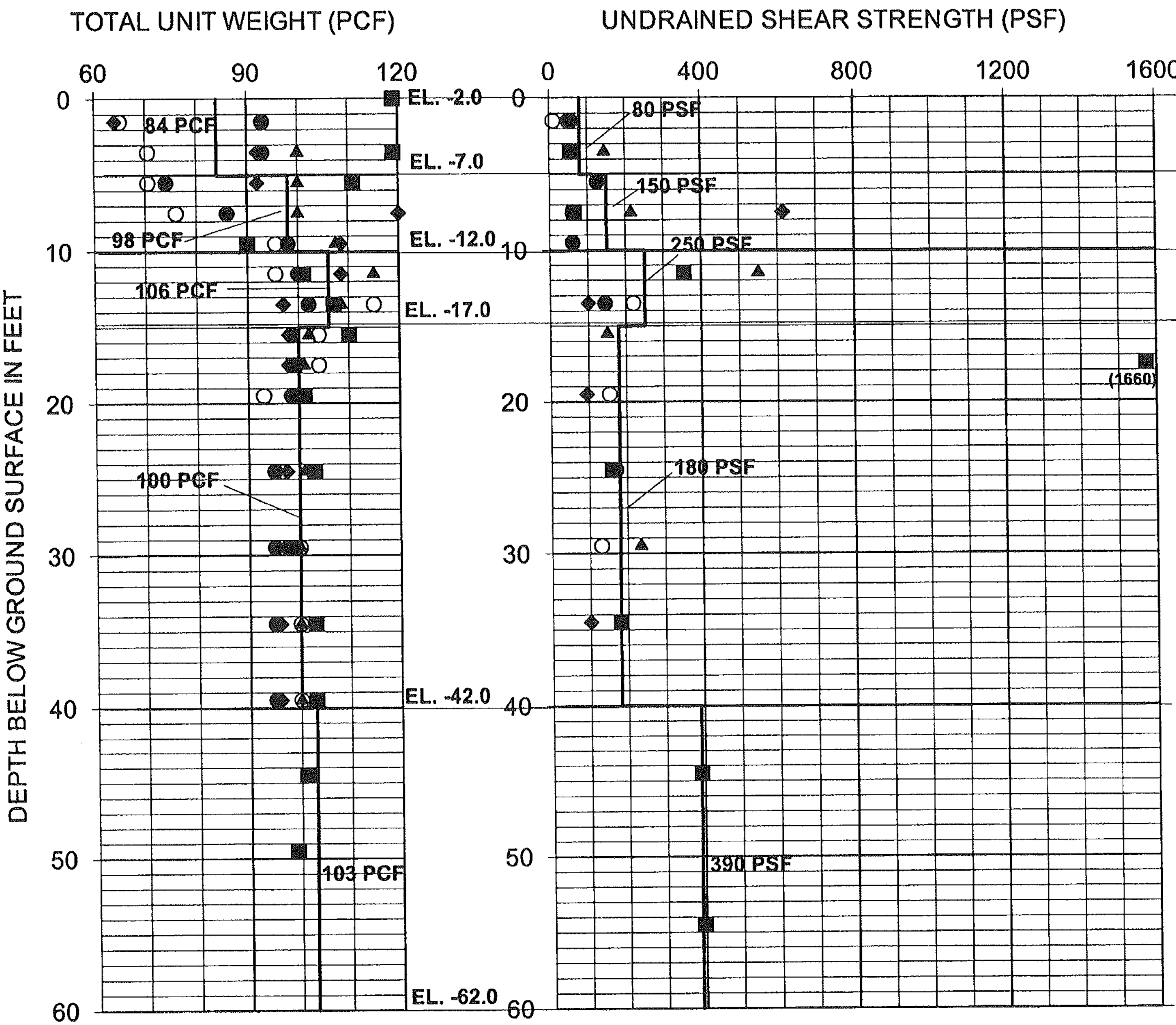


EUSTIS ENGINEERING COMPANY, INC.
GEOTECHNICAL ENGINEERS
3011 28TH STREET METAIRIE, LOUISIANA

GEOLOGIC SUBSOIL PROFILE B -B'

STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM
BAYOU DUPONT
PLAQUEMINES AND JEFFERSON PARISHES, LOUISIANA

DRAWN BY: J.L.S.	PLOT DATE: 10 JULY 06	CADD FILE: B4TOB8PROFILE.DGN
CHECKED BY: J.J.H.	JOB NO.: 19183	FIGURE 3



NOTE: THE AVERAGE ELEVATION WITHIN THE PROPOSED MARSH CREATION AREAS IS APPROXIMATELY EL -2 (NAVD88) BASED ON FURNISHED TOPOGRAPHIC DATA.

LEGEND

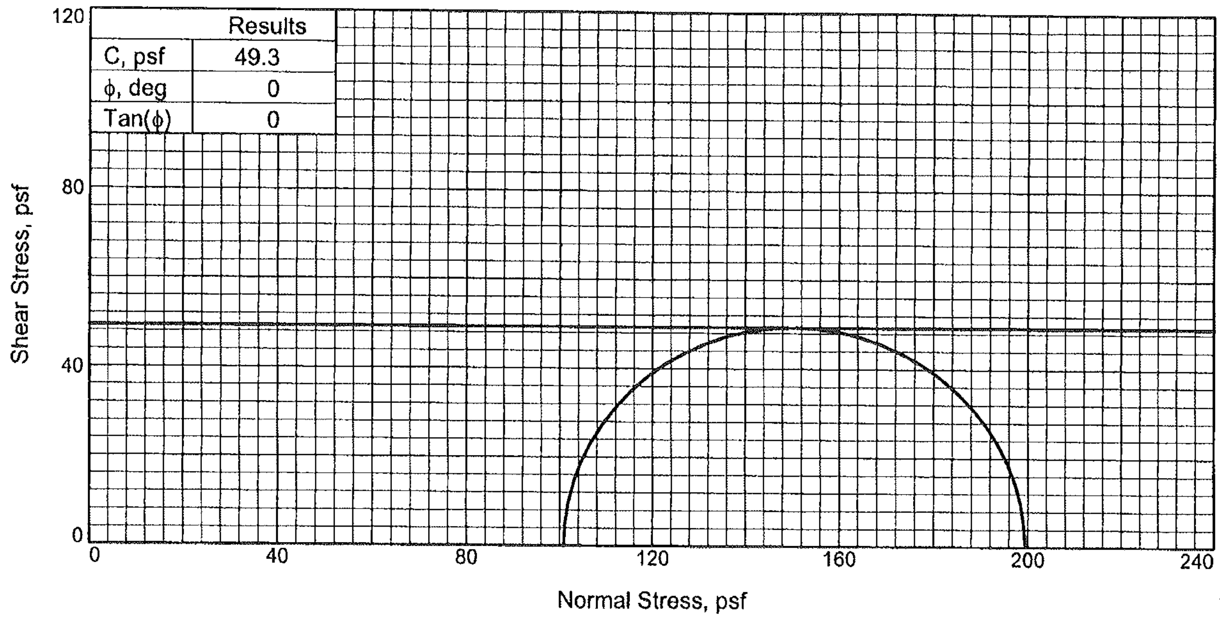
- ● BORING 4
- ■ BORING 5
- ▲ BORING 6
- ◆ BORING 7
- ○ BORING 8

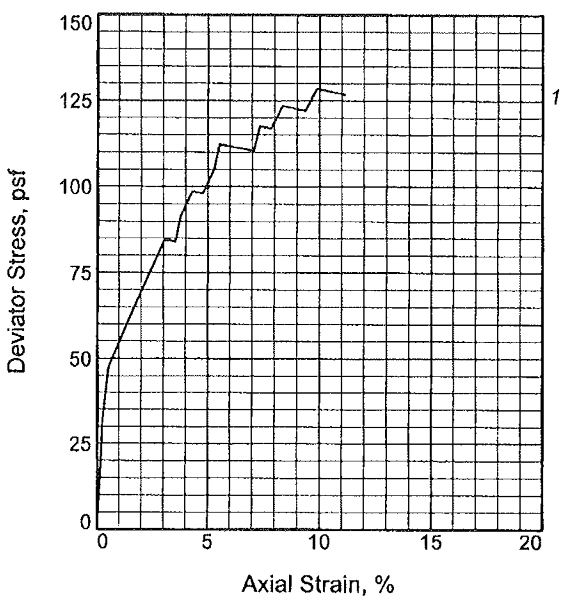
EUSTIS ENGINEERING COMPANY, INC.
GEOTECHNICAL ENGINEERS
3011 28TH STREET METAIRIE, LOUISIANA

SOIL DESIGN PARAMETERS
BORINGS 4 THROUGH 8

STATE OF LOUISIANA
DEPARTMENT OF NATURAL RESOURCES
COASTAL RESTORATION AND MANAGEMENT
MISSISSIPPI RIVER SEDIMENT DELIVERY SYSTEM
BAYOU DUPONT, LOUISIANA

DRAWN BY: D.B.P.	18 JULY 2005	Soil Parameters_Figure 4.grf
CHECKED BY: J.J.H.	JOB NO.: 19183	FIGURE 4





	Specimen No.	1
Initial	Water Content,	87.0
	Dry Density, pcf	49.7
	Saturation,	97.7
	Void Ratio	2.4388
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	89.0
	Dry Density, pcf	49.7
	Saturation,	100.0
	Void Ratio	2.4388
	Diameter, in.	1.391
	Height, in.	2.929
	Strain rate, in./min.	0.029
	Back Pressure, psf	0.0
	Cell Pressure, psf	100.8
	Fail. Stress, psf	98.6
	Ult. Stress, psf	126.7
	σ_1 Failure, psf	199.4
	σ_3 Failure, psf	100.8

Type of Test:
Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: XSO G CL W/ RTS

Assumed Specific Gravity= 2.74
Remarks: TORVANE = 0.100 TSF

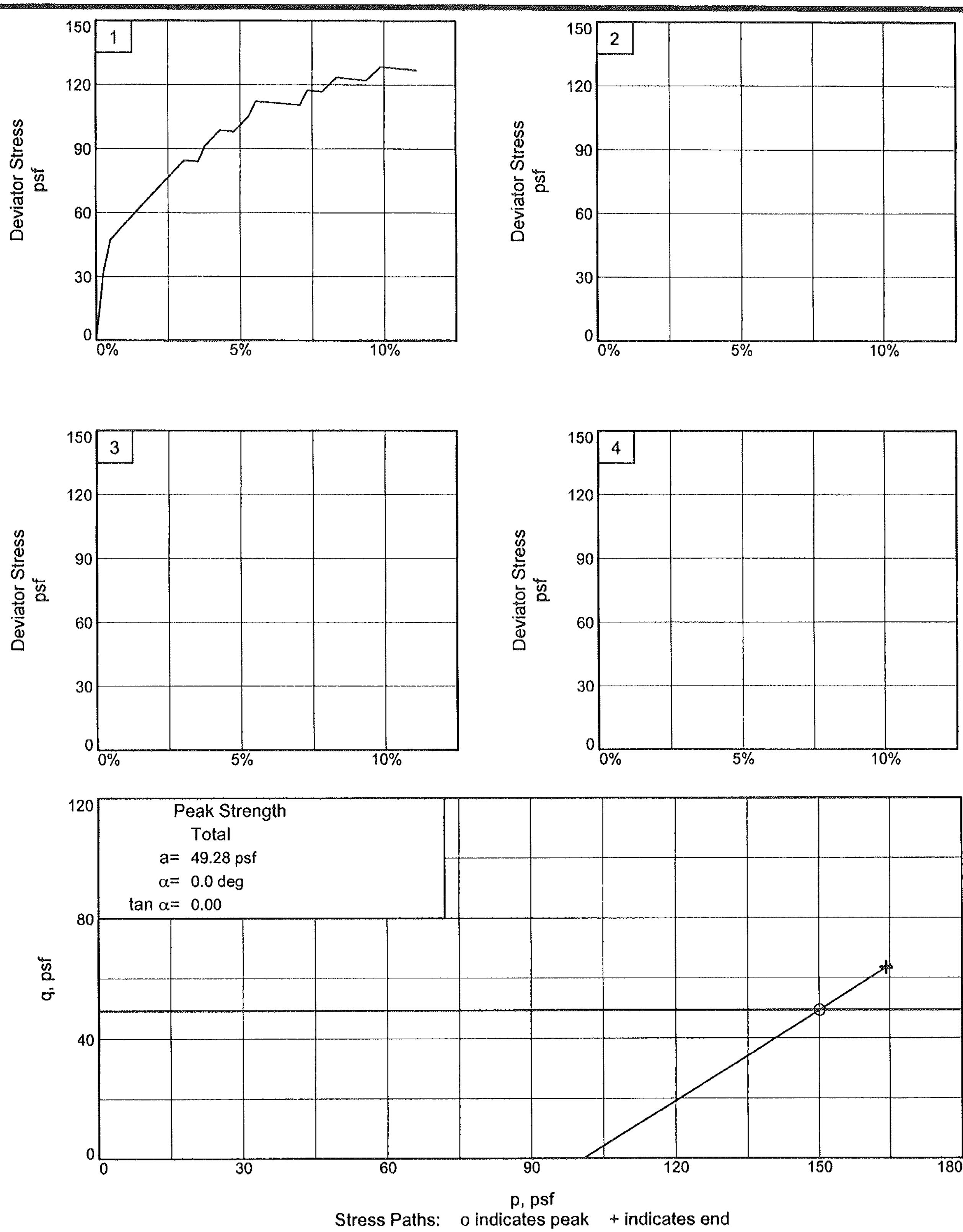
Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 4 **Depth:** 1
Sample Number: 1
Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT
EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD



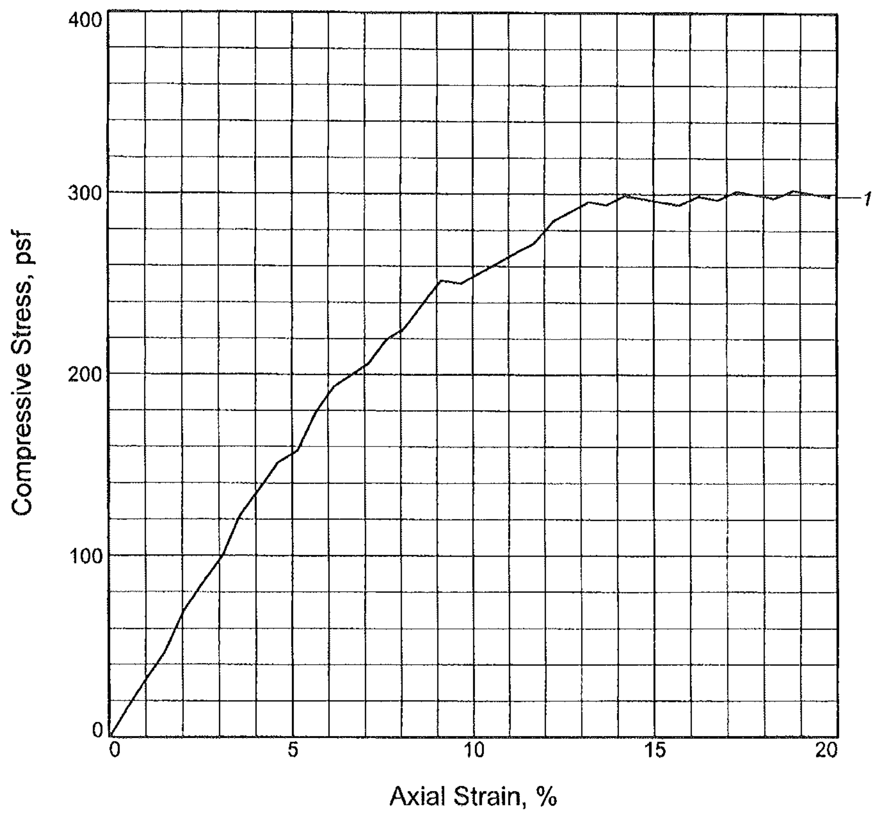
Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 4 **Depth:** 1 **Sample Number:** 1
Project No.: 19183

Figure 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	252.0			
Undrained shear strength, psf	126.0			
Failure strain, %	9.1			
Strain rate, in./min.	0.058			
Water content, %	262.3			
Wet density, pcf	73.8			
Dry density, pcf	20.4			
Saturation, %	97.8			
Void ratio	6.9740			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: VSO G & DK G ORG CL W/ RTS

LL =	PL =	PI =	Assumed GS= 2.60	Type: UNDISTURBED

Project No.: 19183
Date:
Remarks:
TORVANE = 0.130 TSF

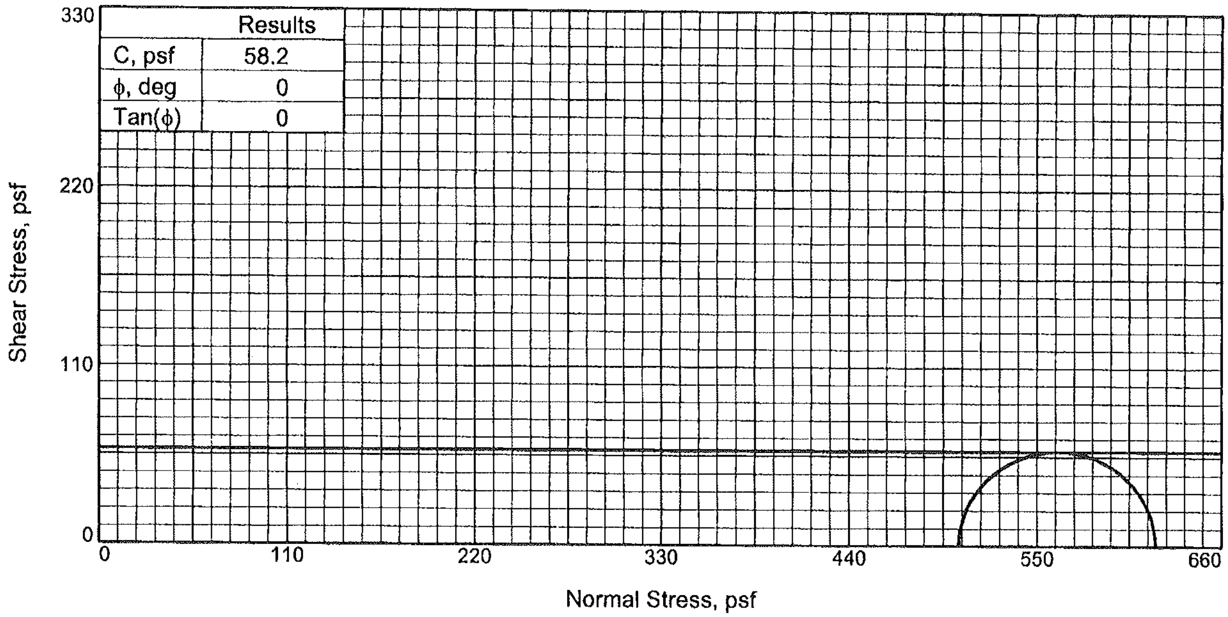
Figure 1

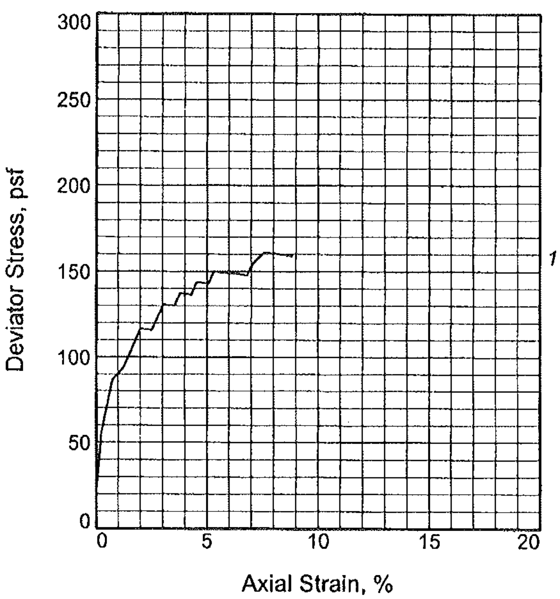
Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 4 **Depth:** 5
Sample Number: 3

UNCONFINED COMPRESSION TEST
EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD





Specimen No.		1
Initial	Water Content,	76.0
	Dry Density, pcf	55.4
	Saturation,	100.2
	Void Ratio	2.0641
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	75.9
	Dry Density, pcf	55.4
	Saturation,	100.0
	Void Ratio	2.0641
	Diameter, in.	1.391
	Height, in.	2.929
Strain rate, in./min.		0.029
Back Pressure, psf		0.0
Cell Pressure, psf		504.0
Fail. Stress, psf		116.4
Ult. Stress, psf		158.9
σ_1 Failure, psf		620.4
σ_3 Failure, psf		504.0

Type of Test:
Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: XSO G CL W/ SASI LEN & POC

Assumed Specific Gravity= 2.72

Remarks: TORVANE = 0.080 TSF

Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source of Sample: 4 **Depth:** 9

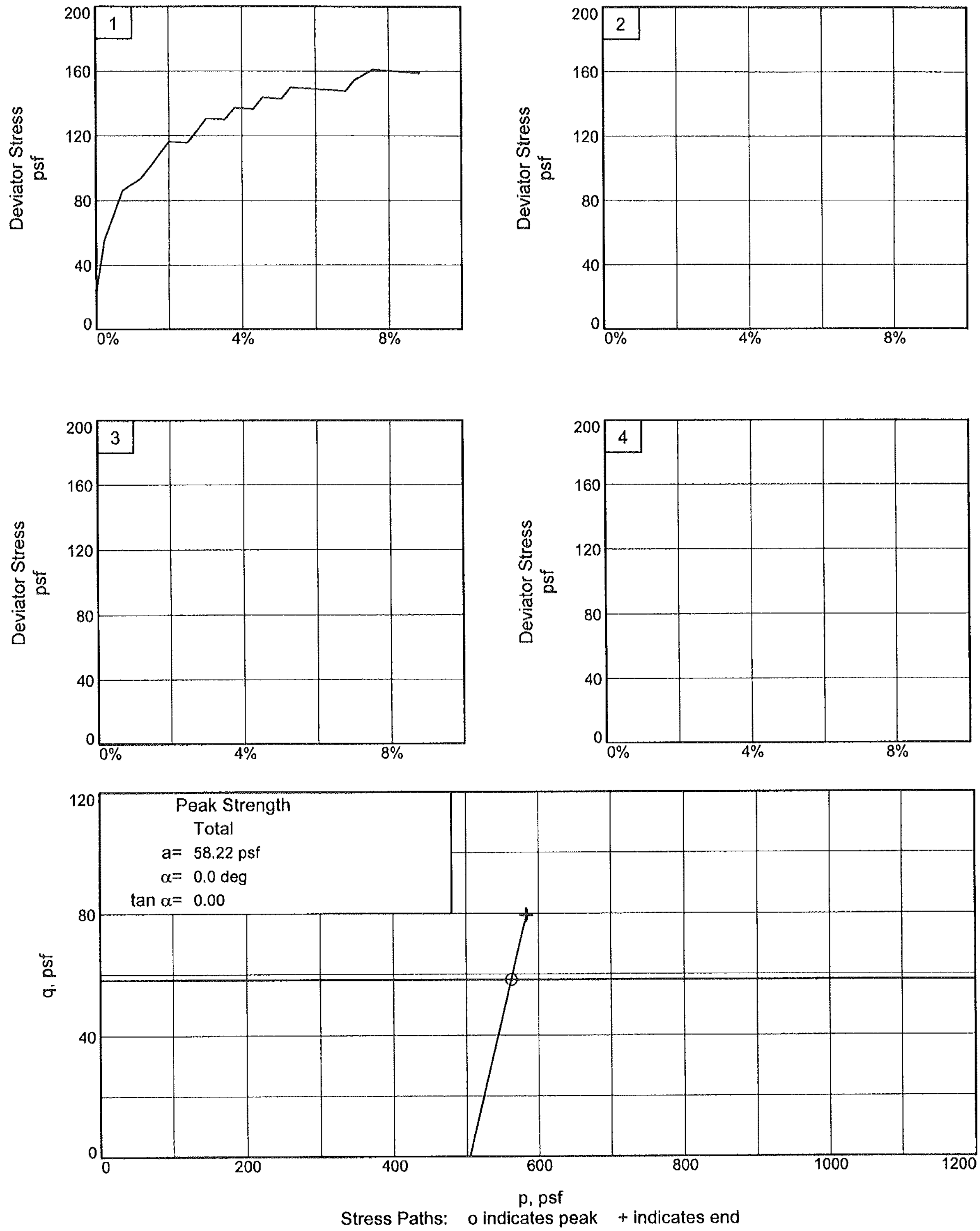
Sample Number: 5

Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD



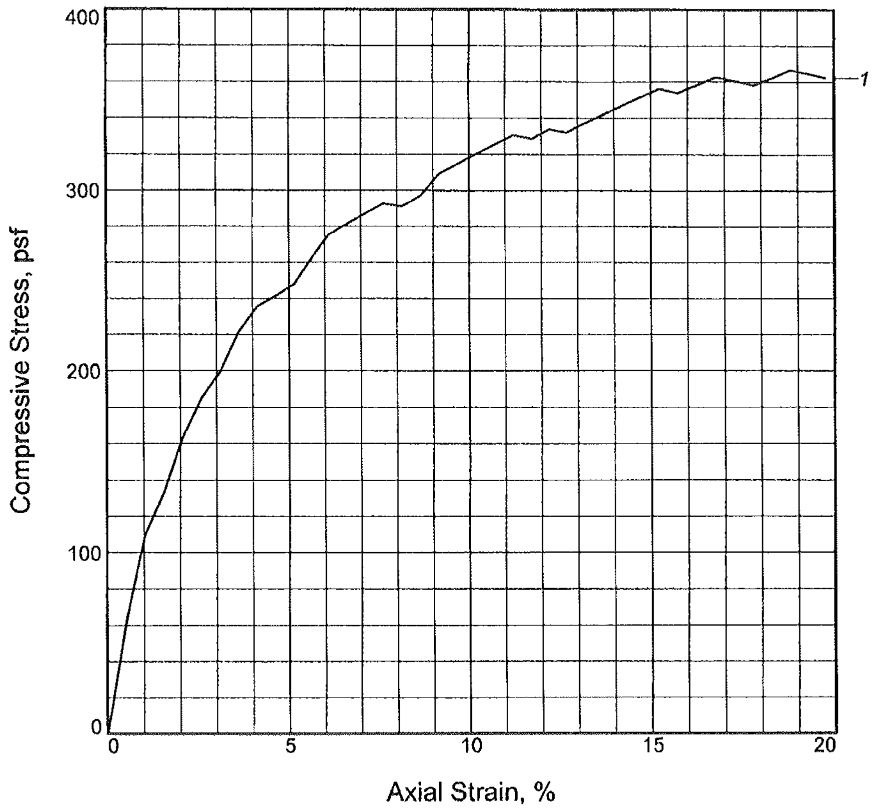
Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 4 **Depth:** 9 **Sample Number:** 5
Project No.: 19183

Figure 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	292.8			
Undrained shear strength, psf	146.4			
Failure strain, %	7.6			
Strain rate, in./min.	0.055			
Water content, %	62.9			
Wet density, pcf	102.3			
Dry density, pcf	62.8			
Saturation, %	100.0			
Void ratio	1.7219			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: VSO G CL W/ SA POC, SH FRAG

LL =	PL =	PI =	Assumed GS= 2.74	Type: UNDISTURBED

Project No.: 19183

Date:

Remarks:
TORVANE = 0.160 TSF

Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source of Sample: 4 **Depth:** 13

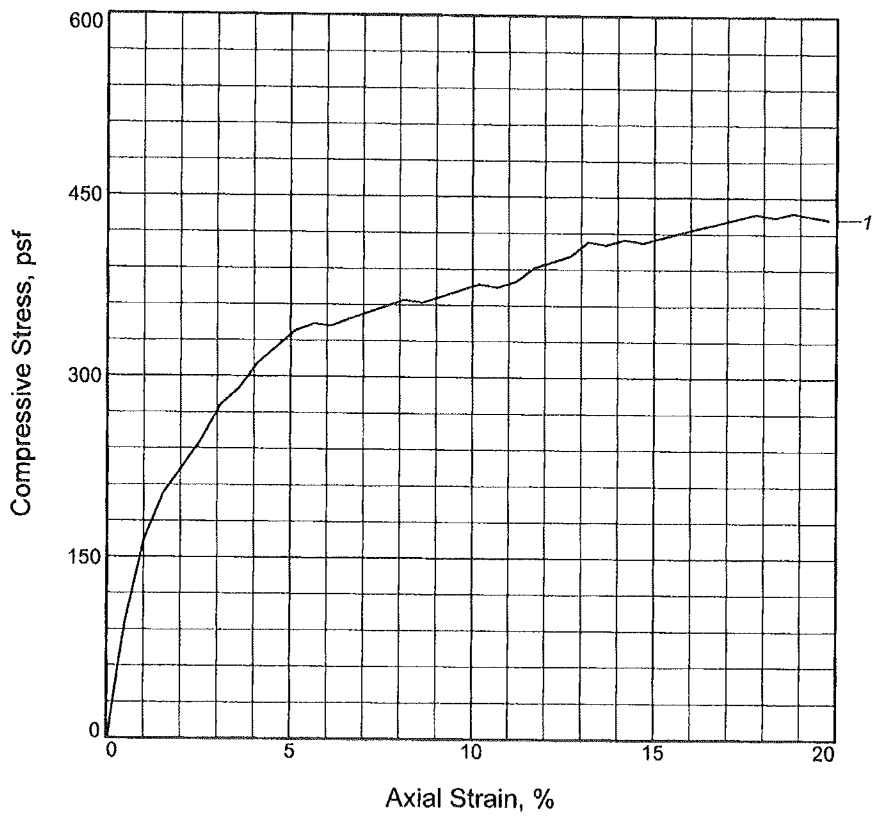
Sample Number: 7

UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	343.9			
Undrained shear strength, psf	171.9			
Failure strain, %	5.6			
Strain rate, in./min.	0.058			
Water content, %	78.5			
Wet density, pcf	94.9			
Dry density, pcf	53.1			
Saturation, %	96.9			
Void ratio	2.2184			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: VSO G CL

LL =	**PL =**	**PI =**	**Assumed GS=** 2.74	**Type:** UNDISTURBED

Project No.: 19183

Date:

Remarks:
TORVANE = 0.160 TSF

Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

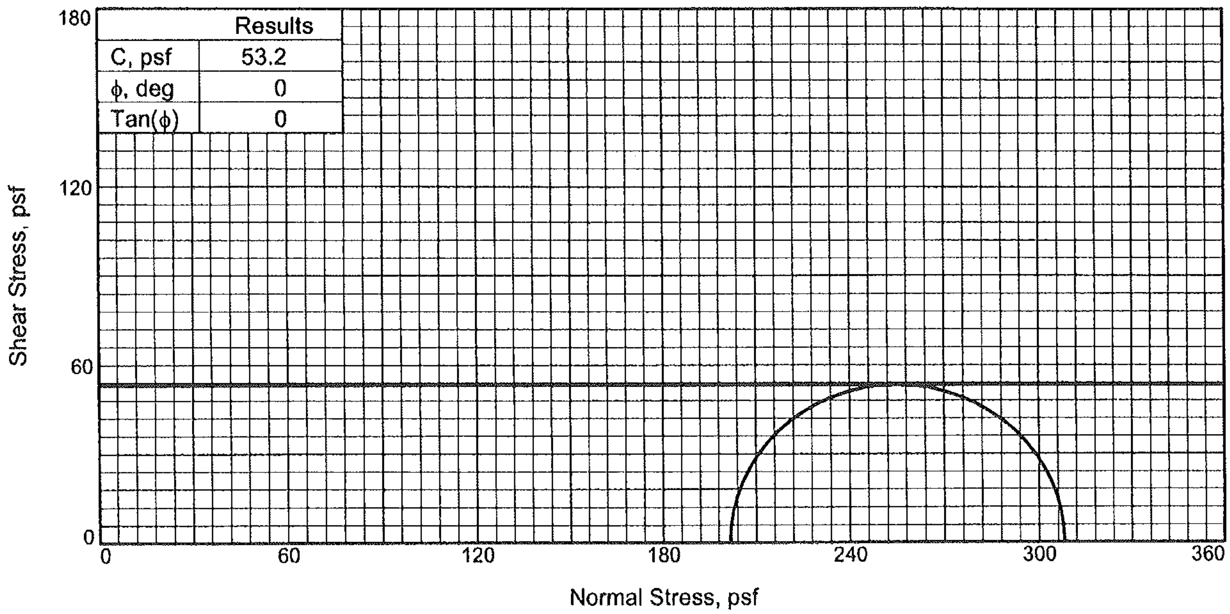
Source of Sample: 4 **Depth:** 24

Sample Number: 11

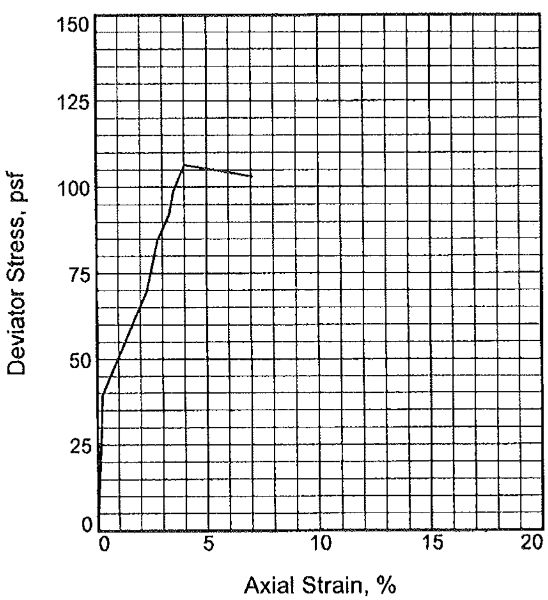
UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD



Results	
C, psf	53.2
ϕ, deg	0
Tan(ϕ)	0



	Specimen No.	1
Initial	Water Content,	664.3
	Dry Density, pcf	8.6
	Saturation,	96.7
	Void Ratio	16.8236
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	664.3
	Dry Density, pcf	8.6
	Saturation,	96.7
	Void Ratio	16.8236
	Diameter, in.	1.391
	Height, in.	2.929
	Strain rate, in./min.	0.029
	Back Pressure, psf	0.0
	Cell Pressure, psf	201.6
	Fail. Stress, psf	106.4
	Ult. Stress, psf	103.1
	σ_1 Failure, psf	308.0
	σ_3 Failure, psf	201.6

Type of Test:
Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: XSO BL PT

Assumed Specific Gravity= 2.45
Remarks: TORVANE = 0.100 TSF

Figure 1

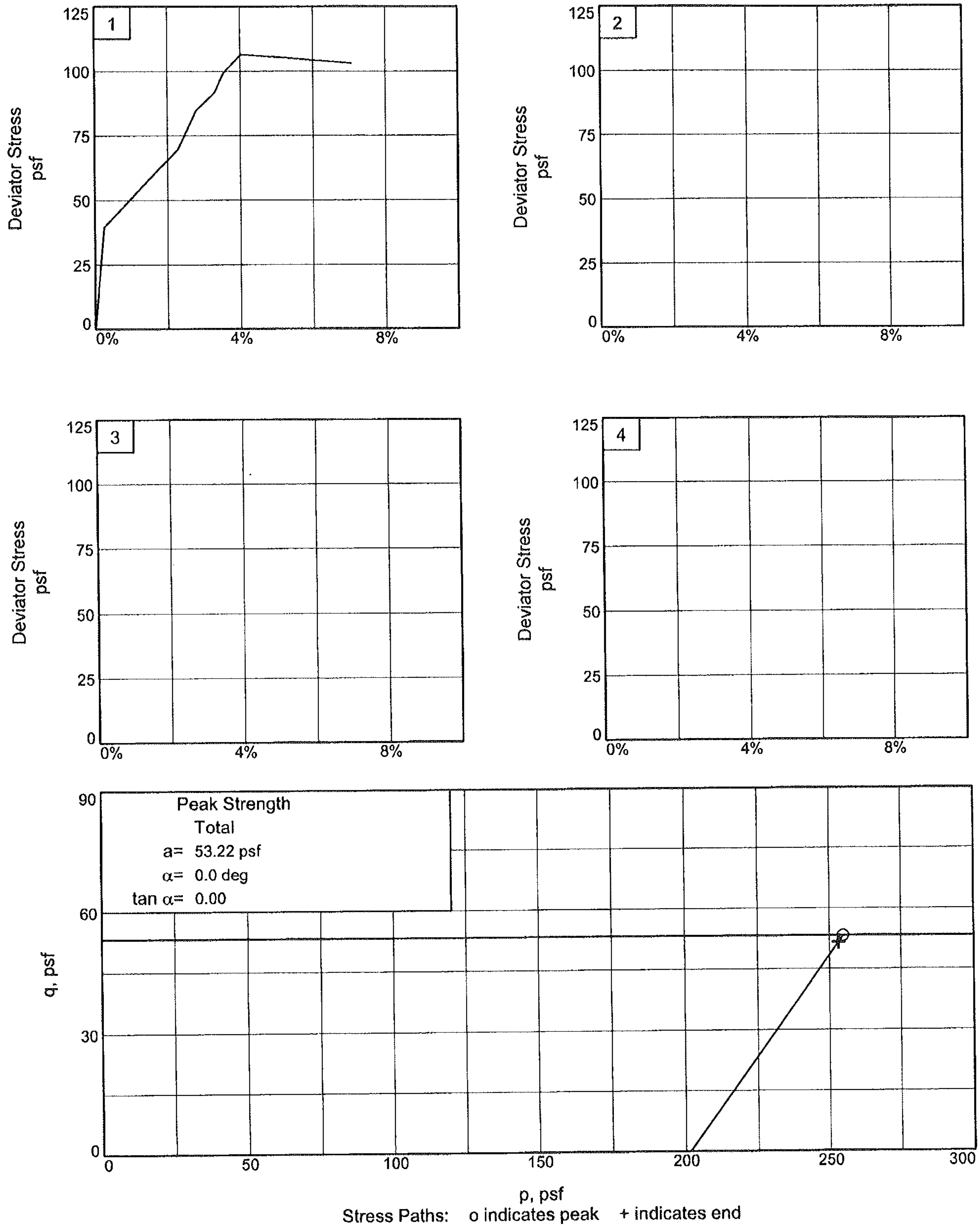
Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 5 **Depth:** 3
Sample Number: 2
Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD



