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December 8, 2020

Coastal Protection and Restoration Authority (CPRA)
150 Terrace Avenue
Baton Rouge, Louisiana 70802

Attn: Ms. April Newman
Project Manager
Phone: (225) 342-6412
Email: April.Newman@la.gov

Re: **Geotechnical Engineering Services Data Report**
Bayou Cane Marsh Creation Project (PO-181)
St. Tammany Parish, LA
PSI Project No. 02541139-1

Dear Ms. Newman:

Professional Service Industries, Inc. (PSI), an Intertek company, is pleased to submit our Geotechnical Data Report for the above-referenced project. This report presents the results of our field exploration and laboratory testing performed for the proposed construction. A final geotechnical report will be submitted once the engineering analyses are performed.

If you have any questions pertaining to this report, please contact our office at (504) 733-9411. PSI would be pleased to continue providing geotechnical and construction material testing services throughout the construction of the project, and we look forward to working with you and your organization on this and future projects.

Respectfully submitted,

PROFESSIONAL SERVICE INDUSTRIES, INC.

Prasoon Tiwari, P.E.(TX)
Geotechnical Department Manager



Nabil S. Mikhail, P.E., D. GE.
Chief Engineer



PSI Project No: 02541139-1
Bayou Cane Marsh Creation Project (PO-181)
St. Tammany Parish, LA
December 8, 2020

GEOTECHNICAL ENGINEERING SERVICES DATA REPORT

**BAYOU CANE MARSH CREATION PROJECT (PO-181)
ST. TAMMANY PARISH, LA**

PSI PROJECT NO. 02541139-1

PREPARED FOR

**COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA)
150 TERRACE AVENUE
BATON ROUGE, LA 70802**

December 8, 2020

BY

**PROFESSIONAL SERVICE INDUSTRIES, INC.
724 CENTRAL AVENUE
JEFFERSON, LOUISIANA 70121
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PROJECT INFORMATION

PROJECT AUTHORIZATION

Professional Service Industries, Inc. (PSI), an Intertek company, has completed the field and laboratory portions of a geotechnical investigation for the proposed Bayou Cane Marsh Creation Project (PO-181) in St. Tammany Parish, Louisiana. Our scope of services is outlined in PSI Proposal 0254-282343 Rev 6, dated June 4, 2020. Our geotechnical services were authorized by Mr. Jerry Carroll, P.E., Lafayette Regional Operations Manager with CPRA, by issuing a Notice to Proceed as Task #3 dated June 10, 2020 under our existing Contract No. 4400015386. A final geotechnical report will be submitted once the engineering analyses are performed.

PROJECT DESCRIPTION

Project information was provided to PSI by Ms. Bevin Barringer, P.E. of CPRA, by an email correspondence dated November 15, 2019. The project information was obtained in detail via email correspondence, in person meeting with CPRA and PSI on November 22, 2019 and a conference call on January 22, 2020. The project information included scope of services for geotechnical investigation and engineering services for Bayou Cane Marsh Creation Project (PO-181) in St. Tammany Parish, Louisiana.

Based on the provided information, PSI understands that the Bayou Cane Marsh Creation Project (PO-181) is located in Region 1 of Pontchartrain. The objective of this project is to create, maintain and nourish existing deteriorating wetland areas. Approximately 384 acres of marsh will be created, and an additional 65 acres nourished by hydraulically dredging material from borrow source located in Lake Pontchartrain.

SITE LOCATION AND DESCRIPTION

The PO-181 project area is located in CWPPRA Planning Region 1 of Pontchartrain, St. Tammany Parish, along the north shore of Lake Pontchartrain between Mandeville and Lacombe on property owned by either the State of Louisiana (Fontainebleau State Park) or the Big Branch National Wildlife Refuge. The project area spans approximately six miles along the shoreline of Lake Pontchartrain and is bordered to the southeast by the existing Goose Point/Point Platte Marsh Creation Project (PO-0033) and to the west by the facilities of Fontainebleau State Park. Approximate coordinates for the center of marsh fill area of the project are shown in the Table1.

Table1: Centerline Coordinates of the PO-181 Project Area

Geographic	
Latitude (DMS)	Longitude (DMS)
30° 18'46.03" N	89° 59'42.42" W

The approximate site location is shown on the Site Vicinity Map included in the Appendix A, which is based on Appendix B of the CPRA Scoping document for Project (PO-181), Proposed Soil Sampling Layout and Permit Drawings.



FIELD AND LABORATORY PROCEDURES

FIELD EXPLORATION

PERMIT ACCESS

Prior to mobilizing for the field work to survey the boring locations and conduct site reconnaissance, PSI coordinated and acquired documented access permission to conduct any work at the subject site. Special Use Permit for Sabine National Wildlife Refuge was secured prior to mobilizing to the project site. Wildlife Refuge Manager for Atchafalaya, Big Branch Marsh, and Bogue Chitto NWR's and Park Manager for Fontainebleau State Park were notified at least one day prior to the mobilization to the project site.

SURVEY POSITIONING AND MAGNETOMETER SURVEY

Prior to mobilizing the drilling equipment to perform the soil borings and Cone Penetration Tests (CPT's), Chustz surveying, LLC performed survey positioning and magnetometer survey at each boring/CPT location in the marsh area and borrow area, as per CPRA guidelines. The horizontal and vertical controls for survey positioning were performed in accordance with secondary monument and data sheets presented in Appendix D of the attached Scope Document. The magnetometer survey at each boring/CPT location in the marsh and borrow area was performed to ensure there are no pipelines or obstructions in the area. The locations in the marsh area were staked in the field by the survey crew. Based on information obtained from the survey work, details for the soil boring and CPT soundings details performed in the borrow area and the marsh area are presented in Table 2 and Table 3 respectively. The survey report is presented in Appendix A.

Table 2. Soil Borings details performed in Borrow area (Datum: NAVD88 GEOID12B)

Boring Location	Northing	Easting	Mudline Elevation (ft)	Water Surface Elevation (ft)
B-1	653592.17	3692783.59	-12.47	-0.02
B-2	654296.28	3690819.59	-12.93	-0.02
B-3	652748.14	3688732.89	-13.84	-0.02
B-4	652972.28	3691044.15	-13.54	-0.02
B-5	652139.05	3693373.58	-12.61	-0.02
B-6	652775.97	3695090.92	-12.34	-0.02
B-7	650812.70	3694361.08	-12.57	-0.02
B-8	651652.76	3691569.40	-13.14	-0.02
B-9	651369.34	3689291.20	-13.99	-0.02
B-10	650982.83	3686457.33	-14.17	-0.02
B-11	649962.50	3689870.89	-13.84	-0.02
B-12	648877.69	3693540.55	-13.44	-0.02
B-13	650171.42	3692404.31	-13.62	-0.02

Note: A water depth of approximately 12 to 15 feet was recorded at boring locations in the Borrow area during our field operations.



Table 3. Soil Borings and CPT Sounding details performed in Marsh area (Datum: NAVD88 GEOID12B)

Boring Location	Northing	Easting	Mudline Elevation (ft)	Water Surface Elevation (ft)
CPT-1	669179.29	3687573.24	0.49	N/A
CPT-2/M-1*	668446.23	3688520.66	-1.34	-0.02
CPT-3	667204.22	3688435.14	-1.48	-0.02
CPT-4	666574.52	3689404.84	-1.29	-0.02
CPT-5	665842.48	3691338.72	-0.97	-0.02
CPT-6/M-2*	667154.95	3691693.24	-1.14	-0.02
CPT-7	665982.13	3692533.45	0.42	N/A
CPT-8	665918.83	3693244.47	-2.14	-0.02
CPT-9/M-3*	665644.37	3694586.10	-2.01	-0.02
CPT-10	664745.33	3695215.87	0.72	N/A
CPT-11	661903.21	3701333.28	-1.44	-0.02
CPT-12	662098.14	3702500.36	-1.49	-0.02
CPT-13/M-4*	661535.73	3703615.42	-2.75	-0.02
CPT-14	659854.65	3703024.00	-1.76	-0.02
CPT-15/M-5*	659157.35	3704594.77	-2.58	-0.02
CPT-16	657716.47	3704969.87	-0.90	-0.02
CPT-17/M-6*	656671.99	3706892.30	-2.18	-0.02
CPT-18	654174.87	3707231.95	0.29	N/A
CPT-19	653210.85	3706520.76	-0.65	-0.02
CPT-20/M-7*	653523.53	3708819.53	-1.60	-0.02
CPT-21/M-8*	651544.24	3709278.43	-0.02	-0.02
CPT-22	650266.17	3709111.32	0.46	N/A
CPT-23/M-9*	648517.00	3710354.60	0.58	N/A

* Co-located CPT sounding with soil boring

SOIL BORINGS

Based on provided information and discussion with CPRA, soil boring and CPT sounding locations were finalized. Boring Location Plans for marsh area and borrow area are attached in the Appendix B.

A total of twenty-three (23) soil borings and CPT soundings were performed for the Marsh creation area. As discussed with CPRA, the soil borings and CPT soundings were explored using an airboat mounted drilling equipment. All the borings and CPT soundings were performed between August 7 and September 2. Detailed description is mentioned below:

- Nine (9) soil borings were drilled to a depth of 20 feet below mudline continuously and then at 5 feet intervals to a depth of 40 feet below the mudline. All the soil borings in the marsh area were co-located with CPT soundings. CPT soundings co-located with soil borings are presented in the



Table 2 above. Drilling and sampling were performed in accordance with applicable ASTM Standards and the furnished Scope of Services Document.

The proposed soil borings were advanced with a 4-inch diameter rotary wash drilling method. Undisturbed samples of cohesive soils were obtained by using three-inch diameter thin-wall tube samplers (Shelby Tube) in general accordance with the procedures for "Thin-Walled Tube Geotechnical Sampling of Soils" (ASTM D1587). For cohesionless soils and semi-cohesive soils, Standard Penetration Test (SPT) was performed to obtain standard penetration values of the soil. The standard penetration value (N) is defined as the number of blows of a 140-pound hammer falling 30 inches that is required to advance the split-barrel sampler one (1) ft. into the soil. The number of blows is recorded for each of three (3) successive increments of six (6) inches penetration. The "N" value is obtained by adding the second and third incremental numbers. The results of the standard penetration test indicate the relative density of cohesionless soils and thereby provide a basis for estimating the relative strength of the soil profile components. Samples of granular soils were obtained utilizing a two (2) inch O.D. split-barrel sampler in general accordance with procedures for "Penetration Test and Split-Barrel Sampling of Soils" (ASTM D1586). The boreholes were grouted upon their completion per LDEQ and LA DOTD regulations.

- Twenty-three (23), Cone Penetration tests to the refusal depth varying from 7 feet to 21 feet. Our CPT utilizes a 60° apex cone, 1.4-inch diameter, Type 2 electric cone penetrometer that meets ASTM D5778 specification. The soil information obtained from CPT soundings are presented in the Appendix B. The CPT was advanced to the shallow refusal depth due to the presence of the Pleistocene. Refusal prior to the depth specified occurs due to achieving maximum tip resistance, a high frictional resistance of the sleeve, or excessive tilt angle of the CPT probe. To prevent damage to the CPT, no attempt was made to advance beyond refusal depth. Due to low penetration depths for CPT soundings, a cut off value of 1.7 was used for overburden correction factors for estimating normalized soil behavior type. For cohesive soils, Shear strength (S_u) was estimated using the total cone resistance (q_t) and cone factor (N_{kt}).

$$S_u = \frac{q_t - \sigma_v}{N_{kt}}$$

- $N_{kt} = 16$, used for marsh area CPT's.

A total of thirteen (13) soil borings were performed for the borrow area. The borings were performed between September 23 to September 30. Detailed description mentioned below:

- Thirteen (13) soil borings were sampled to a depth of 20 feet below mudline continuously. Drilling and sampling were performed in accordance with applicable ASTM Standards and the furnished Scope of Services Document.

The proposed soil borings were advanced with a 4-inch diameter rotary wash drilling method. Undisturbed samples of cohesive soils were obtained by using three-inch diameter thin-wall tube samplers (Shelby Tube) in general accordance with the procedures for "Thin-Walled Tube Geotechnical Sampling of Soils" (ASTM D1587). For cohesionless soils and semi-cohesive soils,



Standard Penetration Test (SPT) was performed to obtain standard penetration values of the soil. The standard penetration value (N) is defined as the number of blows of a 140-pound hammer falling 30 inches that is required to advance the split-barrel sampler one (1) ft. into the soil. The number of blows is recorded for each of three (3) successive increments of six (6) inches penetration. The “N” value is obtained by adding the second and third incremental numbers. The results of the standard penetration test indicate the relative density of cohesionless soils and thereby provide a basis for estimating the relative strength of the soil profile components. Samples of granular soils were obtained utilizing a two (2) inch O.D. split-barrel sampler in general accordance with procedures for “Penetration Test and Split-Barrel Sampling of Soils” (ASTM D1586). The boreholes were grouted upon their completion per LDEQ and LA DOTD regulations.

GEOTECHNICAL LABORATORY TESTING

During the field work, the samples were identified according to boring number and depth and were transported to PSI’s laboratory for classification and testing. A geotechnical laboratory testing program was developed after reviewing the field logs. The laboratory testing program was provided to CPRA for review and approval prior to testing. For borrow area borings, majority of laboratory tests were approved for borings B-02, B-04, B-06, B-08, B-10, and B-12. After approval, laboratory tests were performed in general accordance with applicable ASTM procedures and the furnished Scope Document. Laboratory testing of the obtained samples has been performed to evaluate the classification, strength and other engineering characteristics of the subsurface materials.

The Shelby tube samples were removed from the sampling tubes in the laboratory using a specially fabricated hydraulic piston-type extruder. The tube samples were extruded, classified and subjected to index and compressive strength tests. Additional estimates of shear strength were obtained through the use of a hand penetrometer (HP) and torvane (TV), respectively. The results of the laboratory tests are presented in the Boring Logs given in the Appendix B. A key to the terms and symbols used on the boring logs is also presented in Appendix B.

An overview of the scope of the laboratory testing phase for both the marsh area borings and borrow area borings are tabulated in Table 4 and Table 5 respectively below. A detailed discussion on laboratory test results is presented in subsequent sections of the data report.

Table 4. Laboratory Testing Summary for Marsh area borings

Test Method	ASTM Reference	No. of Tests performed
Moisture Content	(ASTM D2216)	126
Visual Classification	(ASTM D2488)	126
Organic Content	(ASTM D2974)	9
Atterberg limits	(ASTM D4318)	67
Percent Passing #200 sieve	(ASTM D1140)	26
Specific Gravity	(ASTM D854)	8
Total Unit Weight Determination	(ASTM D2937)	33
Unconsolidated Undrained Triaxial Test	(ASTM D2850)	33
Consolidation Test	(ASTM D2435)	8



Table 5. Laboratory Testing Summary for Borrow area borings

Test Method	ASTM Reference	No. of Tests performed
Moisture Content	(ASTM D2216)	130
Visual Classification	(ASTM D2488)	130
Atterberg limits	(ASTM D4318)	26
Hydrometer Analysis	(ASTM D7928)	6
Total Unit Weight Determination	(ASTM D2937)	25
Column Settling Test	USACE 1110-2-5027	2
Low Stress Consolidation Test	ASTM D2345 (mod.)	2

CLASSIFICATION AND INDEX TESTS

To determine the soil classification and geotechnical index properties, various classification and index tests were performed. The following classification and index tests for the marsh and borrow area were performed:

- Visual Classification (ASTM D2488)
- Moisture Content (ASTM D2216)
- Atterberg Limits (ASTM D4318)
- Percent Passing #200 sieve (ASTM D1140)
- Particle-Size Analysis and Hydrometer (ASTM D7928)
- Specific Gravity (ASTM D854)
- Organic Content (ASTM D2974)
- Total Unit Weight Determination (ASTM D2937)

1. Visual Classification

Visual classification was performed based on ASTM D2488 and incorporated into the soil boring logs in Appendix B. It includes the description of soil color, consistency and type, and identification of variations (organics, layers, seams, etc.).

2. Moisture Content

More than two hundred and fifty six (256) moisture content determinations (ASTM D2216) were performed in conjunction with the sample extrusion process and preparation of test specimens. Moisture content determinations were made for each extruded sample, and total densities were computed for each sample. Moisture content for each sample are included on the soil boring logs in Appendix B.

3. Atterberg Limits

A total of ninety-three (93) Atterberg limit determinations (ASTMD4318) were performed on selected samples to assist in soil classification and to enable correlation to pertinent clay behavior properties. The Atterberg limit data consist of measured liquid limit (LL) and plastic limit (PL) values from which the plasticity index ($PI = LL - PL$) is derived. The individual test data are included on the boring logs in Appendix B.

4. Percent Passing #200 sieve and Hydrometer Analysis



A total of six (6) hydrometer particle size analysis tests (ASTM D7928) along with sieve analyses and 26 fines content determinations (ASTM D 1140) were performed on selected samples. The test results, in terms of percent fines (i.e., percent by dry weight finer than the U.S. No. 200 sieve size, 0.074 mm, or silt and clay fraction) are included on the soil boring logs in Appendix B. The hydrometer test with sieve analyses results are included in Appendix C.

5. Specific Gravity

A total of eight (8) specific gravity tests based on ASTM D854 were performed on selected samples and results are included in Appendix C.

6. Organic Content

A total of nine (9) organic tests based on ASTM D2974 were performed on selected samples. The organic content results are included in Appendix C.

7. Total Unit Weight Determination

Total unit weights of the tube samples were computed based on (ASTM D2937) sample volume and weight measurements taken after exclusion of any materials that appeared to have been disturbed during the sampling or extrusion process.

STRENGTH TESTS

To determine the strength characteristics of the marsh area soil deposits, various compression tests and index strength tests were performed. The following tests for the selected marsh area soil samples were performed:

1. Unconsolidated-Undrained Triaxial Compression Tests

Unconsolidated-undrained (UU) triaxial compression tests (ASTM D2850) were performed on specimens trimmed from selected samples. Results of these strength tests are included on the soil boring logs in Appendix B. Individual UU test stress-strain curves are included in Appendix D.

2. Hand penetrometer and Torvane Index Strength Tests

Hand-operated Torvane (TV) and hand penetrometer index strength tests were performed in conjunction with the sample extrusion process and are included in soil boring logs. These test results are considered to be index strengths in that the absolute value of the measured undrained shear strength is generally not considered adequately reliable for use in design. The test results, however, are useful in identifying soil strength variability and trends with respect to depth, material type, etc.

CONSOLIDATION TESTS

To determine the stress history, stress deformation, and time rate settlement characteristics of the marsh area soil deposits, consolidation tests were performed on selected clay samples from marsh area soil borings. A total of 8 tests were performed.



Additionally, to determine the settlement criteria for the dredged soil from the borrow area, settling column tests and low stress consolidation tests were performed. A total of 2 tests each are being conducted in our geotechnical lab.

1. Consolidation Test (ASTM D2435)

Incremental consolidation tests (ASTM D 2435) were included in the laboratory testing program to enable assessment of stress history and determination of stress history, stress deformation, and time rate settlement characteristics of the marsh clay deposits that will dictate post-construction settlement of the marsh creation area. A total of eight (8) tests were conducted.

During each load increment, the accumulation of vertical displacement with time is measured. In general, each load increment was sustained for a period of 12 to 36 hours. The vertical displacement versus time data was evaluated using the conventional log-time and square-root time curve fitting techniques to determine the end of primary consolidation (i.e., the point in time at which dissipation of load-induced excess pore water pressures in the sample had dissipated and drained creep ensued for each load increment). The individual test results, in terms of coefficient of consolidation versus effective vertical stress and void ratio versus effective vertical stress are presented in Appendix E.

2. Settling Column Test

Two representative composite samples were prepared using samples obtained from the nine (9) sampling locations performed within the proposed borrow area. Laboratory settling tests were performed in a 20-cm diameter graduated plexi-glass settling columns.

A water sample from the Lake Pontchartrain was collected to obtain the salinity of the dredging site water. Salinity test was performed by gravimetric method and was also further confirmed by a digital salinity meter. The salinity of dredging site water was calculated about 2 g/l. The composite test samples were mixed with synthesized water having a salinity, in terms of total dissolved solids, of 2 g/l.

As requested, settling column tests with two different solid concentrations, 150 g/l and 300 g/l, are being conducted in our geotechnical lab.

Settling Colum #1 (150 g/l): The initial solids content of the settling test samples was about 13 percent, with a corresponding initial total suspended solids concentration of approximately 150 g/l. The slurry was mixed with a hand-held stirrer to provide a homogeneous sample and remove any segregation of particles which may have occurred during placement of the slurry into the column.

Settling Colum #2 (300 g/l): The initial solids content of the settling test samples was about 25 percent, with a corresponding initial total suspended solids concentration of approximately 300 g/l. The slurry was mixed with a hand-held stirrer to provide a homogeneous sample and remove any segregation of particles which may have occurred during placement of the slurry into the column.

A 7-foot settling column was prepared to run the tests on composite samples. The settling column tests simulate the sedimentation of a dredged material in a quiescent environment. This test



provides information regarding two types of settling regimes which occur; zone settling, and compression settling. The settling tests consist of measuring the fall of the liquid-solid interface over time. These data are plotted in terms of height of the interface over time in Appendix E.

3. Low Stress Consolidation Test (Modified ASTM D2435)

One-dimensional incremental slurry consolidation tests were performed on two (2) composite samples, with solid concentrations 150 g/l and 300 g/l, prepared from the dredge borrow area. The change in specimen height with time under each load is monitored and evaluated to characterize the one-dimensional compressibility, consolidation and drained creep properties of the sediment.

It should be noted that some differences were observed in the void ratio versus vertical effective stress relationship between the settling column and the slurry consolidation test as can be seen by the variance in void ratio for a similar vertical effective stress. This difference may be due in part to the difference in diameter of the testing equipment, which may lead to arching effects. Another possible reason for the observed discrepancy may be due in part to the salinity of the settling column. The addition of salt into the water may cause dispersion and or swelling of the clay particles, which would result in a change in void ratio.

DISCUSSION ON SUBSURFACE CONDITIONS

MARSH FILL AREA SUBSURFACE CONDITIONS

Based on the field observations, CPT soundings and results of the geotechnical laboratory testing, the soils were classified, and boring logs and CPT sounding plots were developed. A generalized subsurface profile for marsh fill area is presented in Table 6.

Table 6: Generalized Soil Profile

Approximate Elevation Range (Datum: NAVD88 GEOID12B, feet)*	Consistency/Relative Density	Material Description
1 to -8	Very Soft to Soft / Very Loose to Medium Dense	Lean Clay (CL), Fat Clay (CH), Sandy Lean Clay (CL), Organic Clay (OH), Peat (PT), Poorly Graded Sand with Clay (SP-SC), Clayey Sand (SC)
-8 to -18	Firm to Stiff/ Loose to Medium Dense	Lean Clay (CL), Fat Clay (CH), Sandy Lean Clay (CL), Poorly Graded Sand with Clay (SP-SC)
-18 to -22	Firm to Stiff / Medium Dense to Dense	Lean Clay (CL), Fat Clay (CH), Poorly Graded Sand (SP), Poorly Graded Sand with Clay (SP-SC)
-22 to -42	Firm to Stiff / Medium Dense to Very Dense	Fat Clay (CH), Sandy Lean Clay (CL), Poorly Graded Sand (SP)



**Referenced from existing mudline elevation at the boring locations performed in marsh fill area, at the time of drilling activities.*

BORROW AREA SUBSURFACE CONDITIONS

Based on the field observations and results of the geotechnical laboratory testing, the soils were classified, and boring logs were developed. A generalized subsurface profile for borrow area is presented in Table 7.

Table 7: Generalized Soil Profile

Approximate Elevation Range (Datum: NAVD88 GEOID12B, feet)*	Consistency/Relative Density	Material Description
-12 to -24	Very Soft to Very Stiff	Lean Clay (CL), Fat Clay (CH), Sandy Lean Clay (CL)
-24 to -34	Soft to Stiff / Medium Dense	Lean Clay (CL), Fat Clay (CH), Poorly Graded Sand (SP), Poorly Graded Sand with Clay (SP-SC)

**Referenced from existing mudline elevation at the boring locations performed in borrow area, at the time of drilling activities.*

The above soil profile is of generalized nature to highlight the major subsurface stratification features and material characteristics of the marsh creation area. The boring logs and CPT sounding plots included in the Appendix A should be reviewed for specific information at individual locations. These records include soil descriptions, stratification, locations of the samples, and laboratory test data. The stratification shown on the boring logs represent the conditions only at the actual boring locations. Variations may occur and should be expected between and away from boring locations. The stratification represents the approximate boundary between subsurface materials and the actual transition may be more distinct or gradual.

TOP OF PLEISTOCENE

Based on information obtained from soil borings and CPT soundings in the marsh areas, the top of Pleistocene may be established at about -8 to -10 feet elevation in the marsh creation area.

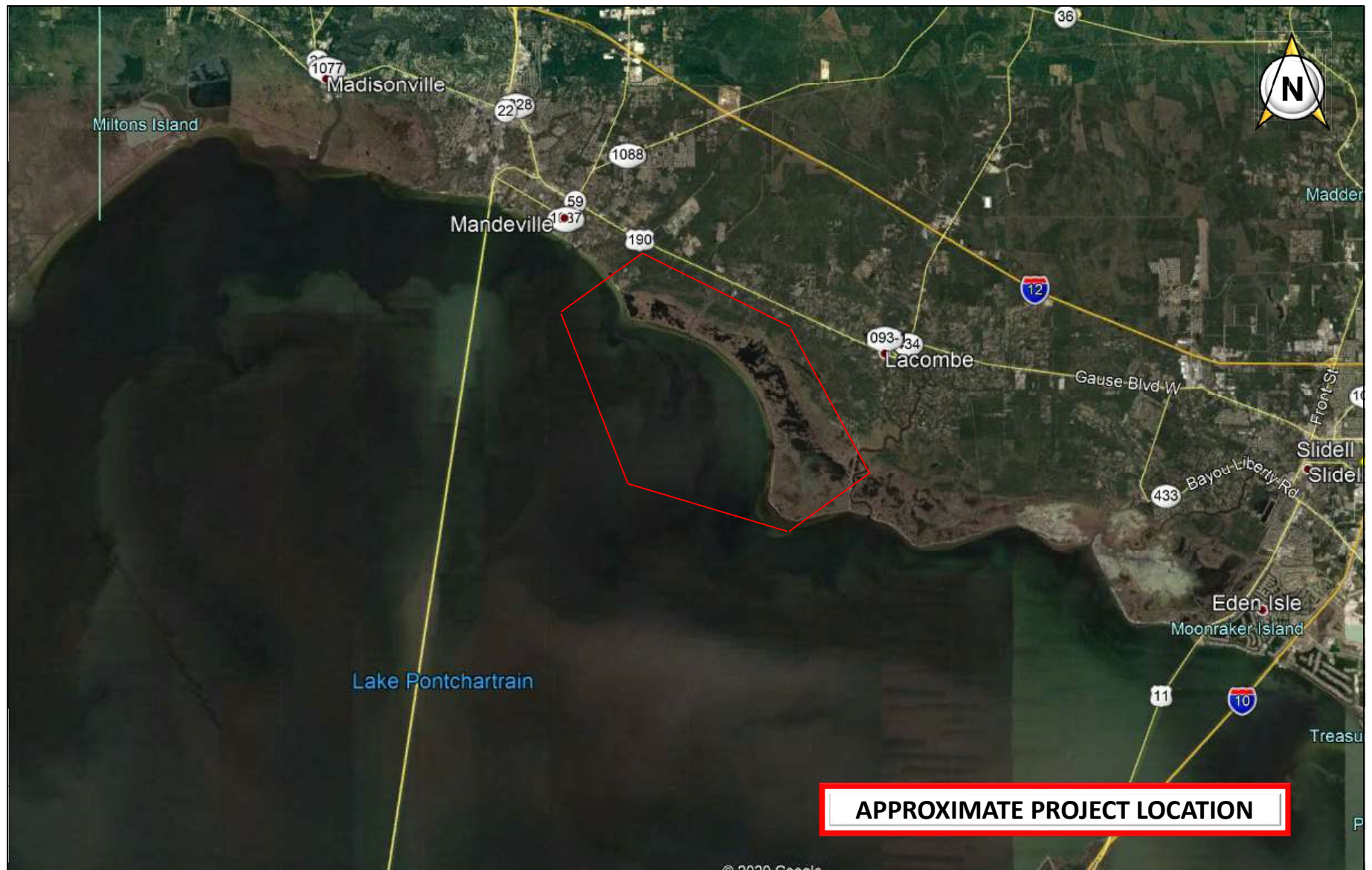
Based on information obtained from soil borings in the borrow area, the top of Pleistocene may be established at about -15 feet elevation in the borrow area.

FUTURE DESIGN REPORT

Based on furnished data from field work and Laboratory test results, soil properties will be established for the design of the proposed project and will be included in the final report.

APPENDIX A

SITE VICINITY MAP SURVEY REPORT



GEOTECHNICAL ENGINEERING SERVICES Bayou Cane Marsh Creation St. Tammany Parish, Louisiana	SITE VICINITY MAP PSI PROJECT NO.: 02541139 GOOGLE EARTH IMAGERY DATE: 1/2018	intertek psi
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Survey Report

PO-181 Bayou Cane Boring Sites Survey St. Tammany Parish, Louisiana

Prepared for Intertek-PSI
724 Central Ave.
Jefferson, LA 70121



Chustz Surveying, LLC
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New Roads, LA 70760
225-638-5949



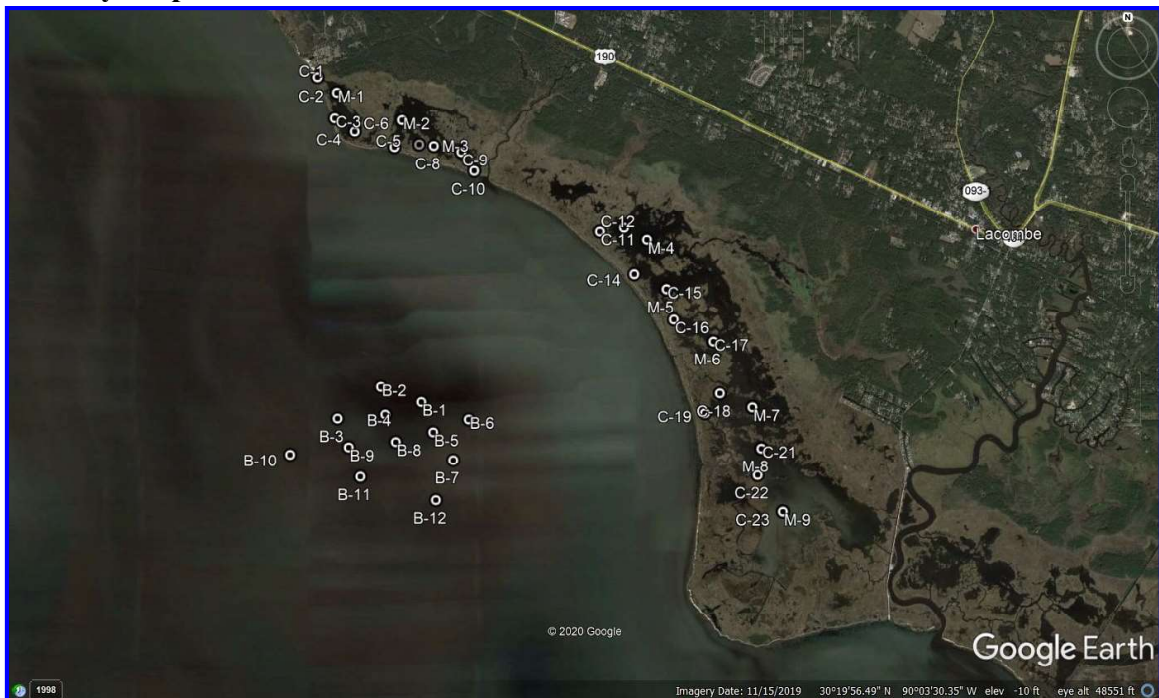
July 2020

Section 1

General Project Description

In July of 2020, Chustz Surveying was tasked by Intertek-PSI to conduct location and magnetometer surveys at 35 boring sites in St. Tammany Parish. The surveys consisted of locating the sites utilizing RTK and multibeam hydrographic methods constrained to the provided project benchmarks (PO33 SM 01 and CRMSPO-SM-11) along with a magnetometer survey.