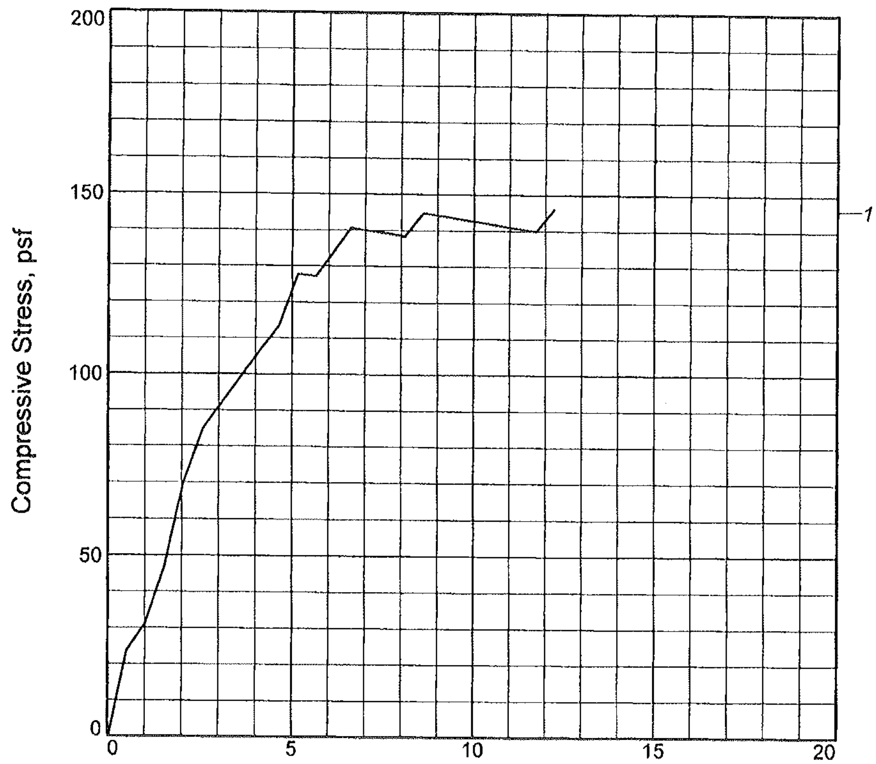
Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 5 **Depth:** 3 **Sample Number:** 2
Project No.: 19183

Figure 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	127.7			
Undrained shear strength, psf	63.9			
Failure strain, %	5.2			
Strain rate, in./min.	0.058			
Water content, %	70.6			
Wet density, pcf	95.8			
Dry density, pcf	56.2			
Saturation, %	94.9			
Void ratio	2.0238			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: XSO G CL W/ SI LEN & POC, RTS

LL =	PL =	PI =	Assumed GS= 2.72	Type: UNDISTURBED

Project No.: 19183
Date:
Remarks:
TORVANE = 0.140 TSF

Figure 1

Client: HNTB CORPORATION

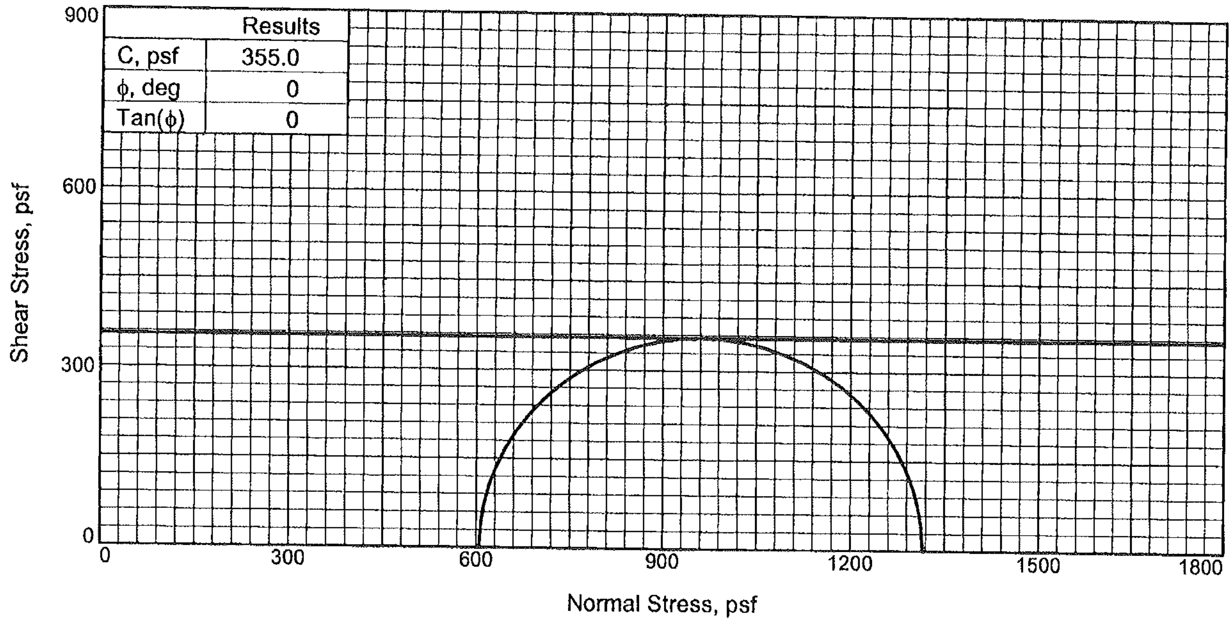
Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 5 **Depth:** 7
Sample Number: 4

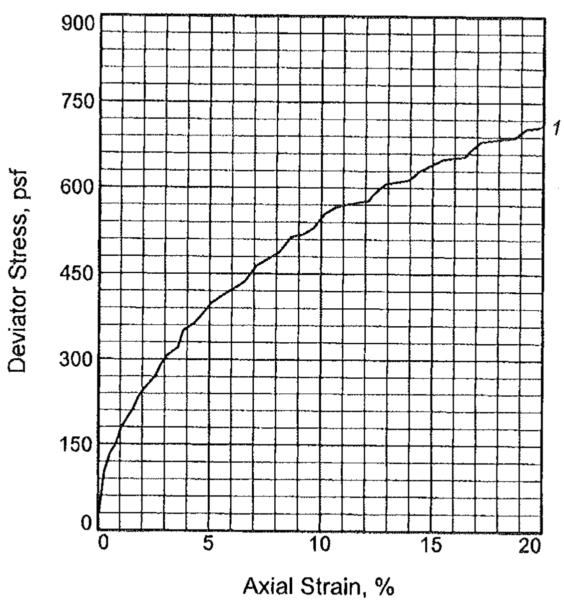
UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

	Results
C, psf	355.0
ϕ, deg	0
Tan(ϕ)	0





	Specimen No.	1
Initial	Water Content,	33.6
	Dry Density, pcf	87.7
	Saturation,	99.8
	Void Ratio	0.8995
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	33.7
	Dry Density, pcf	87.7
	Saturation,	100.0
	Void Ratio	0.8995
	Diameter, in.	1.391
	Height, in.	2.929
	Strain rate, in./min.	0.029
	Back Pressure, psf	0.0
	Cell Pressure, psf	604.8
	Fail. Stress, psf	709.9
	Ult. Stress, psf	709.9
	σ_1 Failure, psf	1314.7
	σ_3 Failure, psf	604.8

Type of Test:
Unconsolidated Undrained
Sample Type: UNDSITURBED
Description: LO G CLSI W/ CL LAY

Assumed Specific Gravity= 2.67
Remarks:

Figure 1

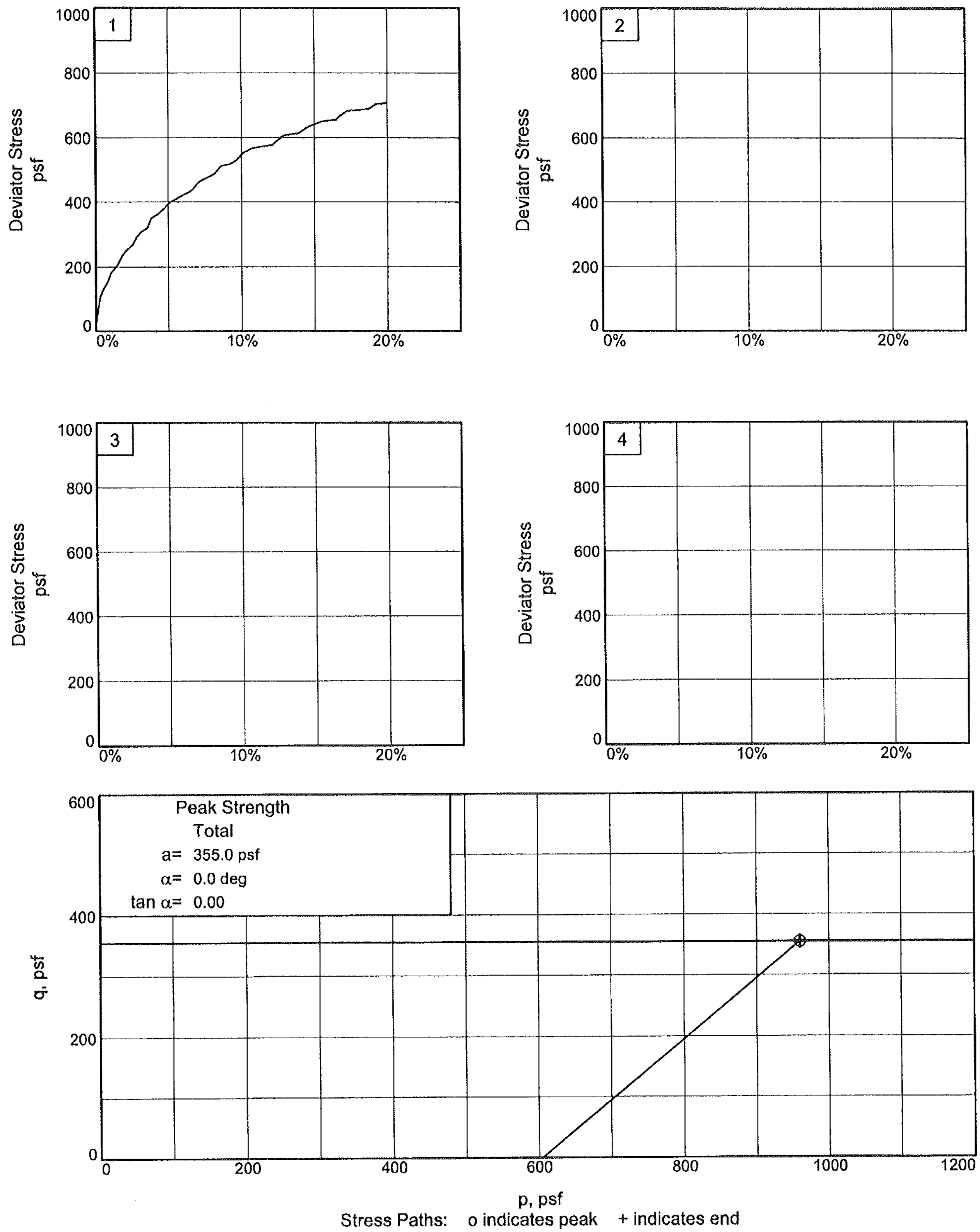
Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 5 **Depth:** 11
Sample Number: 6
Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

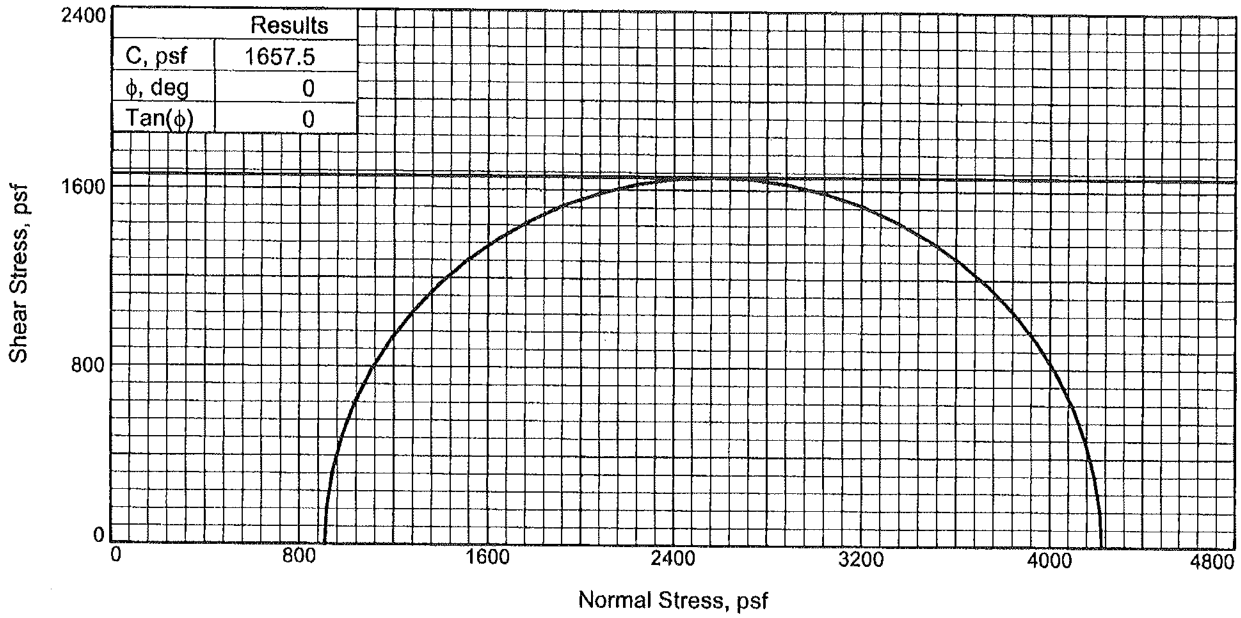
Tested By: RA **Checked By:** BD

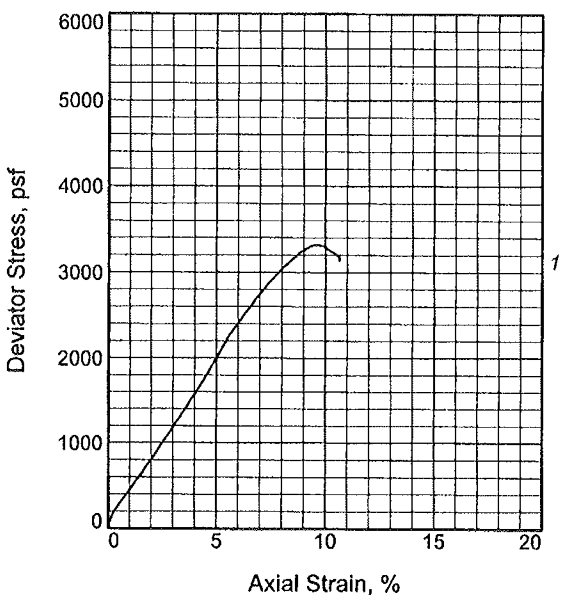


Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 5 **Depth:** 11 **Sample Number:** 6
Project No.: 19183 **Figure** 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD





Specimen No.		1
Initial	Water Content,	24.4
	Dry Density, pcf	98.0
	Saturation,	92.8
	Void Ratio	0.7004
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	26.2
	Dry Density, pcf	98.0
	Saturation,	100.0
	Void Ratio	0.7004
	Diameter, in.	1.391
	Height, in.	2.929
Strain rate, in./min.		0.029
Back Pressure, psf		0.0
Cell Pressure, psf		907.2
Fail. Stress, psf		3315.0
Ult. Stress, psf		3136.0
σ_1 Failure, psf		4222.2
σ_3 Failure, psf		907.2

Type of Test:
Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: MC G SASI W/ CL LAY

LL= 35 **PL=** 26 **PI=** 9

Assumed Specific Gravity= 2.67

Remarks:

Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source of Sample: 5 **Depth:** 17

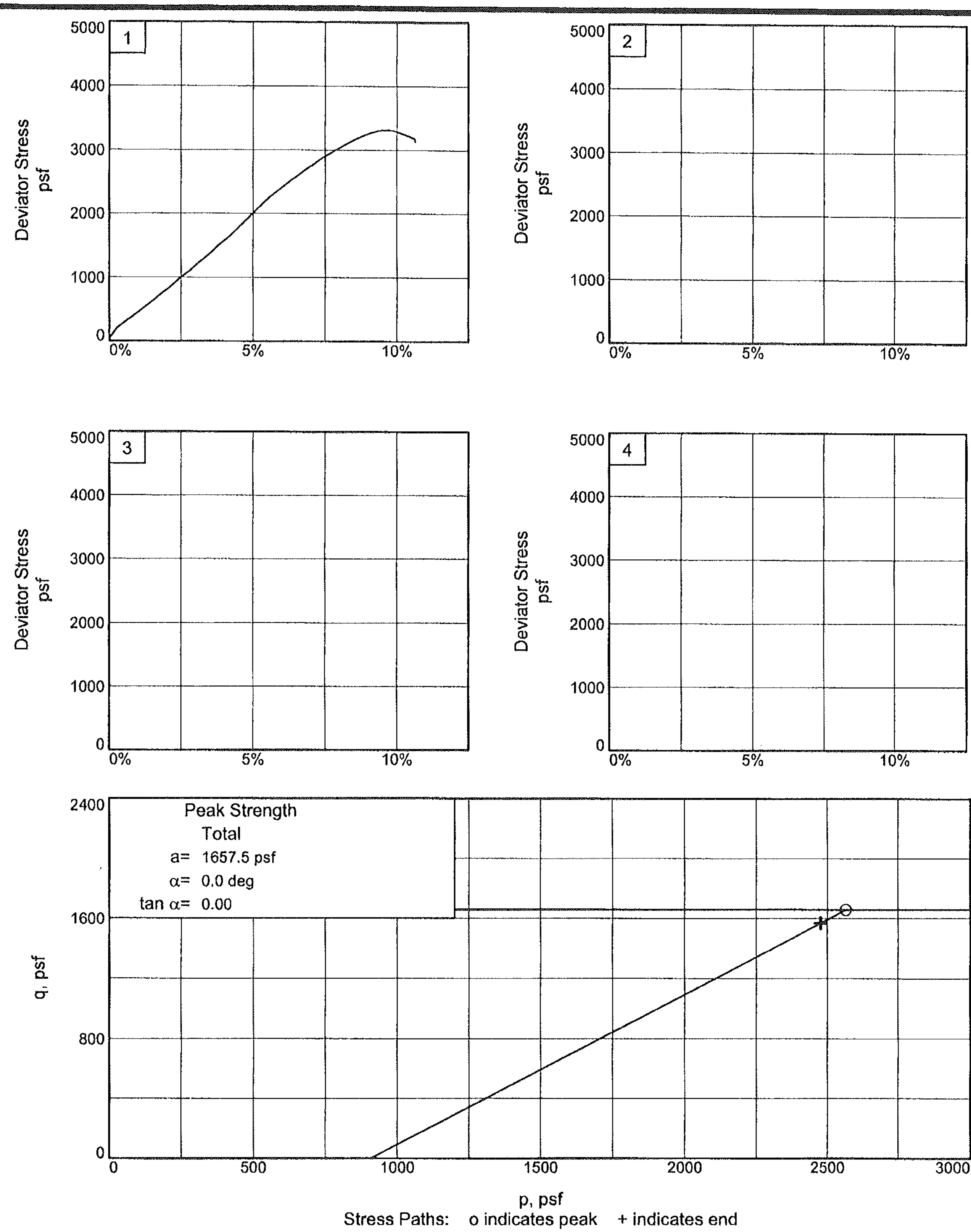
Sample Number: 9

Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

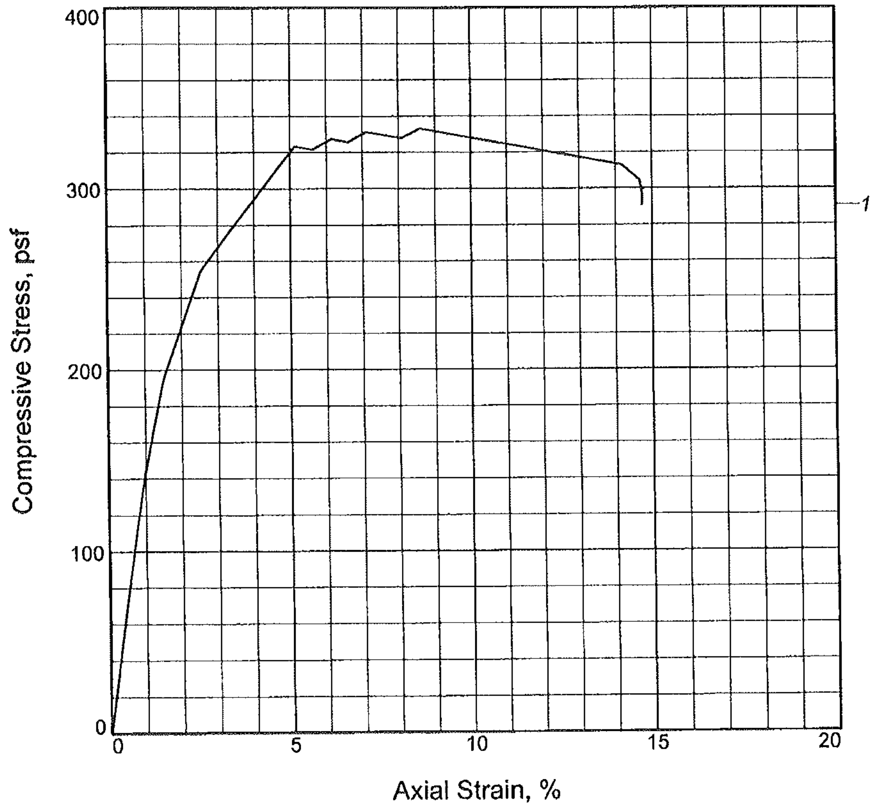


Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 5 **Depth:** 17 **Sample Number:** 9
Project No.: 19183 **Figure** 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	323.2			
Undrained shear strength, psf	161.6			
Failure strain, %	5.1			
Strain rate, in./min.	0.058			
Water content, %	81.8			
Wet density, pcf	94.1			
Dry density, pcf	51.8			
Saturation, %	97.2			
Void ratio	2.3040			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: VSO G CL

LL =	**PL =**	**PI =**	**Assumed GS=** 2.74	**Type:** UNDISTURBED

Project No.: 19183

Date:

Remarks:
TORVANE = 0.160 TSF

Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source of Sample: 5 **Depth:** 24

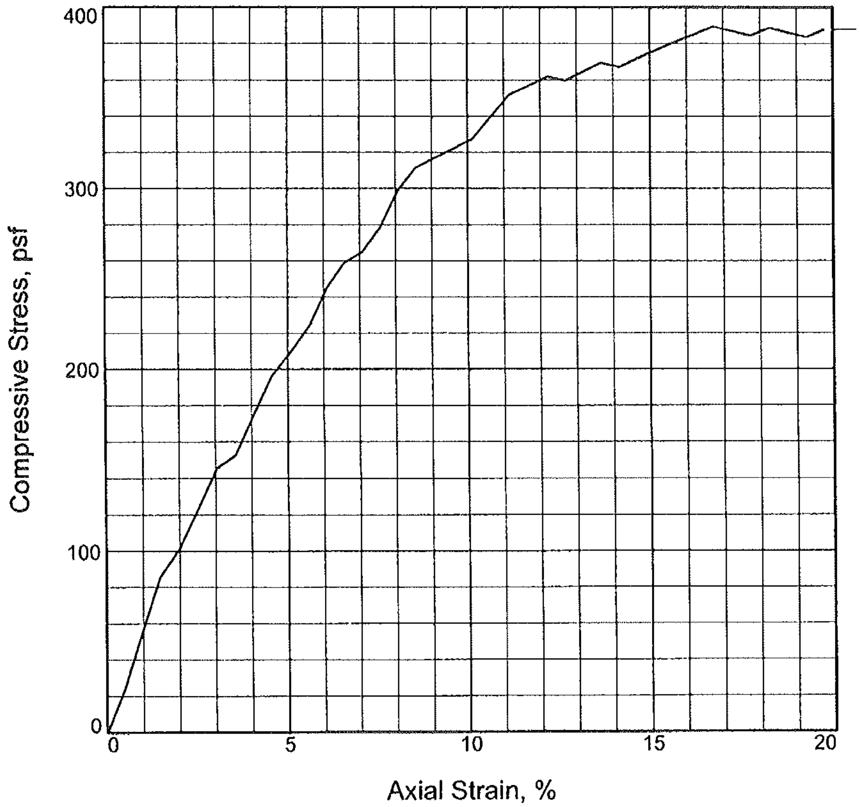
Sample Number: 11

UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	361.7			
Undrained shear strength, psf	180.8			
Failure strain, %	12.2			
Strain rate, in./min.	0.059			
Water content, %	63.2			
Wet density, pcf	99.1			
Dry density, pcf	60.8			
Saturation, %	95.7			
Void ratio	1.7944			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: VSO G CL W/ SASI LEN & LAY

LL =	PL =	PI =	Assumed GS= 2.72	Type: UNDISTURBED

Project No.: 19183
Date:
Remarks:
TORVANE = 0.160 TSF

Figure 1

Client: HNTB CORPORATION

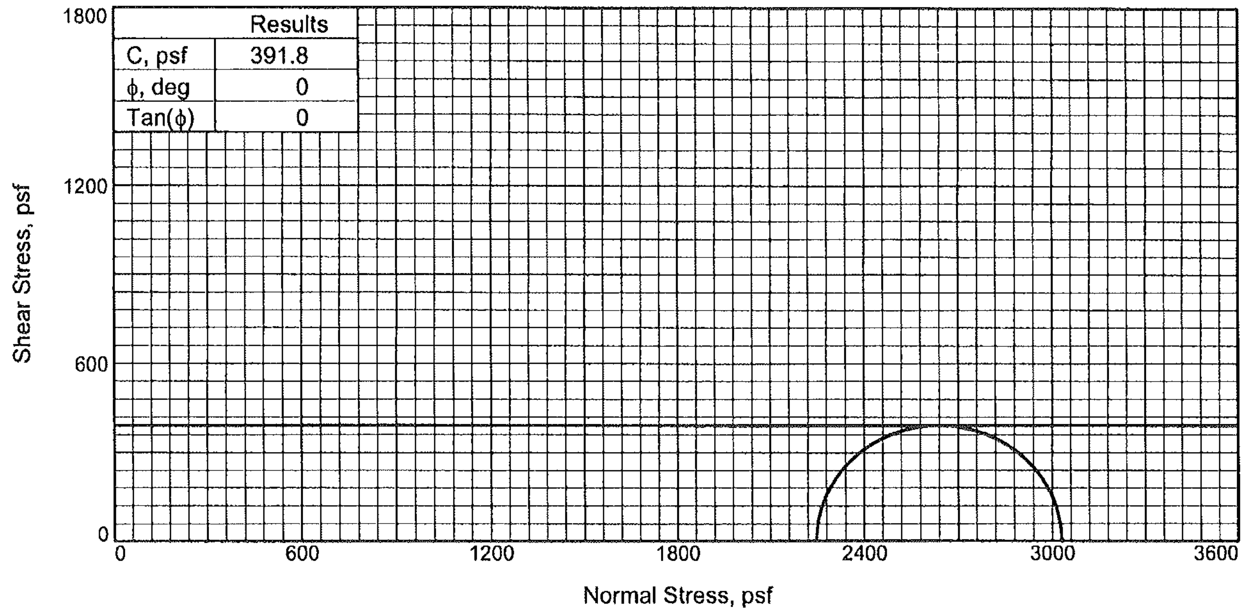
Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 5 **Depth:** 34
Sample Number: 13

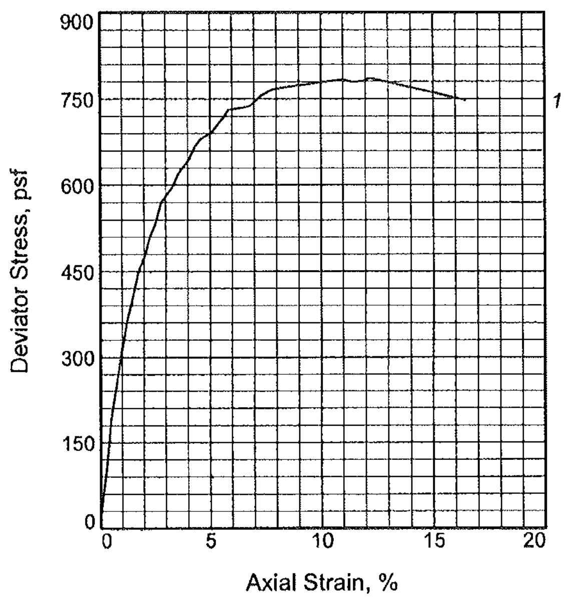
UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

Results	
C, psf	391.8
ϕ, deg	0
Tan(ϕ)	0





Specimen No.		1
Initial	Water Content,	60.3
	Dry Density, pcf	62.7
	Saturation,	95.7
	Void Ratio	1.7271
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	63.0
	Dry Density, pcf	62.7
	Saturation,	100.0
	Void Ratio	1.7271
	Diameter, in.	1.391
	Height, in.	2.929
Strain rate, in./min.		0.029
Back Pressure, psf		0.0
Cell Pressure, psf		2246.4
Fail. Stress, psf		783.6
Ult. Stress, psf		747.9
σ_1 Failure, psf		3030.0
σ_3 Failure, psf		2246.4

Type of Test:
Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: SO G CL W/ SA LEN

Assumed Specific Gravity= 2.74
Remarks: TORVANE = 0.240 TSF

Figure 1

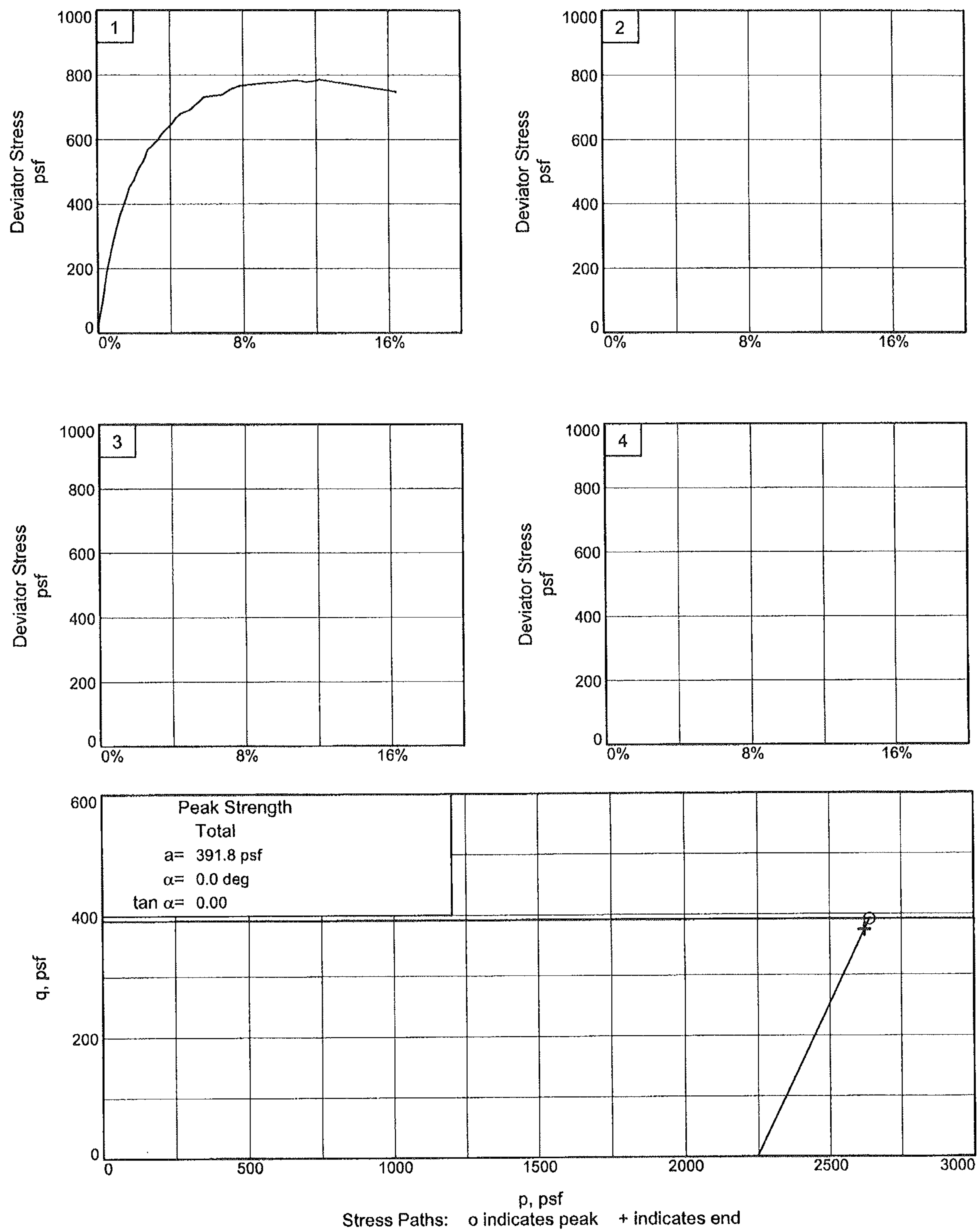
Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 5 **Depth:** 44
Sample Number: 15
Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

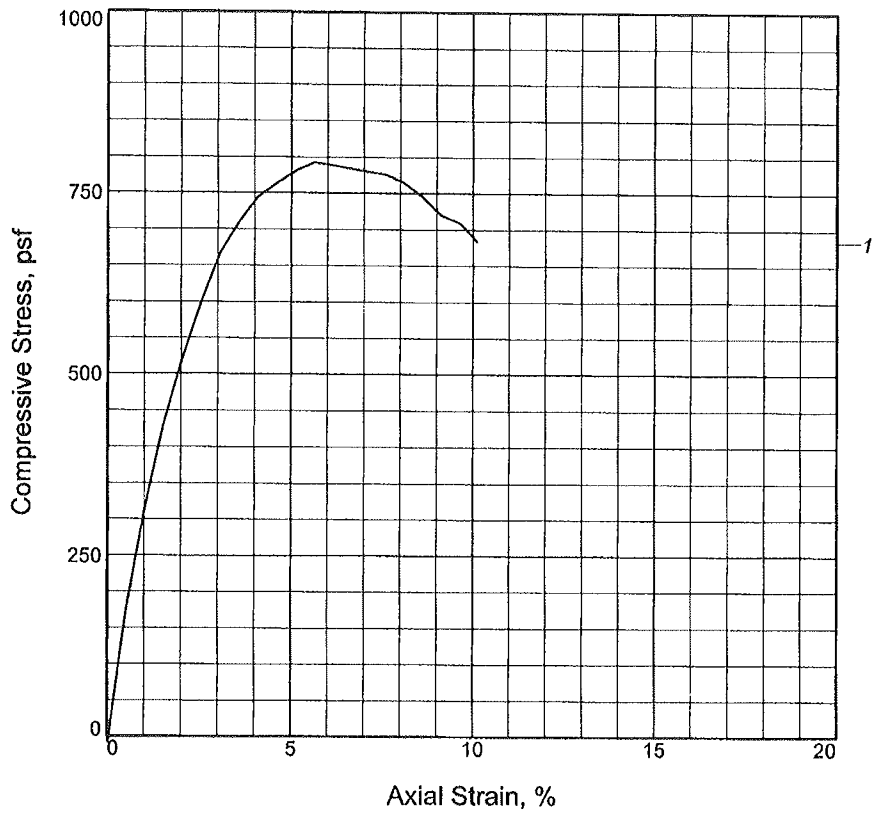


Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 5 **Depth:** 44 **Sample Number:** 15
Project No.: 19183 **Figure 2**

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	792.4			
Undrained shear strength, psf	396.2			
Failure strain, %	5.6			
Strain rate, in./min.	0.058			
Water content, %	63.7			
Wet density, pcf	102.3			
Dry density, pcf	62.5			
Saturation, %	100.4			
Void ratio	1.7385			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: SO G CL (FLOC)

LL =	PL =	PI =	Assumed GS= 2.74	Type: UNDISTURBED

Project No.: 19183

Date:

Remarks:
TORVANE = 0.250 TSF

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source of Sample: 5 **Depth:** 54

Sample Number: 17

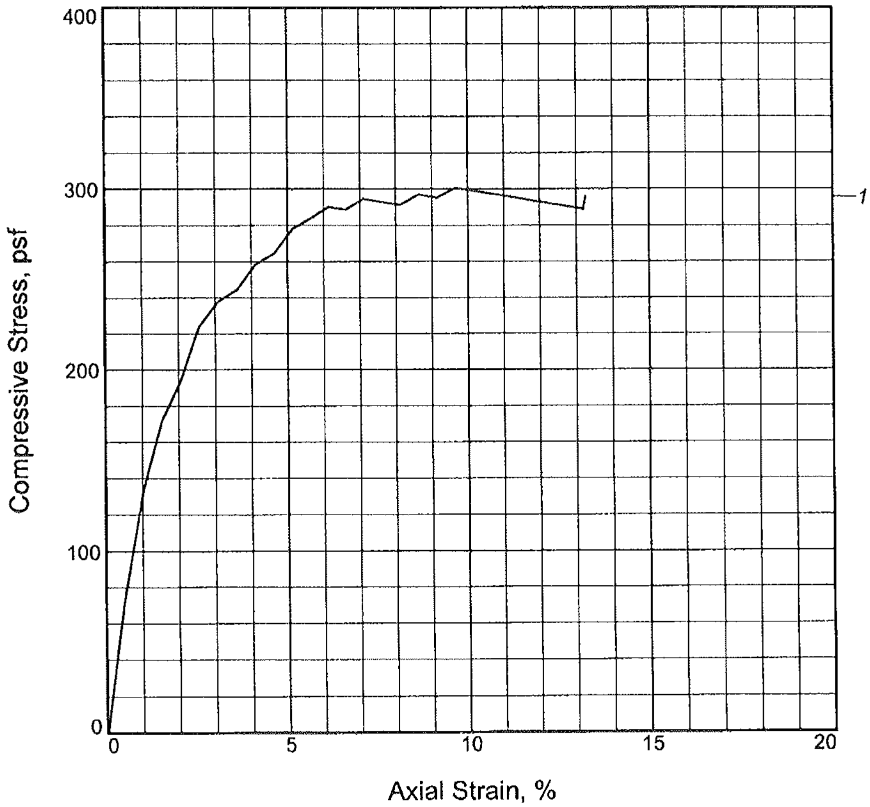
UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Figure 1

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	290.0			
Undrained shear strength, psf	145.0			
Failure strain, %	6.1			
Strain rate, in./min.	0.058			
Water content, %	62.8			
Wet density, pcf	100.0			
Dry density, pcf	61.4			
Saturation, %	96.5			
Void ratio	1.7843			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: VSO G CL W/ RTS

LL = 88	**PL** = 27	**PI** = 61	**Assumed GS=** 2.74	**Type:** UNDISTURBED

Project No.: 19183
Date:
Remarks:
TORVANE = 0.210 TSF

Figure 1

Client: HNTB CORPORATION

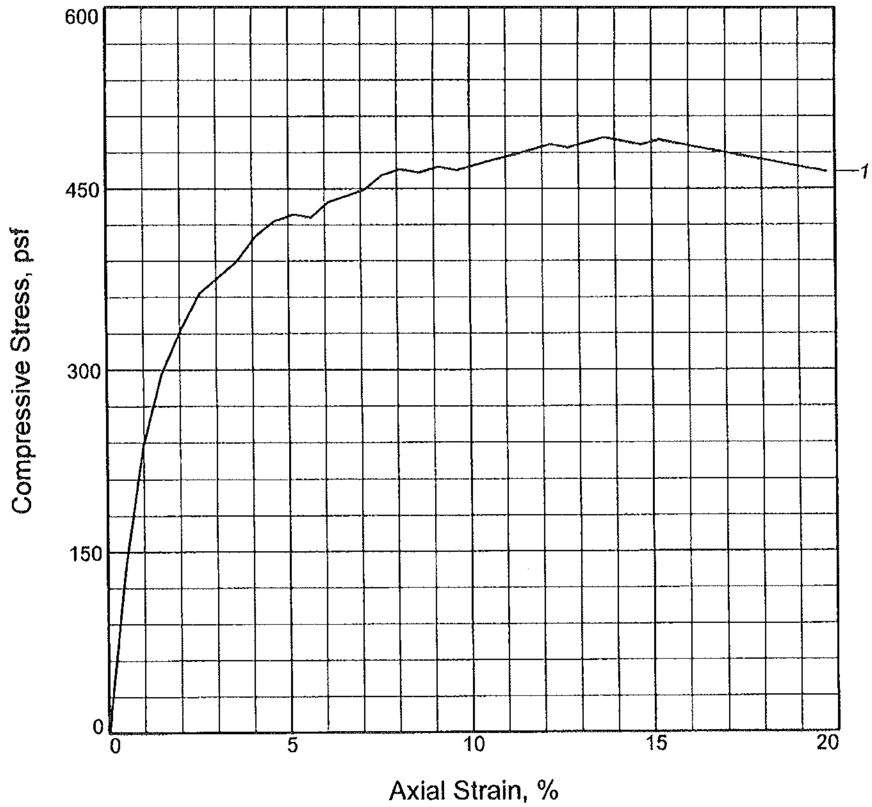
Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 6 **Depth:** 3
Sample Number: 2

UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	428.3			
Undrained shear strength, psf	214.2			
Failure strain, %	5.1			
Strain rate, in./min.	0.056			
Water content, %	62.6			
Wet density, pcf	100.1			
Dry density, pcf	61.6			
Saturation, %	96.4			
Void ratio	1.7787			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: VSO G CL W/ RTS

LL =	PL =	PI =	Assumed GS= 2.74	Type: UNDISTURBED

Project No.: 19183
Date:
Remarks:
TORVANE = 0.210 TSF

Figure 1

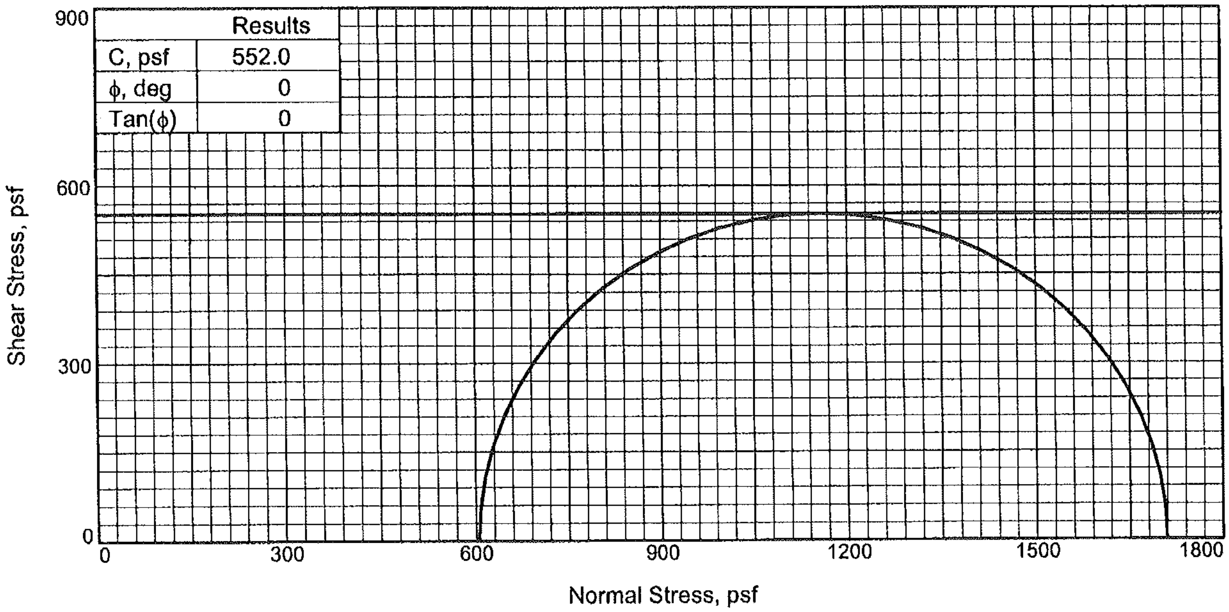
Client: HNTB CORPORATION

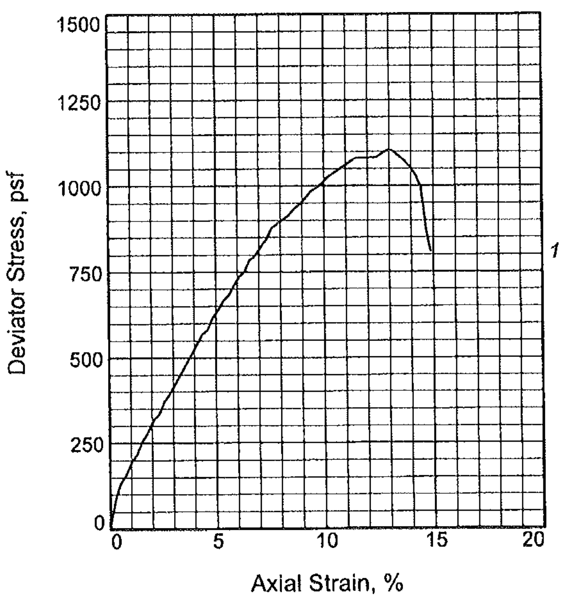
Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 6 **Depth:** 7
Sample Number: 4

UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD





Specimen No.		1
Initial	Water Content,	38.0
	Dry Density, pcf	83.3
	Saturation,	101.3
	Void Ratio	1.0009
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	37.5
	Dry Density, pcf	83.3
	Saturation,	100.0
	Void Ratio	1.0009
	Diameter, in.	1.391
	Height, in.	2.929
Strain rate, in./min.		0.029
Back Pressure, psf		0.0
Cell Pressure, psf		604.8
Fail. Stress, psf		1103.9
Ult. Stress, psf		809.3
σ_1 Failure, psf		1708.7
σ_3 Failure, psf		604.8

Type of Test:
Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: MC G CLSI W/ CL LEN

Assumed Specific Gravity= 2.67
Remarks:

Figure 1

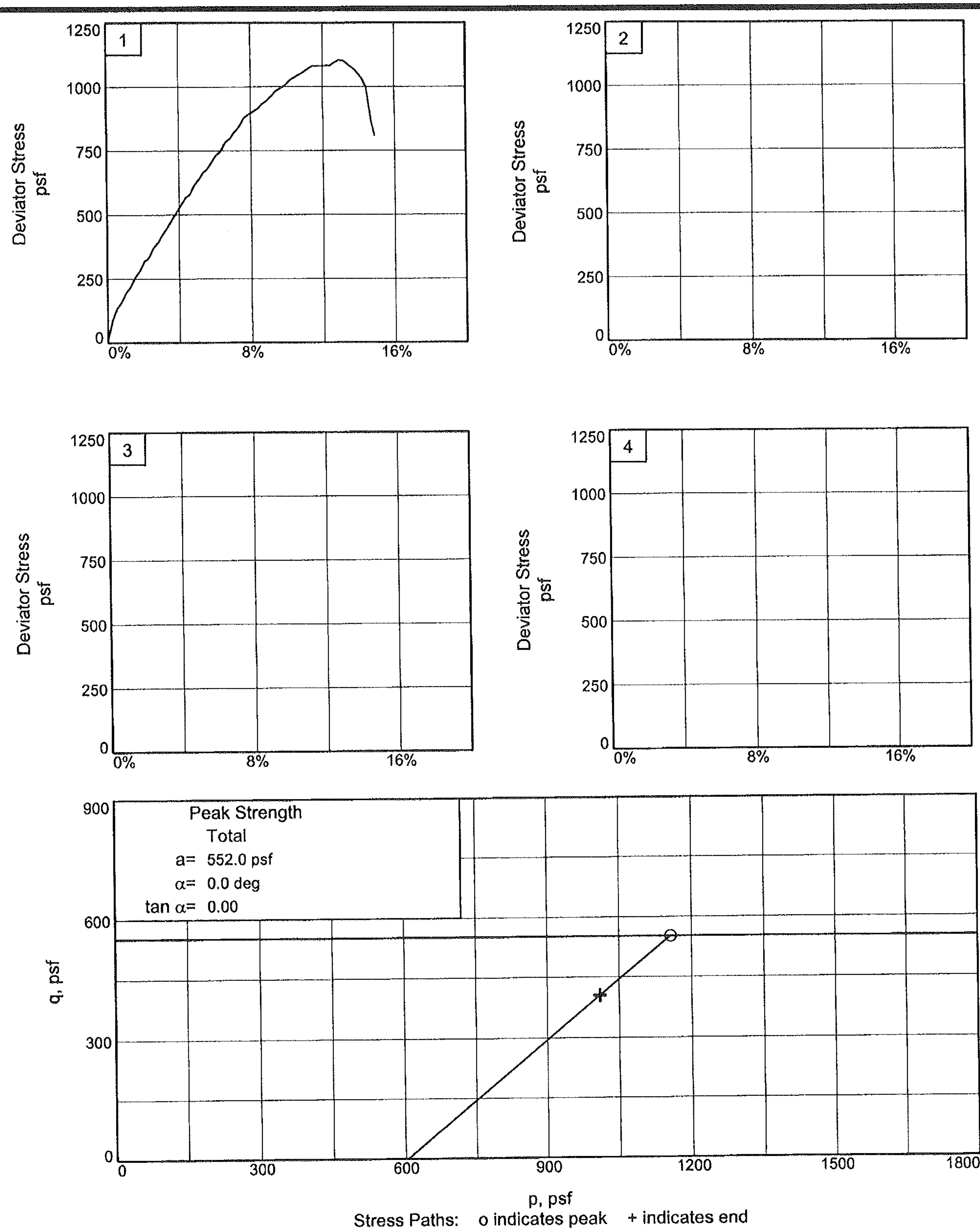
Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 6 **Depth:** 11
Sample Number: 6
Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

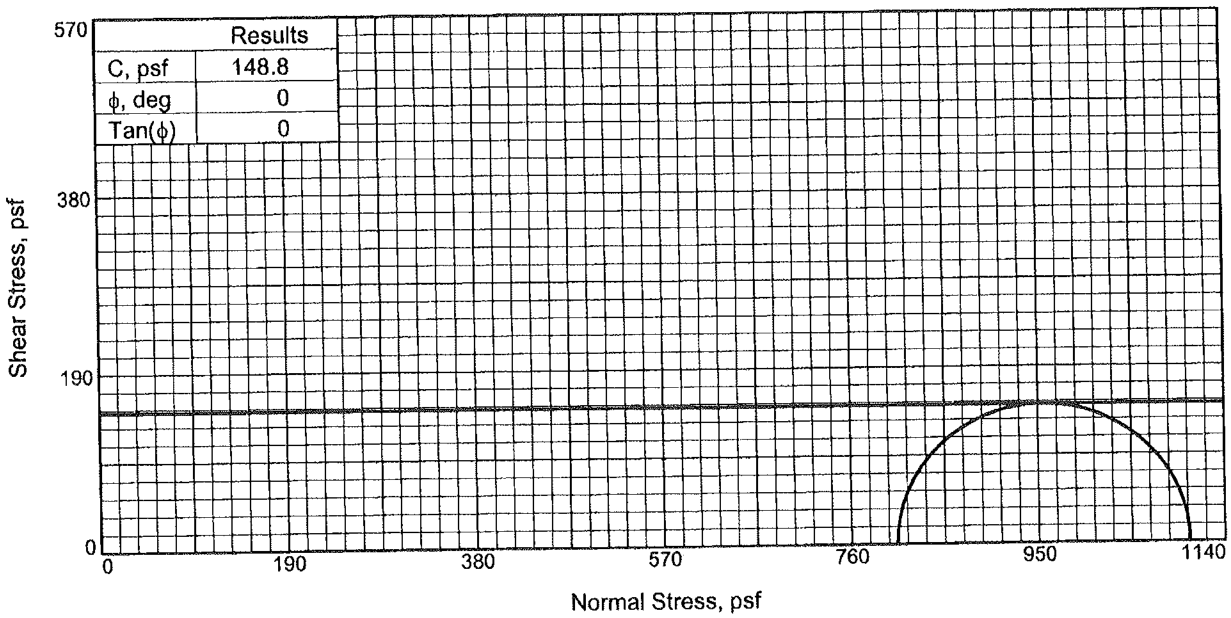
Tested By: RA **Checked By:** BD

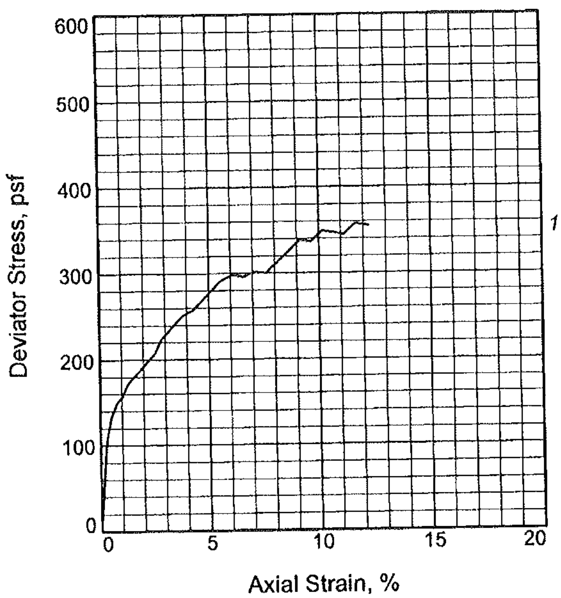


Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 6 **Depth:** 11 **Sample Number:** 6
Project No.: 19183 **Figure** 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD





Specimen No.		1
Initial	Water Content,	56.5
	Dry Density, pcf	65.1
	Saturation,	95.4
	Void Ratio	1.6095
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	59.2
	Dry Density, pcf	65.1
	Saturation,	100.0
	Void Ratio	1.6095
	Diameter, in.	1.391
	Height, in.	2.929
Strain rate, in./min.		0.029
Back Pressure, psf		0.0
Cell Pressure, psf		806.4
Fail. Stress, psf		297.6
Ult. Stress, psf		354.5
σ_1 Failure, psf		1104.0
σ_3 Failure, psf		806.4

Type of Test:
Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: VSO G CL W/ SI LEN & LAY

Assumed Specific Gravity= 2.72

Remarks: TORVANE = 0.130 TSF

Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source of Sample: 6 **Depth:** 15

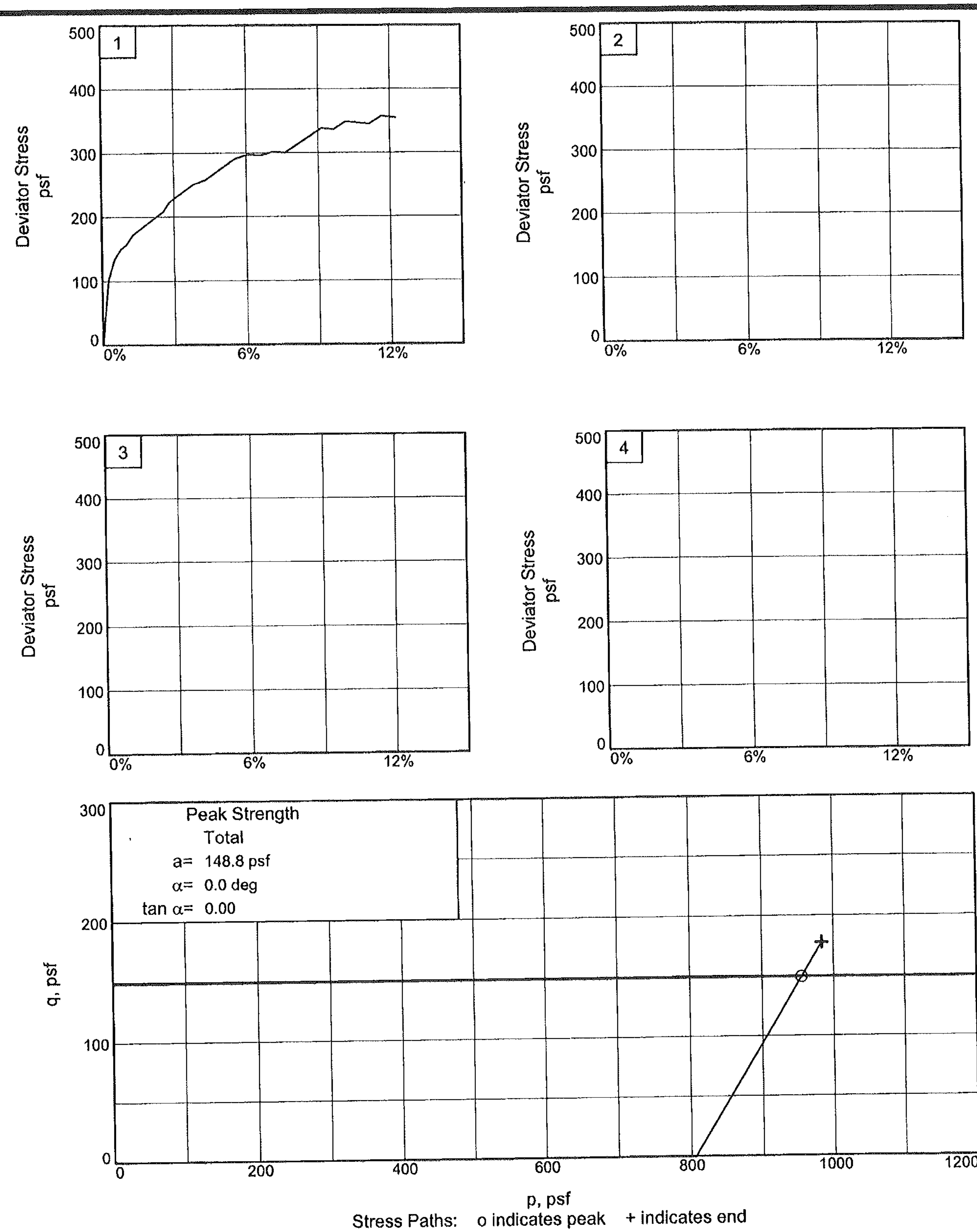
Sample Number: 8

Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

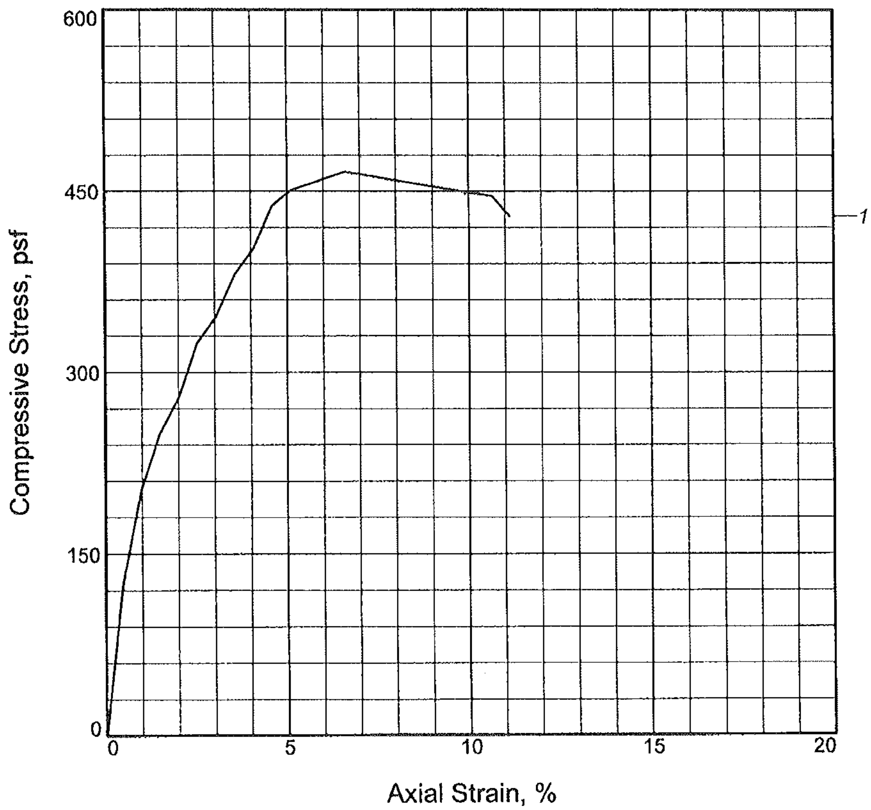


Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 6 **Depth:** 15 **Sample Number:** 8
Project No.: 19183 Figure 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	466.2			
Undrained shear strength, psf	233.1			
Failure strain, %	6.6			
Strain rate, in./min.	0.058			
Water content, %	64.2			
Wet density, pcf	99.5			
Dry density, pcf	60.6			
Saturation, %	96.9			
Void ratio	1.8027			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: VSO G CL W/ SASI POC

LL =	**PL =**	**PI =**	**Assumed GS=** 2.72	**Type:** UNDISTURBED

Project No.: 19183
Date:
Remarks:
TORVANE = 0.220 TSF

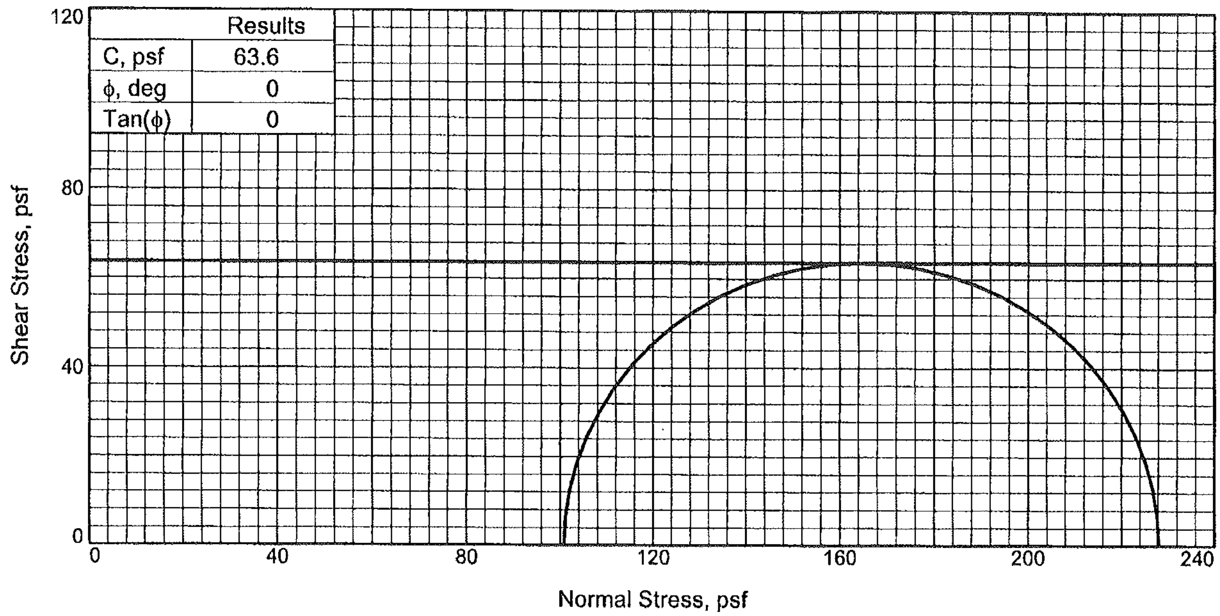
Figure 1

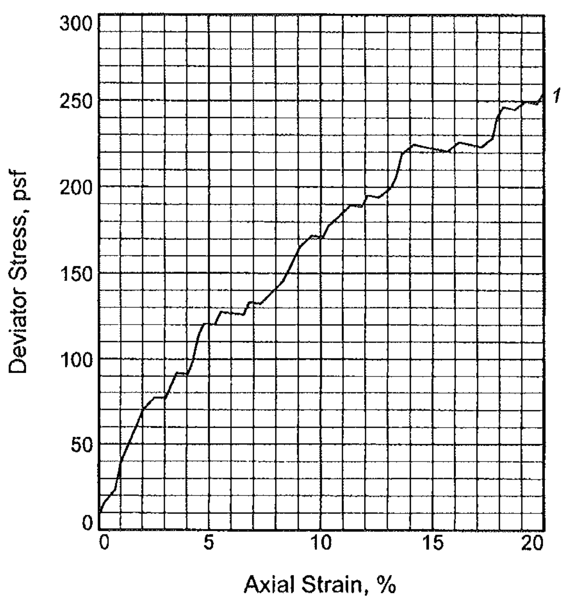
Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 6 **Depth:** 29
Sample Number: 12

UNCONFINED COMPRESSION TEST
EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD





Specimen No.		1
Initial	Water Content,	700.4
	Dry Density, pcf	8.0
	Saturation,	94.3
	Void Ratio	18.1947
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	742.6
	Dry Density, pcf	8.0
	Saturation,	100.0
	Void Ratio	18.1947
	Diameter, in.	1.391
	Height, in.	2.929
Strain rate, in./min.		0.029
Back Pressure, psf		0.0
Cell Pressure, psf		100.8
Fail. Stress, psf		127.2
Ult. Stress, psf		253.7
σ_1 Failure, psf		228.0
σ_3 Failure, psf		100.8

Type of Test:
Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: XSO BL PT W/ RTS

Assumed Specific Gravity= 2.45
Remarks: TORVANE = 0.028 TSF

Figure 1

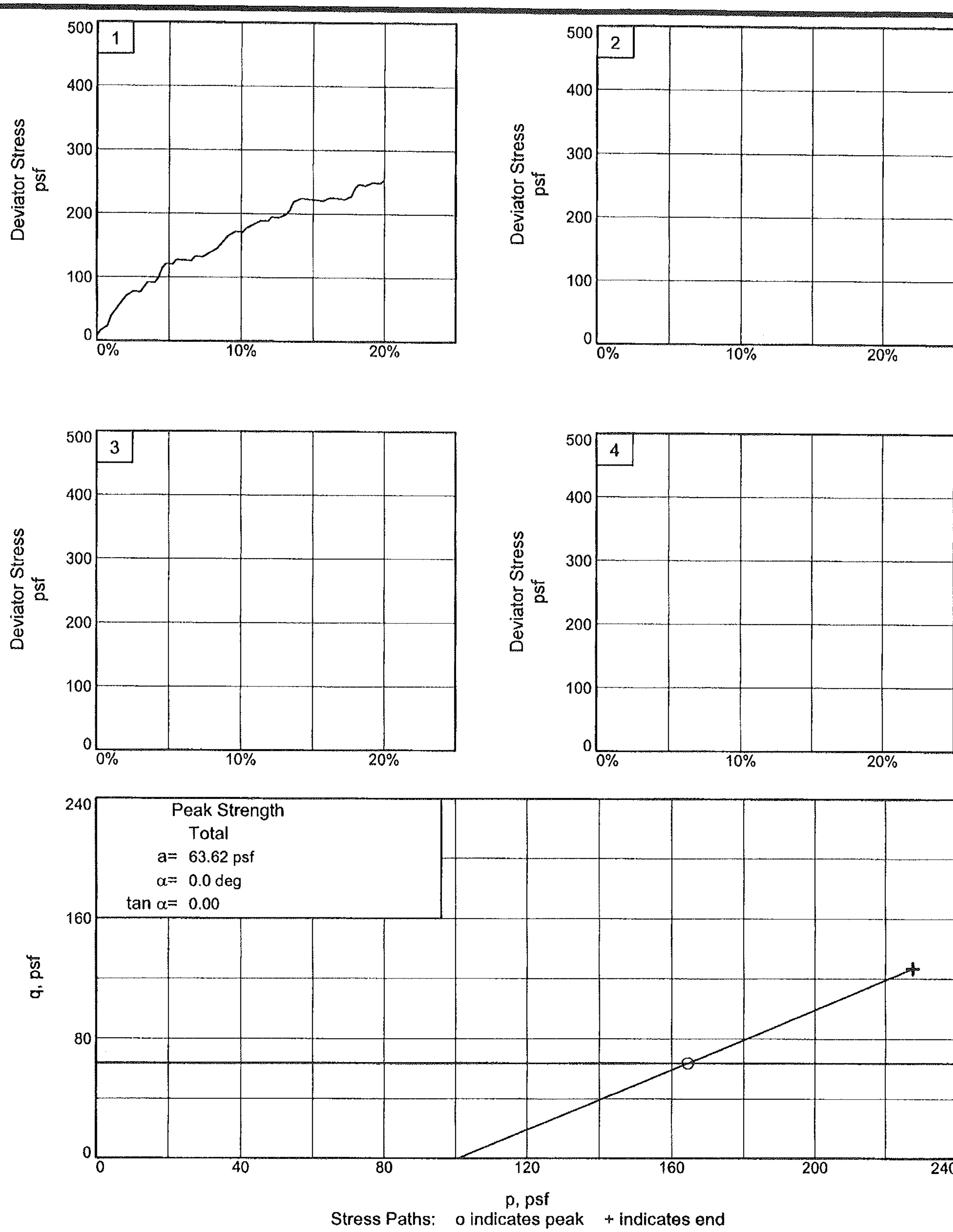
Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 7 **Depth:** 1
Sample Number: 1
Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

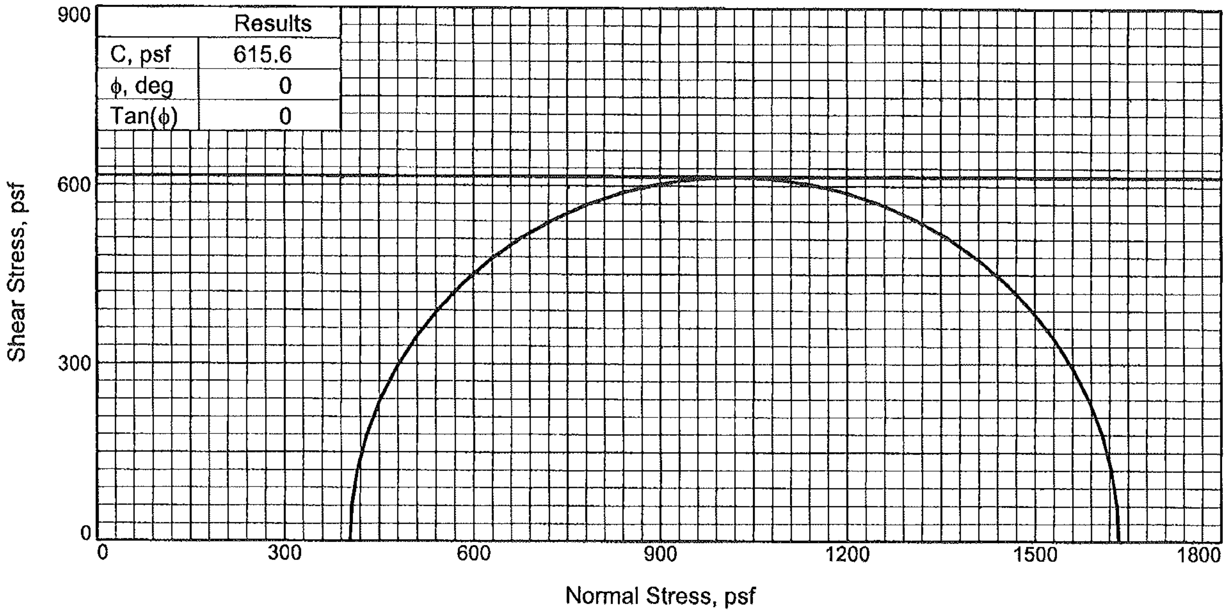


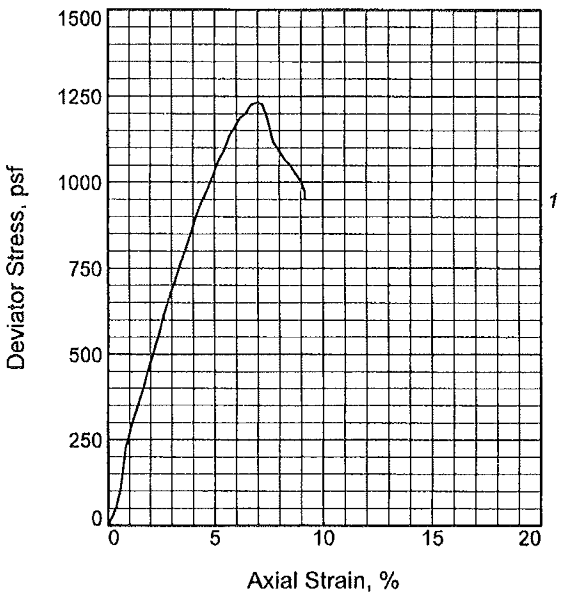
Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 7 **Depth:** 1 **Sample Number:** 1
Project No.: 19183

Figure 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD





Specimen No.		1
Initial	Water Content,	28.6
	Dry Density, pcf	93.2
	Saturation,	96.6
	Void Ratio	0.7893
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	29.6
	Dry Density, pcf	93.2
	Saturation,	100.0
	Void Ratio	0.7893
	Diameter, in.	1.391
	Height, in.	2.929
Strain rate, in./min.		0.029
Back Pressure, psf		0.0
Cell Pressure, psf		403.2
Fail. Stress, psf		1231.1
Ult. Stress, psf		949.8
σ_1 Failure, psf		1634.3
σ_3 Failure, psf		403.2