Vicinity Map



Section 2

Data Collection

On July 6, 2020, Chustz Surveying deployed a two person crew to the site to begin the survey. First they located the provided benchmark CRMSPO-SM-11 and took photos. The crew was initially unable to locate the PO33 SM 01. They then set a temporary gauge and traveled to the sites in the lake to begin the survey. Due to bad weather, the crew was unable to return until July 13, 2020. Upon arrival, they traveled to the sites in the lake and finished the hydrographic survey. Next, utilizing two Trimble R6 GPS receivers, the crew set the base up on CRMSPO-SM-11 and set two 60D nails on site to establish temporary control points for the survey. They then moved the base to one of the nails and made an RTK tie to the other to confirm the temporary control points. The crew then ran a level loop from PO33 SM01 to the temporary gauge to establish the water surface elevation. The crew returned the next day with a third person and a Geometrics G882 cesium magnetometer and established additional control points throughout the site



while the magnetometer was set up and all systems were checked. The next day, they began locating the boring sites in the marsh area with RTK while collecting the magnetometer data. They drove ¾" pvc pipes with pink flagging into the ground at each location in the marsh. In Hypack, they set up a grid around each location to cover a 25 ft. radius for the magnetometer survey. On July 16th, the crew returned to collect the remaining magnetometer data in the lake and completed the survey that day. No significant magnetometer anomalies were detected as part of this survey.

Section 3 Data Processing

The data was transferred to our office for initial processing once the surveys were complete. The RTK data files were loaded into Trimble Business Center where they were processed and checked for correct rod and instrument heights. All RTK ties were checked to ensure they were within the allowed accuracies. The hydrographic and magnetometer data was processed in Hypack utilizing the temporary gauge readings.

Section 6 Project Summary and Conclusion

The field effort was completed ahead schedule on Thursday, July 16, 2020, and all data was processed and deliverables were completed for Friday, July 24, 2020. No safety incidents occurred as part of this work effort.





VICINITY MAP

1 inch = 3,000 feet

Reproduced from NAIP Louisiana 2013 1m Aerial Imagery

Station Name: "PO33 SM 01"

Location: From the intersection of I-12 and Hwy 434 go south 2.7 miles to Hwy 190. Turn right (west) for 0.3 miles to Hwy 434 (Lake road) for 3.4 miles to a bridge and a boat launch. The monument is 177 feet east of the centerline of Lake road, 59 feet N/E of the N/E corner of the concrete boat launch, and 13 feet westerly from the top bank of Bayou Lacome.

Monument Description: Monument is a 9/16 steel rod driven to ?? feet to refusal within a 6" PVC sleeve set in concrete with protective metal accesscover stamped 'PO SM 01'

Stamping: PO SM 01

Installation Date: 2005 **Date of Survey:** 13-May-14

Monument Established By: C&C Technologies

NAD83 (2011) Epoch 2010.00 Geodetic Position

Lat: 30°16'06.92777"N

Long: 89°57'20.77295"W

NAD83 (2011) Epoch 2010.00 Datum LSZ (1702) Ft

N= 645,784.85

E= 3,715,691.24

Adjusted NAVD88 Height

Elevation = 1.19 feet (0.363 mtrs)

Ellipsoid Height (2011) = -26.241 mtrs.

Geoid12A Height = -26.604 mtrs.



FOR REFERENCE ONLY

LCZ Adjusted NAVD88 Height

Elevation (Geoid09)= N/A

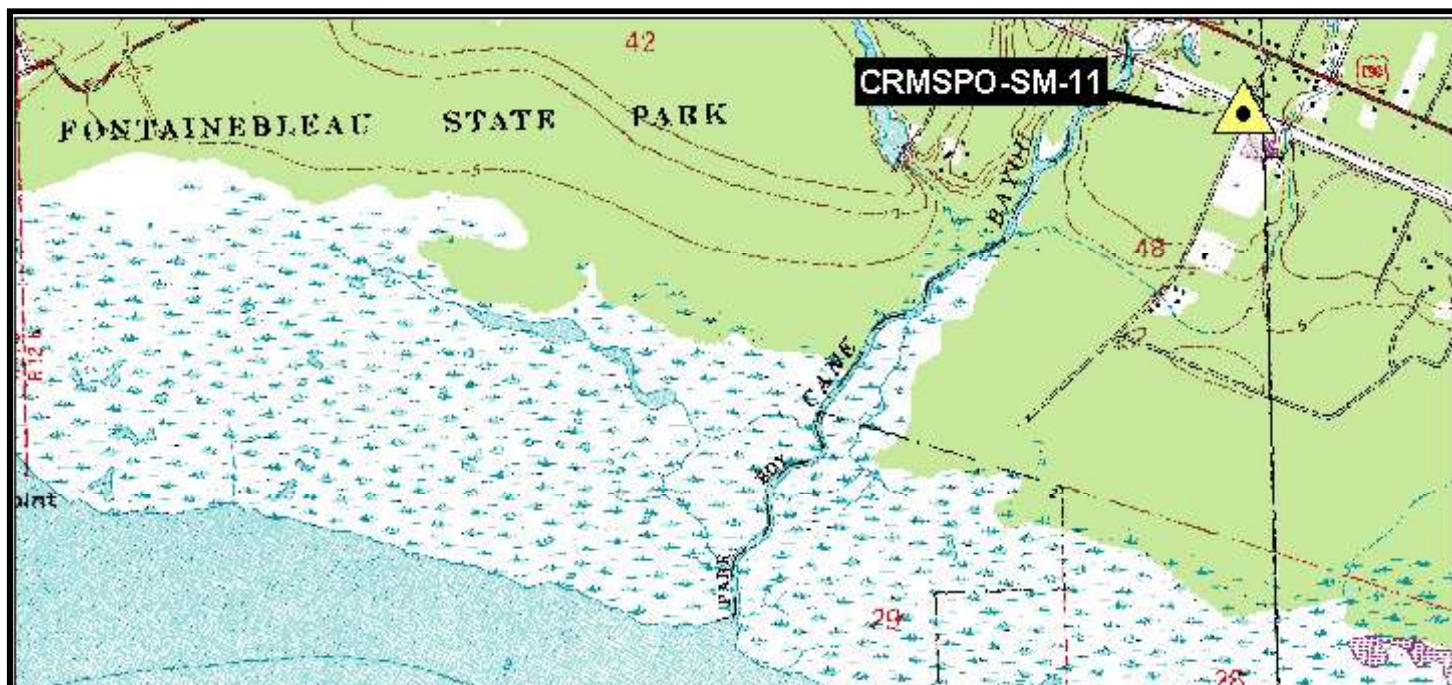
Ellipsoid Height = N/A

Elevation (Geoid03)= 1.76 feet (0.537 mtrs)

Ellipsoid Height = -26.253 mtrs.

Elevation (Geoid99)= N/A

Ellipsoid Height = N/A



VICINITY MAP

Scale: 1" = 2000'

Reproduced from USC&GS "MANDEVILLE & LACOMBE"

Quadrangle

Station Name: CRMSPO-SM-11

Monument Location: From the intersection of Hwy US Hwy 190 and the Lake Pontchartrain Causeway in Mandeville, go 6.8 miles west on Hwy 190 to Lemieux Blvd. Turn right on Lemieux Blvd. and go south 0.1 miles to a bike trail and the Monument. It is located 7' south of the centerline of a bicycle trail and 43' west of the centerline of Lemieux Blvd.

Monument Description: 9/16" steel rod driven to refusal within a 6" PVC sleeve set in concrete with protective metal access cover stamped **CRMSPO-SM-11**.

Date: July 2005

Monument Established By:

Chustz Surveying, Inc. for
Louisiana Department of
Natural Resources

Adjusted NAD 83 Geodetic Position*

Lat. 30°20'05.83357"N

Long. 90°00'02.48142"W

Adjusted NAD 1983 Datum*

LSZ (1702) Feet

N= 669750.54

E= 3701231.54

Adjusted NAVD88 (Feet) /Geoid99*

Elevation = 18.02



* As surveyed for LaDNR CRMS project, contract NO: 2503-05-56

PO-181 Bayou Cane Survey Locations (NAVD88-NAD83 US Survey Ft)

Site	Northing	Easting	Elevation	Water Surface	Depth
C-1	669179.29	3687573.24	0.49	-0.02	N/A
C-2/M-1	668446.23	3688520.66	-1.34	-0.02	1.32
C-3	667204.22	3688435.14	-1.48	-0.02	1.46
C-4	666574.52	3689404.84	-1.29	-0.02	1.27
C-5	665842.48	3691338.72	-0.97	-0.02	0.95
C-6/M-2	667154.95	3691693.24	-1.14	-0.02	1.12
C-7	665982.13	3692533.45	0.42	-0.02	N/A
C-8	665918.83	3693244.47	-2.14	-0.02	2.12
C-9/M-3	665644.37	3694586.10	-2.01	-0.02	1.99
C-10	664745.33	3695215.87	0.72	-0.02	N/A
C-11	661903.21	3701333.28	-1.44	-0.02	1.42
C-12	662098.14	3702500.36	-1.49	-0.02	1.47
C-13/M-4	661535.73	3703615.42	-2.75	-0.02	2.73
C-14	659854.65	3703024.00	-1.76	-0.02	1.74
C-15/M-5	659157.35	3704594.77	-2.58	-0.02	2.56
C-16	657716.47	3704969.87	-0.90	-0.02	0.88
C-17/M-6	656671.99	3706892.30	-2.18	-0.02	2.16
C-18	654174.87	3707231.95	0.29	-0.02	N/A
C-19	653210.85	3706520.76	-0.65	-0.02	0.63
C-20/M-7	653523.53	3708819.53	-1.60	-0.02	1.58
C-21/M-8	651544.24	3709278.43	-0.02	-0.02	0.00
C-22	650266.17	3709111.32	0.46	-0.02	N/A
C-23/M-9	648517.00	3710354.60	0.58	-0.02	N/A
B-1	653592.17	3692783.59	-12.47	-0.02	12.45
B-2	654296.28	3690819.59	-12.93	-0.02	12.91
B-3	652748.14	3688732.89	-13.84	-0.02	13.82
B-4	652972.28	3691044.15	-13.54	-0.02	13.52
B-5	652139.05	3693373.58	-12.61	-0.02	12.59
B-6	652775.97	3695090.92	-12.34	-0.02	12.32
B-7	650812.70	3694361.08	-12.57	-0.02	12.55
B-8	651652.76	3691569.40	-13.14	-0.02	13.12
B-9	651369.34	3689291.20	-13.99	-0.02	13.97
B-10	650982.83	3686457.33	-14.17	-0.02	14.15
B-11	649962.50	3689870.89	-13.84	-0.02	13.82
B-12	648877.69	3693540.55	-13.44	-0.02	13.42
B-13	650171.42	3692404.31	-13.62	-0.02	13.60



ELP2

Mag Hit	Latitude	Longitude	X Coord.	Y Coord.	Peak Min	Peak Max	Peak Spread	Distance Over Ground	Time Elapsed
1	30 19 29.9534 N	090 01 42.6568 W	3,692,495	666,025	47,006.00	47,013.50	7.44	8.15	6.10
2	30 18 27.5485 N	089 59 43.9758 W	3,702,969	659,841	46,948.70	46,985.10	36.41	36.08	19.00
3	30 18 27.3648 N	089 59 44.4118 W	3,702,931	659,822	46,942.20	46,981.80	39.61	19.15	8.81
4	30 18 27.4583 N	089 59 43.8629 W	3,702,979	659,832	46,957.40	46,983.30	25.90	38.49	18.88
5	30 18 49.8561 N	089 59 49.2893 W	3,702,477	662,089	46,956.90	46,987.80	30.86	47.72	28.24
6	30 18 49.9790 N	089 59 48.7171 W	3,702,527	662,102	46,958.60	46,977.30	18.77	52.69	22.30

• Denotes Mag Hit
• Denotes Bore Location

ALL INFORMATION SHOWN HEREON IS BASED ON EVIDENCE FOUND DURING FIELD SURVEY AND PERMITTED INFORMATION; OTHER HAZARDS/PIPELINES MAY EXIST WITHIN THE SURVEYED AREA THAT WERE NOT DISCOVERED DURING THE COURSE OF THE SURVEY.

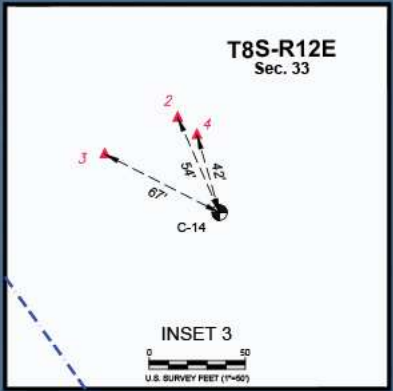
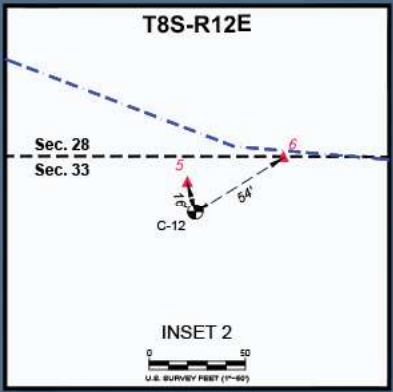
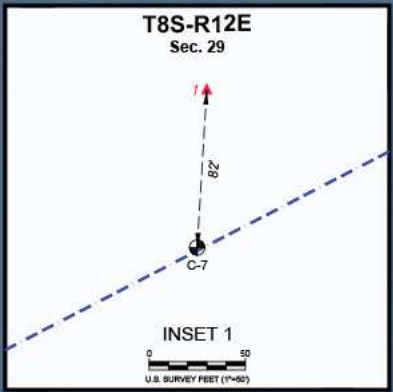
1,000 0 1,000
U.S. SURVEY FEET (1"=1,000')

DATES OF SURVEY 7/15/2020 & 7/16/2020

PREPARED BY:
 CHUSTZ SURVEYING

BAYOU CANE MARSH CREATION (PO-181)
MAGNETOMETER SURVEY
Lake Pontchartrain Area
St. Tammy Parish, Louisiana