Type of Test:
Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: MC G CLSI

Assumed Specific Gravity= 2.67

Remarks:

Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source of Sample: 7 **Depth:** 7

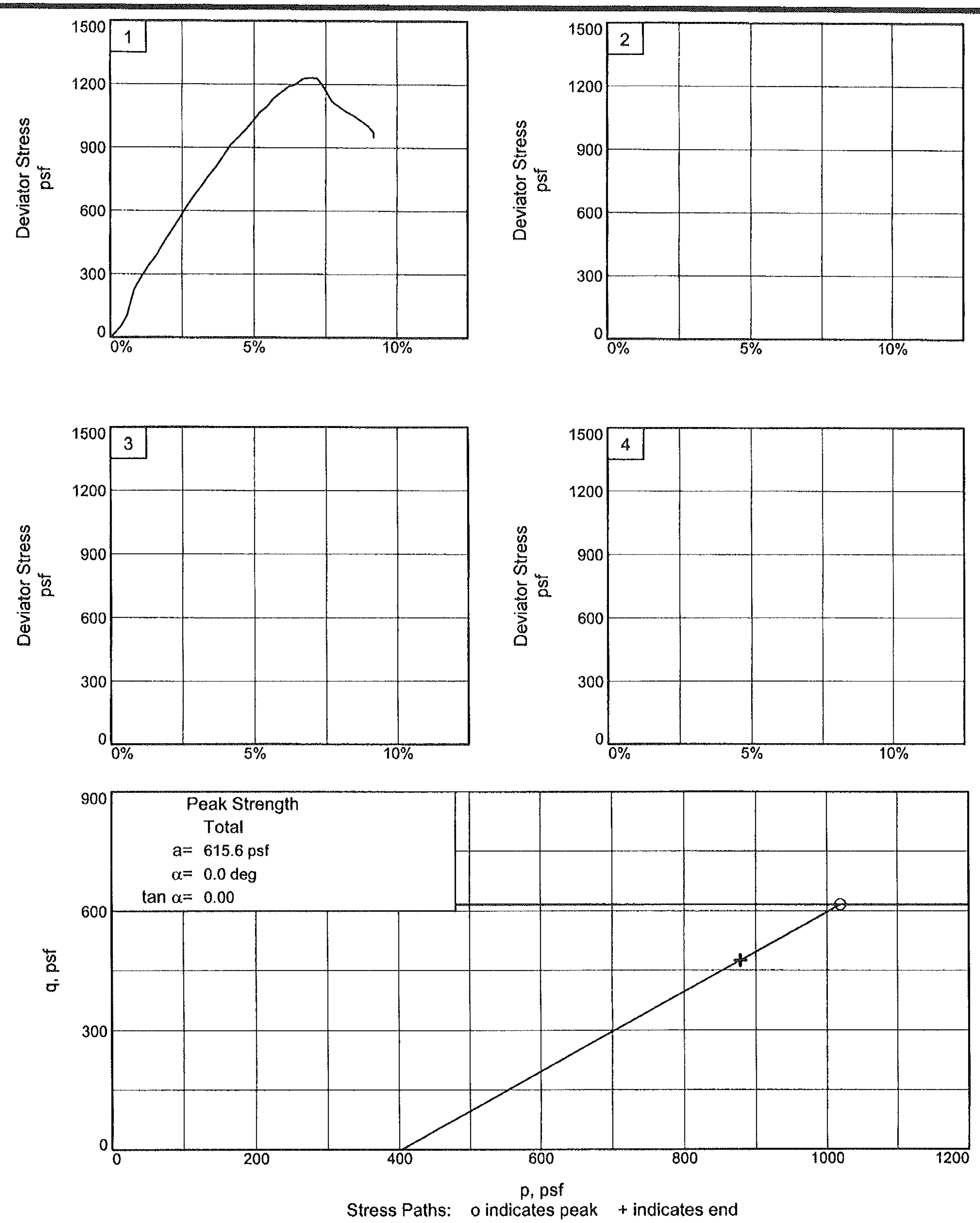
Sample Number: 4

Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

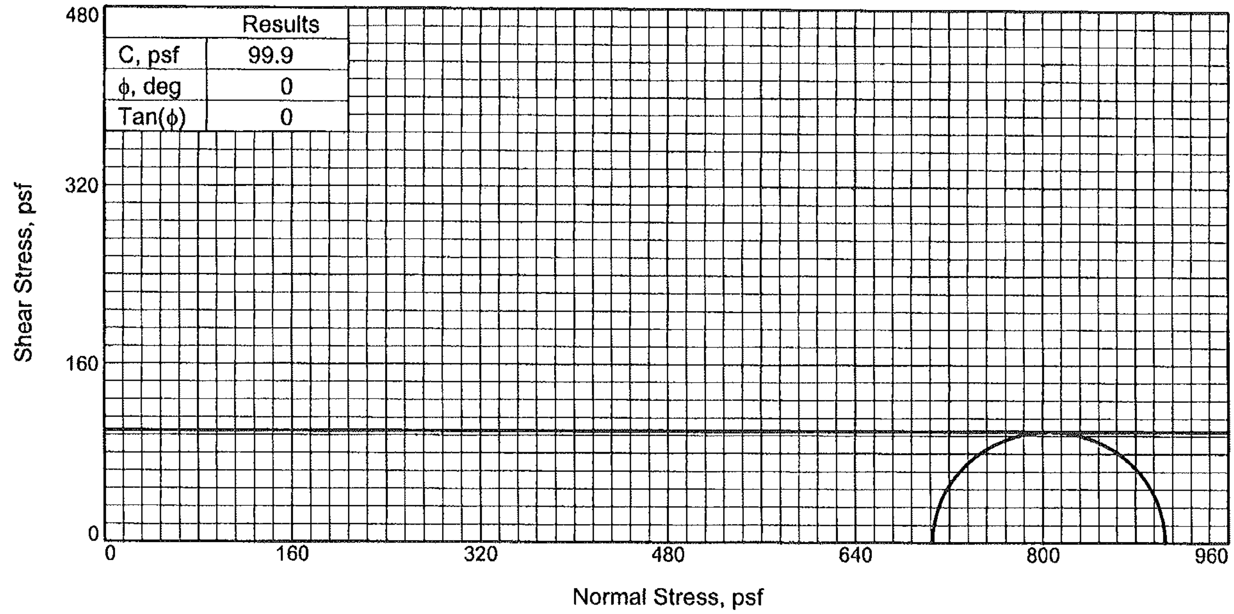


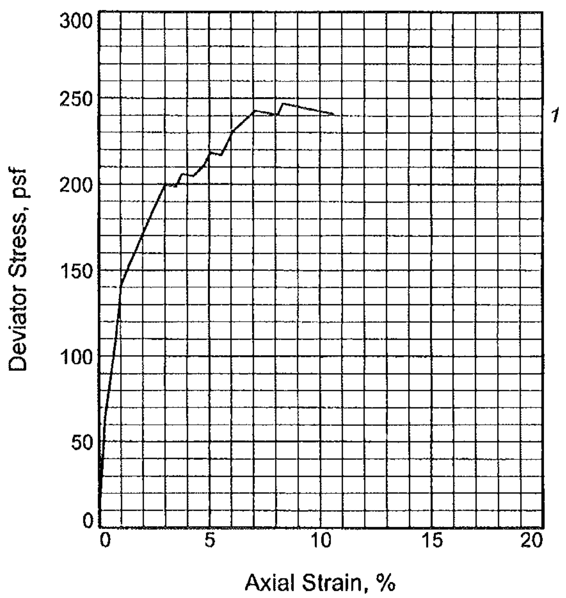
Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 7 **Depth:** 7 **Sample Number:** 4
Project No.: 19183

Figure 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD





Specimen No.		1
Initial	Water Content,	79.2
	Dry Density, pcf	54.0
	Saturation,	100.6
	Void Ratio	2.1429
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	78.8
	Dry Density, pcf	54.0
	Saturation,	100.0
	Void Ratio	2.1429
	Diameter, in.	1.391
	Height, in.	2.929
Strain rate, in./min.		0.029
Back Pressure, psf		0.0
Cell Pressure, psf		705.6
Fail. Stress, psf		199.8
Ult. Stress, psf		240.8
σ_1 Failure, psf		905.4
σ_3 Failure, psf		705.6

Type of Test:
Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: XSO G CL W/ SA LEN & POC

Assumed Specific Gravity= 2.72

Remarks: TORVANE = 0.076 TSF

Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source of Sample: 7 **Depth:** 13

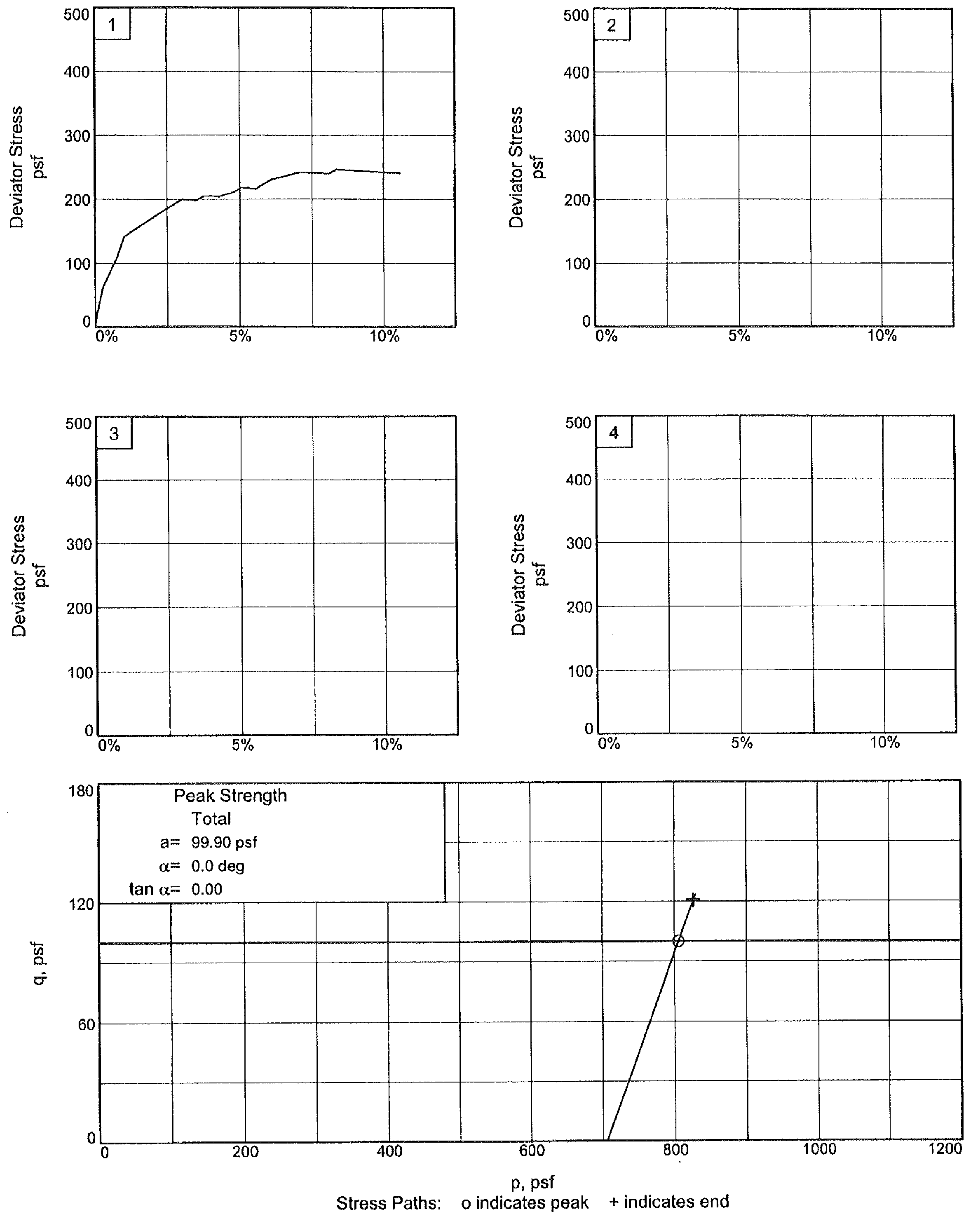
Sample Number: 7

Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

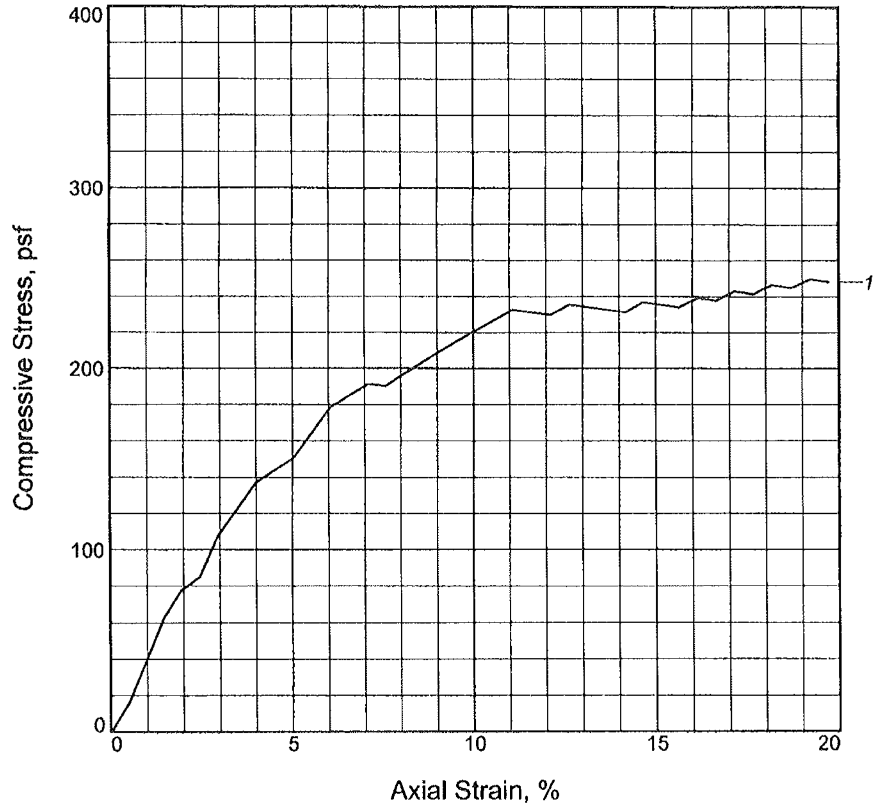


Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 7 **Depth:** 13 **Sample Number:** 7
Project No.: 19183 **Figure** 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	191.4			
Undrained shear strength, psf	95.7			
Failure strain, %	7.1			
Strain rate, in./min.	0.056			
Water content, %	74.0			
Wet density, pcf	99.3			
Dry density, pcf	57.1			
Saturation, %	101.6			
Void ratio	1.9966			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: XSO G CL W/ SA POC

LL =	PL =	PI =	Assumed GS= 2.74	Type: UNDISTURBED

Project No.: 19183

Date:

Remarks:
TORVANE = 0.190 TSF

Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source of Sample: 7 **Depth:** 19

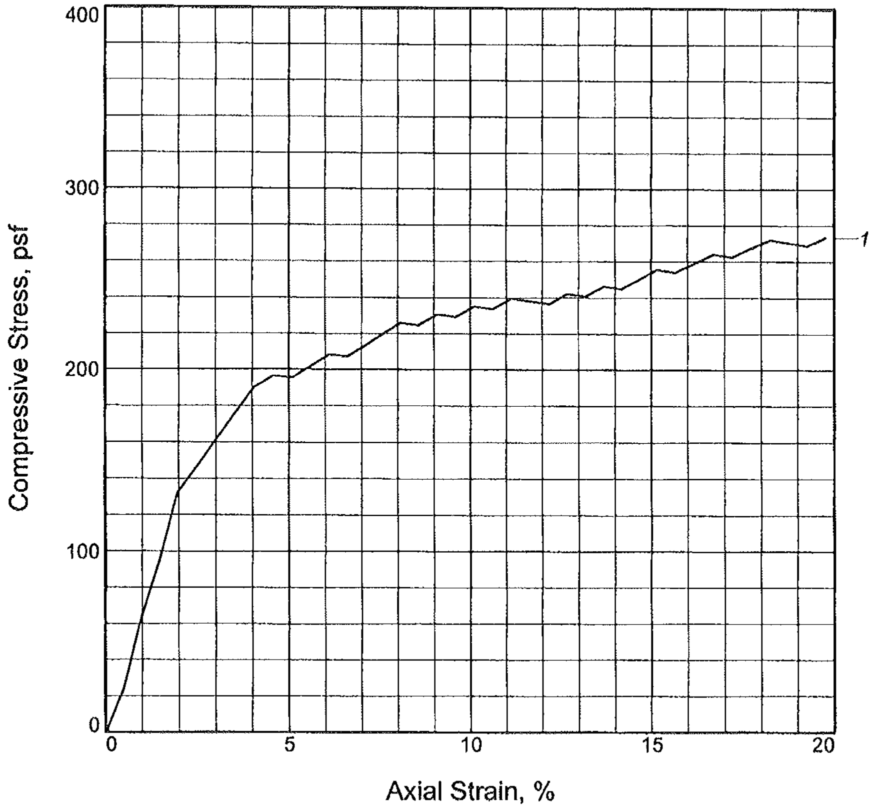
Sample Number: 10

UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	196.6			
Undrained shear strength, psf	98.3			
Failure strain, %	4.6			
Strain rate, in./min.	0.058			
Water content, %	75.6			
Wet density, pcf	96.3			
Dry density, pcf	54.8			
Saturation, %	98.1			
Void ratio	2.0969			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: XSO G CL W/ SA POC

LL = 74	PL = 22	PI = 52	Assumed GS= 2.72	Type: UNDISTURBED

Project No.: 19183
Date:

Remarks:
TORVANE = 0.170 TSF

Figure 1

Client: HNTB CORPORATION

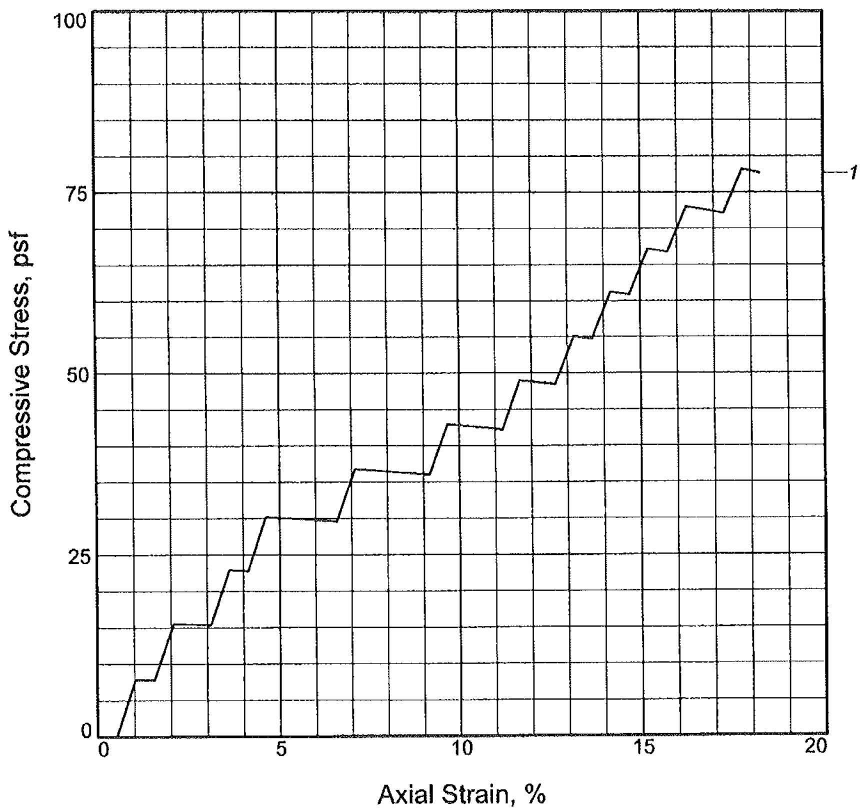
Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 7 **Depth:** 34
Sample Number: 13

UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	22.9			
Undrained shear strength, psf	11.5			
Failure strain, %	3.6			
Strain rate, in./min.	0.059			
Water content, %	528.0			
Wet density, pcf	64.6			
Dry density, pcf	10.3			
Saturation, %	93.2			
Void ratio	13.8763			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: XSO BL PT W/ RTS

LL =	PL =	PI =	Assumed GS= 2.45	Type: UNDISTRBED

Project No.: 19183
Date:
Remarks:
TORVANE = 0.050 TSF

Figure 1

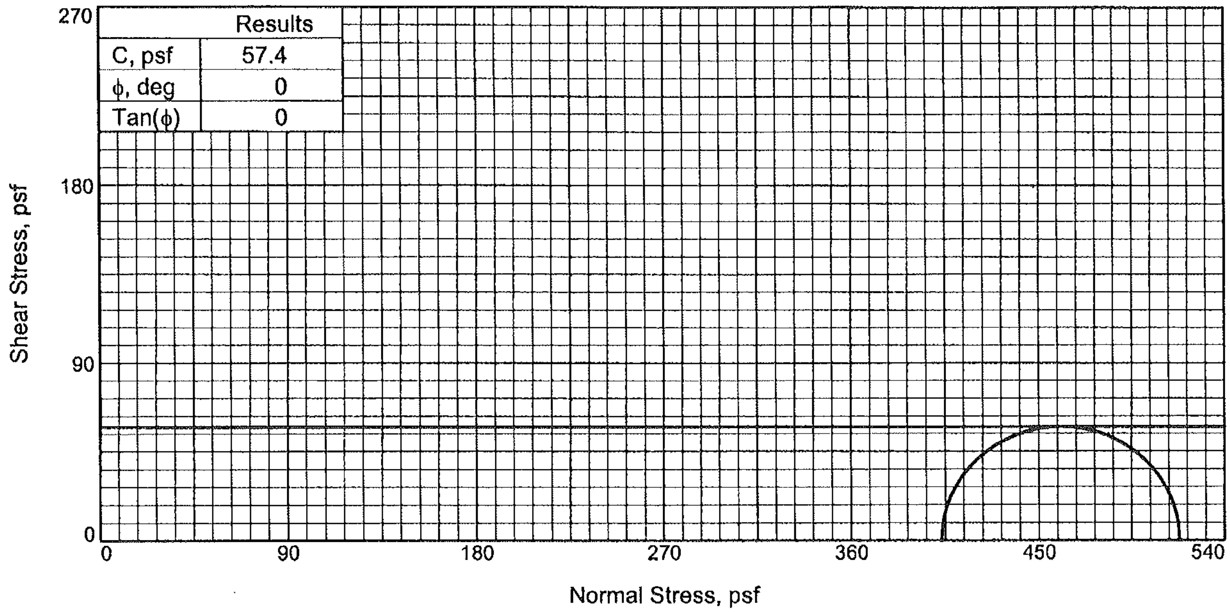
Client: HNTB CORPORATION

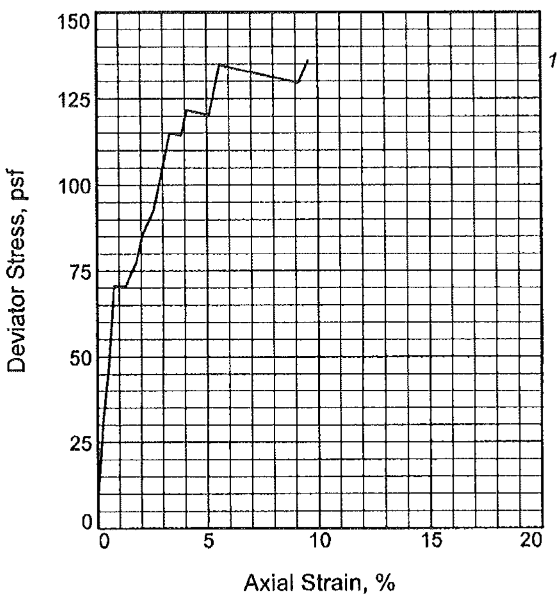
Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 8 **Depth:** 1
Sample Number: 1

UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD





Specimen No.		1
Initial	Water Content,	208.2
	Dry Density, pcf	24.6
	Saturation,	96.9
	Void Ratio	5.5850
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	214.8
	Dry Density, pcf	24.6
	Saturation,	100.0
	Void Ratio	5.5850
	Diameter, in.	1.391
	Height, in.	2.929
Strain rate, in./min.		0.029
Back Pressure, psf		0.0
Cell Pressure, psf		403.2
Fail. Stress, psf		114.9
Ult. Stress, psf		136.1
σ_1 Failure, psf		518.1
σ_3 Failure, psf		403.2

Type of Test:
Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: XSO DK G ORG CL

LL= 224 **PL=** 67 **PI=** 157
Assumed Specific Gravity= 2.60
Remarks: TORVANE = 0.100 TSF

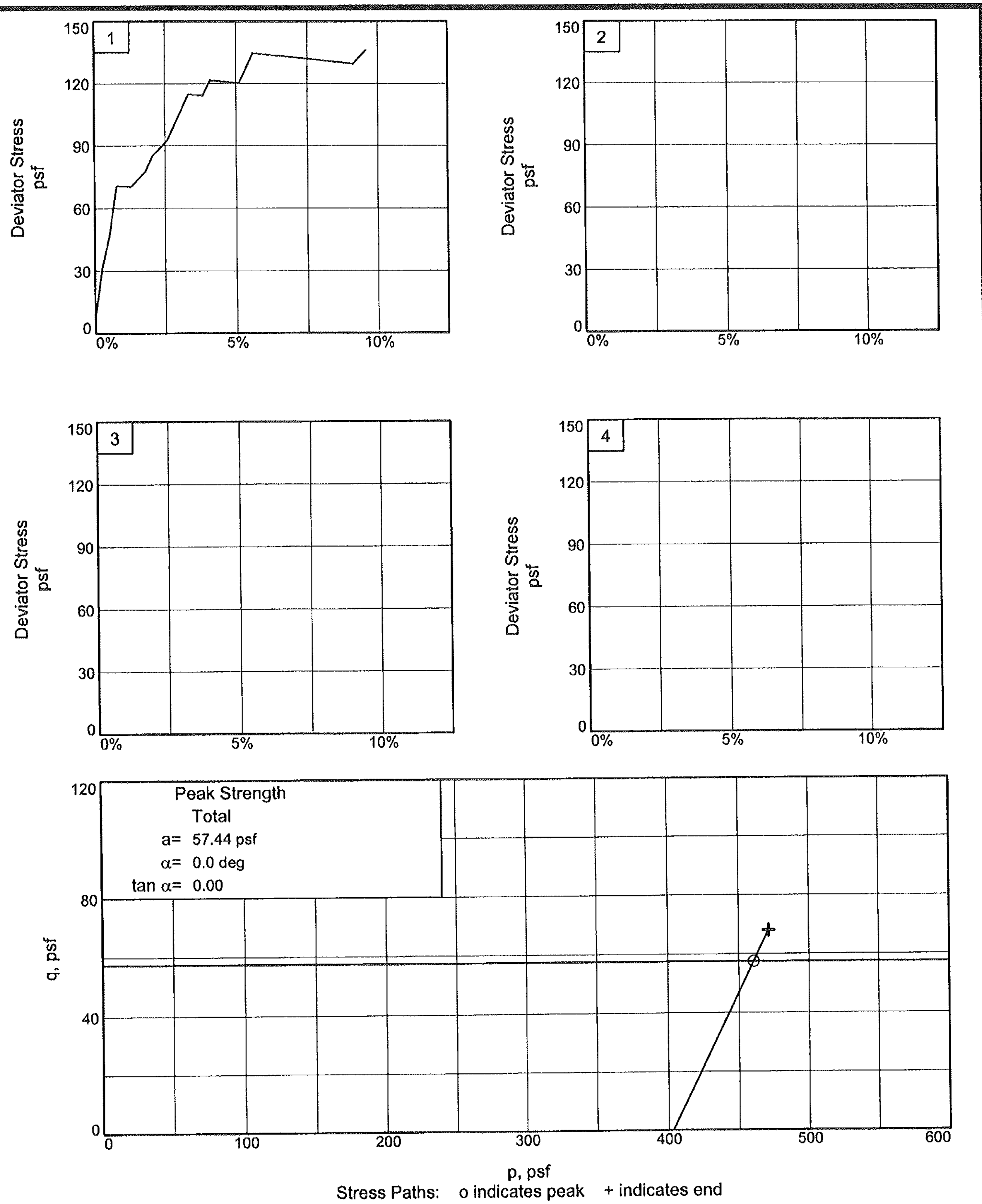
Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 8 **Depth:** 7
Sample Number: 4
Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT
EUSTIS ENGINEERING COMPANY, INC.

Figure 1

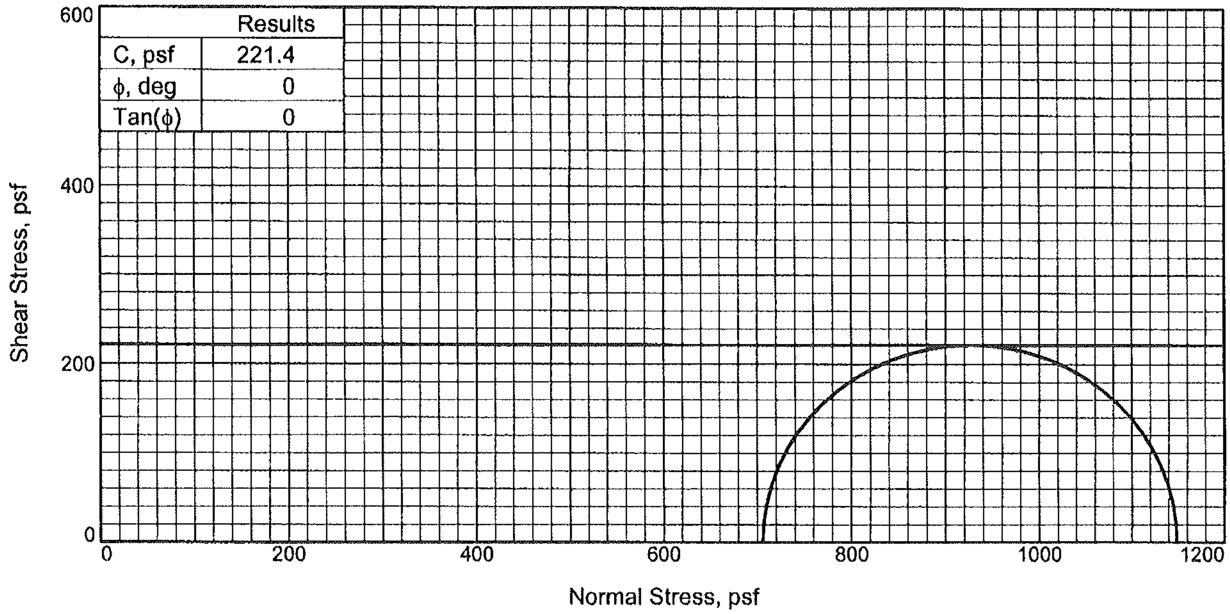
Tested By: RA **Checked By:** BD



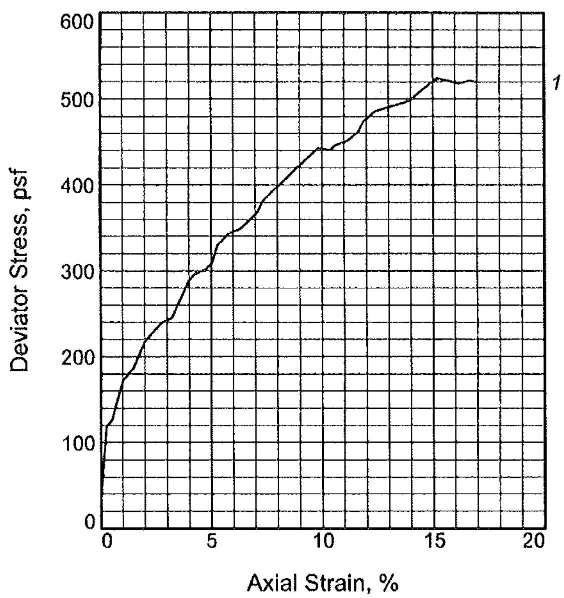
Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 8 **Depth:** 7 **Sample Number:** 4
Project No.: 19183 **Figure 2**

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD



Results	
C, psf	221.4
ϕ, deg	0
Tan(ϕ)	0



Specimen No.		1
Initial	Water Content,	38.4
	Dry Density, pcf	82.8
	Saturation,	101.2
	Void Ratio	1.0129
	Diameter, in.	1.391
	Height, in.	2.929
At Test	Water Content,	37.9
	Dry Density, pcf	82.8
	Saturation,	100.0
	Void Ratio	1.0129
	Diameter, in.	1.391
	Height, in.	2.929
Strain rate, in./min.		0.029
Back Pressure, psf		0.0
Cell Pressure, psf		705.6
Fail. Stress, psf		442.9
Ult. Stress, psf		519.9
σ_1 Failure, psf		1148.5
σ_3 Failure, psf		705.6

Type of Test:
Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: VLO G CLSI

Assumed Specific Gravity= 2.67
Remarks:

Figure 1

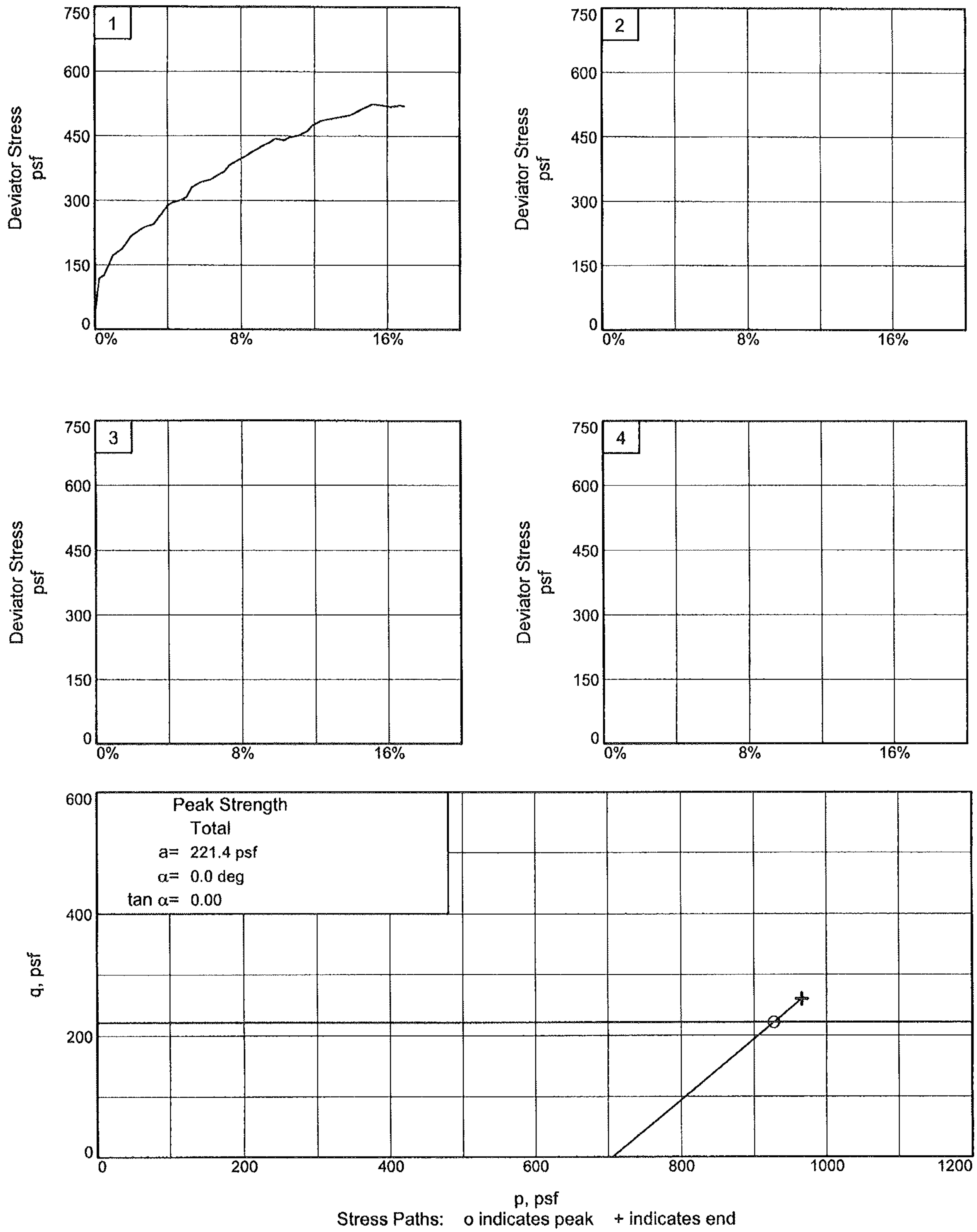
Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 8 **Depth:** 13
Sample Number: 7
Proj. No.: 19183 **Date:**

TRIAXIAL SHEAR TEST REPORT

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD



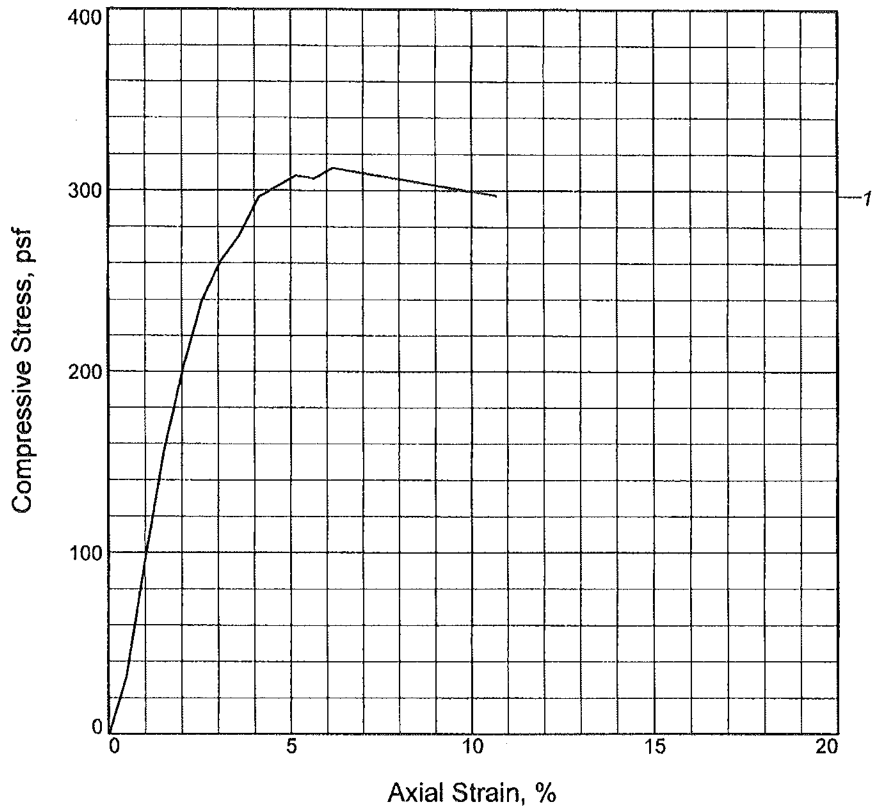
Client: HNTB CORPORATION
Project: STATE OF LOUISIANA
Source of Sample: 8 **Depth:** 13 **Sample Number:** 7
Project No.: 19183

Figure 2

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	308.1			
Undrained shear strength, psf	154.1			
Failure strain, %	5.1			
Strain rate, in./min.	0.058			
Water content, %	80.6			
Wet density, pcf	93.4			
Dry density, pcf	51.7			
Saturation, %	95.7			
Void ratio	2.3089			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: VSO G CL (FLOC)

LL =	**PL =**	**PI =**	**Assumed GS=** 2.74	**Type:** UNDISTURBED

Project No.: 19183
Date:

Remarks:
TORVANE = 0.150 TSF

Figure 1

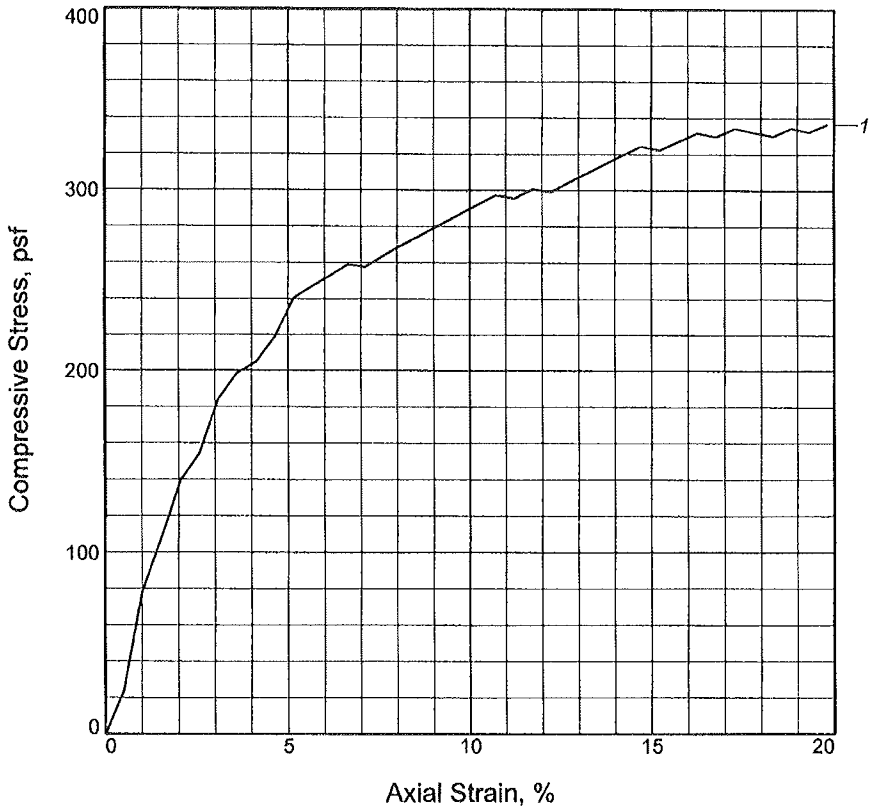
Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT
Source of Sample: 8 **Depth:** 19
Sample Number: 10

UNCONFINED COMPRESSION TEST
EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

UNCONFINED COMPRESSION TEST



Specimen No.	1			
Unconfined strength, psf	258.9			
Undrained shear strength, psf	129.4			
Failure strain, %	6.6			
Strain rate, in./min.	0.059			
Water content, %	56.7			
Wet density, pcf	100.2			
Dry density, pcf	64.0			
Saturation, %	93.2			
Void ratio	1.6534			
Specimen diameter, in.	1.391			
Specimen height, in.	2.929			
Height/diameter ratio	2.11			

Description: VSO G CL W/ SA LEN & POC (FLOC)

LL =	PL =	PI =	Assumed GS= 2.72	Type: UNDISTURBED

Project No.: 19183

Date:

Remarks:
TORVANE = 0.140 TSF

Figure 1

Client: HNTB CORPORATION

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source of Sample: 8 **Depth:** 29

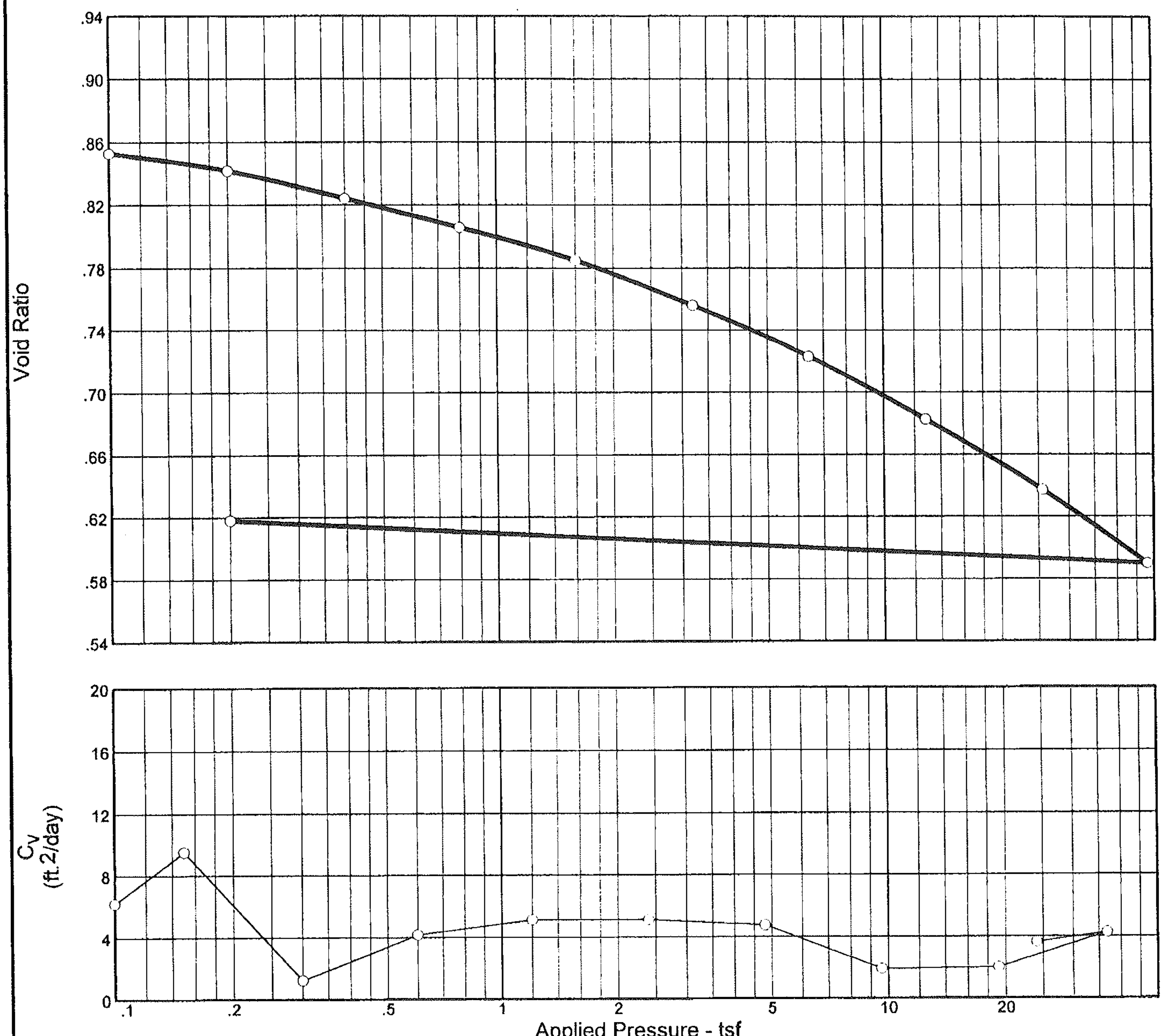
Sample Number: 12

UNCONFINED COMPRESSION TEST

EUSTIS ENGINEERING COMPANY, INC.

Tested By: RA **Checked By:** BD

CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	P_c (tsf)	C_c	Initial Void Ratio
Saturation	Moisture							
87.7 %	28.6 %	89.0	32	7	2.67	8.97	0.17	0.872

MATERIAL DESCRIPTION	USCS	AASHTO
Loose gray sandy silt	ML	

Project No. 19183 **Client:**

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source: 4 **Sample No.:** 4 **Elev./Depth:** 7.0

EUSTIS ENGINEERING COMPANY, INC.

METAIRIE, LA

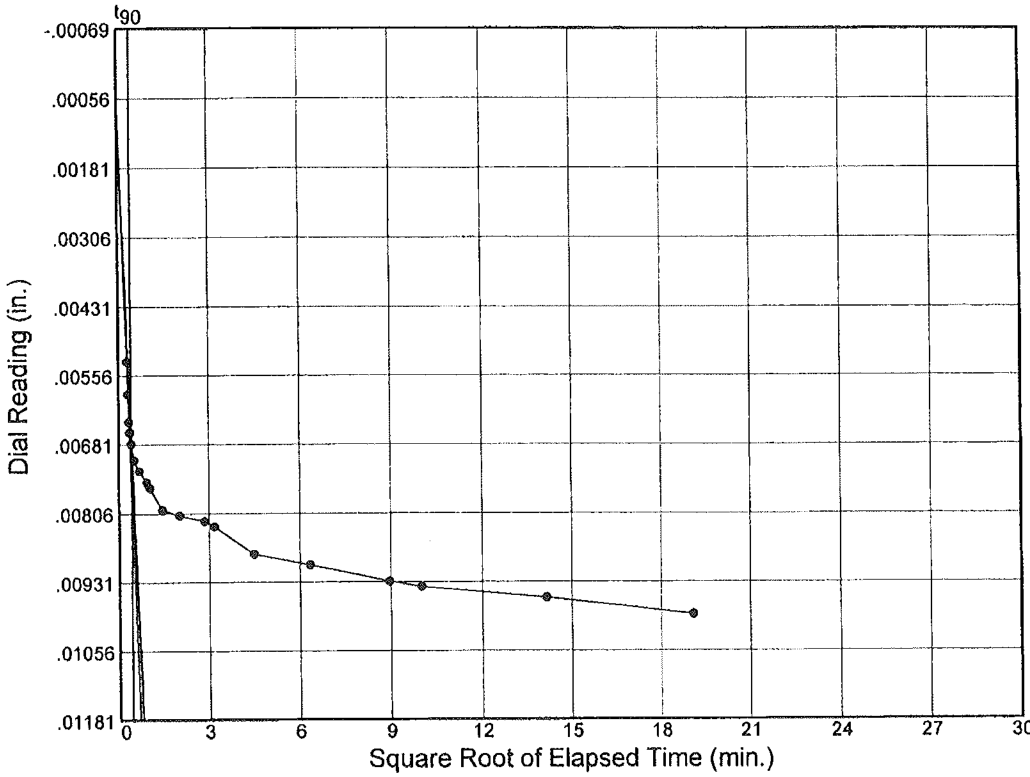
Remarks:
Tested by : BD, RNE

Figure

Dial Reading vs. Time

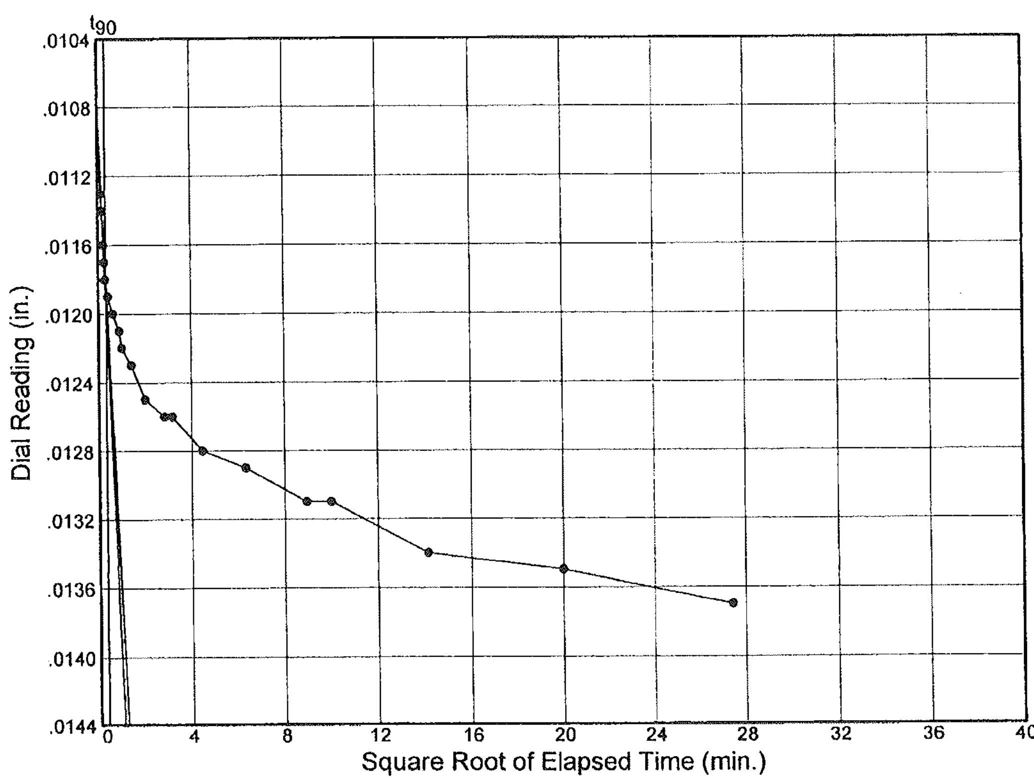
Project No.: 19183
Project: STATE OF LOUISIANA

Source: 4 Sample No.: 4 Elev./Depth: 7



Load No.= 1
Load= 0.10 tsf
D_0 = 0.00056
D_{90} = 0.00685
D_{100} = 0.00755
T_{90} = 0.19 min.

C_v @ T_{90}
6.20 ft.^{2}/day



Load No.= 2
Load= 0.20 tsf
D_0 = 0.01081
D_{90} = 0.01184
D_{100} = 0.01196
T_{90} = 0.12 min.

C_v @ T_{90}
9.51 ft.^{2}/day

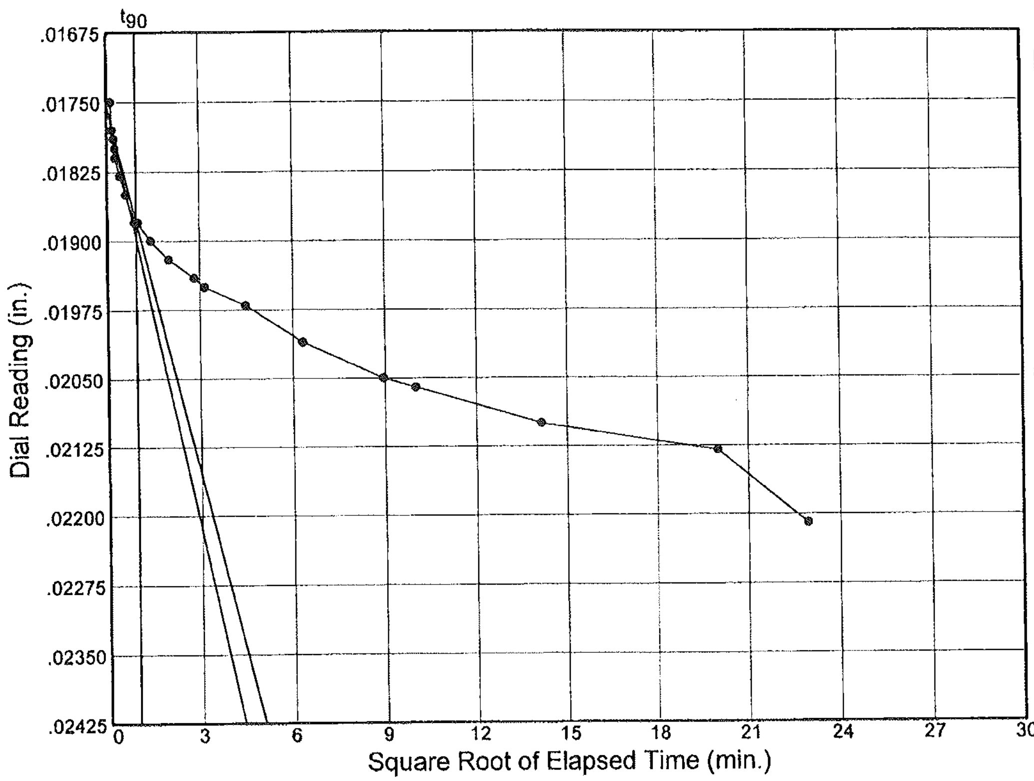
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

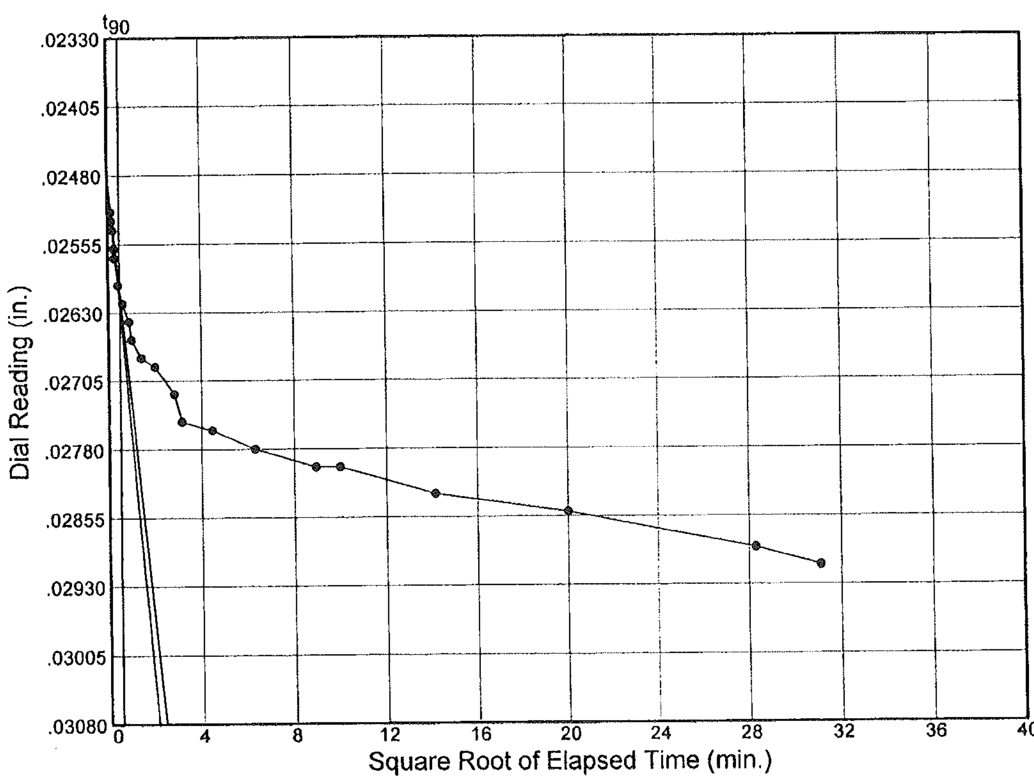
Project No.: 19183
Project: STATE OF LOUISIANA

Source: 4 | Sample No.: 4 | Elev./Depth: 7



Load No.= 3
Load= 0.40 tsf
D_0 = 0.01753
D_{90} = 0.01880
D_{100} = 0.01894
T_{90} = 0.90 min.

C_v @ T_{90}
1.23 ft.^{2}/day



Load No.= 4
Load= 0.80 tsf
D_0 = 0.02480
D_{90} = 0.02609
D_{100} = 0.02623
T_{90} = 0.26 min.

C_v @ T_{90}
4.17 ft.^{2}/day

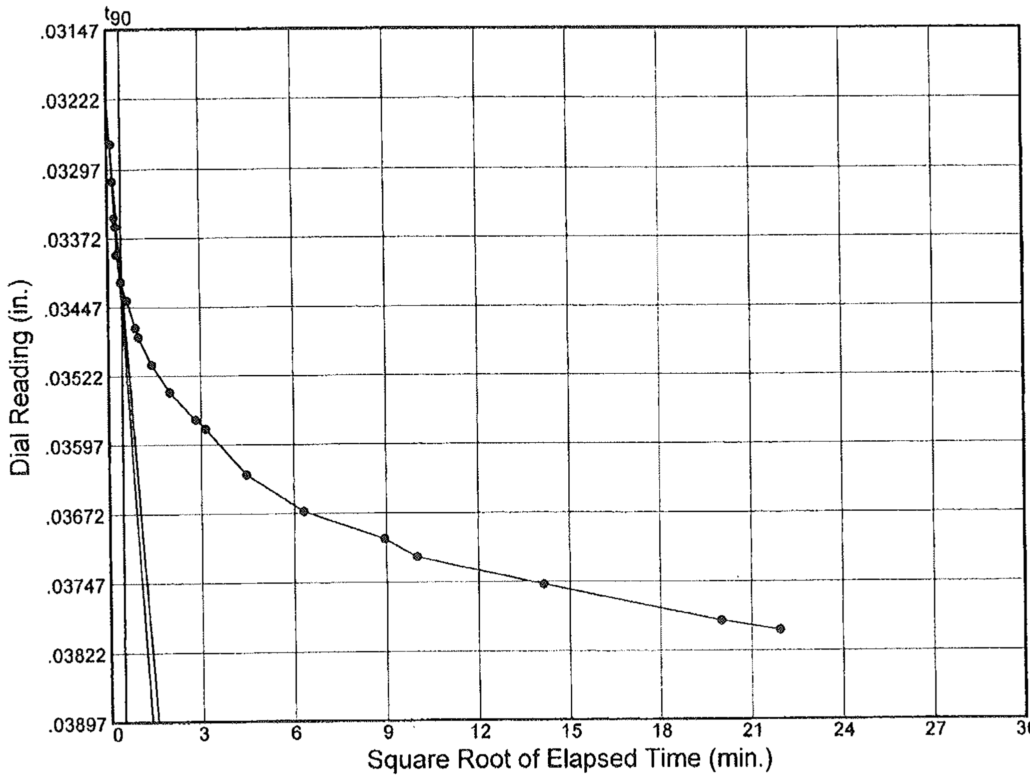
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

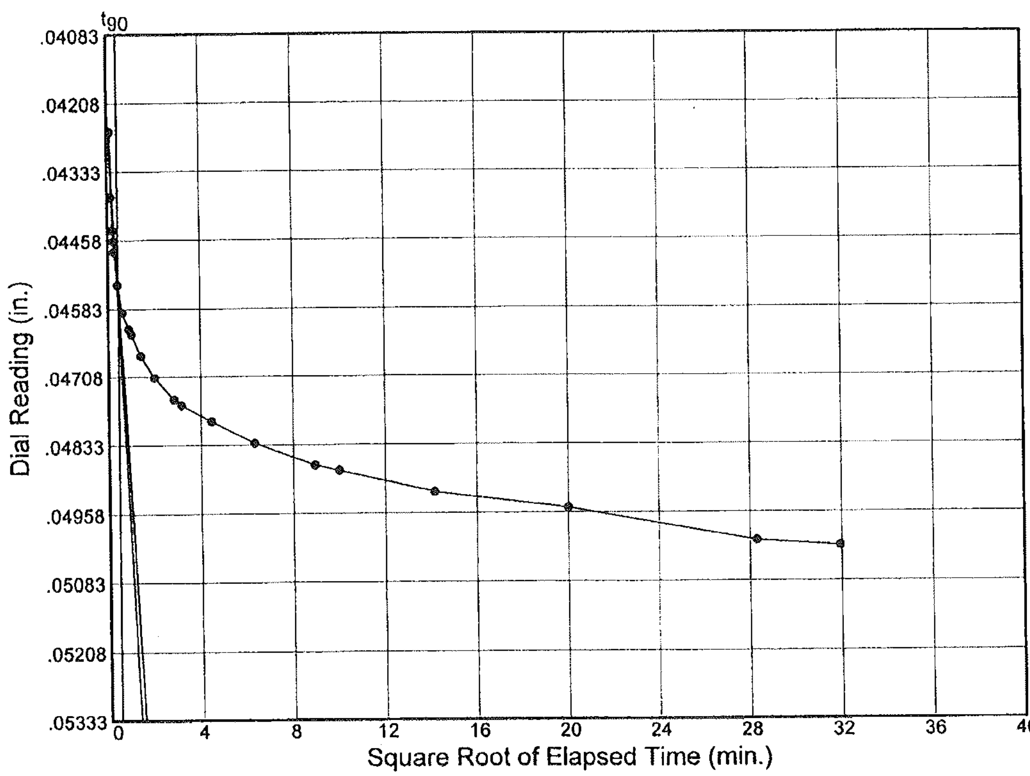
Project No.: 19183
Project: STATE OF LOUISIANA

Source: 4 Sample No.: 4 Elev./Depth: 7



Load No.= 5
Load= 1.60 tsf
D_0 = 0.03222
D_{90} = 0.03423
D_{100} = 0.03446
T_{90} = 0.21 min.

C_v @ T_{90}
5.10 ft.^{2}/day



Load No.= 6
Load= 3.20 tsf
D_0 = 0.04209
D_{90} = 0.04546
D_{100} = 0.04584
T_{90} = 0.20 min.

C_v @ T_{90}
5.11 ft.^{2}/day

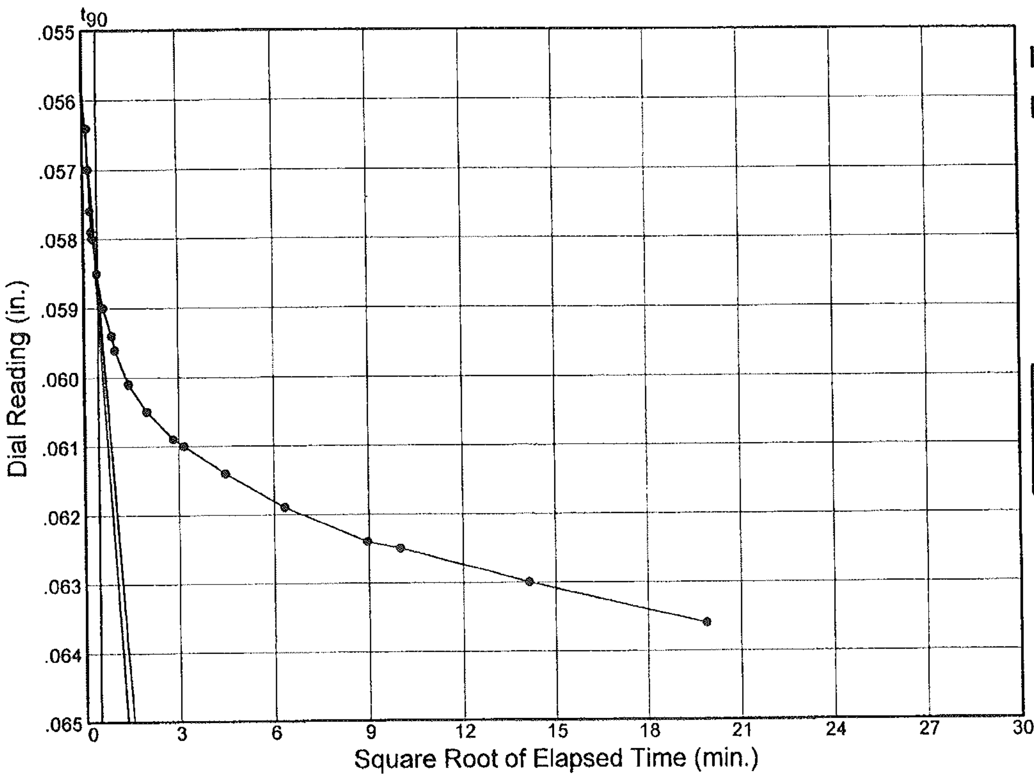
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

Source: 4 Sample No.: 4 Elev./Depth: 7



Load No.= 7

Load= 6.40 tsf

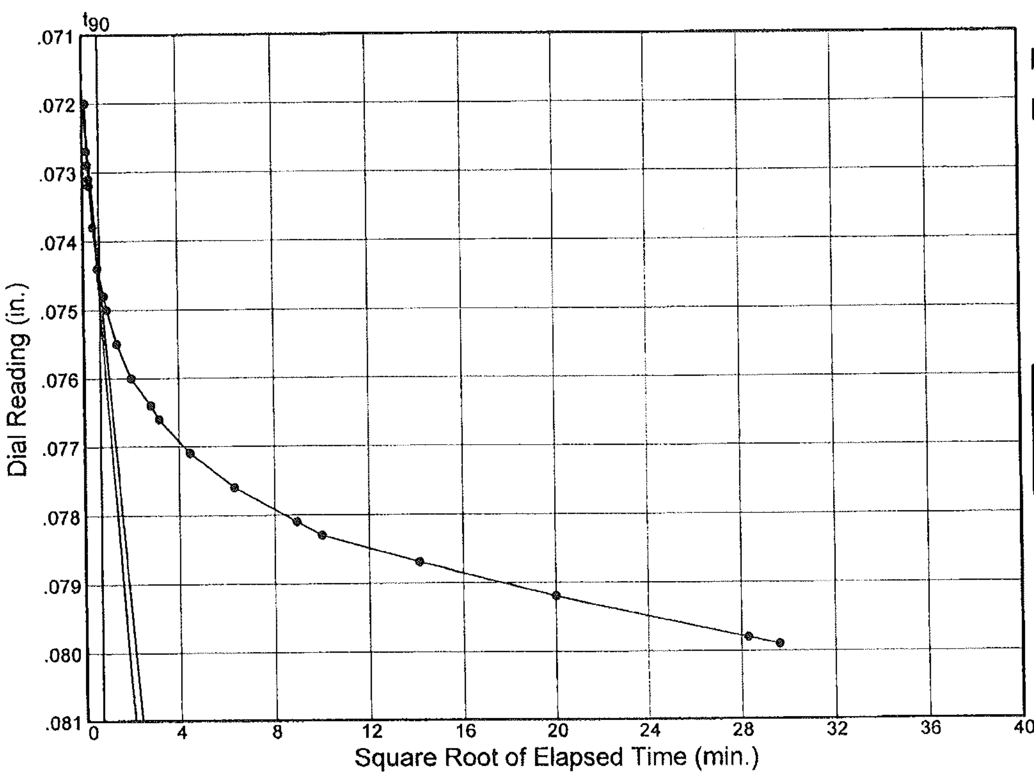
D_0 = 0.05580

D_{90} = 0.05858

D_{100} = 0.05889

T_{90} = 0.21 min.

C_v @ T_{90}
4.75 ft.^{2}/day



Load No.= 8

Load= 12.80 tsf

D_0 = 0.07180

D_{90} = 0.07453

D_{100} = 0.07483

T_{90} = 0.49 min.

C_v @ T_{90}
1.92 ft.^{2}/day

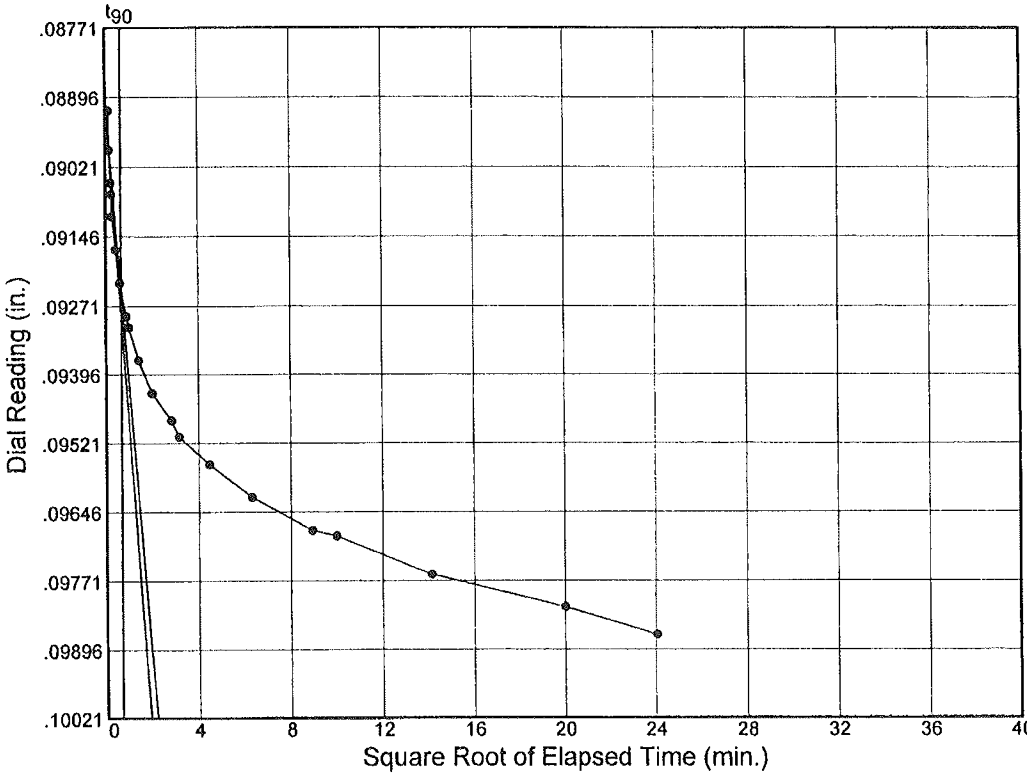
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

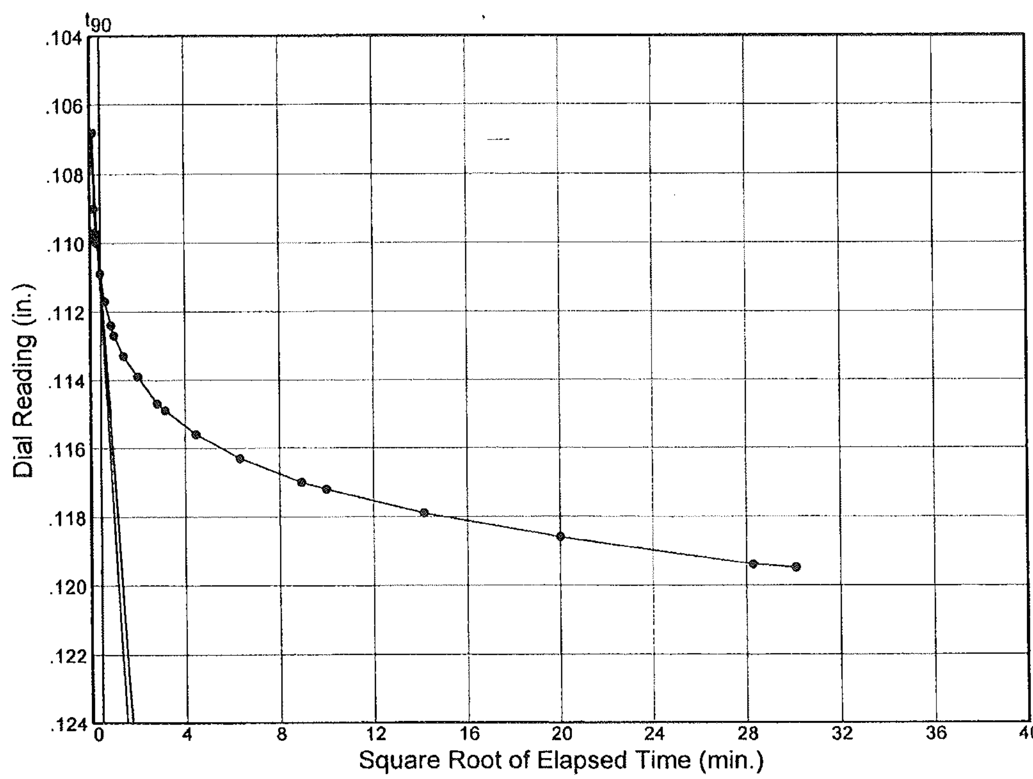
Project No.: 19183
Project: STATE OF LOUISIANA

Source: 4 | Sample No.: 4 | Elev./Depth: 7



Load No.= 9
Load= 25.60 tsf
D_0 = 0.08896
D_{90} = 0.09241
D_{100} = 0.09280
T_{90} = 0.44 min.