JOB NO.	200154	SIZE	34x22	FILENAME	200154-BASE.DWG
CHK	BCH	DWG REF	DATE	7/20/2020	SHEET: 1 OF 1
					REV: 0



SEE
INSET 1

25' RADIUS SURVEYED
AROUND EACH MARSH
LOCATION

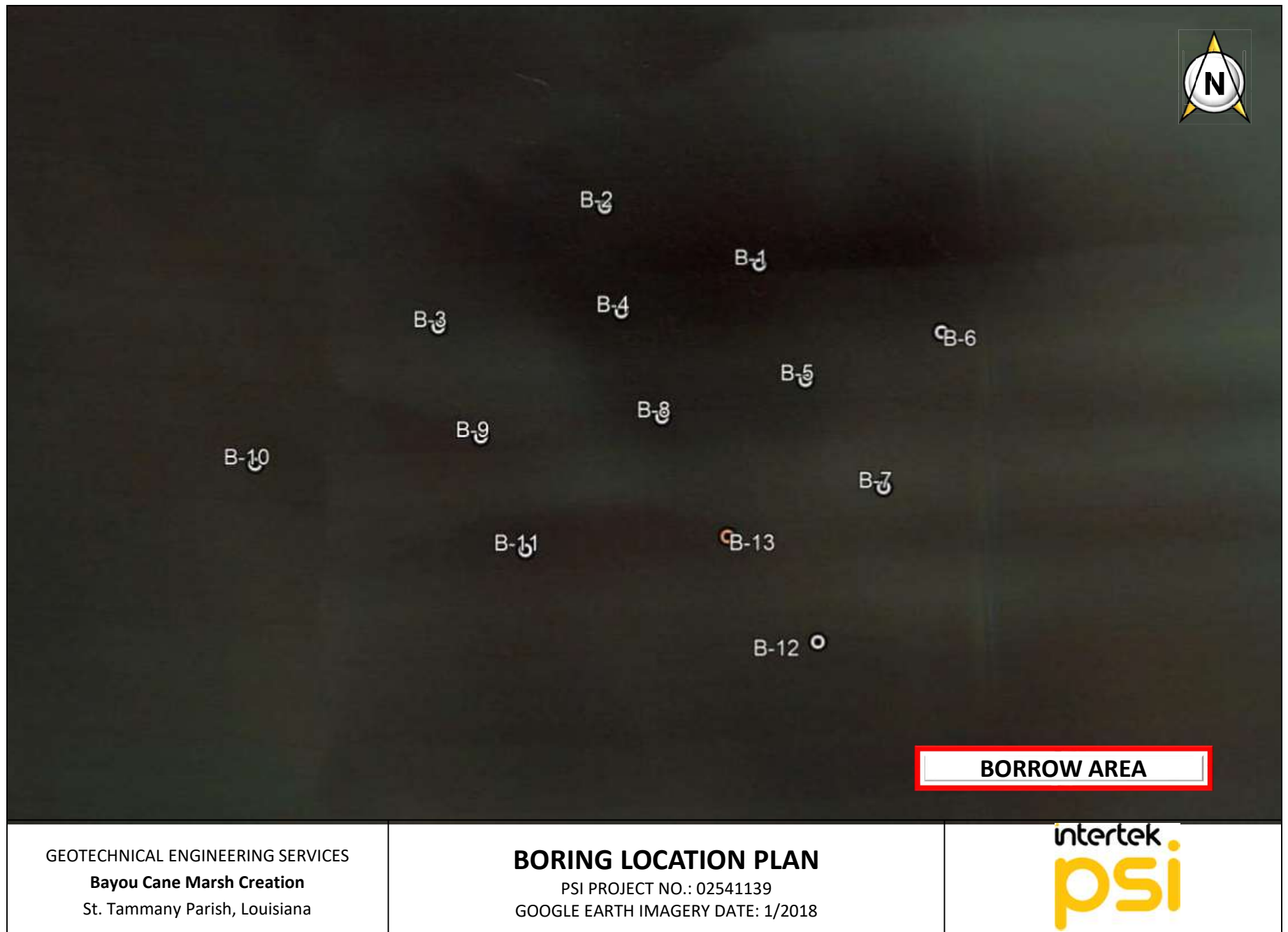
SEE
INSET 2

SEE
INSET 3

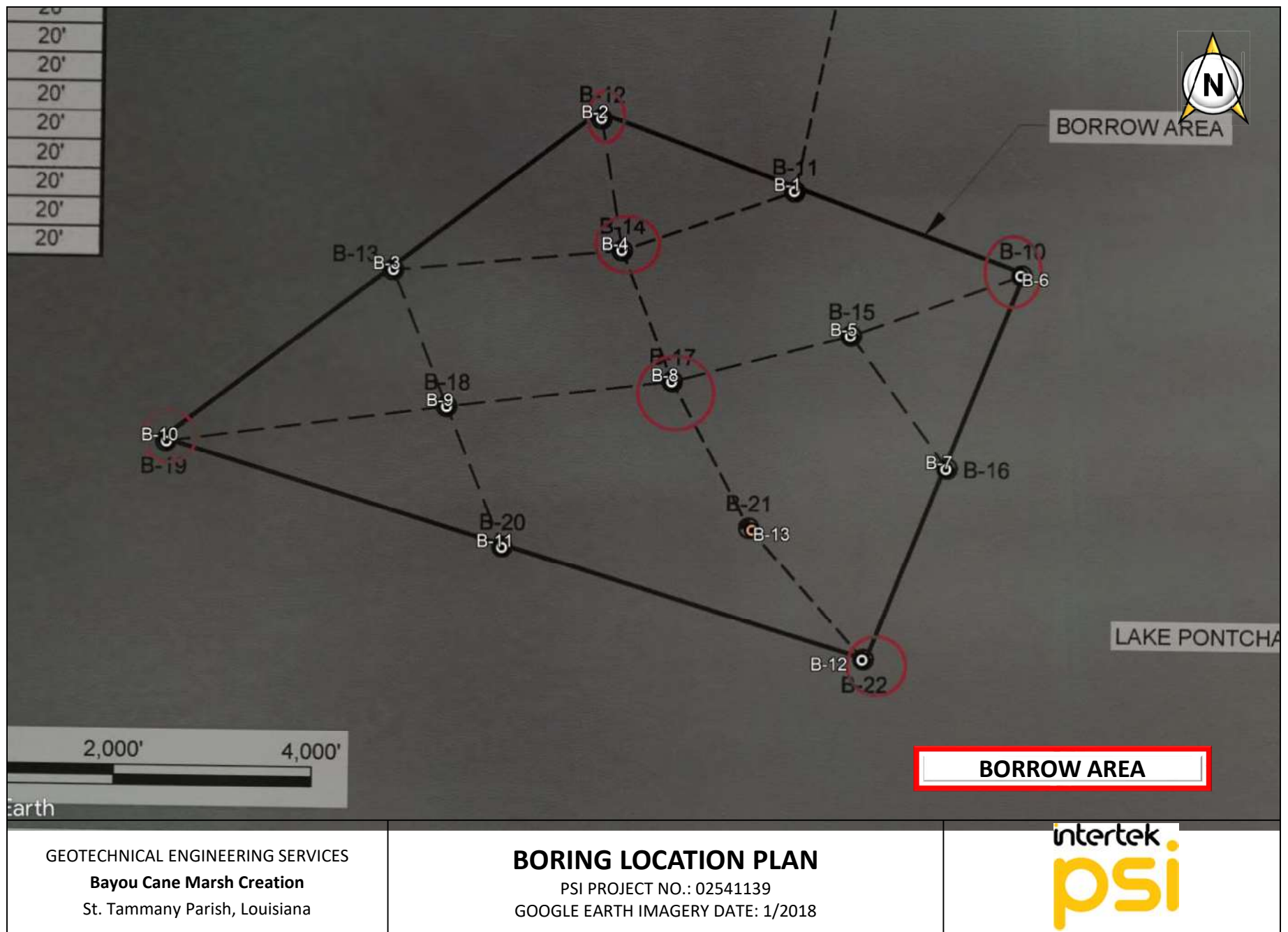
MAGNETOMETER SURVEY
PERFORMED ON PERIMETER
OF BORROW AREA. NO
ANOMALIES DETECTED.

Appendix Figure No. G.1-22

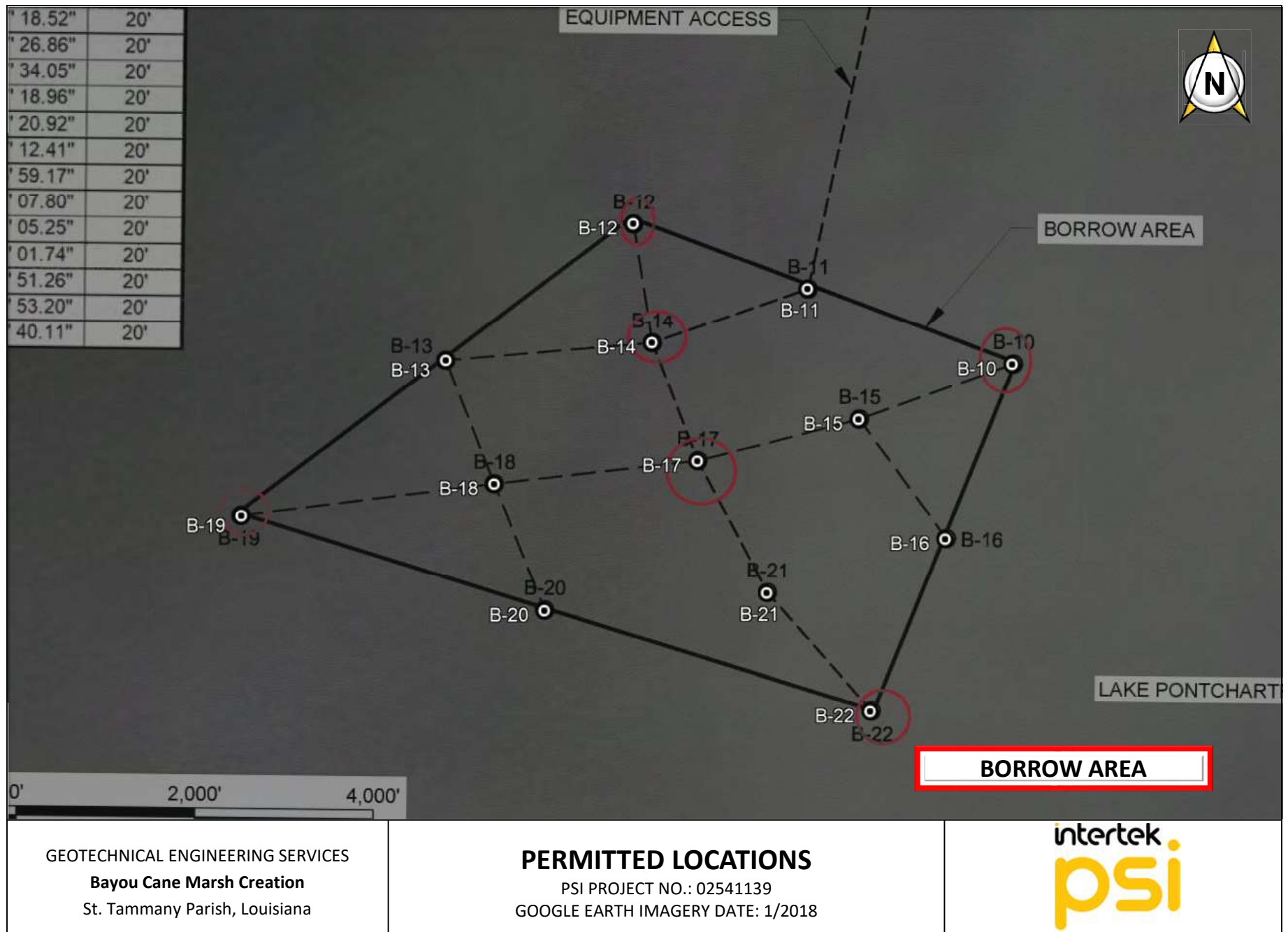
APPENDIX B
BORING LOCATION PLAN
BORING LOGS AND CPT SOUNDING PLOTS
KEY TO TERMS AND SYMBOLS USED ON BORING LOGS



Appendix Figure No. G.1-24



Appendix Figure No. G.1-25







GEOTECHNICAL ENGINEERING SERVICES
Bayou Cane Marsh Creation
 St. Tammany Parish, Louisiana

BORING LOCATION PLAN

PSI PROJECT NO.: 02541139
 GOOGLE EARTH IMAGERY DATE: 1/2018





GEOTECHNICAL ENGINEERING SERVICES
Bayou Cane Marsh Creation
 St. Tammany Parish, Louisiana

BORING LOCATION PLAN
 PSI PROJECT NO.: 02541139
 GOOGLE EARTH IMAGERY DATE: 1/2018

intertek
psi



GEOTECHNICAL ENGINEERING SERVICES
Bayou Cane Marsh Creation
 St. Tammany Parish, Louisiana

PERMITTED LOCATIONS

PSI PROJECT NO.: 02541139
 GOOGLE EARTH IMAGERY DATE: 1/2018



LOG OF BORING B-1

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)
												○ HP ● UC △ TV ◆ MV ▲ UU				HP	UC	TV	UU	
												0.0	0.5	1.0	1.5					
		CH		Very soft gray FAT CLAY , with less than 3/4" shell fragments		84	60													
		CH		Stiff gray FAT CLAY		75	63	52	15	37										93
-2.5				- less than 3/4" shell fragments , 3 to 6 feet																
						79	27													
-5.0																				
						75	33													
-7.5																				
		CH			Stiff light gray/brown FAT CLAY		71	45												
-10.0			CH		Firm to stiff gray FAT CLAY		67	44												
-12.5		CH		Soft to firm gray FAT CLAY , with sand partings		79	45													
-15.0						79	47													
-17.5		CH		Soft gray/brown FAT CLAY		75	45													
		CH		Stiff to very stiff gray/brown FAT CLAY		71	49													
-20.0				BORING TERMINATED AT 20 FEET																

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/25/20

NOTE: ELEVATION OF BORING: -12.47 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:49 - 0254

LOG OF BORING B-2

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
												SHEAR STRENGTH (tsf)								
												HP	UC	TV	UU					
								LL	PL	PI		0.0	0.5	1.0	1.5					
		CL		Firm to stiff gray LEAN CLAY WITH SAND		79	24													
-2.5				- Hydrometer Results: 41.93% clay & 58.06% silt, 2 to 4 feet		71	26	44	14	30	79						0.42		0.45	
																	0.33		0.55	99
		CH		Stiff light gray/brown FAT CLAY , with sand partings		71	26	50	14	36							0.33		0.45	100
-5.0																				
-7.5						67	24										0.67		0.75	
						75	23	56	16	40							0.67		0.70	99
-10.0		CH		Stiff to very stiff gray/tan FAT CLAY																
				- silt partings and sand pockets, 10 to 12 feet		67	35										0.83		1.15	
-12.5																				
				- sand partings and fine gravel pockets, 12 to 14 feet		71	36										0.58		0.60	
-15.0																				
				- silt partings, 14 to 18 feet		75	38	84	28	56							0.75		0.88	79
-17.5						83	42										0.58		1.00	
		CH		Firm gray/tan FAT CLAY , with sand partings		75	46										0.33		0.45	
-20.0																				
				BORING TERMINATED AT 20 FEET																

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/23/20

NOTE: ELEVATION OF BORING: -12.93 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:49 - 0254

LOG OF BORING B-3

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
												SHEAR STRENGTH (tsf)								
												HP	UC	TV	UU					
												0.0	0.5	1.0	1.5					
		CH		Firm gray FAT CLAY , with sand partings			26										0.33		0.43	
2.5				- sand pockets and less than 3/4" shell fragments, 2 to 3 feet			26										0.25		0.30	
		CH		Firm to stiff gray/brown FAT CLAY			37										0.25		0.35	
							46										0.58		0.94	
7.5		CH		Stiff brown/gray FAT CLAY			49										0.58		1.00	
							47										0.58		0.88	
12.5		CH		Stiff gray/brown FAT CLAY , with silt partings			36										0.58		0.75	
		CH		Firm to stiff gray FAT CLAY			42										0.50		0.75	
				- less than 3/4" shell fragments and silt partings, 14 to 16 feet			31										0.33		0.45	
17.5				- sand pockets, 16 to 20 feet			40										0.25		0.30	
20.0				BORING TERMINATED AT 20 FEET																

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/23/20

NOTE: ELEVATION OF BORING: -13.84 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:49 - 0254

LOG OF BORING B-4

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)						
												SHEAR STRENGTH (tsf)										
								HP	UC	TV		UU										
													0.0	0.5	1.0	1.5						
		CH		Stiff gray FAT CLAY WITH SAND , with sand partings - Hydrometer Results: (43.65% clay & 56.34% silt), 0 to 2 feet		75	23	50	15	35	75							0.67		0.75		104
-2.5						75	30										0.75		0.88			
-5.0		CH		Stiff to very stiff gray/brown FAT CLAY - sand pockets and silt partings, 4 to 6 feet		71	39	73	25	48							0.92		1.13		84	
-7.5						67	43										0.50		0.63			
-10.0		CH		Firm to stiff gray FAT CLAY - silt partings, 8 to 10 feet		63	43	83	28	55							0.75		0.75		78	
-12.5						71	46										0.50		0.25			
-15.0				- sand partings and less than 3/4" shell fragments, 12 to 14 feet		67	43										0.25		0.33			
-17.5				- trace of less than 3/4" shell fragments, 16 to 18 feet		71	44										0.50		0.75			
-20.0				- sand partings, 18 to 20 feet		50	32	52	18	34							0.58		0.88			
																					90	
				BORING TERMINATED AT 20 FEET																		

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/22/20

NOTE: ELEVATION OF BORING: -13.54 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:49 - 0254