C_v @ T_{90}
2.01 ft.^{2}/day



Load No.= 10
Load= 48.00 tsf
D_0 = 0.10640
D_{90} = 0.11098
D_{100} = 0.11149
T_{90} = 0.20 min.

C_v @ T_{90}
4.26 ft.^{2}/day

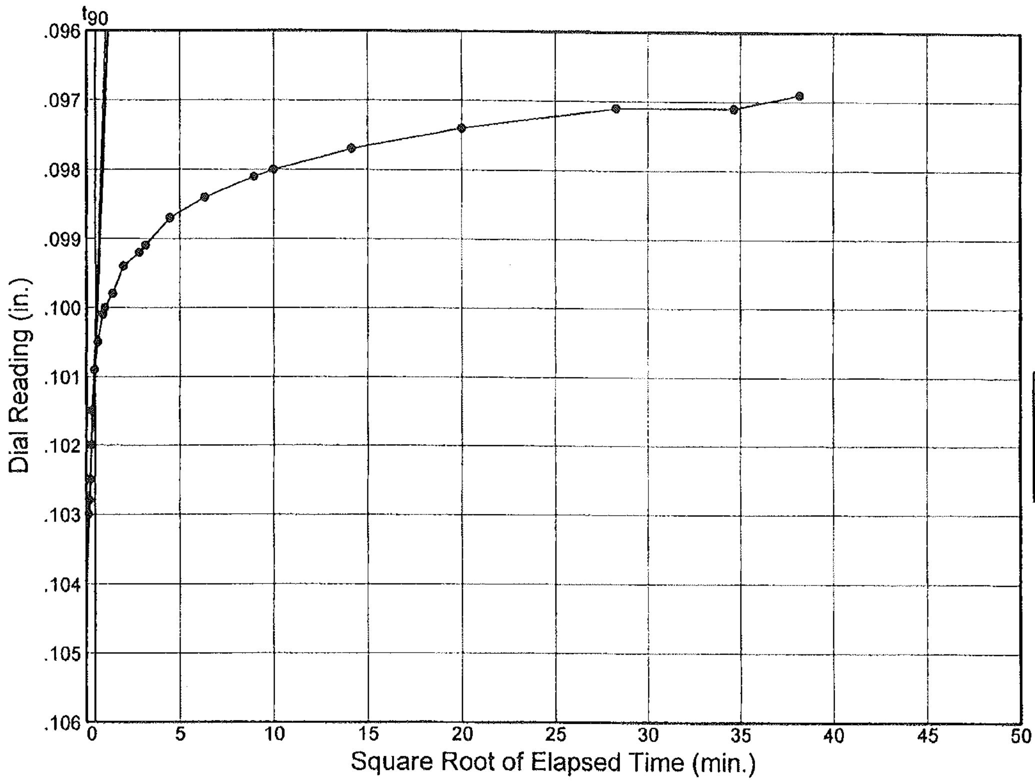
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

Source: 4 | Sample No.: 4 | Elev./Depth: 7



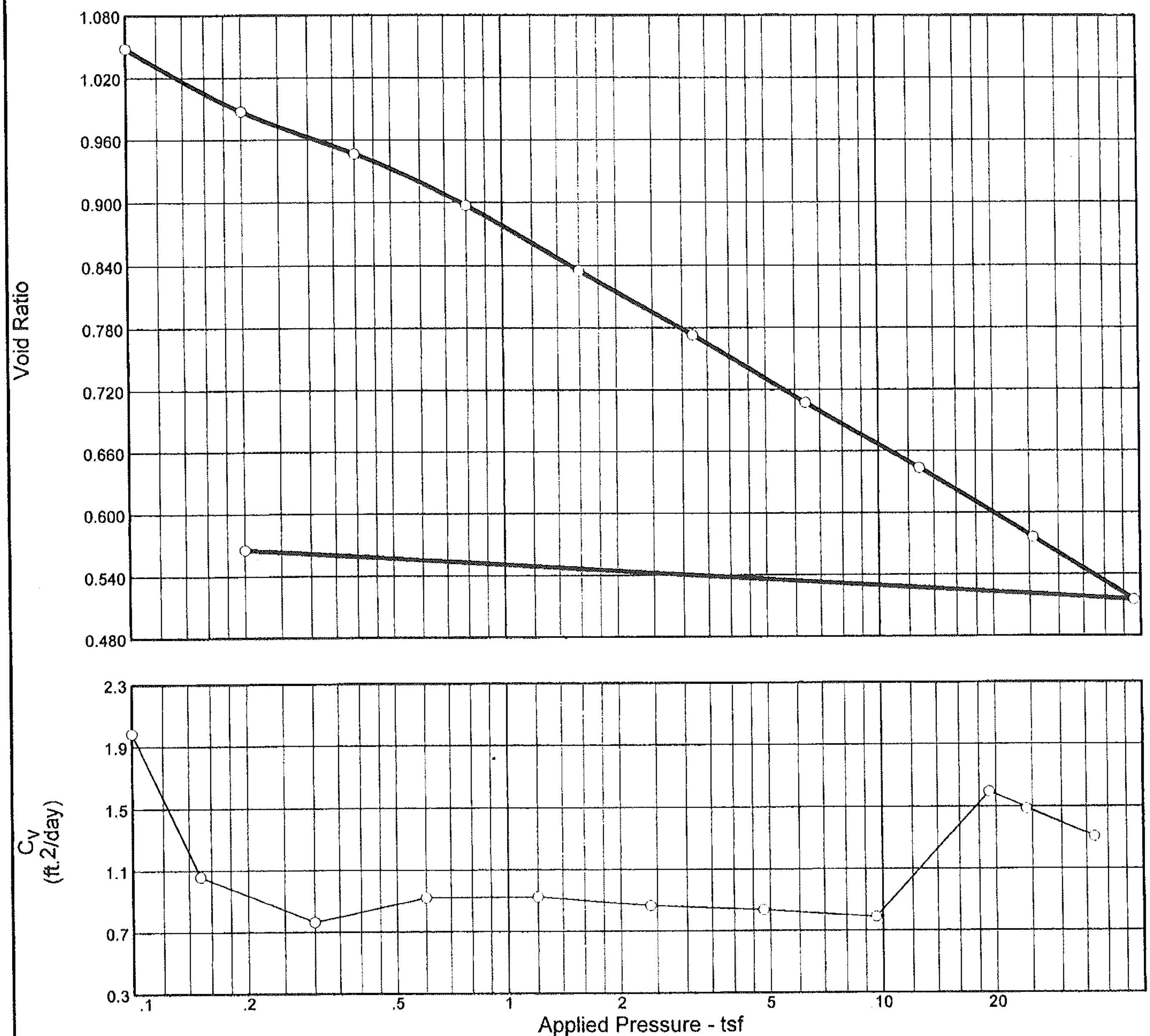
Load No.= 11
Load= 0.20 tsf
D_0 = 0.10409
D_{90} = 0.10077
D_{100} = 0.10040
T_{90} = 0.24 min.

C_v @ T_{90}
3.63 ft.^{2}/day

EUSTIS ENGINEERING COMPANY, INC.

Figure

CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	P_c (tsf)	C_c	Initial Void Ratio
Saturation	Moisture							
86.1 %	33.9 %	81.6	35	9	2.69	1.01	0.22	1.058

MATERIAL DESCRIPTION	USCS	AASHTO
medium compact gray clayey silt	ML	

Project No. 19183 **Client:**

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source: 5 **Sample No.:** 9 **Elev./Depth:** 17

EUSTIS ENGINEERING COMPANY, INC.

METAIRIE, LA

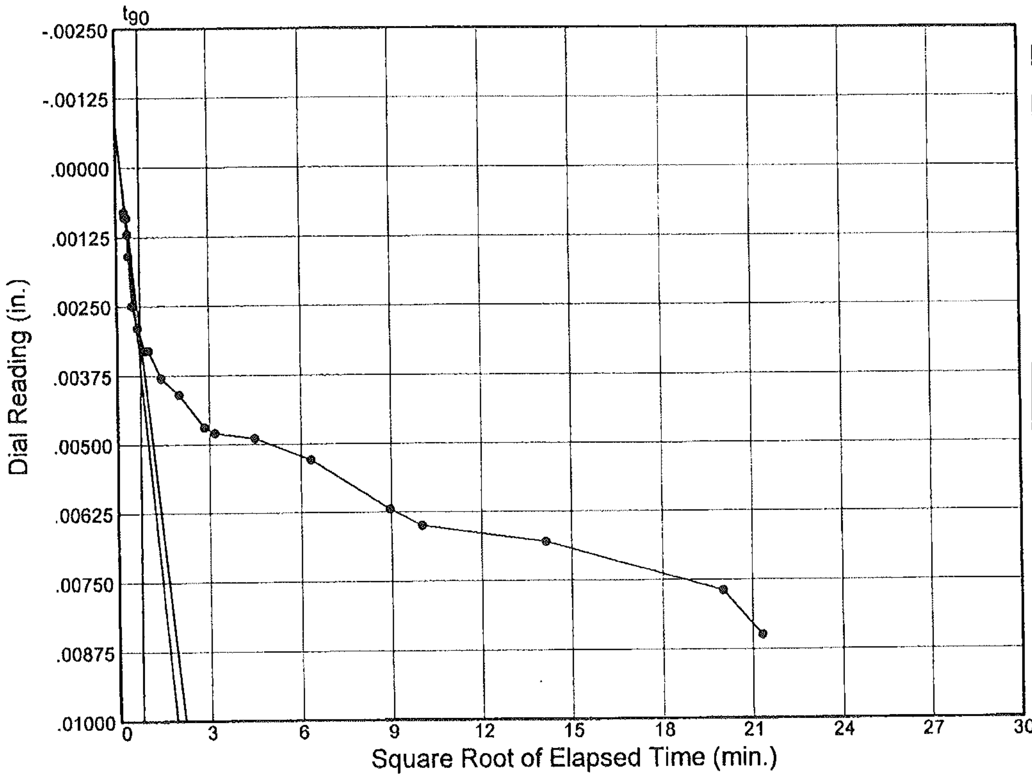
Remarks:
Tested by : BD, RNE

Figure

Dial Reading vs. Time

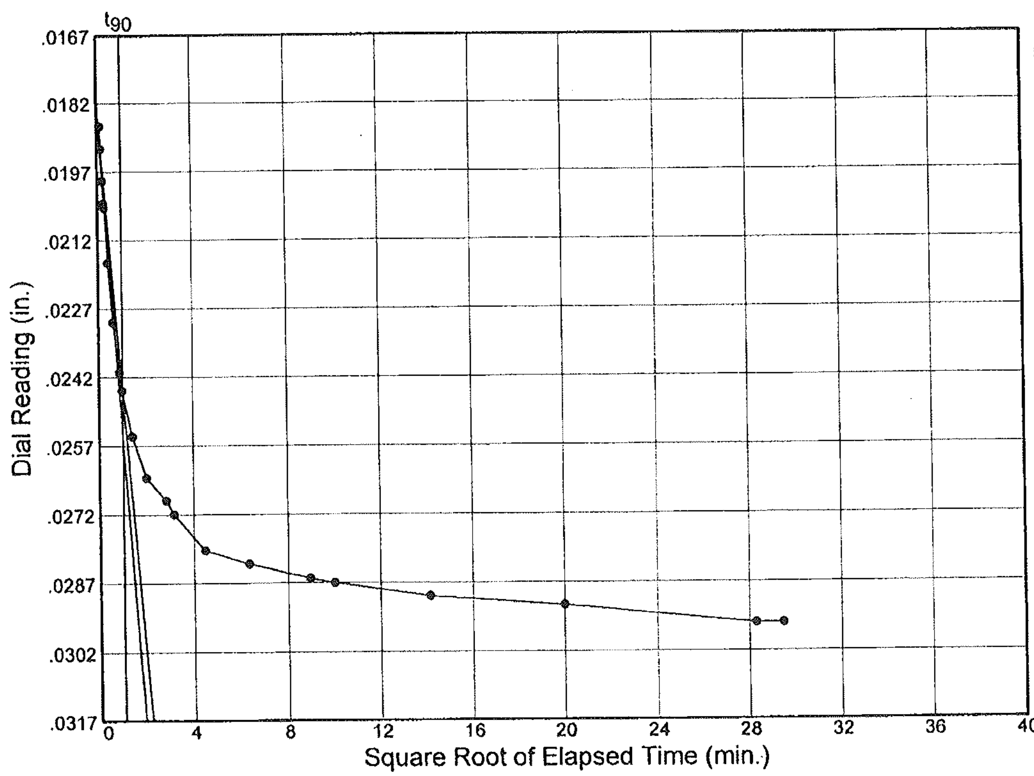
Project No.: 19183
Project: STATE OF LOUISIANA

Source: 5 Sample No.: 9 Elev./Depth: 17



Load No.= 1
Load= 0.10 tsf
D_0 = -0.00081
D_{90} = 0.00305
D_{100} = 0.00348
T_{90} = 0.58 min.

C_v @ T_{90}
1.98 ft.^{2}/day



Load No.= 2
Load= 0.20 tsf
D_0 = 0.01825
D_{90} = 0.02456
D_{100} = 0.02526
T_{90} = 1.03 min.

C_v @ T_{90}
1.06 ft.^{2}/day

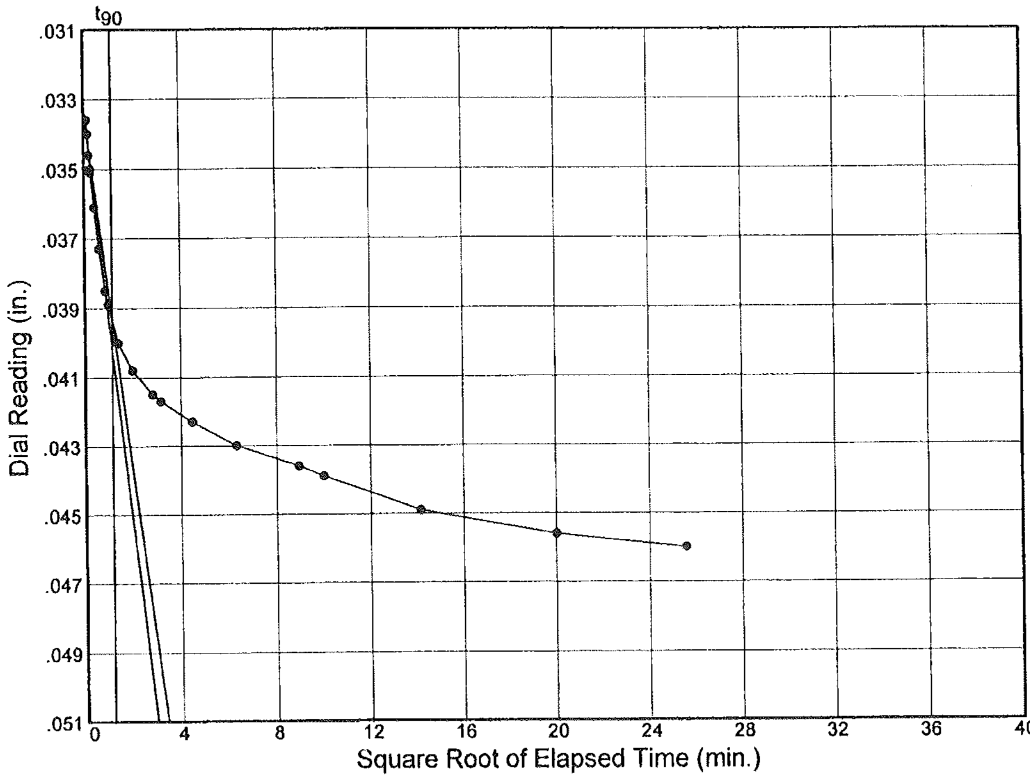
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

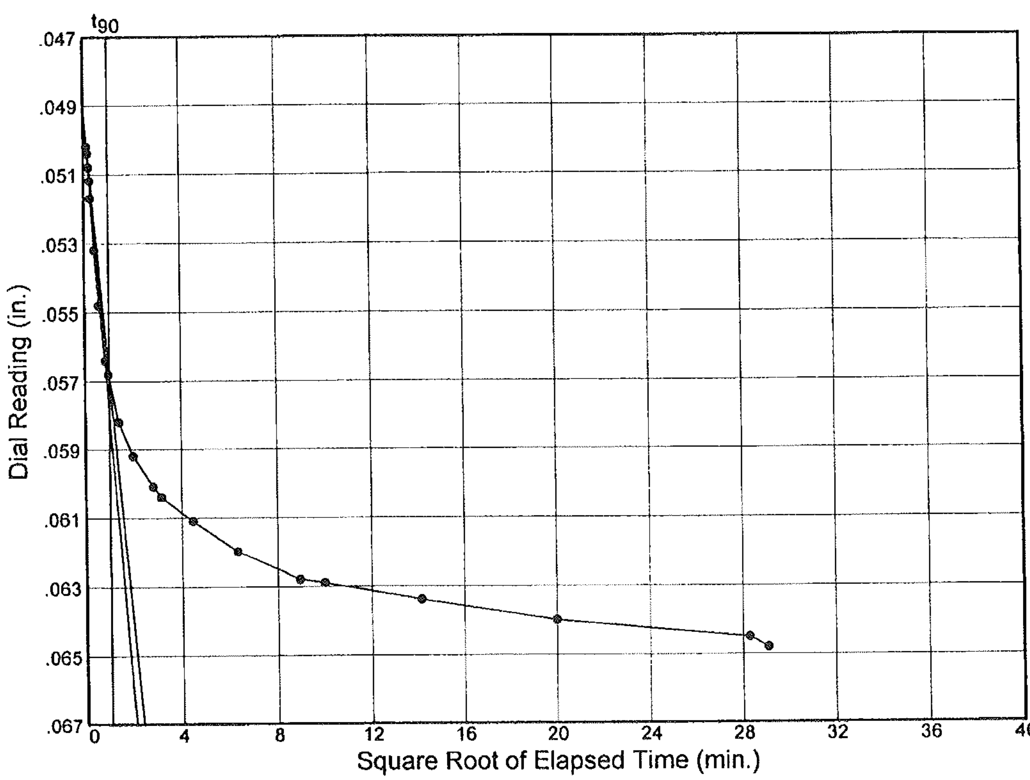
Project No.: 19183
Project: STATE OF LOUISIANA

Source: 5 Sample No.: 9 Elev./Depth: 17



Load No.= 3
Load= 0.40 tsf
D_0 = 0.03321
D_{90} = 0.03938
D_{100} = 0.04006
T_{90} = 1.37 min.

C_v @ T_{90}
0.76 ft.^{2}/day



Load No.= 4
Load= 0.80 tsf
D_0 = 0.04904
D_{90} = 0.05697
D_{100} = 0.05785
T_{90} = 1.08 min.

C_v @ T_{90}
0.92 ft.^{2}/day

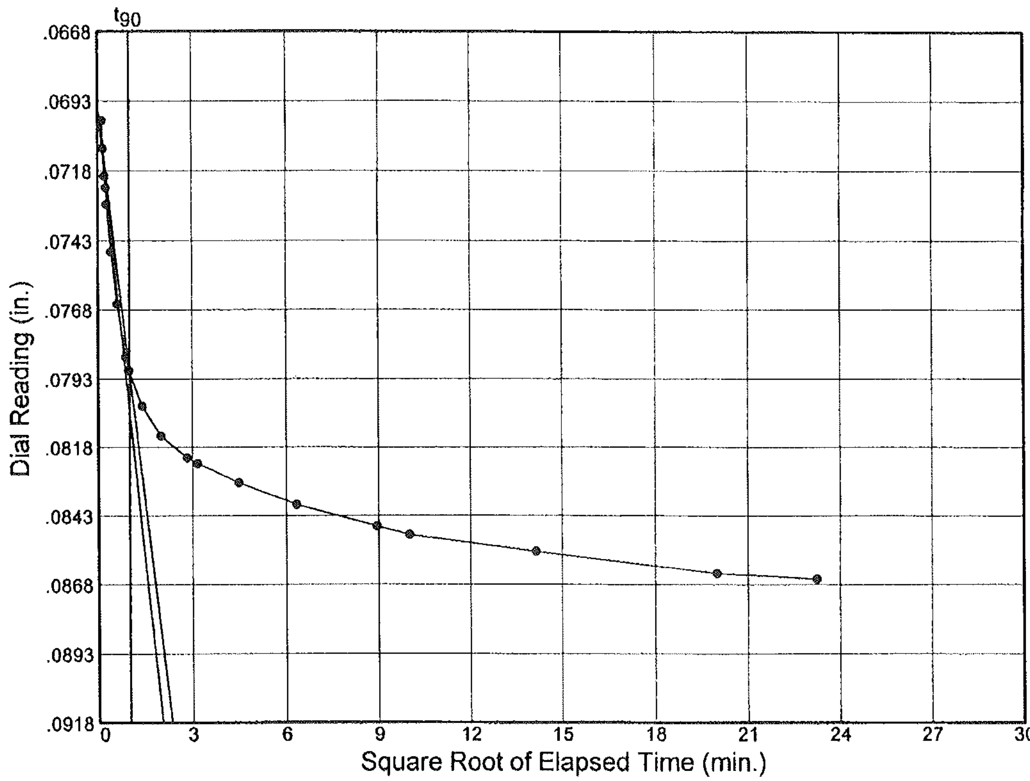
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

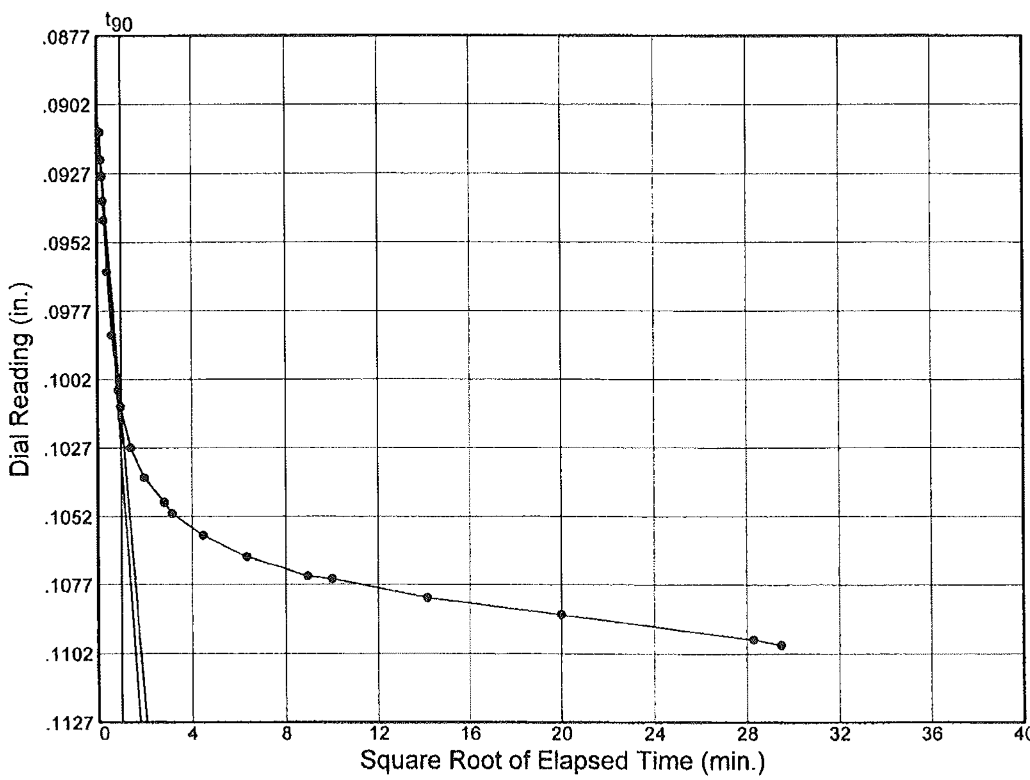
Project No.: 19183
Project: STATE OF LOUISIANA

Source: 5 Sample No.: 9 Elev./Depth: 17



Load No.= 5
Load= 1.60 tsf
D_0 = 0.06936
D_{90} = 0.07905
D_{100} = 0.08013
T_{90} = 1.01 min.

C_v @ T_{90}
0.92 ft.^{2}/day



Load No.= 6
Load= 3.20 tsf
D_0 = 0.09024
D_{90} = 0.10126
D_{100} = 0.10249
T_{90} = 1.01 min.

C_v @ T_{90}
0.87 ft.^{2}/day

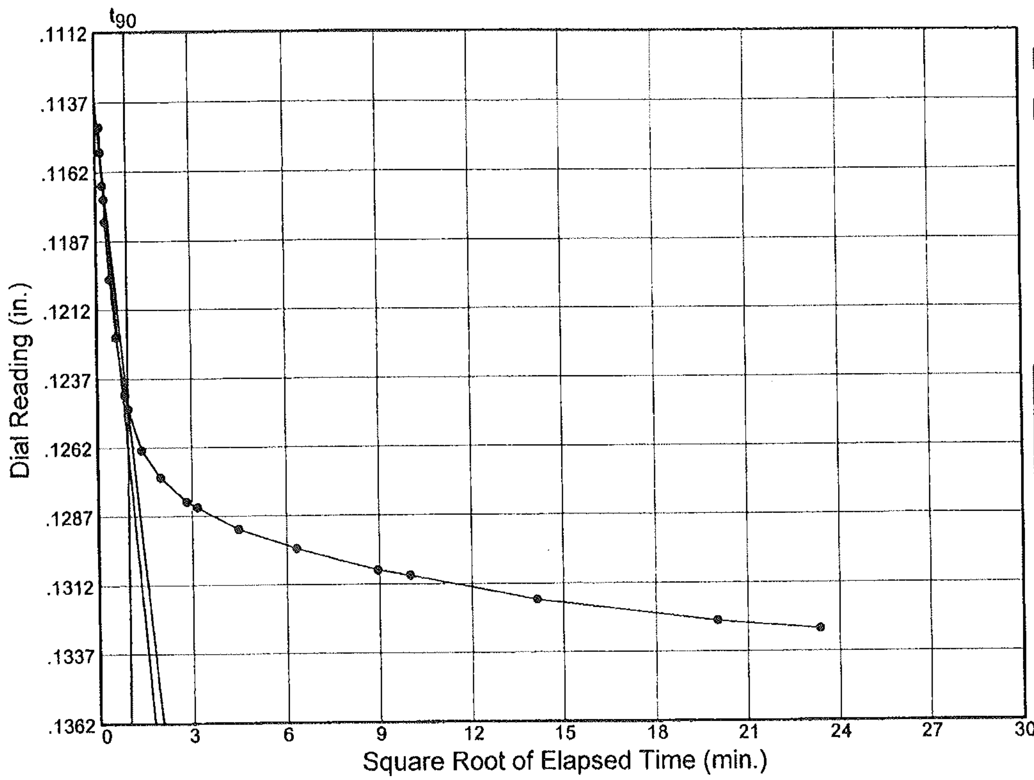
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

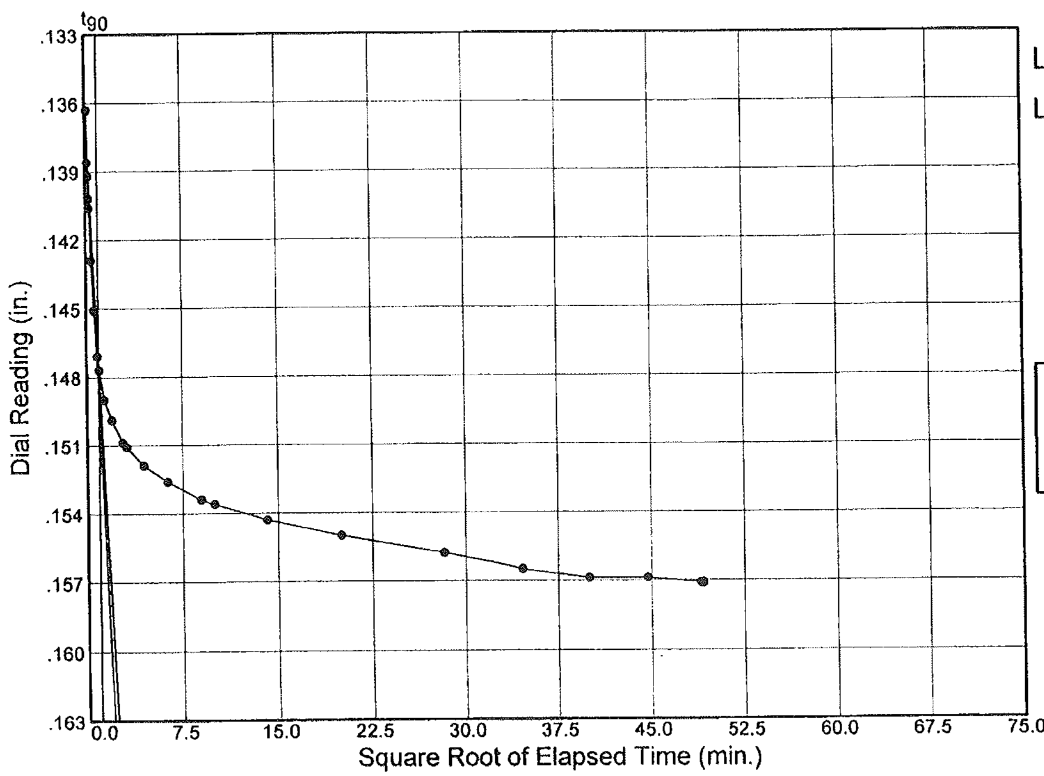
Project No.: 19183
Project: STATE OF LOUISIANA

Source: 5 Sample No.: 9 Elev./Depth: 17



Load No.= 7
Load= 6.40 tsf
D_0 = 0.11373
D_{90} = 0.12479
D_{100} = 0.12602
T_{90} = 0.97 min.

C_v @ T_{90}
0.84 ft.^{2}/day



Load No.= 8
Load= 12.80 tsf
D_0 = 0.13619
D_{90} = 0.14763
D_{100} = 0.14891
T_{90} = 0.96 min.

C_v @ T_{90}
0.79 ft.^{2}/day

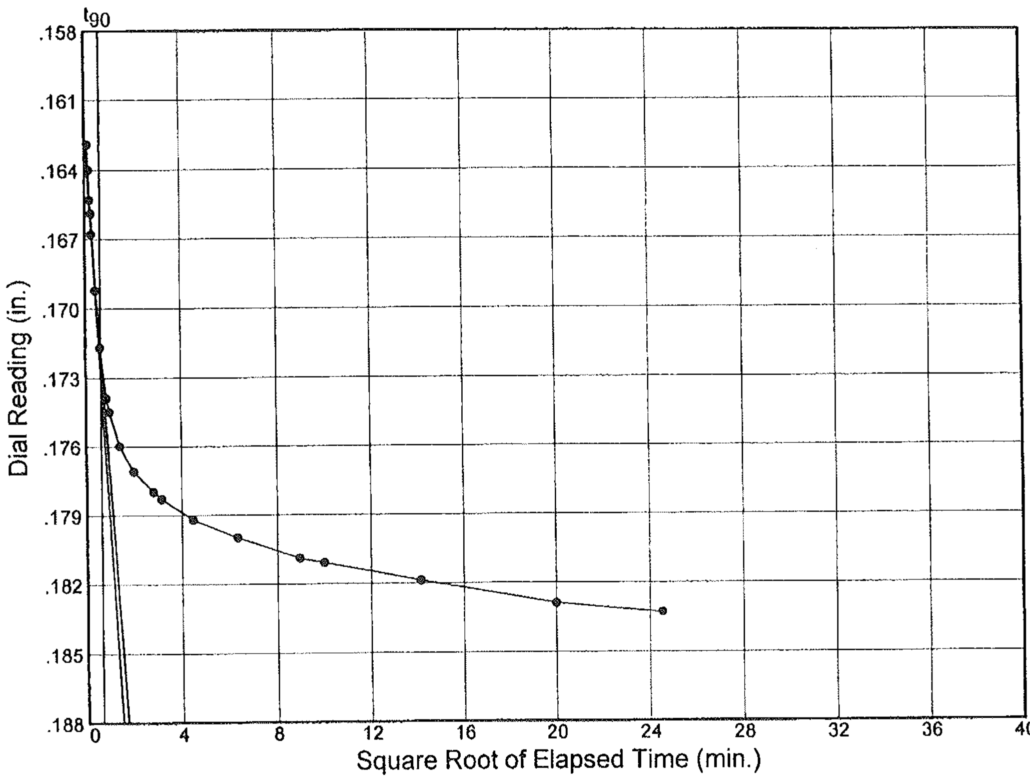
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

Source: 5 | Sample No.: 9 | Elev./Depth: 17



Load No.= 9

Load= 25.60 tsf

D_0 = 0.16195

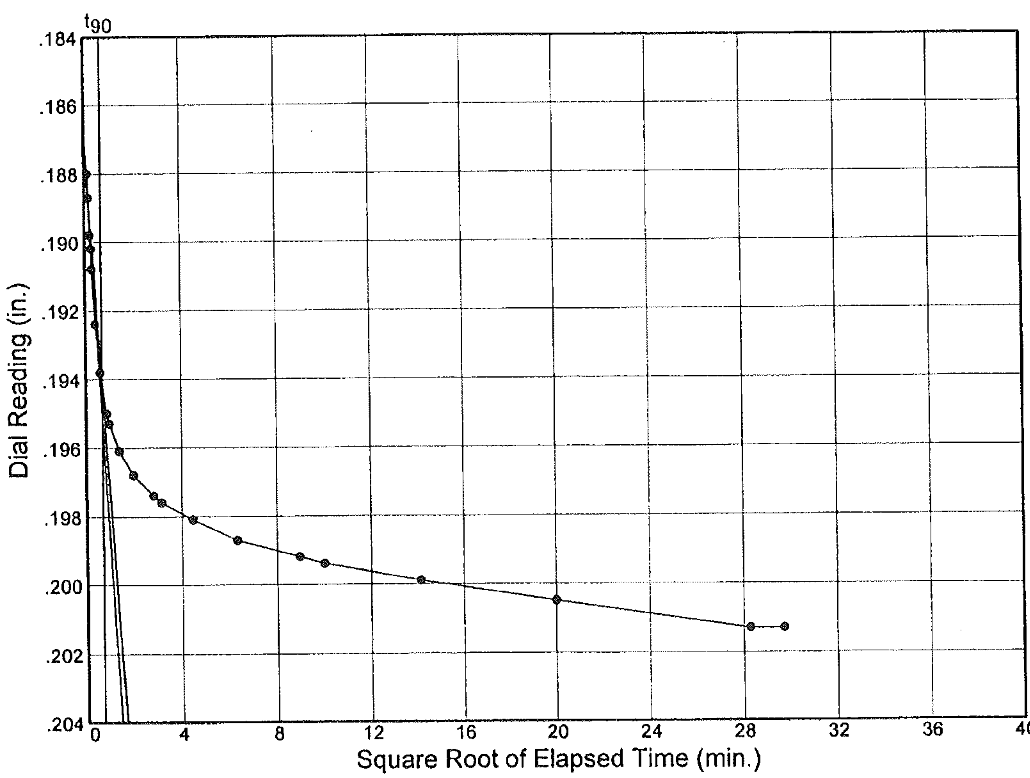
D_{90} = 0.17205

D_{100} = 0.17318

T_{90} = 0.43 min.

C_v @ T_{90}

1.60 ft.^{2}/day



Load No.= 10

Load= 48.00 tsf

D_0 = 0.18689

D_{90} = 0.19418

D_{100} = 0.19498

T_{90} = 0.49 min.