LOG OF BORING B-5

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
												SHEAR STRENGTH (tsf)								
												HP	UC	TV	UU					
								LL	PL	PI		0.0	0.5	1.0	1.5					
		CH		Stiff gray/orange FAT CLAY , with sand partings		71	23													
		CH		Firm to stiff gray/brown FAT CLAY		75	40													
-2.5				- silt partings, 2 to 4 feet																
-5.0				- silt pockets , 4 to 6 feet		83	39													
-7.5						75	44													
		CH		Firm to stiff gray FAT CLAY		63	42													
-10.0				- sand partings, 10 to 12 feet		79	32													
		SP-SC		Loose gray POORLY GRADED SAND WITH CLAY		75	34													
-12.5																				
		SP		Medium dense gray/brown POORLY GRADED SAND , trace of clay, 14 to 16 feet	5-8-9	42	29													
-15.0					17															
					6-8-10	67	26													
-17.5					18															
					7-9-11	67	26													
-20.0					20															
				BORING TERMINATED AT 20 FEET																

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/27/20

NOTE: ELEVATION OF BORING: -12.61 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:49 - 0254

LOG OF BORING B-6

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)					
												SHEAR STRENGTH (tsf)									
												HP	UC	TV	UU						
												0.0	0.5	1.0	1.5						
		CL		Stiff to very stiff gray LEAN CLAY		71	25	42	13	29							0.33		0.30		103
2.5				- sand partings, 0 to 4 feet		51	16									1.25					
5.0						75	30									0.67		1.00			
		CH		Stiff to very stiff gray FAT CLAY WITH SAND		83	41	68	19	49	82					0.58		0.94		83	
7.5				- Hydrometer Results: (57.74% clay & 42.26% silt), 6 to 8 feet		94	38									0.75		1.13			
10.0																					
						43	89	28	61							0.58		0.88		80	
12.5						45										0.58		0.88			
		SP		Medium dense brown POORLY GRADED SAND	5-8-9	51	24														
15.0					17																
					6-9-9	63	25														
17.5					18																
					7-9-21	50	27	NP	NP	NP											
20.0					21																
				BORING TERMINATED AT 20 FEET																	

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/26/20

NOTE: ELEVATION OF BORING: -12.34 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:50 - 0254

LOG OF BORING B-7

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
												SHEAR STRENGTH (tsf)								
								HP	UC	TV		UU								
								LL	PL	PI		0.0	0.5	1.0	1.5					
		CL		Very soft gray SANDY LEAN CLAY , with less than 3/4" shells fragments		67	36	32	17	15									0.03	68
-2.5		CH		Firm to stiff gray FAT CLAY		42	29											0.67	0.88	
				- sand partings, 2 to 6 feet																
-5.0						75	31											0.75	0.88	
						42	39											0.58	1.00	
-7.5						46	40											0.58		
						75	45												0.80	
-12.5				- trace of less than 3/4" shell fragments, 12 to 14 feet		71	46											0.33	0.55	
-15.0						67	30											0.33	0.40	
		CH		Stiff gray FAT CLAY , with silt partings		71	32											0.75		
-17.5																				
		SP		Medium dense brown POORLY GRADED SAND	7-9-13		29													
					22															
-20.0				BORING TERMINATED AT 20 FEET																

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/27/20

NOTE: ELEVATION OF BORING: -12.57 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:50 - 0254

LOG OF BORING B-8

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
																	HP	UC	TV	UU
								LL	PL	PI		0.0	0.5	1.0	1.5					
		CL		Firm to stiff gray LEAN CLAY WITH SAND - Hydrometer Results: (44.37% clay & 55.63% silt), 0 to 2 feet		75	25	38	13	25	74						0.42			97
-2.5						79	23									0.75				
-5.0						75	27									0.83				
-7.5		CH		Stiff gray/tan FAT CLAY - silt lenses, 6 to 8 feet		75	34	78	23	55						0.92			87	
							41									0.83				
-10.0		CH		Stiff tan/gray FAT CLAY		79	43	76	25	51						0.67			78	
-12.5		CH		Stiff tan/orange FAT CLAY		71	43									0.58				
-15.0		CL		Stiff gray/tan LEAN CLAY , with sand partings		67	31									0.67				
-17.5						75	31	46	15	31						0.67			91	
						71	29									0.58				
-20.0				BORING TERMINATED AT 20 FEET																

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/28/20

NOTE: ELEVATION OF BORING: -13.14 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:50 - 0254

LOG OF BORING B-9

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)
												○ HP ● UC △ TV ◆ MV ▲ UU				HP	UC	TV	UU	
												0.0	0.5	1.0	1.5					
		CH		Firm to stiff gray FAT CLAY																
						71	24													
						75	22													
-2.5																				
						67	36													
-5.0																				
						75	37													
-7.5																				
						67	44													
-10.0																				
						75	45													
-12.5																				
						71	36													
-15.0																				
						67	44													
-17.5																				
						75	33													
						71	40													
				- silt seams, 18 to 20 feet																
-20.0				BORING TERMINATED AT 20 FEET																

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/28/20

NOTE: ELEVATION OF BORING: -13.99 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

⚡ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:50 - 0254



Professional Service Industries, Inc.
724 Central Avenue
Jefferson Louisiana 70121
(504) 733-9411

LOG OF BORING B-10

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
												SHEAR STRENGTH (tsf)								
												HP	UC	TV	UU					
												0.0	0.5	1.0	1.5					
		CL		Stiff to very stiff gray LEAN CLAY		71	22	44	11	33						1.08				106
-2.5						67	26									0.67				
				- Hydrometer Results: (39.39% clay & 60.61% silt), 4 to 6 feet		63	28	46	17	29	94					0.50				97
-5.0						42	25									0.83				
-7.5																				
		CH		Stiff gray/brown FAT CLAY		67	36									0.67				
-10.0				- silt seams, 10 to 12 feet		71	39	84	28	56						0.75				84
-12.5						67	41									0.67				
		CH		Stiff brown FAT CLAY		75	41									0.67				
-15.0						75	46	94	29	65						0.67				78
-17.5						71	30									0.58				
-20.0				BORING TERMINATED AT 20 FEET																

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/29/20

NOTE: ELEVATION OF BORING: -14.17 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:49 - 0254

LOG OF BORING B-11

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
												SHEAR STRENGTH (tsf)					HP	UC	TV	UU
												○ HP	● UC	△ TV	◆ MV					
													0.0	0.5	1.0	1.5				
		CL		Firm to stiff gray LEAN CLAY		75	28										0.42	0.38		
-2.5				- silt partings, 2 to 4 feet		75	25										0.58	0.40		
-5.0		CH		Firm to stiff gray FAT CLAY		71	44										0.50	0.88		
-7.5						67	43										0.67	0.63		
-10.0						58	46										0.25	0.30		
-12.5						67	37										0.67	0.98		
-15.0		CH		Firm brown FAT CLAY		71	29										0.25	0.30		
-17.5				- sand partings, 16 to 18 feet		67	31										0.33	0.30		
-20.0		SP		Medium dense brown POORLY GRADED SAND	7-9-12 21	67	22													
BORING TERMINATED AT 20 FEET																				

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/29/20

NOTE: ELEVATION OF BORING: -13.84 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

⚡ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:49 - 0254

LOG OF BORING B-12

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	SHEAR STRENGTH (tsf)			SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)					
								LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	HP	UC	TV		UU				
																	○ HP	● UC	△ TV	◆ MV
								LL	PL	PI		0.0	0.5	1.0	1.5					
		CL		Very soft gray LEAN CLAY WITH SAND ,with less than 3/4" shell fragments - Hydrometer Results: (13.87% clay & 86.13% silt), 0 to 2 feet		71	32	38	12	26	78	○					0.00			101
-2.5		CH		Firm gray FAT CLAY		67	28						○				0.42			
-5.0						38	26	61	19	42			○				0.50			98
-7.5		CH		Stiff gray/brown FAT CLAY		75	48						○				0.58			
						71	46	90	29	61			○				0.50			75
-10.0						79	46						○				0.42			
-12.5						75	46						○				0.67			
-15.0						71	31						○				0.50			
-17.5		CH		Firm gray FAT CLAY , with sand partings		75	44	53	20	33			○				0.25			
						71	48						○				0.42			73
-20.0				BORING TERMINATED AT 20 FEET																

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/30/20

NOTE: ELEVATION OF BORING: -13.44 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:49 - 0254

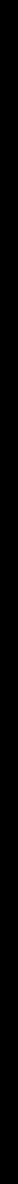
LOG OF BORING B-13

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
								LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)								
												HP	UC	TV	UU					
								LL	PL	PI		0.0	0.5	1.0	1.5	HP	UC	TV	UU	
		CH		Stiff gray FAT CLAY , with sand partings		75	23									0.67		0.63		
2.5					67	22										0.92		1.13		
5.0					38	21										1.00		1.50		
7.5		CH		Stiff light gray/brown FAT CLAY - silt parting, 6 to 8 feet	42	42										0.58		0.88		
10.0					67	32										0.58		0.88		
12.5	CH	Stiff light gray FAT CLAY		71	46										0.58		0.75			
15.0	CH	Firm to stiff gray/brown FAT CLAY		67	46										0.67		0.88			
17.5				- sand partings, 16 to 18 feet		75	36									0.42		0.63		
20.0		SP		Medium dense brown POORLY GRADED SAND	8-10-14 24	63	23													
				BORING TERMINATED AT 20 FEET																

DEPTH OF BORING: 20 FEET

DATE DRILLED: 9/30/20

NOTE: ELEVATION OF BORING: -13.62 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 16:49 - 0254

LOG OF BORING M-1

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
												○ HP ● UC △ TV ◆ MV ▲ UU					HP	UC	TV	UU
												0.0	0.5	1.0	1.5					
		CL		Very soft to soft gray LEAN CLAY , with sand pockets - traces of organic, 0 to 2 feet		95	20	24	12	12		▲		0.08		0.10	0.10	102		
-2.5										71	26					○		0.17		
		CL		Soft to firm light gray SANDY LEAN CLAY		100	27	33	13	20		●		0.25			0.28	107		
-5.0										79	30				50	○		0.17		
-7.5																				
		CL		Stiff light gray SANDY LEAN CLAY		100	24	36	16	20		▲		0.58			0.62	100		
-10.0																				
		CL		Very soft light gray/tan LEAN CLAY		100	30					○		0.08						
-12.5																				
		CH		Stiff tan/light gray FAT CLAY , with sand partings		77	31	66	23	43		○		0.50						
-15.0																				
		CL		Stiff light gray LEAN CLAY , with sand partings		38	30	44	20	24	100	▲		0.50			0.59	92		
-17.5										98	28					○		0.58		
		CL		Soft light gray/tan LEAN CLAY , with sand partings		44	25	34	19	15		○		0.17						
-20.0																				

DEPTH OF BORING: 40 FEET

DATE DRILLED: 9/2/20

NOTE: ELEVATION OF BORING: -1.34 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-1

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)
																HP	UC	TV	UU	
								LL	PL	PI		0.0	0.5	1.0	1.5					
22.5																				
		CH		Stiff light gray FAT CLAY , with sand partings and shell fragments		79	32									0.83			0.64	90
25.0																				
27.5																				
30.0						79	36	67	23	44						0.83				
32.5																				
35.0						29	40									0.83				
37.5																				
40.0						96	32	83	27	56						0.67				
				BORING TERMINATED AT 40 FEET																

DEPTH OF BORING: 40 FEET

DATE DRILLED: 9/2/20

NOTE: ELEVATION OF BORING: -1.34 feet

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-2

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)	
												○ HP ● UC △ TV ◆ MV ▲ UU				HP	UC	TV	UU		
								0.0	0.5	1.0		1.5									
		OH		Very soft dark gray/black ORGANIC CLAY		58	195	207	78	129											
-2.5						67	639	213	67	146											
		CL		Very soft dark gray LEAN CLAY			37	35	16	19											
		CH		Firm to stiff light gray SANDY FAT CLAY		73	23														
-5.0																					
						98	35	58	16	42	61					0.42			0.50	0.74	90
-7.5																					
		CL		Firm light gray LEAN CLAY , with sand pockets		81	22										0.25				
-10.0																					
						98	25	29	21	8							0.33			0.41	96
-12.5																					
						89	36										0.33				
-15.0				- Possible clay stone layer, 14 to 16 feet		100	30	38	22	16											
-17.5						69	37										0.50				
		CH		Stiff tan FAT CLAY WITH SAND		96	27	60	22	38	84									0.66	89
-20.0																					

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/18/20

NOTE: ELEVATION OF BORING: -1.14 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-2

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)
								LL	PL	PI		HP	UC	TV	UU	
22.5																
25.0		CH		Firm to stiff light gray/brown FAT CLAY , with silt partings and shells fragments		81	33				95	0.50				
27.5																
30.0						100	41	82	27	55		0.58				
32.5																
35.0						100	46					0.42				
37.5																
40.0						100	42	86	31	55		0.67				
				BORING TERMINATED AT 40 FEET												

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/18/20

NOTE: ELEVATION OF BORING: -1.14 feet

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-3

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
												SHEAR STRENGTH (tsf)								
								HP	UC	TV		UU								
													0.0	0.5	1.0	1.5				
		OH		Very soft black ORGANIC CLAY		83	438	392	131	261		▲						0.03	0.03	12
2.5		CL		Very soft gray LEAN CLAY , with sand partings		21	33					△					0.05			
		OH		Very soft dark gray/black ORGANIC CLAY		100	460	314	95	219		▲					0.03	0.04	16	
5.0						69	529	394	117	277		△					0.02			
7.5																				
		CL		Firm to stiff light gray LEAN CLAY WITH SAND		36	21	42	12	30		○			0.42					
10.0						83	27				80	▲				0.50	0.55	93		
12.5				- sand partings, 12 to 14 feet		94	36	46	19	27		⊕			0.25					
		CH		Firm to stiff tan/light gray FAT CLAY , sand		100	40					⊕			0.50					
15.0				- sand seams, 14 to 16 feet																
				- calcareous framents, 16 to 18 feet		63	32	61	23	38		⊕			0.50					
17.5				- silt partings and calcareous fragments, 18 to 30 feet		69	33				97	⊕			0.58					
20.0																				

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/19/20

NOTE: ELEVATION OF BORING: -2.01 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-3

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)													
												○ HP ● UC △ TV ◆ MV ▲ UU				HP	UC	TV	UU														
												0.0	0.5	1.0	1.5																		
22.5						63	37	76	26	50			0.50																				
25.0																																	
27.5																																	
30.0																			77	36				0.75		0.74	79						
32.5																																	
35.0																			100	38	86	30	56		0.50								
37.5																																	
40.0																			100	49				0.42	0.45	0.45	69						
BORING TERMINATED AT 40 FEET																																	

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/19/20

NOTE: ELEVATION OF BORING: -2.01 feet

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-4

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
												SHEAR STRENGTH (tsf)								
								HP	UC	TV		UU								
								LL	PL	PI		0.0	0.5	1.0	1.5					
		OH		Very soft dark gray/black ORGANIC CLAY		100	659	425	114	311										
2.5						56	45	53	21	32										
		CL		Soft to firm light gray LEAN CLAY WITH SAND			28	34	14	20								0.15	0.12	94
5.0						98	26				77					0.25				
						51	33	45	15	30						0.25				
7.5																				
		CL		Firm to stiff light gray LEAN CLAY WITH SAND		83	28	49	17	32						0.58		0.75	0.68	94
10.0						92	25				81					0.50				
12.5						92	30									0.50				
				- sand partings, 12 to 14 feet																
15.0		CH		Firm light gray FAT CLAY , with silt partings		90	41	58	20	38	100					0.42				
		CL		Firm to stiff light gray LEAN CLAY , with silt partings		31	32	36	23	13						0.50			0.60	94
17.5																				
		CH		Firm to stiff gray/brown FAT CLAY , with silt and sand partings		48	46									0.33				
20.0																				

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/13/20

NOTE: ELEVATION OF BORING: -2.75 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-4

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	SHEAR STRENGTH (tsf)			% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)	
								LL	PL	PI		SHEAR STRENGTH (tsf)					
												HP	UC	TV	UU		
22.5						100	49										
25.0																	
27.5																	
30.0				- shells, 28 to 30 feet		96	35	73	26	47					0.42		
32.5																	
35.0						58	45								0.42		
37.5																	
40.0				- silt partings, 38 to 40 feet		100	54	78	26	52						0.50 0.57	70
				BORING TERMINATED AT 40 FEET													