C_v @ T_{90}

1.30 ft.^{2}/day

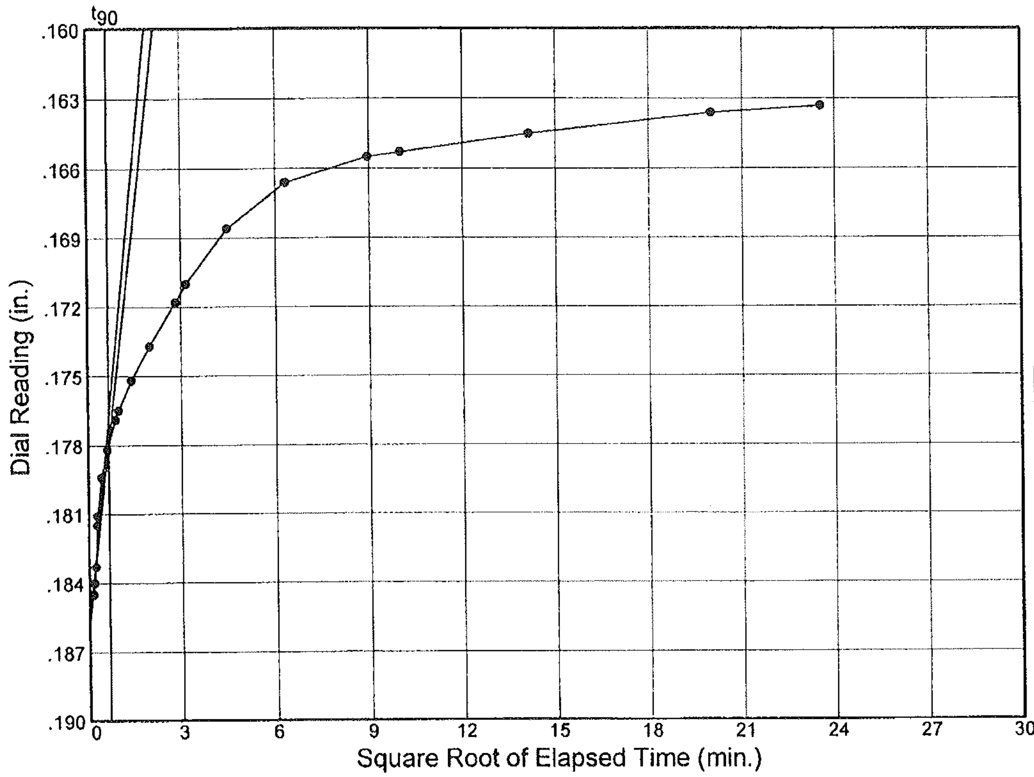
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

Source: 5 | Sample No.: 9 | Elev./Depth: 17



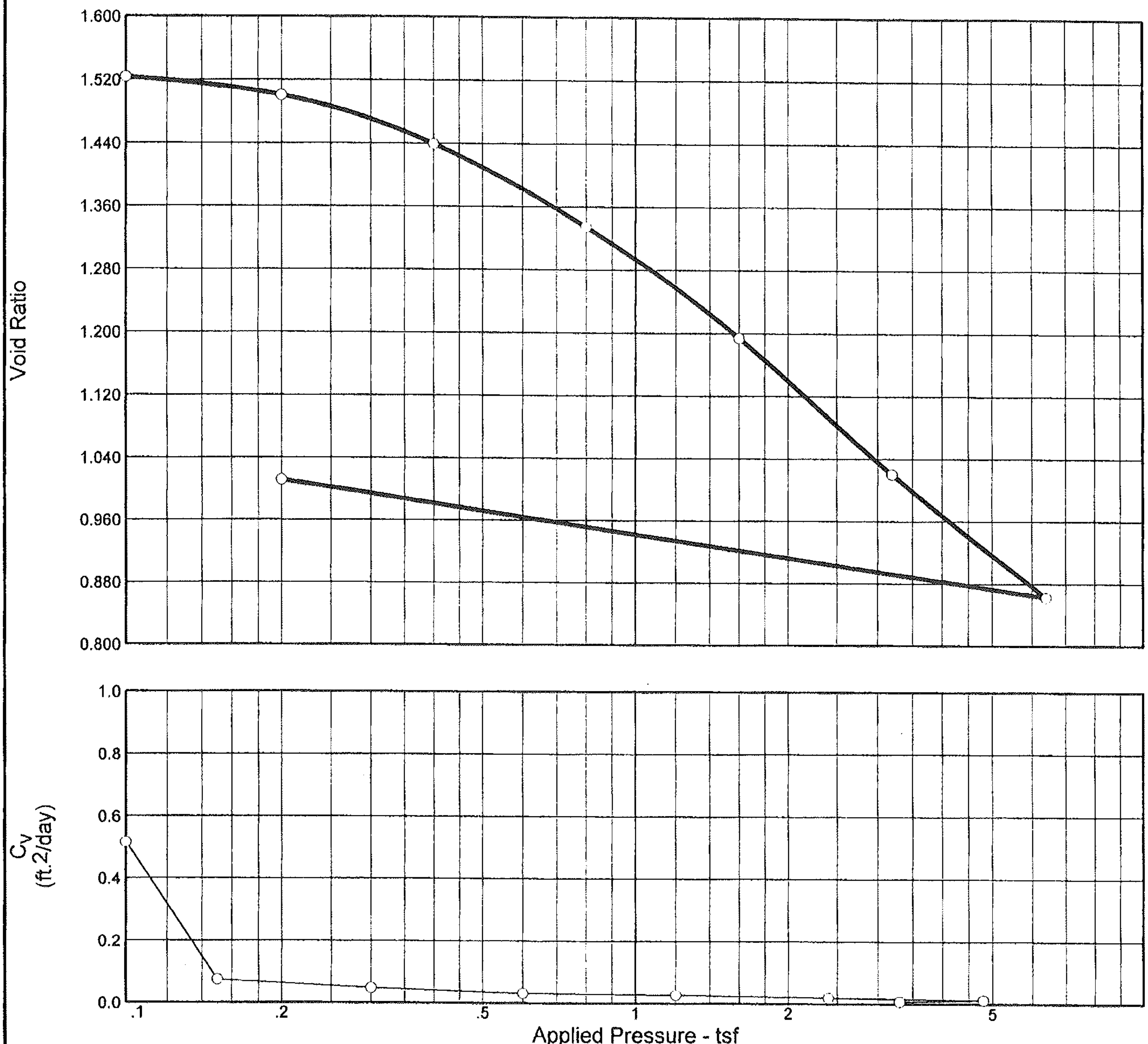
Load No.= 11
Load= 0.20 tsf
D_0 = 0.18585
D_{90} = 0.17795
D_{100} = 0.17708
T_{90} = 0.44 min.

C_v @ T_{90}
1.49 ft.^{2}/day

Figure

EUSTIS ENGINEERING COMPANY, INC.

CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	P_c (tsf)	C_c	Initial Void Ratio
Saturation	Moisture							
99.1 %	56.2 %	66.8	88	61	2.72	0.82	0.59	1.541

MATERIAL DESCRIPTION	USCS	AASHTO
very soft gray clay with silt pockets	CH	

Project No. 19183 **Client:**

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source: 6 **Sample No.:** 2 **Elev./Depth:** 3

EUSTIS ENGINEERING COMPANY, INC.

METAIRIE, LA

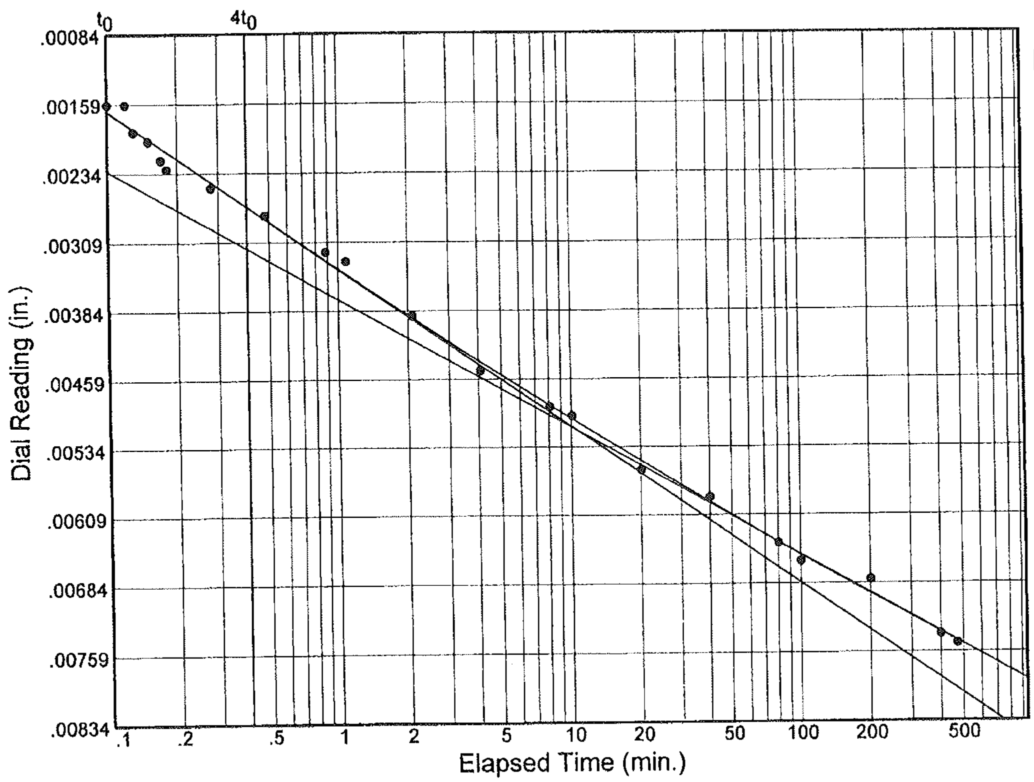
Remarks:
Tested by : BD, RNE

Figure

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

Source: 6 | Sample No.: 2 | Elev./Depth: 3



Load No.= 1

Load= 0.10 tsf

D_0 = 0.00063

D_{50} = 0.00290

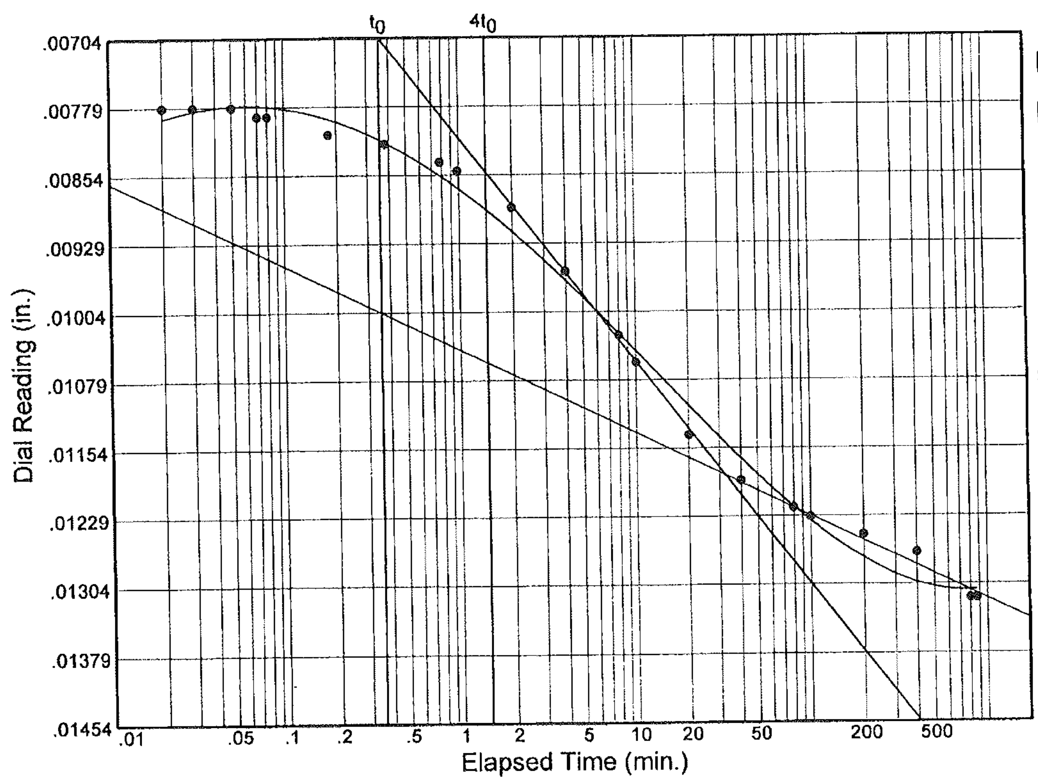
D_{100} = 0.00516

T_{50} = 0.52 min.

C_v @ T_{50}

0.52 ft.^{2}/day

C_α = 0.002



Load No.= 2

Load= 0.20 tsf

D_0 = 0.00739

D_{50} = 0.00962

D_{100} = 0.01184

T_{50} = 3.61 min.

C_v @ T_{50}

0.07 ft.^{2}/day

C_α = 0.001

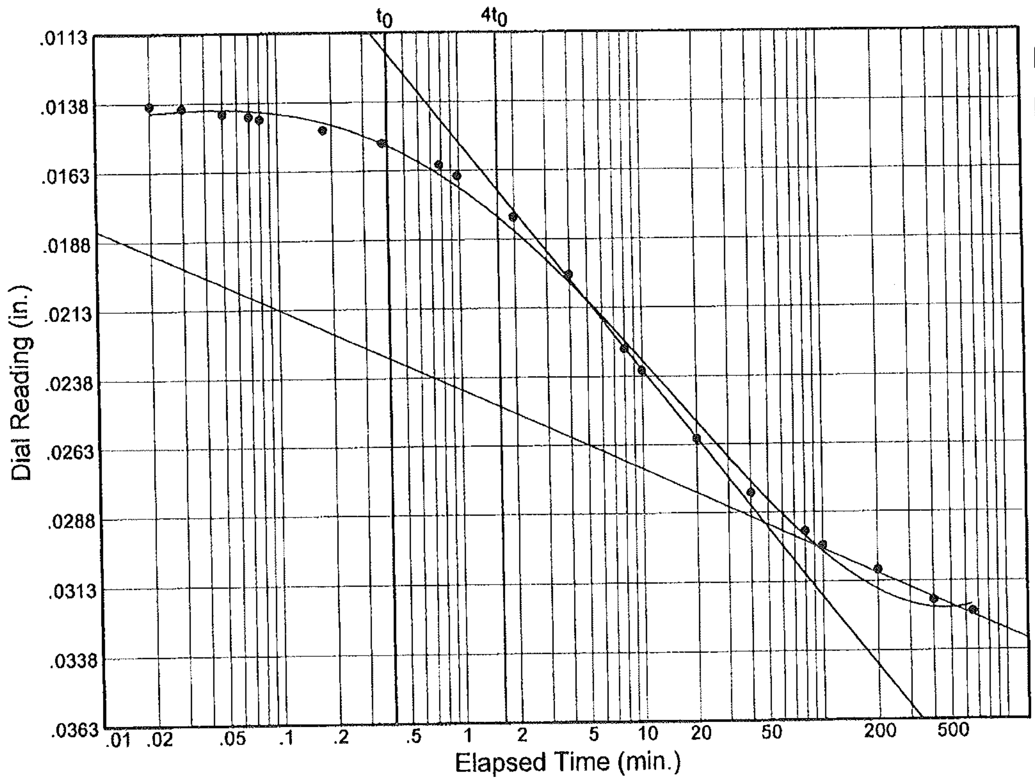
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

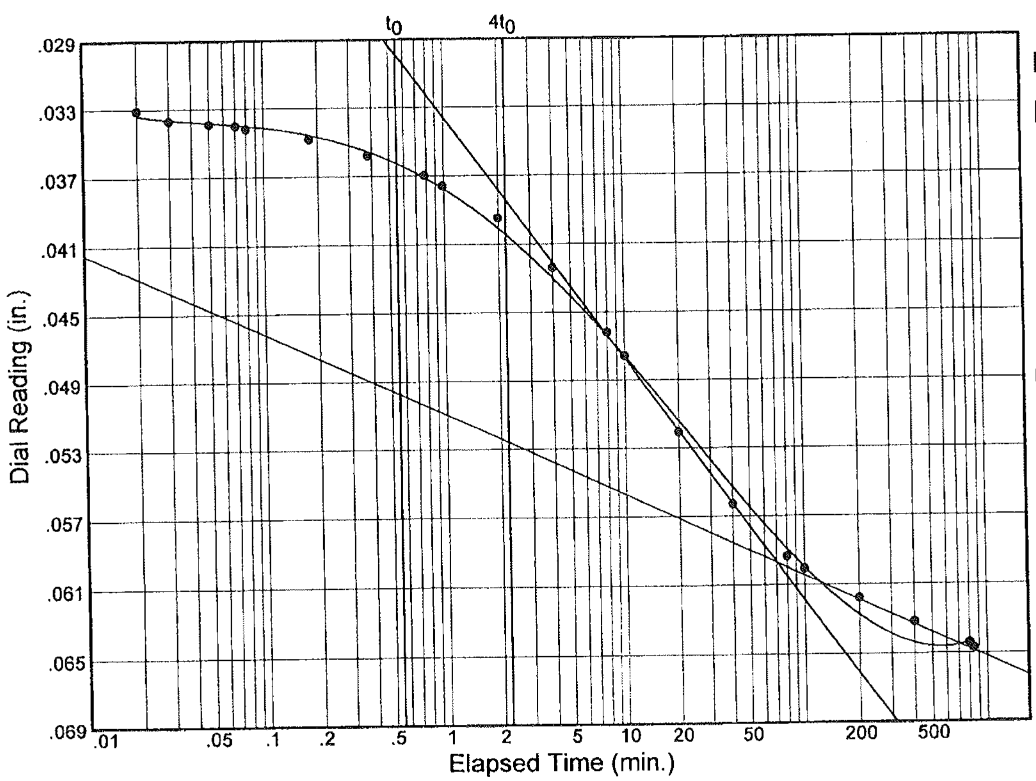
Source: 6 Sample No.: 2 Elev./Depth: 3



Load No.= 3
Load= 0.40 tsf
D_0 = 0.01279
D_{50} = 0.02099
D_{100} = 0.02918
T_{50} = 4.89 min.

C_v @ T_{50}
0.05 ft.^{2}/day

C_α = 0.004



Load No.= 4
Load= 0.80 tsf
D_0 = 0.03208
D_{50} = 0.04592
D_{100} = 0.05975
T_{50} = 7.26 min.

C_v @ T_{50}
0.03 ft.^{2}/day

C_α = 0.007

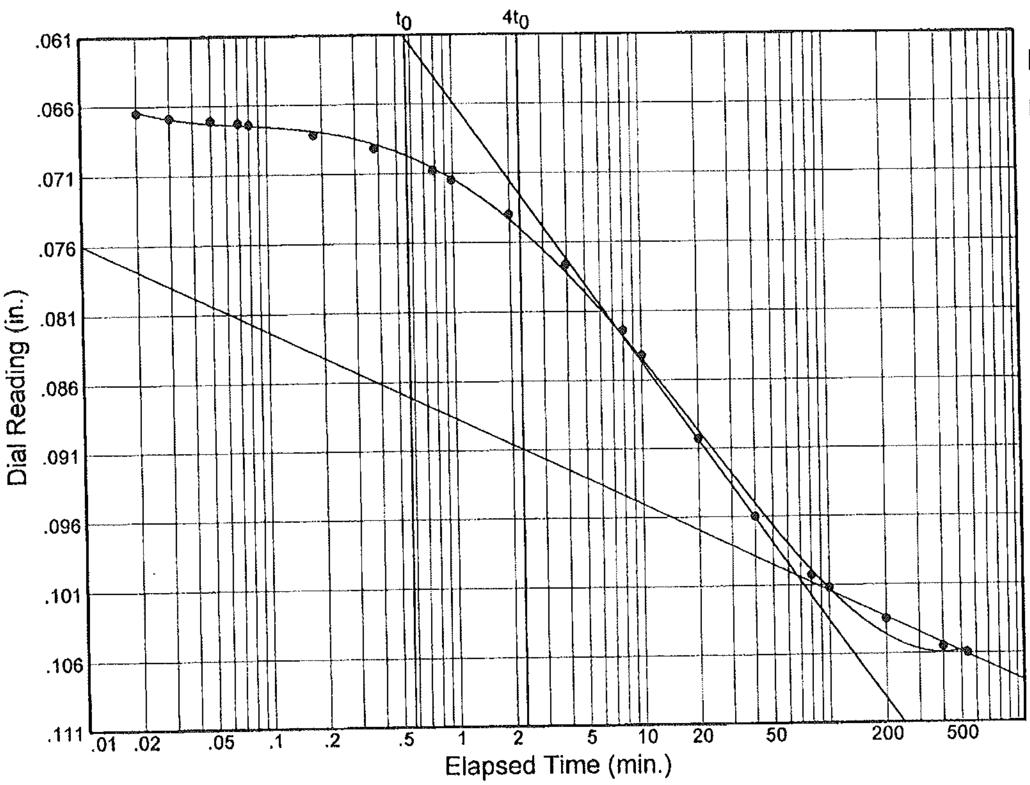
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

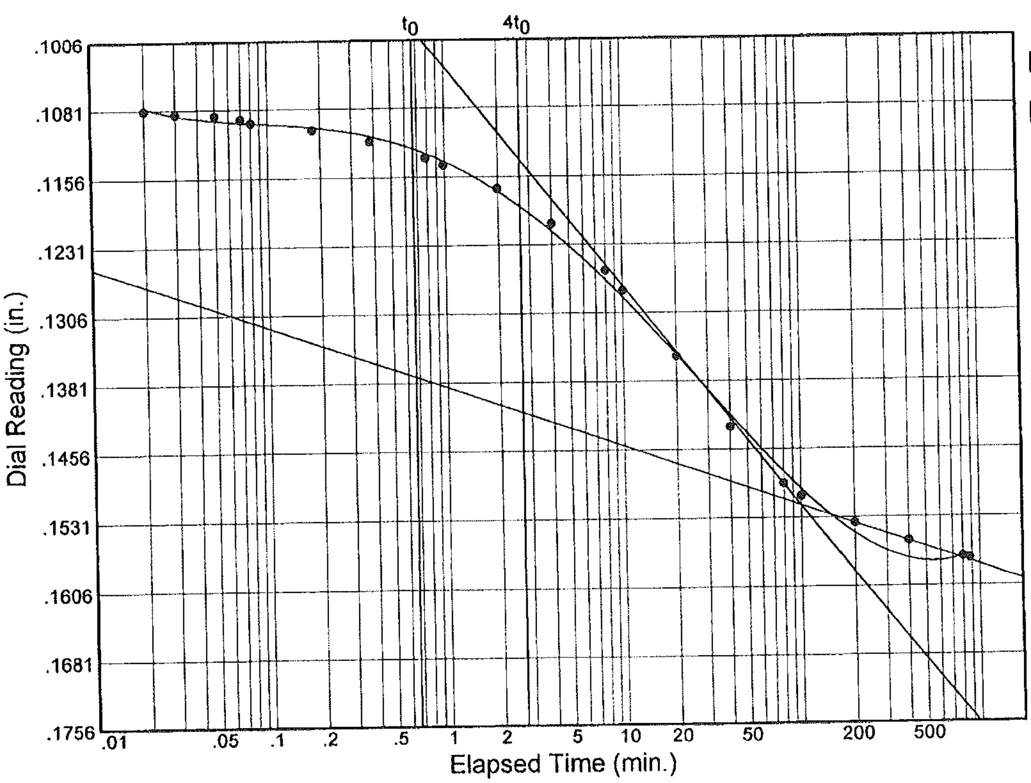
Source: 6 | Sample No.: 2 | Elev./Depth: 3



Load No.= 5
Load= 1.60 tsf
D_0 = 0.06446
D_{50} = 0.08239
D_{100} = 0.10032
T_{50} = 7.51 min.

C_v @ T_{50}
0.03 ft.^{2}/day

C_α = 0.009



Load No.= 6
Load= 3.20 tsf
D_0 = 0.10614
D_{50} = 0.12895
D_{100} = 0.15177
T_{50} = 10.02 min.

C_v @ T_{50}
0.02 ft.^{2}/day

C_α = 0.010

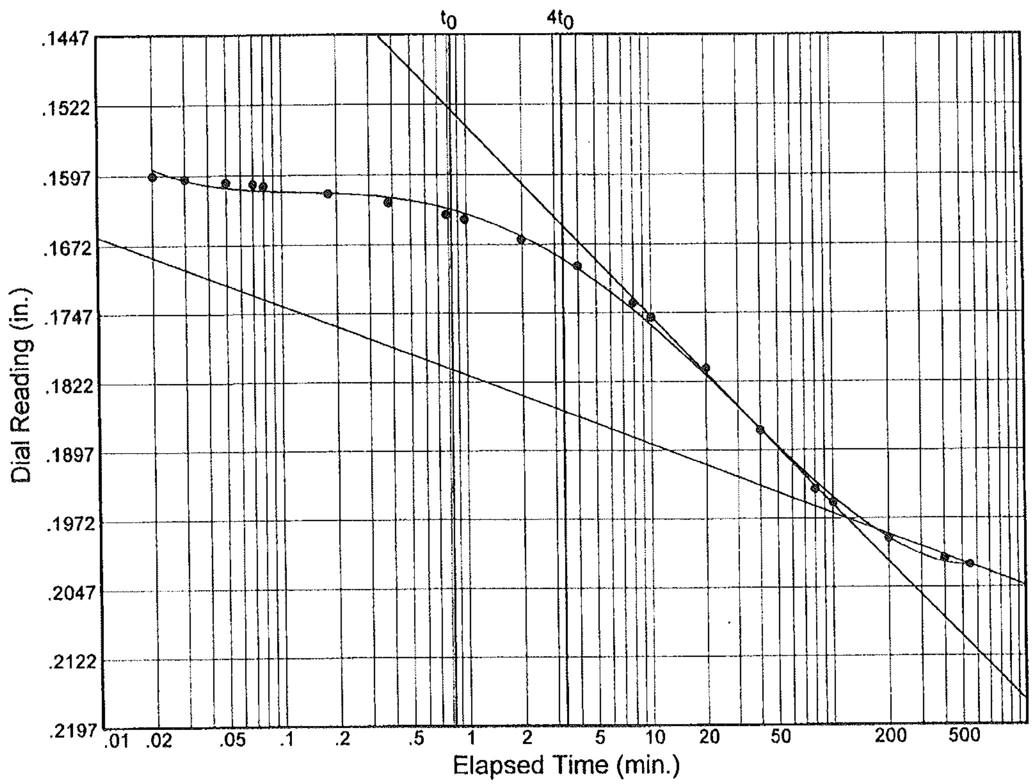
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

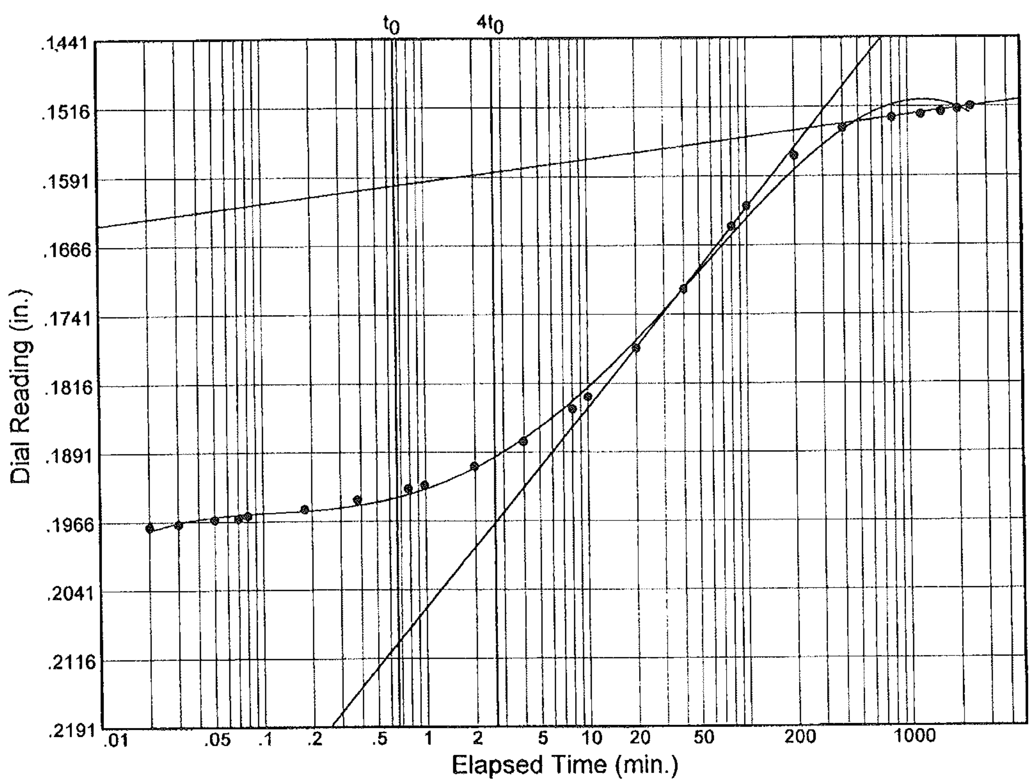
Source: 6 | Sample No.: 2 | Elev./Depth: 3



Load No.= 7
Load= 6.40 tsf
D_0 = 0.15803
D_{50} = 0.17763
D_{100} = 0.19723
T_{50} = 12.31 min.

C_v @ T_{50}
0.01 ft.^{2}/day

C_α = 0.013



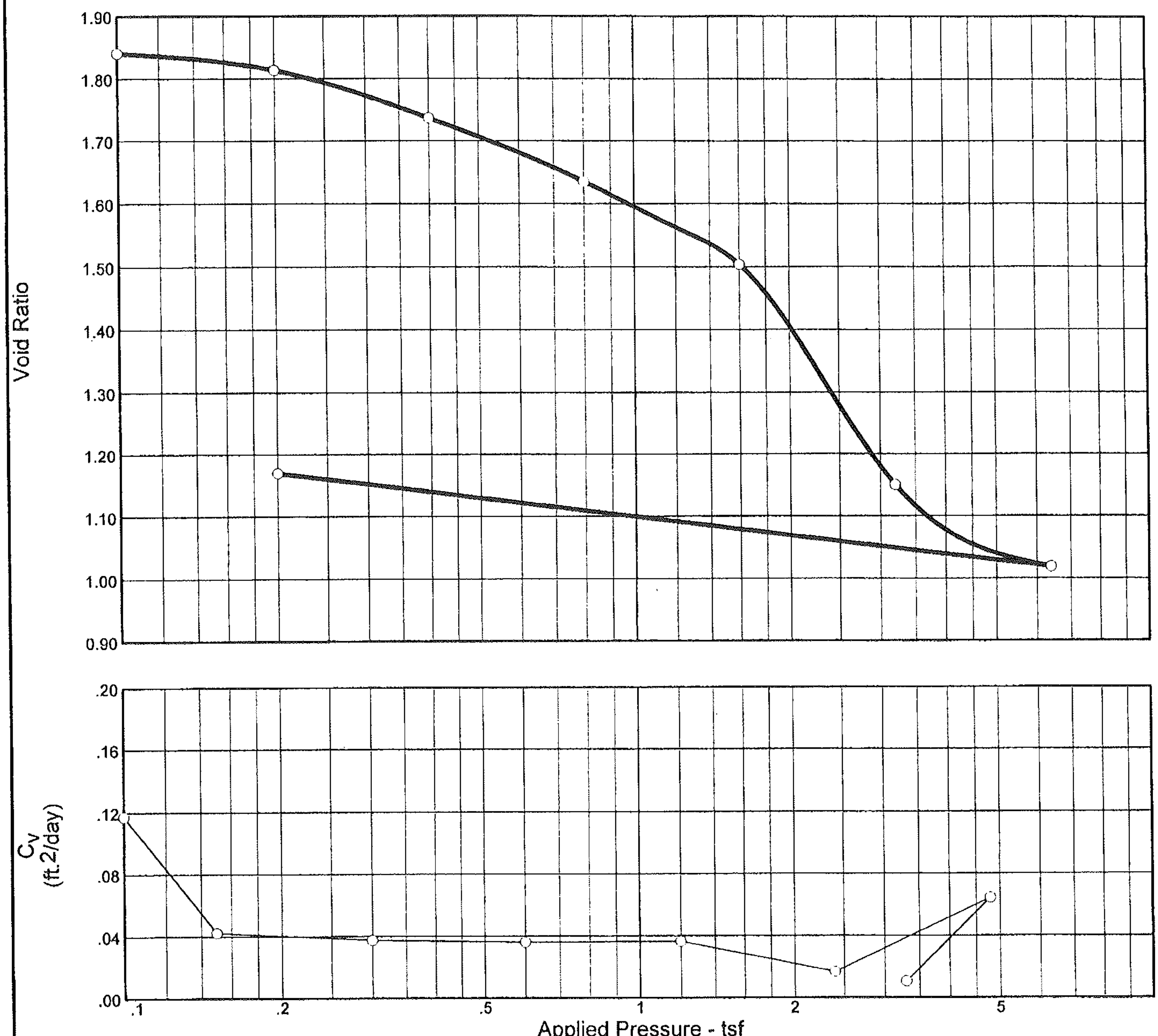
Load No.= 8
Load= 0.20 tsf
D_0 = 0.19844
D_{50} = 0.17618
D_{100} = 0.15391
T_{50} = 21.97 min.

C_v @ T_{50}
0.01 ft.^{2}/day

Figure

EUSTIS ENGINEERING COMPANY, INC.

CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	P_c (tsf)	C_c	Initial Void Ratio
Saturation	Moisture							
100.1 %	69.6 %	58.7	74	52	2.72	1.59	1.27	1.892

MATERIAL DESCRIPTION	USCS	AASHTO
extremely soft gray clay with sand pockets	CH	

Project No. 19183 **Client:**

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source: 7 **Sample No.:** 13 **Elev./Depth:** 34

EUSTIS ENGINEERING COMPANY, INC.

METAIRIE, LA

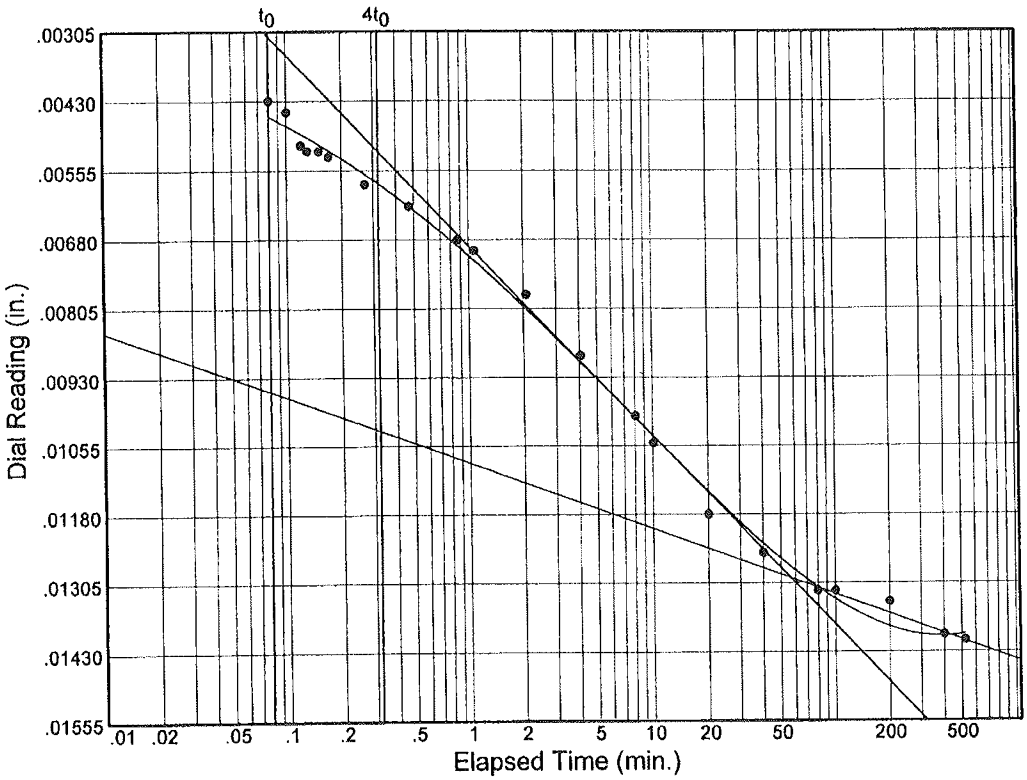
Remarks:
Tested by : BD, RNE

Figure

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

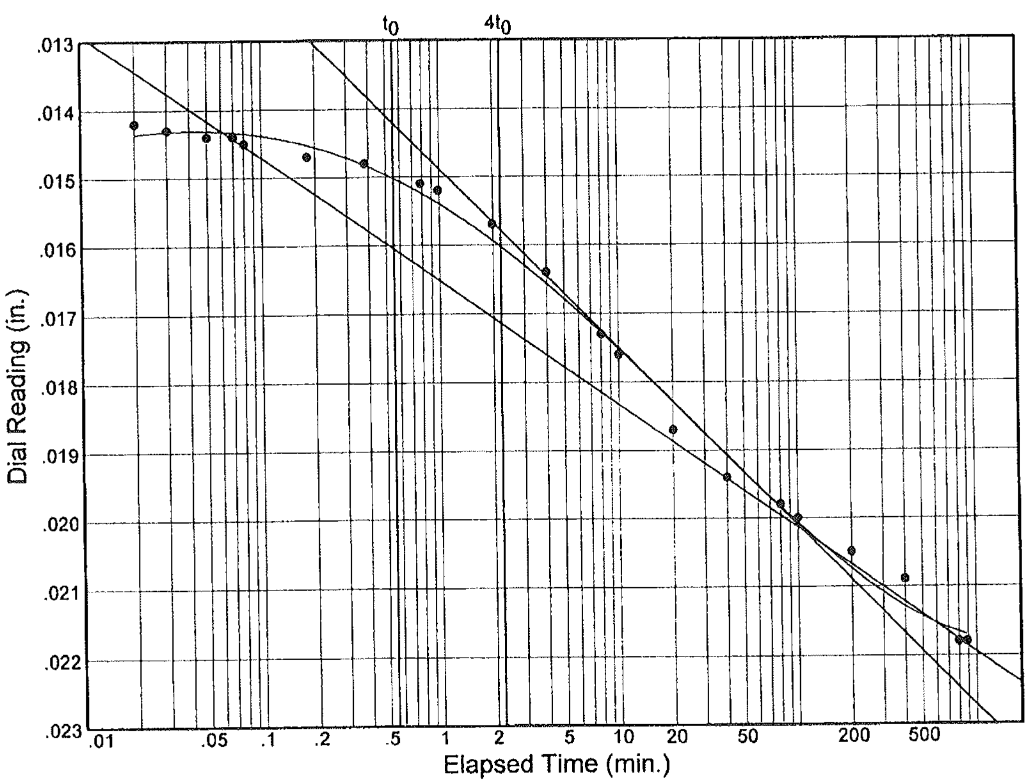
Source: 7 | Sample No.: 13 | Elev./Depth: 34



Load No.= 1
Load= 0.10 tsf
D_0 = 0.00338
D_{50} = 0.00817
D_{100} = 0.01296
T_{50} = 2.25 min.