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/13/20

NOTE: ELEVATION OF BORING: -2.75 feet

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-5

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)
												○ HP ● UC △ TV ◆ MV ▲ UU				HP	UC	TV	UU	
								0.0	0.5	1.0		1.5								
		PT		Very soft black PEAT			442													
		OH		Very soft dark gray ORGANIC CLAY			159	135	34	101		▲					0.05	0.04	30	
2.5		CL		Very soft to soft light gray SANDY LEAN CLAY		64	25					△					0.15			
5.0						92	24	34	15	19	70	○			0.17					
7.5		CL		Soft light gray LEAN CLAY WITH SAND		83	23					⊕			0.25					
10.0		CL		Stiff light gray LEAN CLAY , silt and trace sand		56	22	40	18	22			▲		0.75		0.75	106		
12.5						58	25				89		○		0.83					
						92	27	41	19	22			⊕		0.50					
15.0		CH		Firm light gray FAT CLAY , silt		42	32	61	22	39	100	●			0.33		0.39	83		
17.5		CH		Firm to stiff light gray/tan/brown FAT CLAY - silt partings 16 to 20 feet		100	41					○			0.50					
20.0						100	40	68	25	43		○			0.50					

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/12/20

NOTE: ELEVATION OF BORING: -2.58 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-5

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)
								LL	PL	PI		SHEAR STRENGTH (tsf)				HP	UC	TV	UU	
												○ HP	● UC	△ TV	◆ MV					
												0.0	0.5	1.0	1.5					
22.5						100	42										0.42			
25.0																				
27.5																				
30.0						100	50	91	31	60	100								0.33	69
32.5																				
35.0		CH		- silt partings 33 to 35 feet		100	54											0.42		
37.5																				
40.0		CH		Very soft gray FAT CLAY		100	61	80	26	54								0.08		
				BORING TERMINATED AT 40 FEET																

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/12/20

NOTE: ELEVATION OF BORING: -2.58 feet

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-6

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)
												○ HP	● UC	△ TV	◆ MV	▲ UU	HP	UC	TV	
								0.0	0.5	1.0		1.5								
		OH		Very soft dark gray/black ORGANIC CLAY			69	93	31	62										
-2.5		PT		Very soft black PEAT		96	588													
		CL		Very soft to soft gray SANDY LEAN CLAY			23	32	13	19							0.15	0.14	106	
-5.0				- silt seams 4 to 6 feet	0-0-1 1		31				64									
-7.5					1-2-2 4		26													
		CH		Firm to stiff light gray FAT CLAY , with sand pockets		60	33	54	22	32						0.33			0.44	89
-10.0						31	29				93					0.42				
-12.5						87	32	60	22	38						0.33			0.44	82
-15.0						60	33	54	21	33						0.42				
-17.5				- shell fragments, 16 to 18 feet		94	39									0.50				
-20.0						77	37	79	26	53						0.33			0.46	78

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/11/20

NOTE: ELEVATION OF BORING: -2.18 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-6

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	SHEAR STRENGTH (tsf)			% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)			
								LL	PL	PI		HP	UC	TV	UU				
																	○ HP	● UC	△ TV
											0.0	0.5	1.0	1.5					
22.5						79	46								0.33				
25.0																			
27.5																			
30.0																			
32.5																			
35.0																			
37.5																			
40.0																			
				BORING TERMINATED AT 40 FEET															

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/11/20

NOTE: ELEVATION OF BORING: -2.18 feet

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-7

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
												SHEAR STRENGTH (tsf)								
								HP	UC	TV		UU								
								LL	PL	PI		0.0	0.5	1.0	1.5					
		OH		Very soft dark gray ORGANIC CLAY		65	99	54	20	34								0.05	0.06	82
-2.5		PT		Very soft black PEAT		33	516	543	200	343										
		SC		Loose light gray/black CLAYEY SAND , traces of organic			21	24	16	8										
-5.0		CH		Firm to stiff light gray FAT CLAY , with sand partings		25	34									0.42				
-7.5						89	26	56	17	39							0.98	1.03	100	
		CL		Firm light gray LEAN CLAY , with sand partings		100	28									0.25				
-10.0						92	30	40	19	21	90					0.42		0.50	0.52	92
-12.5						87	28									0.42				
-15.0		CH		Stiff light gray FAT CLAY		83	32	74	25	49	100					0.67				
				- silt partings and shell fragments, 14 to 16 feet		67	39	76	28	48							0.88	0.72	80	
-17.5						98	44									0.42				
-20.0				- silt partings, 18 to 20 feet																

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/10/20

NOTE: ELEVATION OF BORING: -1.60 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-7

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)
												○ HP ● UC △ TV ◆ MV ▲ UU				HP	UC	TV	UU	
												0.0	0.5	1.0	1.5					
22.5		CH	Firm to stiff gray FAT CLAY - silt partings, 23 to 25 feet - sand pockets and shell fragments, 33 to 35 feet - sand pockets , 38 to 40 feet																	
					63	55	82	28	54					○	0.42					
25.0																				
27.5																				
30.0																				
32.5																				
35.0																				
37.5																				
40.0																				
				BORING TERMINATED AT 40 FEET																

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/10/20

NOTE: ELEVATION OF BORING: -1.60 feet

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-8

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)
												○ HP ● UC △ TV ◆ MV ▲ UU				HP	UC	TV	UU	
												0.0	0.5	1.0	1.5					
		OH		Very soft black SANDY ORGANIC CLAY																
					WOH		52	32	20	12	57									
2.5					0-0-1		19													
		CL		Very soft to soft gray SANDY LEAN CLAY	1-2-1		18	42	12	30										
5.0					3															
					WOH		25				67									
7.5																				
		CL		Firm to stiff gray SANDY LEAN CLAY	2-2-4		24	40	12	28										
10.0					6															
					3-5-6		26				60									
12.5					11															
						58	24	44	18	26		▲		0.58			0.60	95		
15.0						92	27					○		0.33						
		CH		Firm light gray/brown FAT CLAY , silt partings		94	36	58	22	36		▲		0.42			0.34	81		
17.5						83	39				100	○		0.42						
20.0																				

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/8/20

NOTE: ELEVATION OF BORING: -0.02 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-8

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)		
												○ HP △ TV ▲ UU	● UC ◆ MV	0.0	0.5	1.0	1.5	HP	UC		TV	UU
								LL	PL	PI												
22.5																						
		CH	⊗	Soft gray FAT CLAY ,with sand seams	1-2-2		46	58	27	31												
			⊗		4																	
25.0																						
27.5																						
			⊗		2-1-2		50															
			⊗		3																	
30.0																						
32.5																						
		SC	⊗	Mwdium dense gray CLAYEY SAND	7-8-4		44				37											
			⊗		12																	
35.0																						
37.5																						
		CH	⊗	Firm light gray SANDY FAT CLAY	3-4-4		41	82	25	57												
			⊗		8																	
40.0				BORING TERMINATED AT 40 FEET																		

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/8/20

NOTE: ELEVATION OF BORING: -0.02 feet

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:42 - 0254

LOG OF BORING M-9

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)
								LL	PL	PI		HP	UC	TV	UU	
		PT		Very soft black PEAT		29	711									
2.5		SC		Very loose CLAYEY SAND	2-2-1 3		18									
5.0					1-2-2 4		23				31					
7.5		SP-SC		Medium dense gray POORLY GRADED SAND WITH CLAY	9-10-11 21		19									
10.0					10-13-13 26		17									
12.5		CL		Very soft to soft tan/gray LEAN CLAY WITH SAND	2-2-2 4		23									
15.0		SP-SC		Medium dense gray POORLY GRADED SAND WITH CLAY	8-10-13 23		18									
17.5					10-12-15 27		22				9					
20.0		SP		Dense to very dense tan POORLY GRADED SAND	10-15-16 31		20									

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/7/20

NOTE: ELEVATION OF BORING: -0.58 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

⚡ DELAYED GROUNDWATER: N / A

JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:43 - 0254

LOG OF BORING M-9

CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: BAYOU CANE

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	RECOVERY (%)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				DRY UNIT WEIGHT (pcf)				
																	HP	UC	TV	UU
												0.0	0.5	1.0	1.5					
22.5					17-50/4" 67/10"		20													
25.0																				
27.5																				
30.0						15-27-29 56		18												
32.5																				
35.0						19-23-25 48		17				1								
37.5																				
40.0						19-25-31 56		17												
			BORING TERMINATED AT 40 FEET																	

DEPTH OF BORING: 40 FEET

DATE DRILLED: 8/7/20

NOTE: ELEVATION OF BORING: -0.58 feet

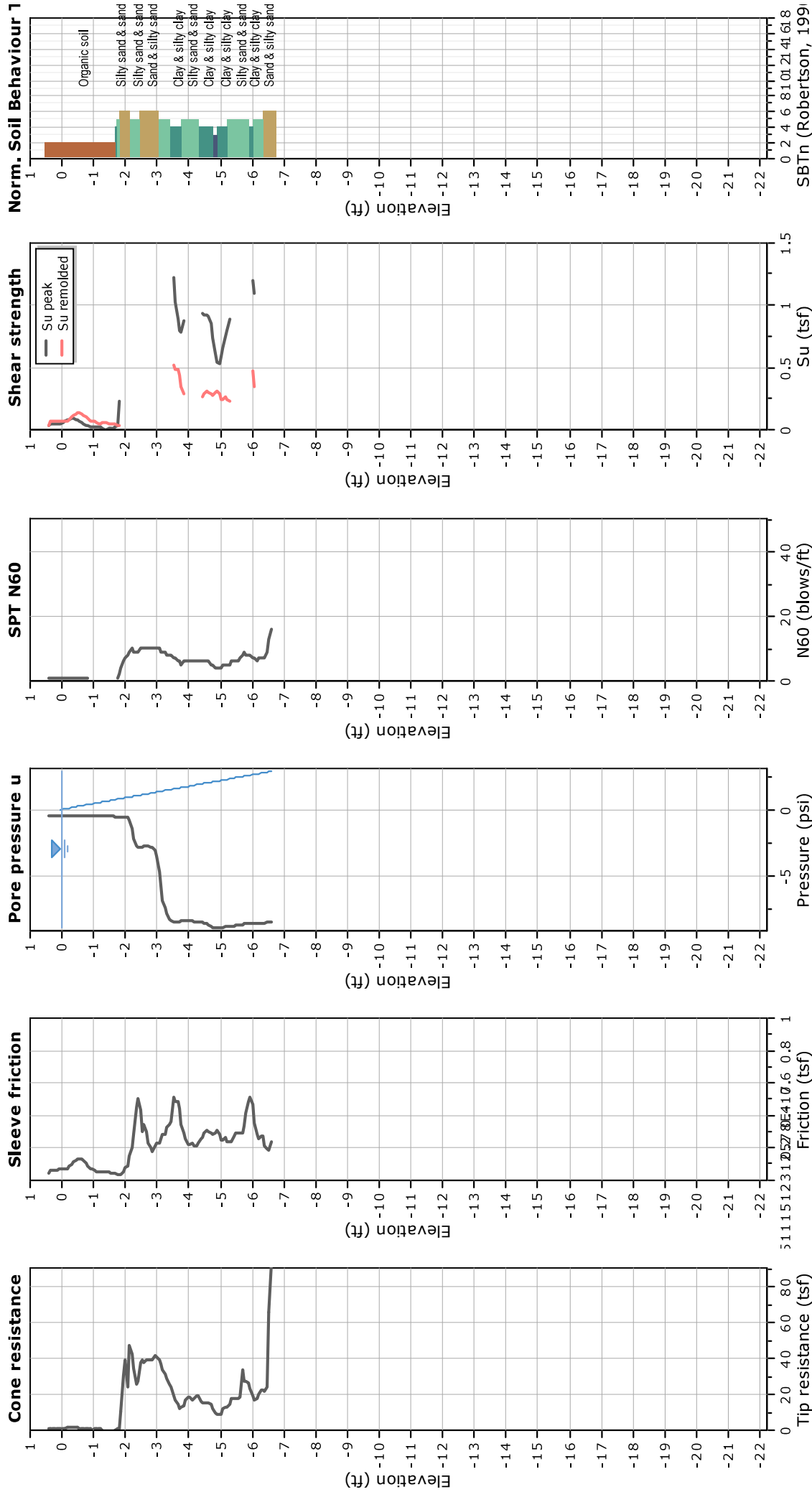
JEFFERSON - LOG WITH RECOVERY - PSIHOUSTON.GDT - 12/8/20 12:43 - 0254



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-01

Total depth: 7.09 ft, Date: 9/1/2020
Surface Elevation: 0.49 ft
Coords: lat 30.333813° lon -90.044002°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



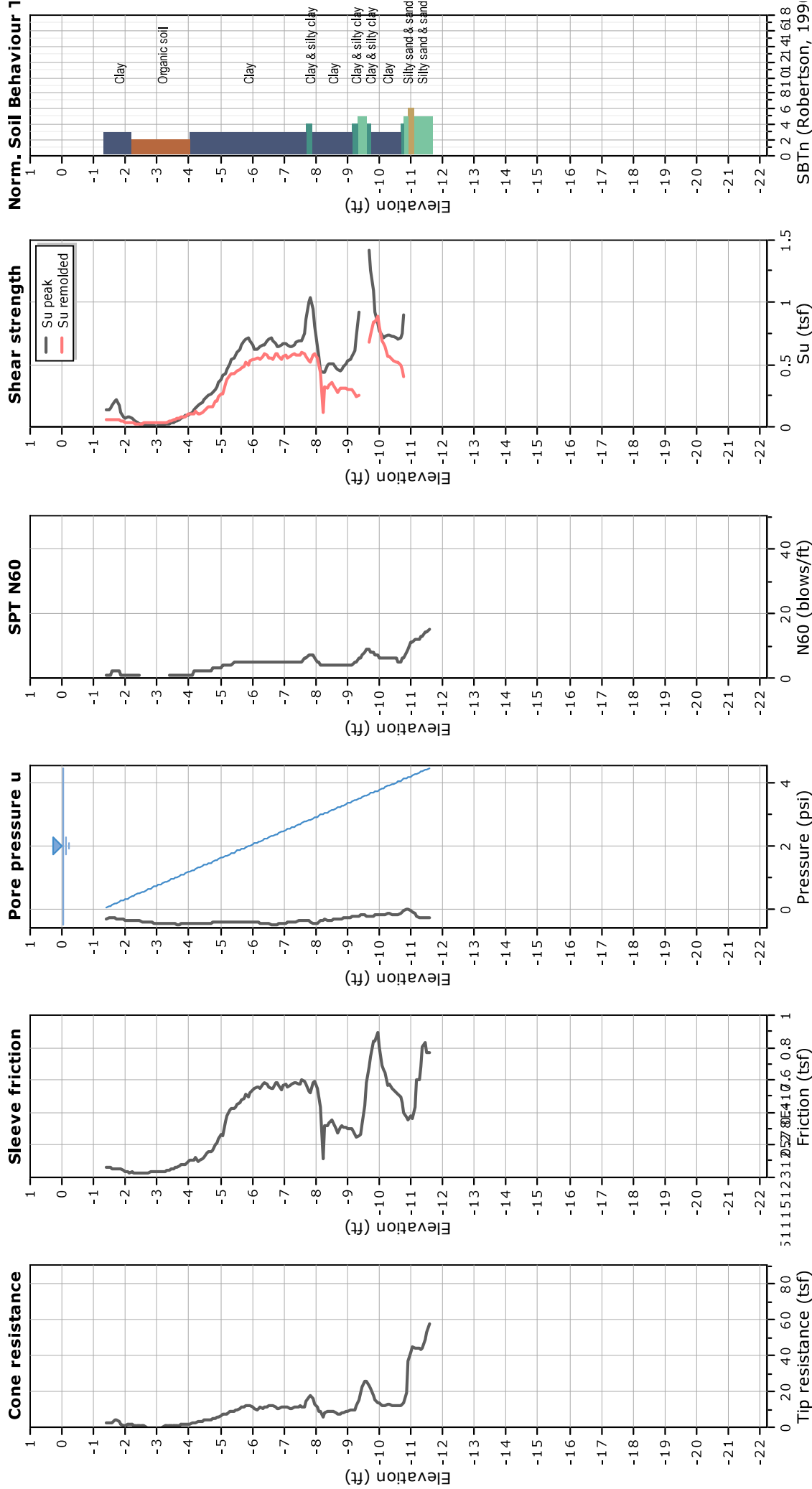
Appendix Figure No. G.1-64



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-02

Total depth: 10.24 ft, Date: 9/1/2020
Surface Elevation: -1.34 ft
Coords: lat 30.331772° lon -90.041027°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