C_v @ T_{50}
0.12 ft.^{2}/day

C_α = 0.002



Load No.= 2
Load= 0.20 tsf
D_0 = 0.01402
D_{50} = 0.01711
D_{100} = 0.02020
T_{50} = 6.83 min.

C_v @ T_{50}
0.04 ft.^{2}/day

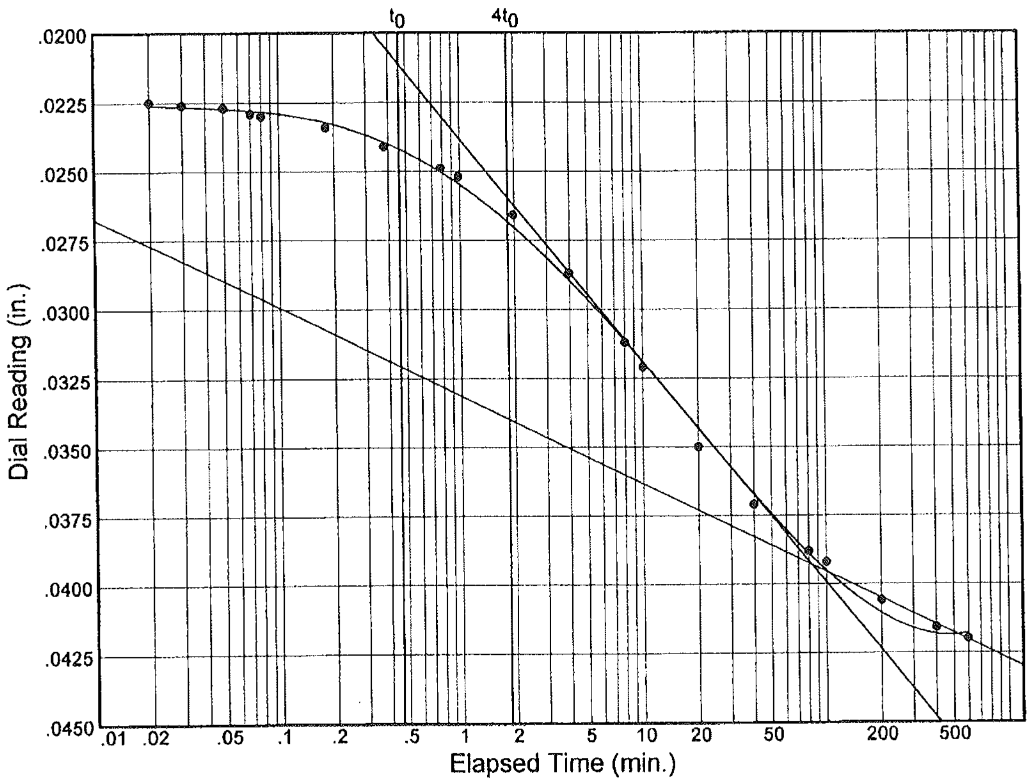
C_α = 0.002

Figure

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

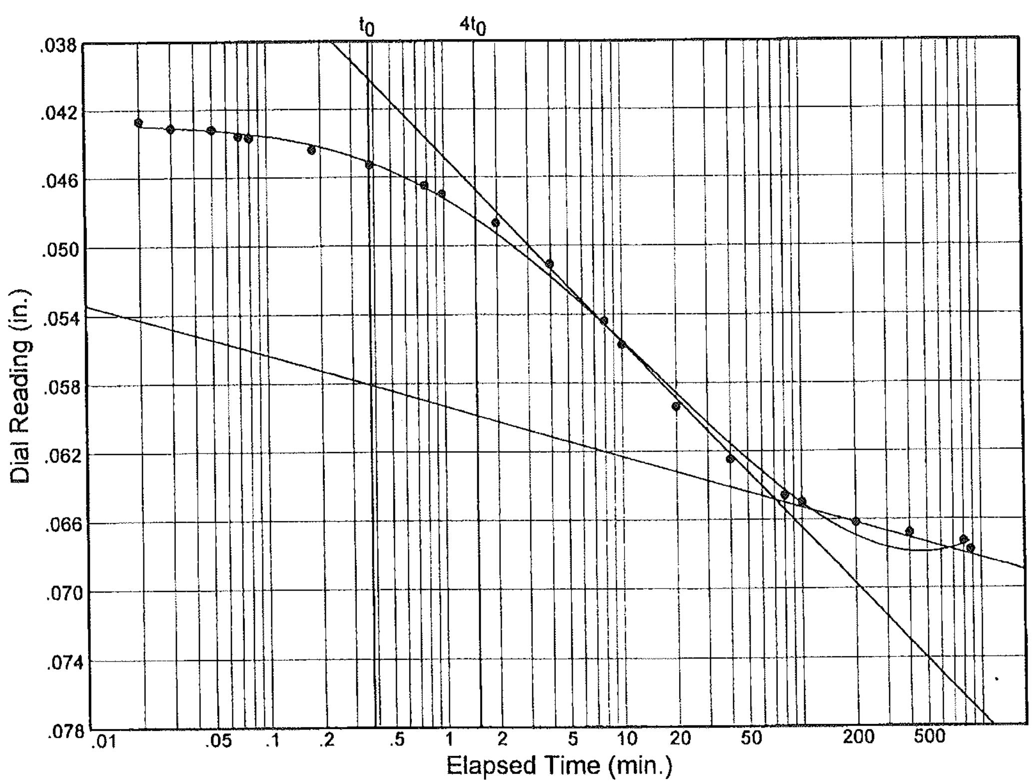
Source: 7 | Sample No.: 13 | Elev./Depth: 34



Load No.= 3
Load= 0.40 tsf
D_0 = 0.02152
D_{50} = 0.03041
D_{100} = 0.03929
T_{50} = 6.34 min.

C_v @ T_{50}
0.04 ft.^{2}/day

C_α = 0.004



Load No.= 4
Load= 0.80 tsf
D_0 = 0.04184
D_{50} = 0.05334
D_{100} = 0.06485
T_{50} = 5.91 min.

C_v @ T_{50}
0.04 ft.^{2}/day

C_α = 0.004

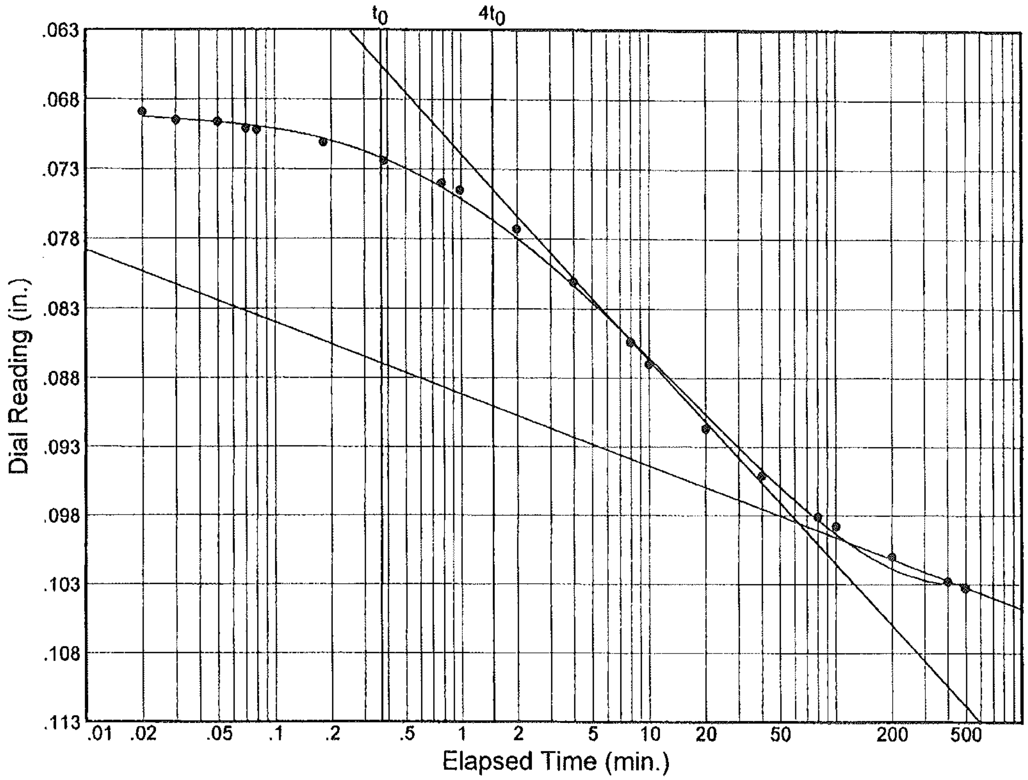
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

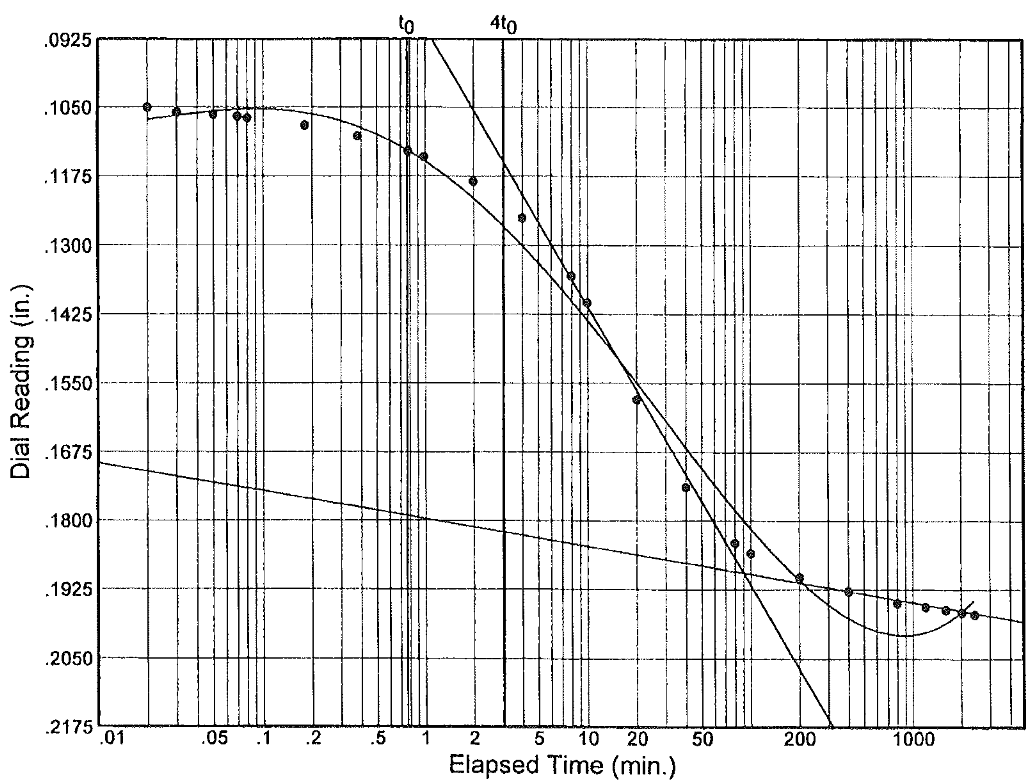
Source: 7 | Sample No.: 13 | Elev./Depth: 34



Load No.= 5
Load= 1.60 tsf
D_0 = 0.06769
D_{50} = 0.08313
D_{100} = 0.09856
T_{50} = 5.53 min.

C_v @ T_{50}
0.04 ft.^{2}/day

C_α = 0.008



Load No.= 6
Load= 3.20 tsf
D_0 = 0.09885
D_{50} = 0.14423
D_{100} = 0.18962
T_{50} = 10.39 min.

C_v @ T_{50}
0.02 ft.^{2}/day

C_α = 0.008

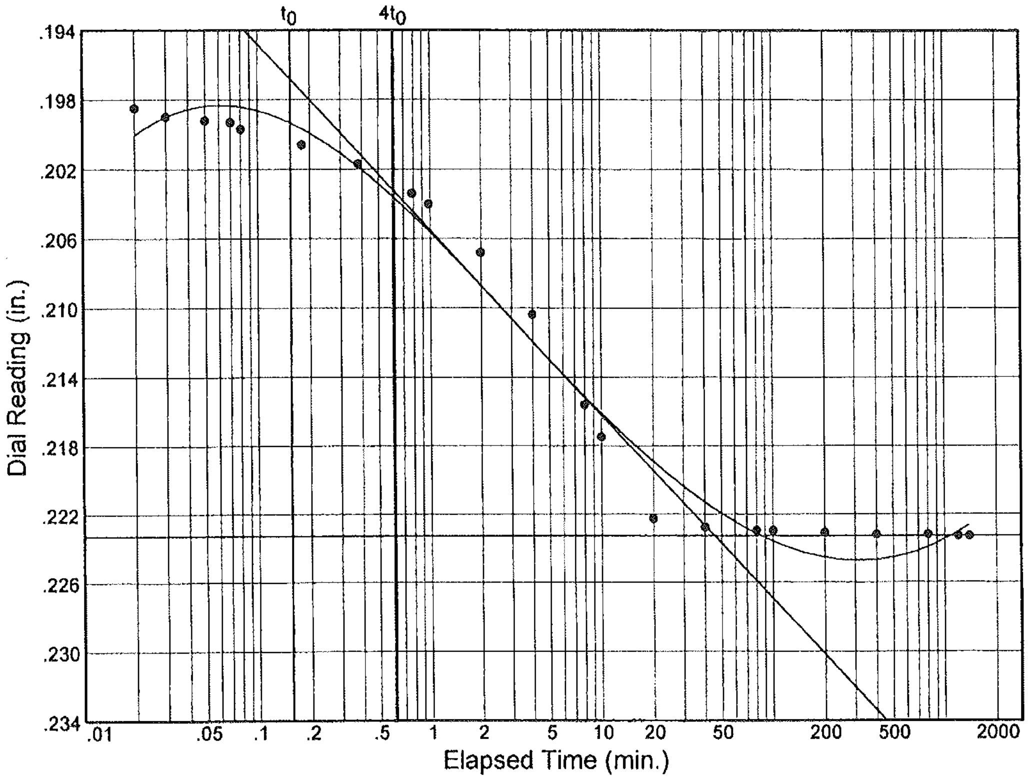
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

Source: 7 | Sample No.: 13 | Elev./Depth: 34



Load No.= 7

Load= 6.40 tsf

D_0 = 0.19504

D_{50} = 0.20917

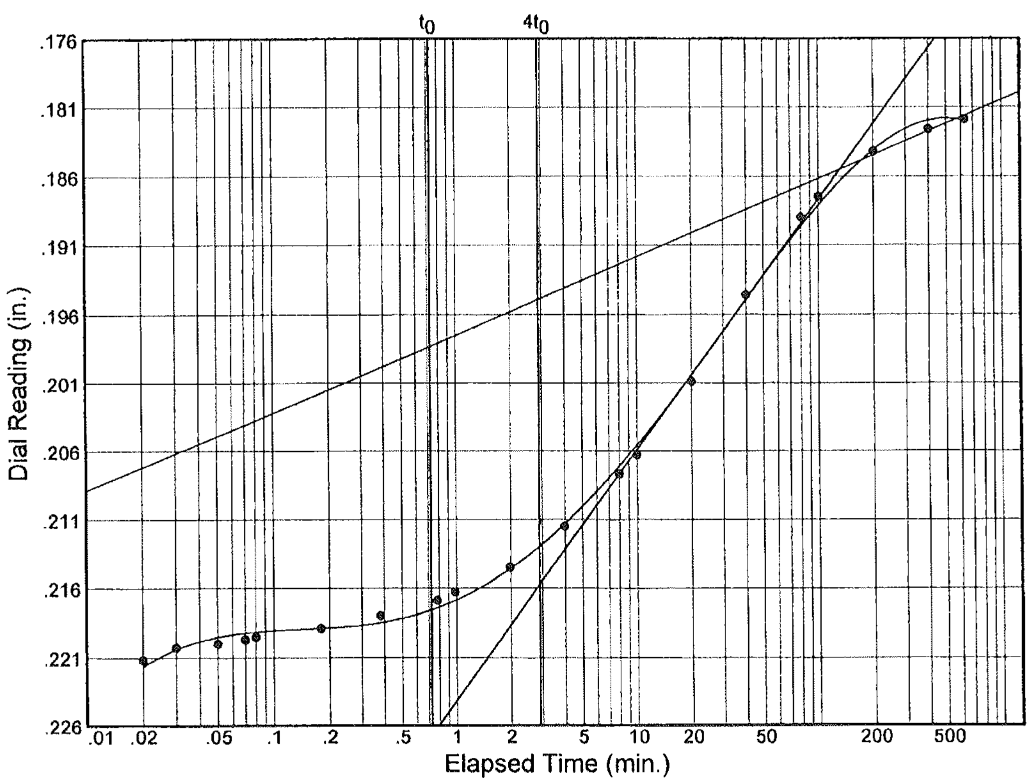
D_{100} = 0.22330

T_{50} = 2.19 min.

C_v @ T_{50}

0.06 ft.^{2}/day

C_α = 0.000



Load No.= 8

Load= 0.20 tsf

D_0 = 0.22219

D_{50} = 0.20383

D_{100} = 0.18547

T_{50} = 12.70 min.

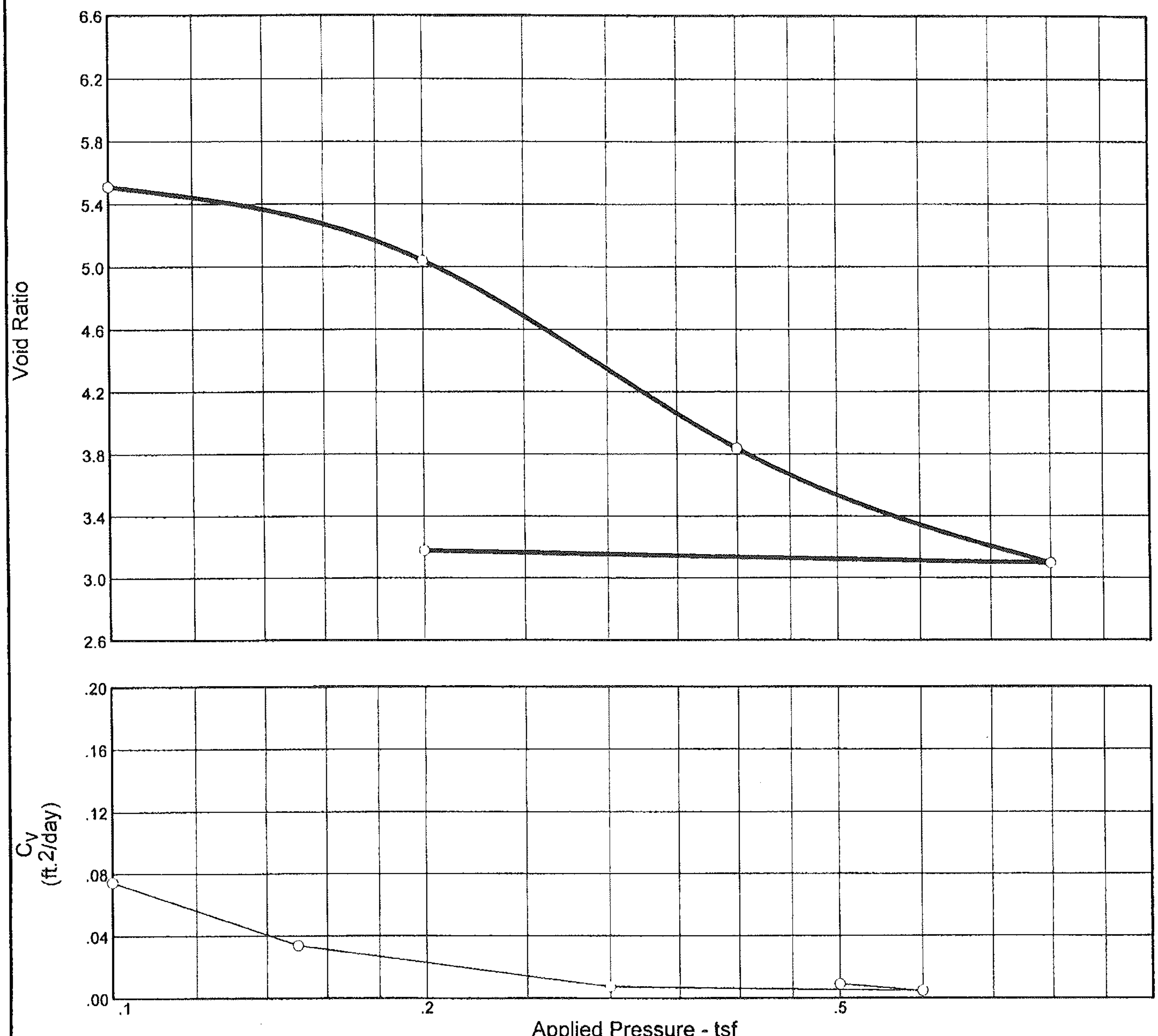
C_v @ T_{50}

0.01 ft.^{2}/day

Figure

EUSTIS ENGINEERING COMPANY, INC.

CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	P_c (tsf)	C_c	Initial Void Ratio
Saturation	Moisture							
96.0 %	219.8 %	23.3	224	157	2.55	0.21	4.35	5.840

MATERIAL DESCRIPTION	USCS	AASHTO
extremely soft dark gray humus wuth roots & clay layers	PT	

Project No. 19183 **Client:**

Project: STATE OF LOUISIANA
COASTAL RESTORATION AND MANAGEMENT

Source: 8 **Sample No.:** 4 **Elev./Depth:** 7

EUSTIS ENGINEERING COMPANY, INC.

METAIRIE, LA

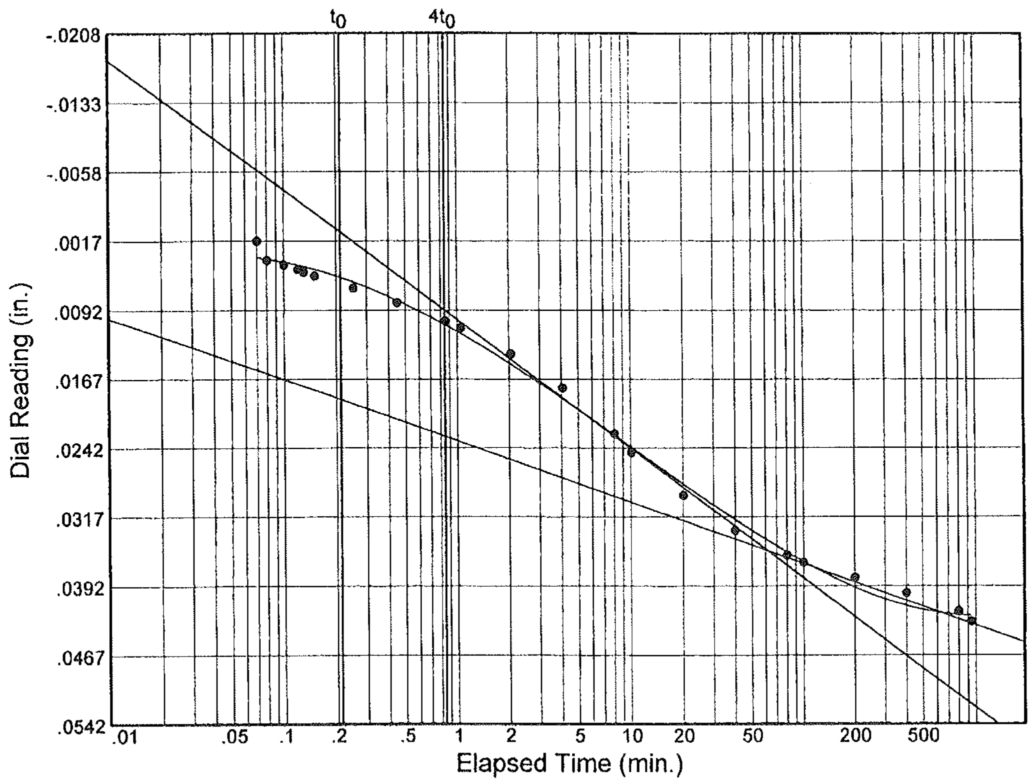
Remarks:
Tested by : BD, RNE

Figure

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

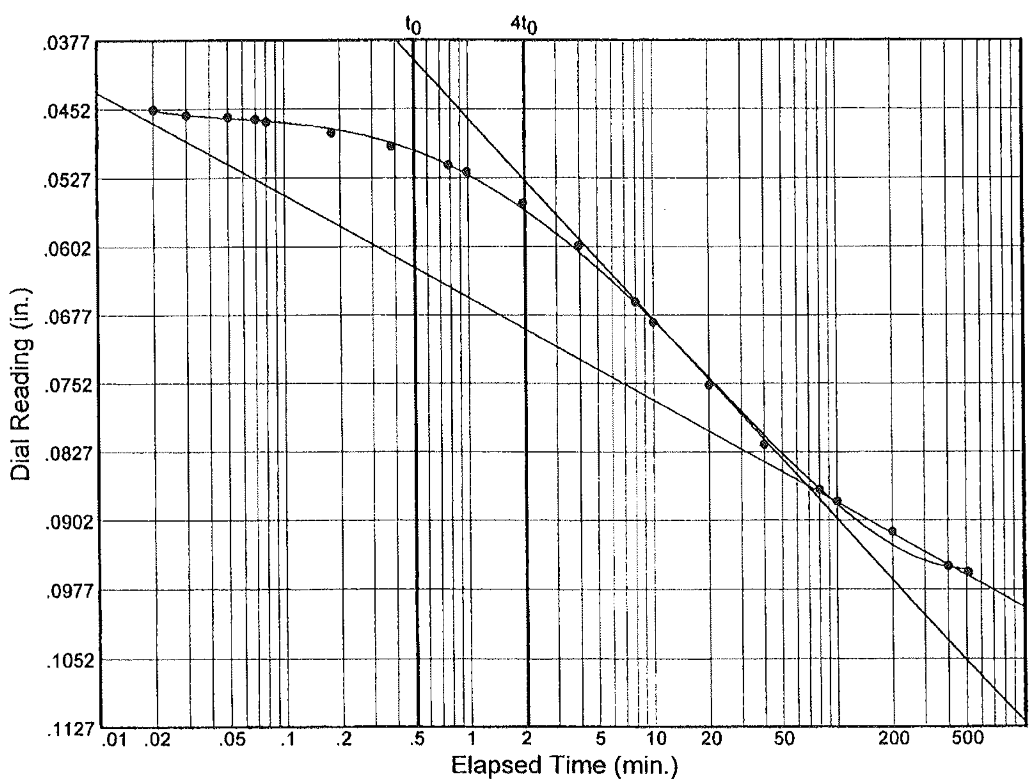
Source: 8 Sample No.: 4 Elev./Depth: 7



Load No.= 1
Load= 0.10 tsf
D_0 = 0.00051
D_{50} = 0.01796
D_{100} = 0.03541
T_{50} = 3.45 min.

C_v @ T_{50}
0.07 ft.^{2}/day

C_α = 0.009



Load No.= 2
Load= 0.20 tsf
D_0 = 0.04327
D_{50} = 0.06484
D_{100} = 0.08641
T_{50} = 6.64 min.

C_v @ T_{50}
0.03 ft.^{2}/day

C_α = 0.016

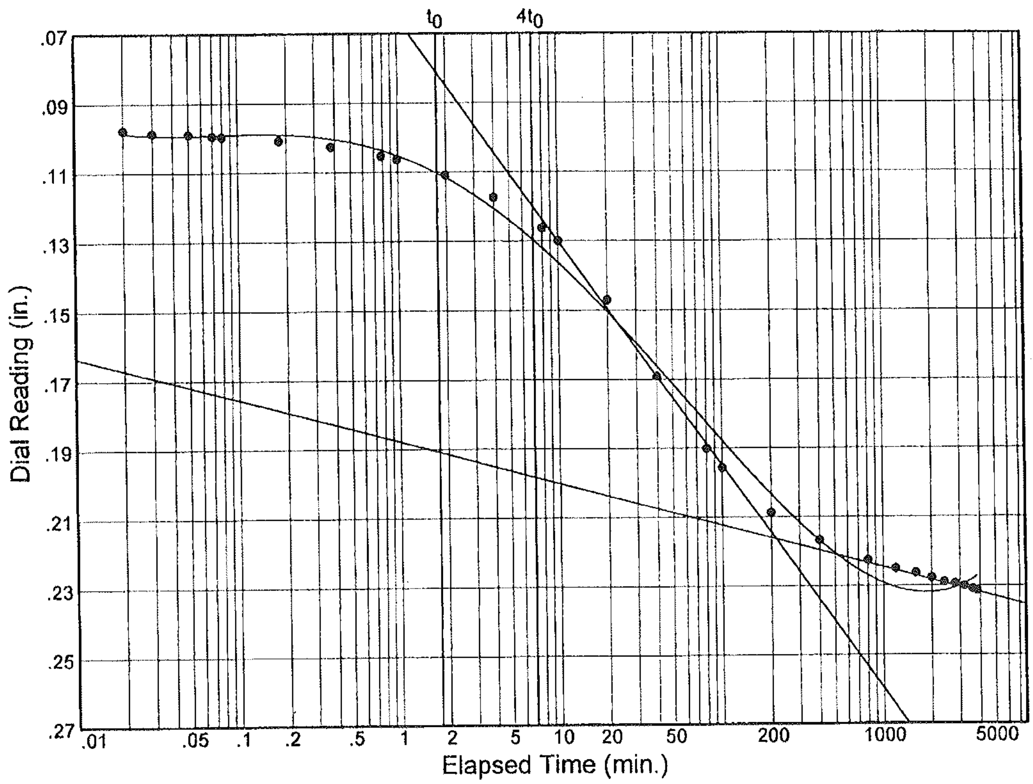
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

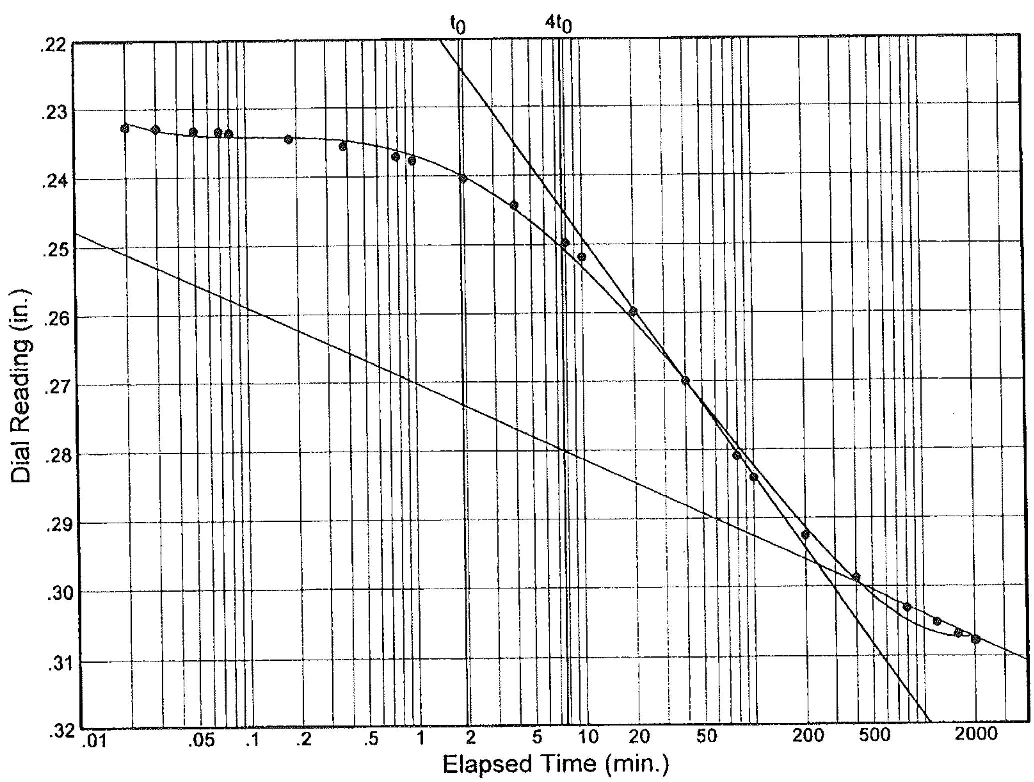
Source: 8 | Sample No.: 4 | Elev./Depth: 7



Load No.= 3
Load= 0.40 tsf
D_0 = 0.09081
D_{50} = 0.15365
D_{100} = 0.21649
T_{50} = 23.08 min.

C_v @ T_{50}
0.01 ft.^{2}/day

C_α = 0.019



Load No.= 4
Load= 0.80 tsf
D_0 = 0.22931
D_{50} = 0.26310
D_{100} = 0.29689
T_{50} = 23.25 min.

C_v @ T_{50}
0.00 ft.^{2}/day

C_α = 0.022

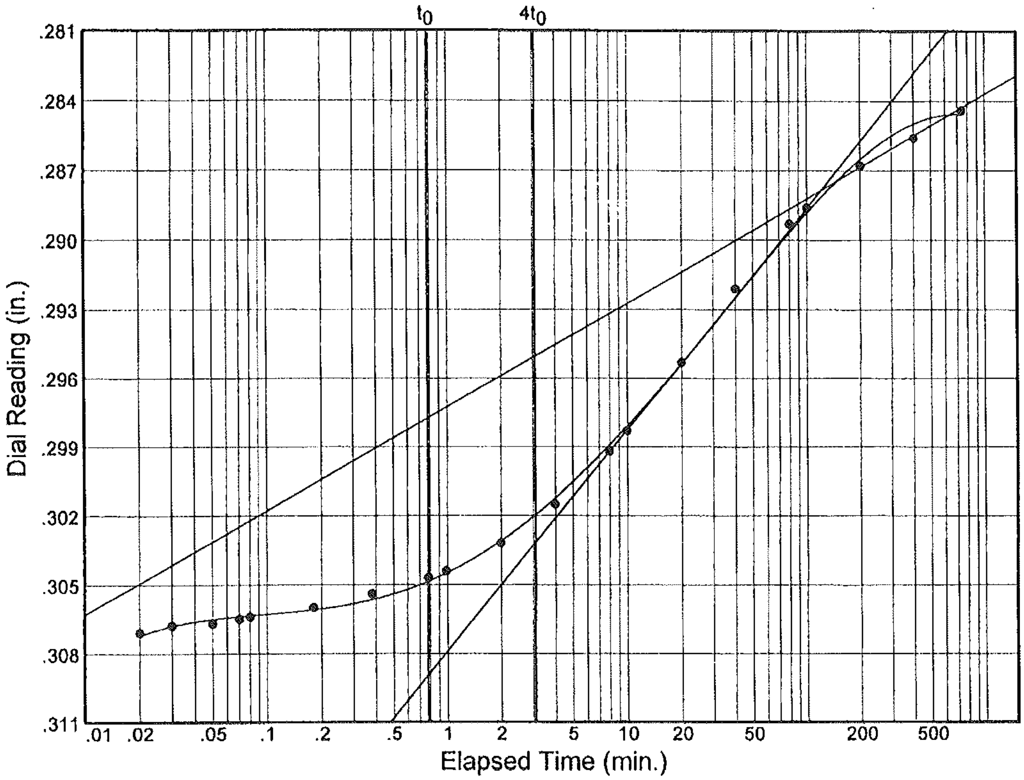
Figure

EUSTIS ENGINEERING COMPANY, INC.

Dial Reading vs. Time

Project No.: 19183
Project: STATE OF LOUISIANA

Source: 8 | Sample No.: 4 | Elev./Depth: 7



Load No.= 5
Load= 0.20 tsf
D_0 = 0.30776
D_{50} = 0.29779
D_{100} = 0.28782
T_{50} = 10.83 min.

C_v @ T_{50}
0.01 ft.^{2}/day

Figure