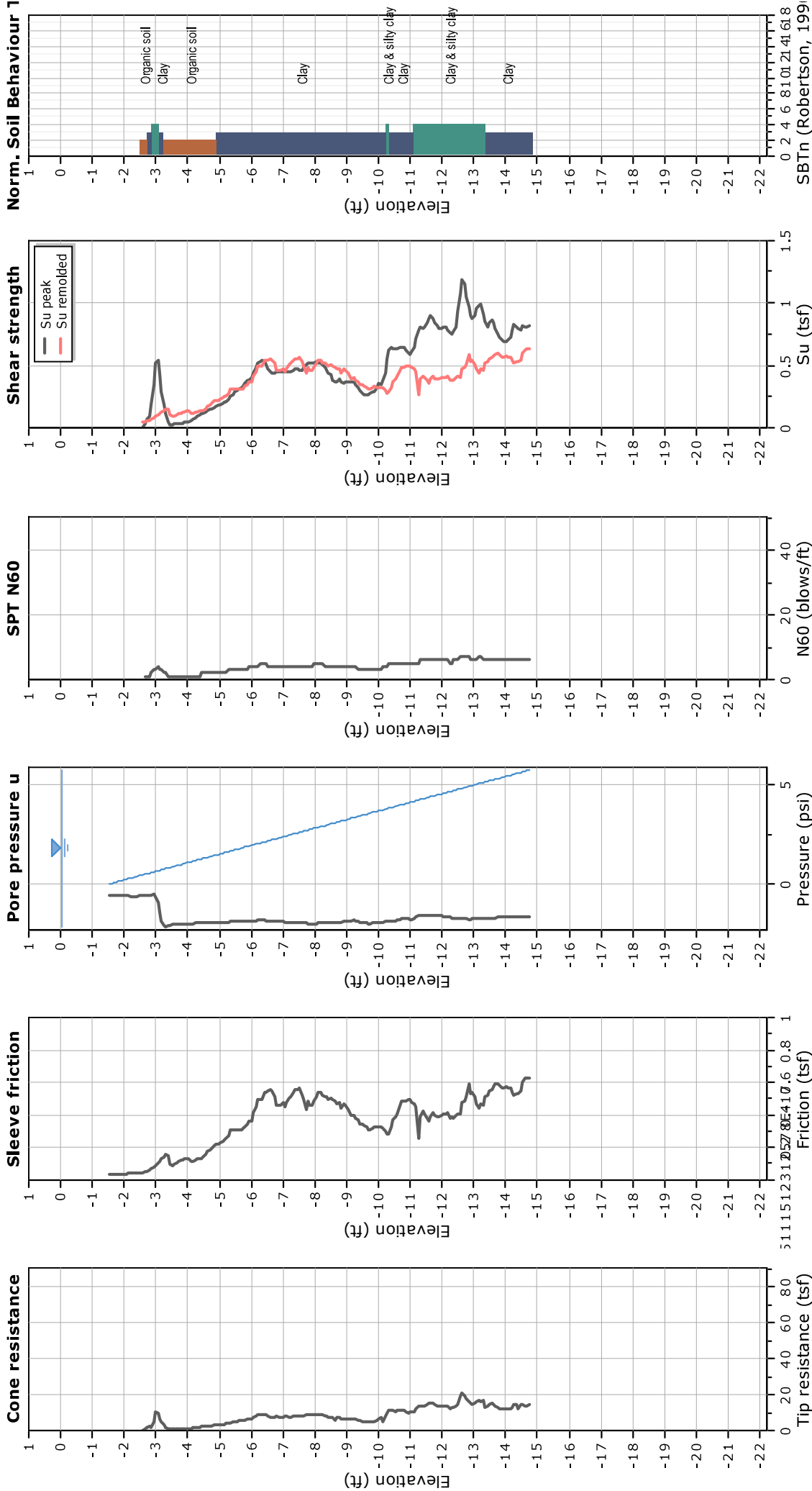
Appendix Figure No. G.1-65



CPT: CPT-03

Total depth: 13.25 ft, Date: 9/1/2020
Surface Elevation: -1.48 ft
Coords: lat 30.328358° lon -90.041344°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker

Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana



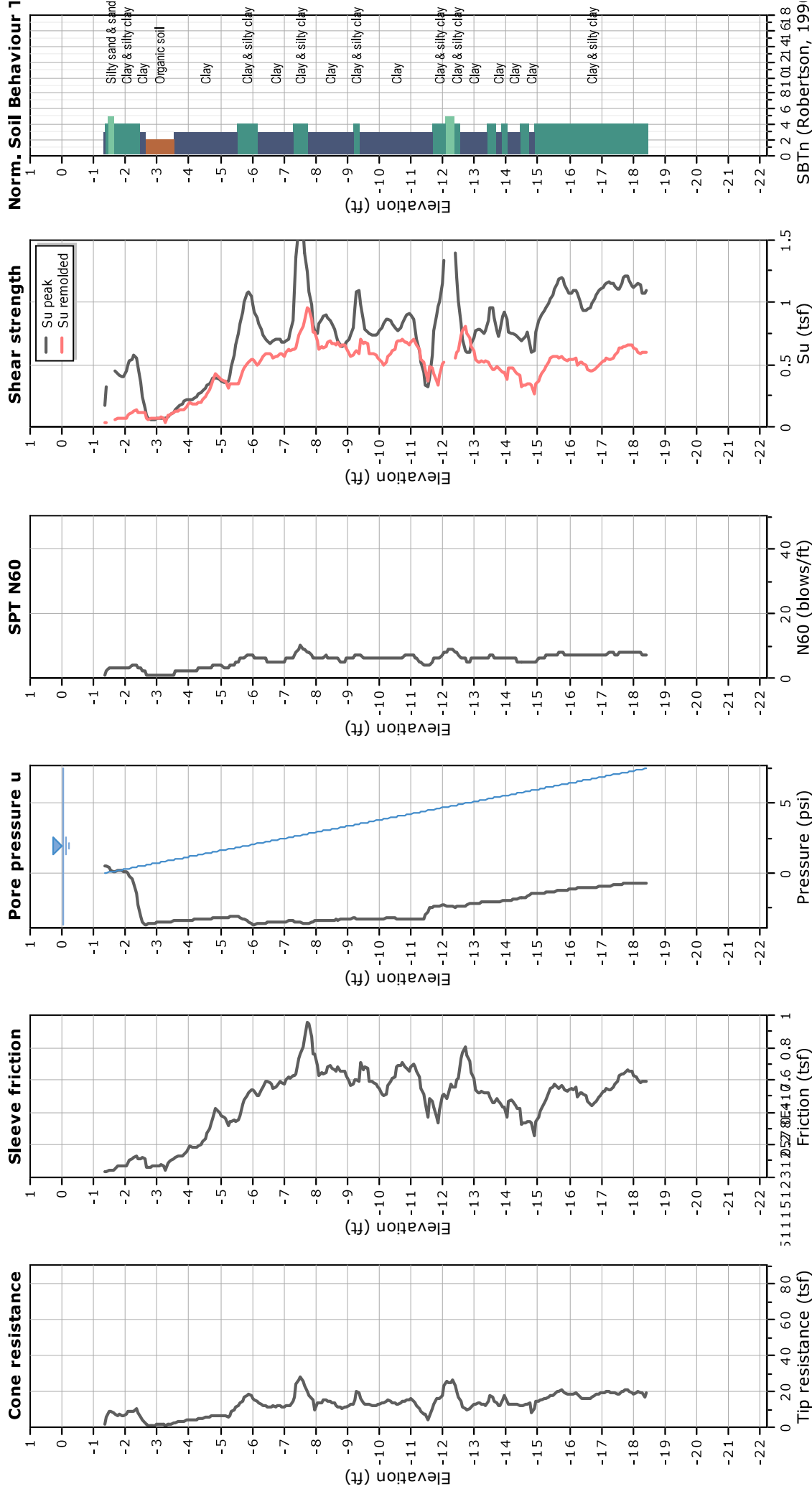
Appendix Figure No. G.1-66



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-04

Total depth: 17.13 ft, Date: 9/1/2020
Surface Elevation: -1.29 ft
Coords: lat 30.3266° lon -90.038286°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker

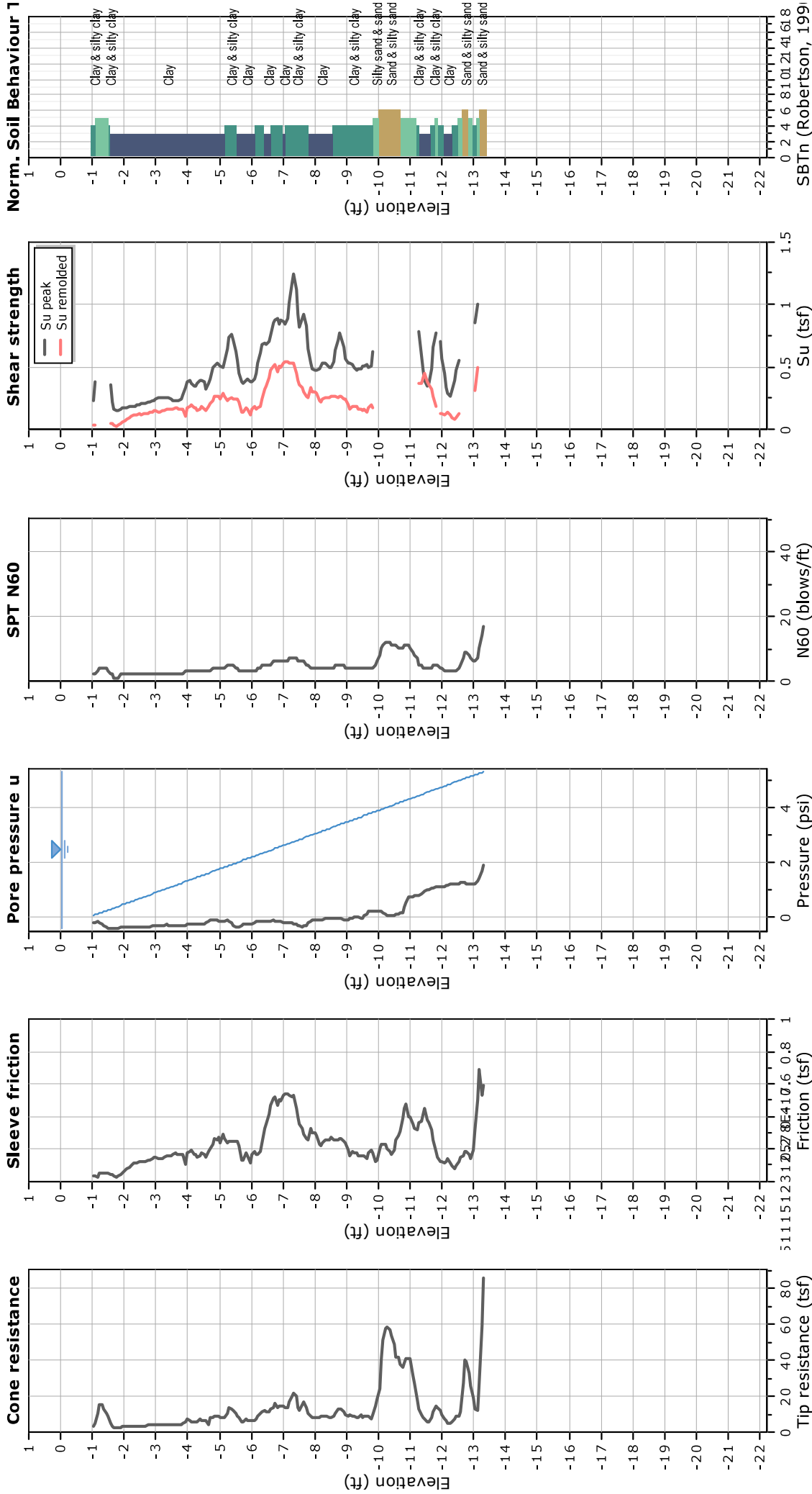


Appendix Figure No. G.1-67



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-05
Total depth: 12.34 ft, Date: 8/19/2020
Surface Elevation: -0.97 ft
Coords: lat 30.324522° lon -90.032183°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker

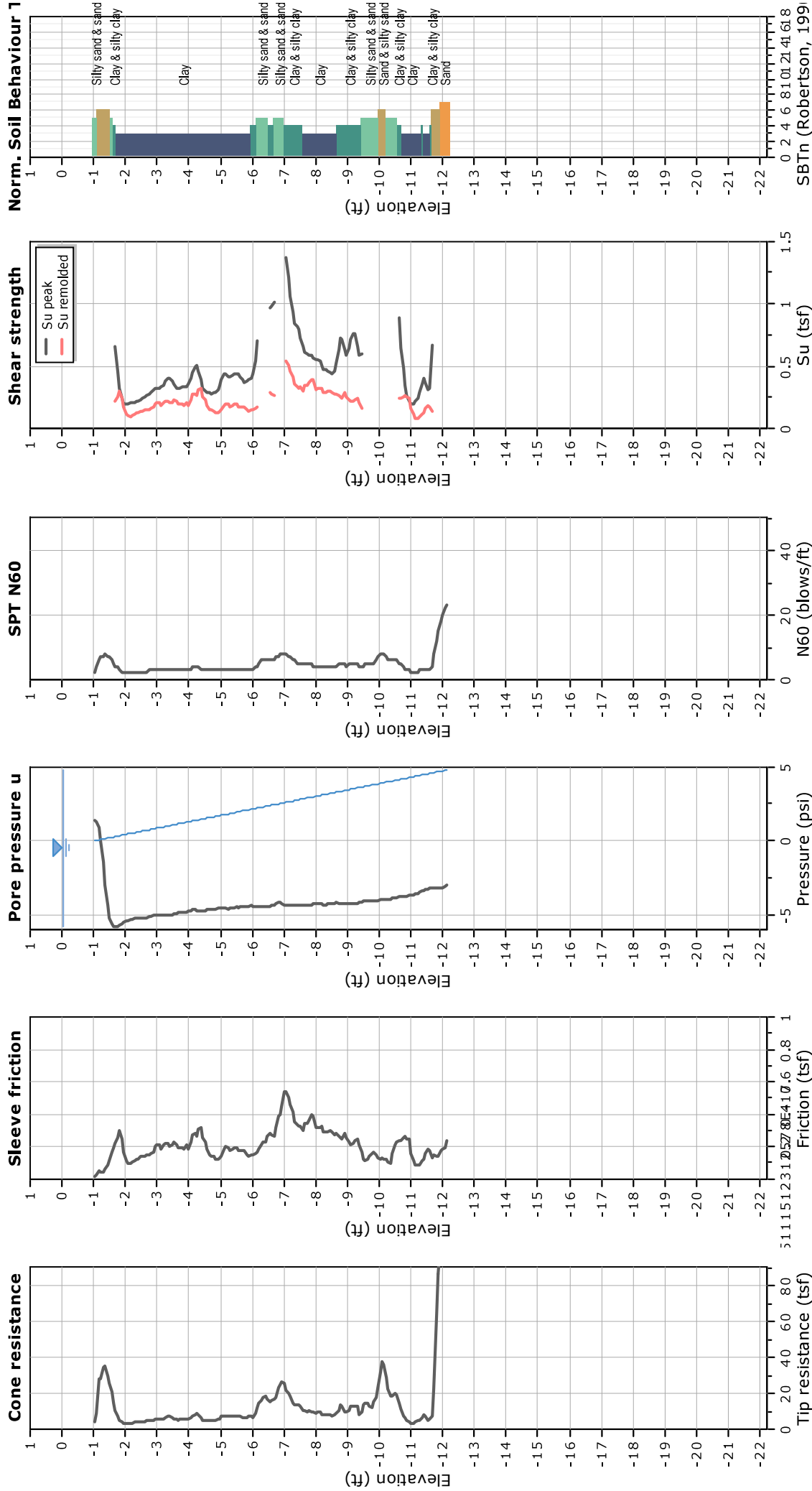


Appendix Figure No. G.1-68



Total depth: 11.15 ft, Date: 8/20/2020
Surface Elevation: -0.97 ft
Coords: lat 30.324522° lon -90.032183°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker

Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana



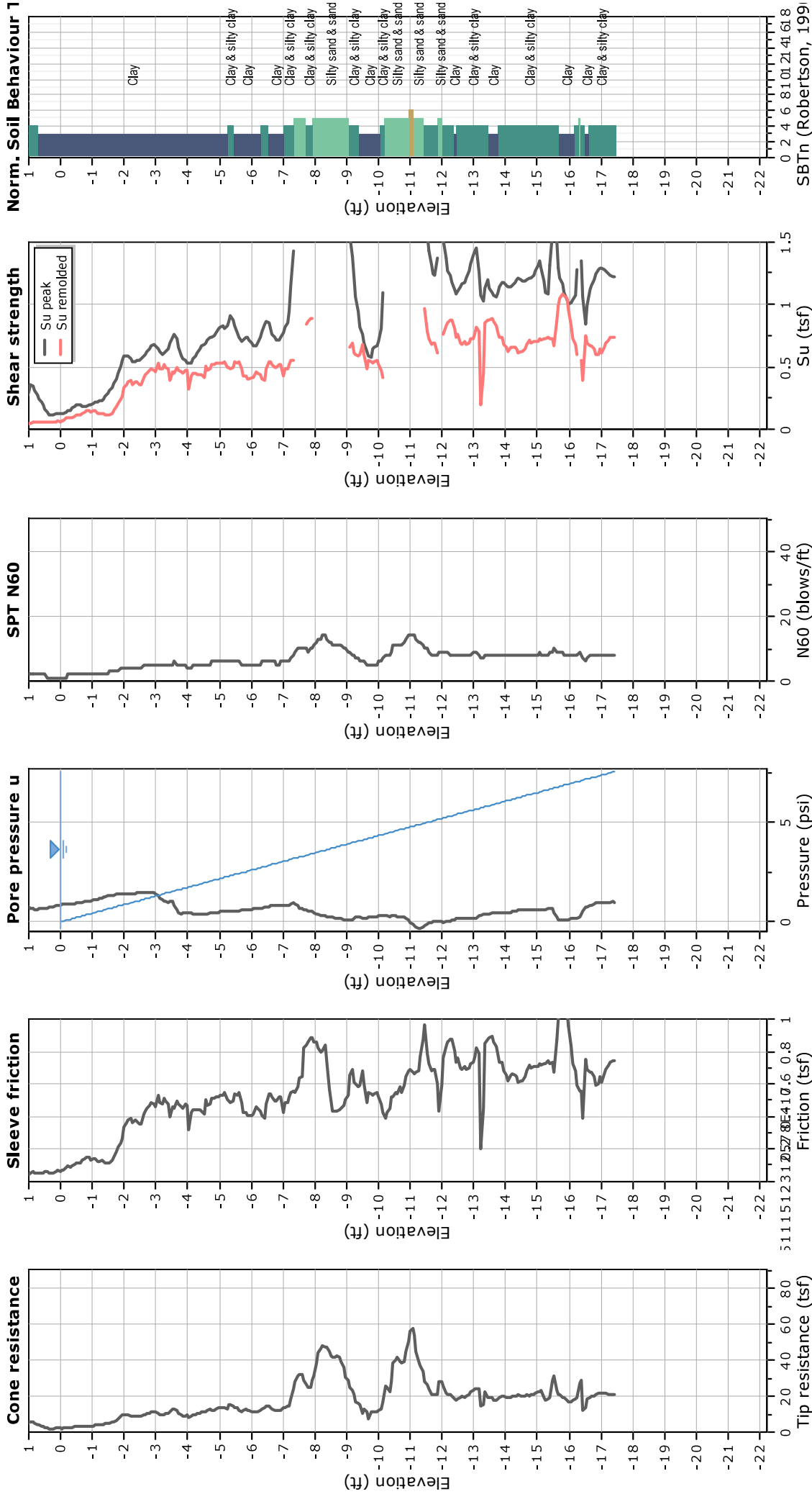
Appendix Figure No. G.1-69



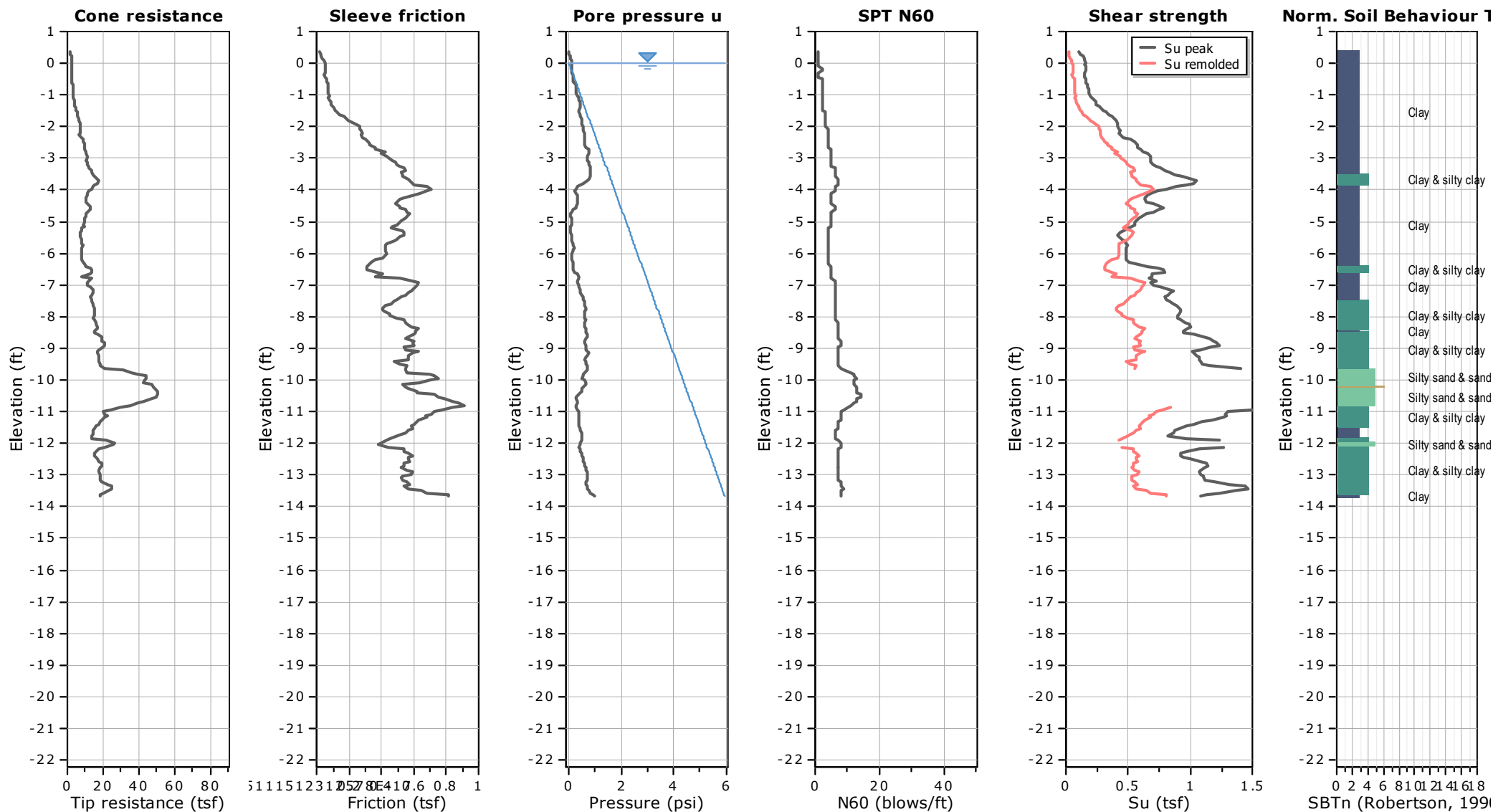
Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-06

Total depth: 18.57 ft, Date: 8/19/2020
Surface Elevation: 1.14 ft
Coords: lat 30.328119° lon -90.031014°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker

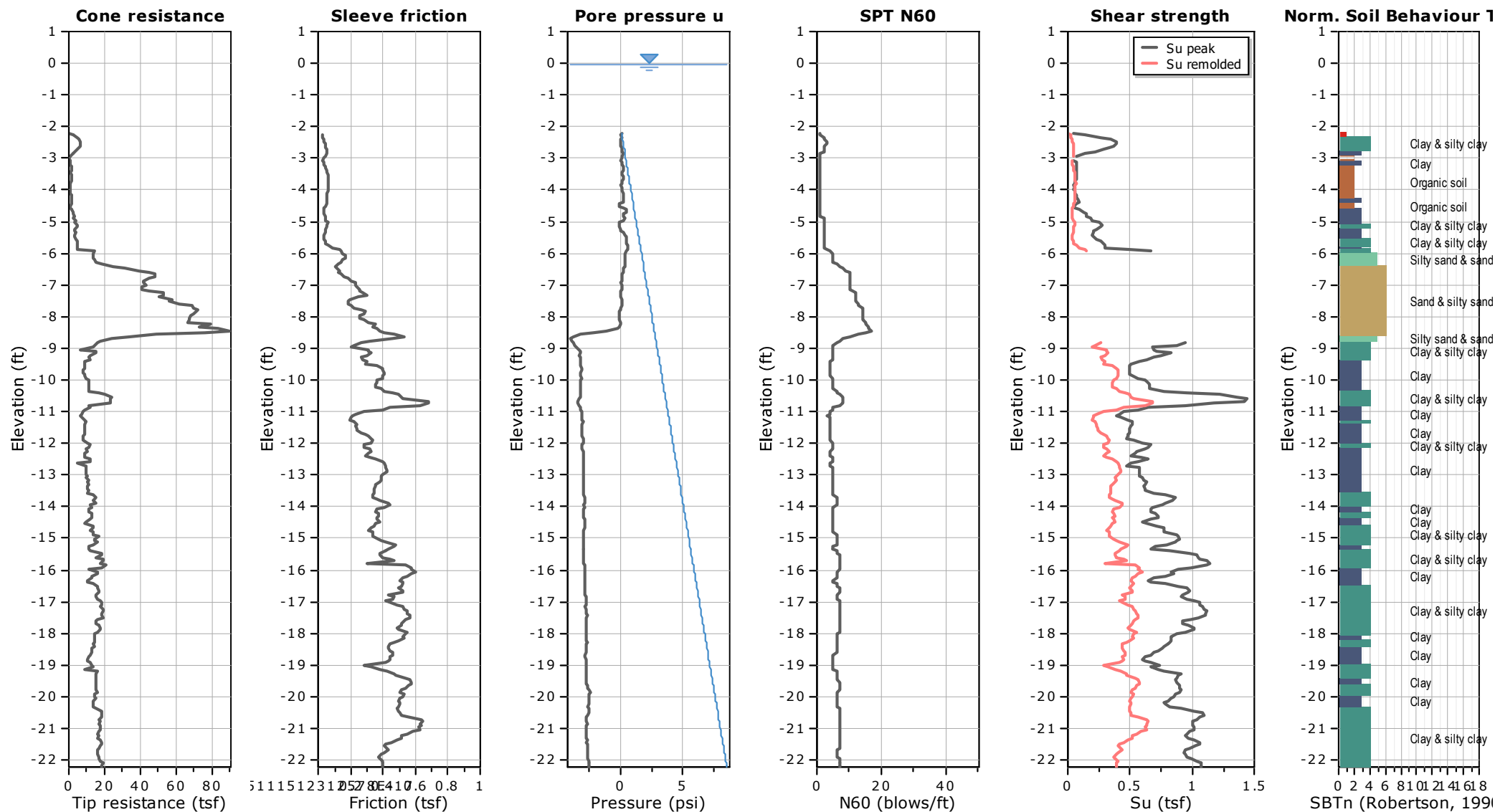


Appendix Figure No. G.1-70



Project: 02541139 - Bayou Cane Marsh Creation

Location: St. Tammany Parish, Louisiana

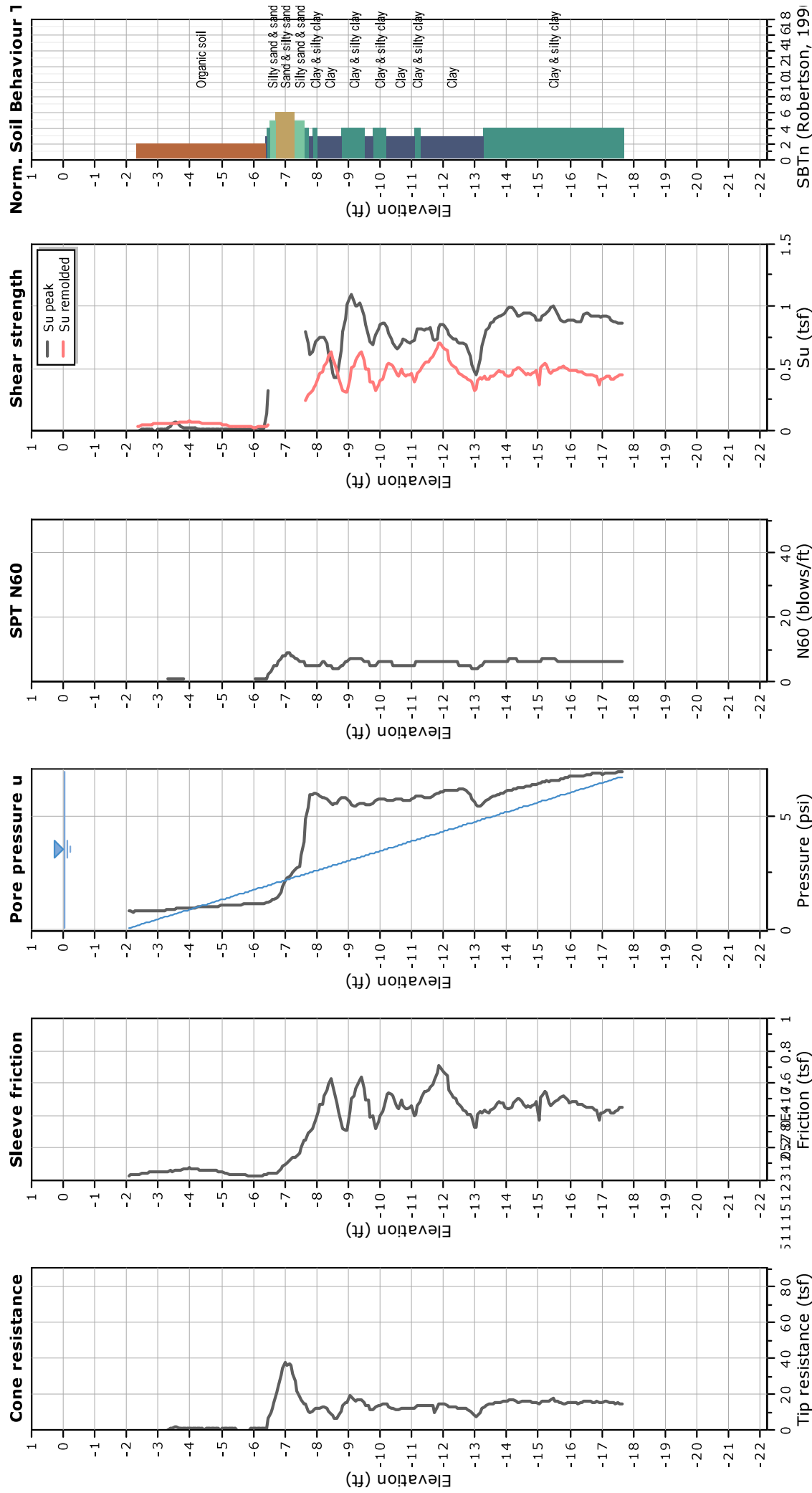




Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-09

Total depth: 15.62 ft, Date: 9/2/2020
Surface Elevation: -2.01 ft
Coords: lat 30.323869° lon -90.021906°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



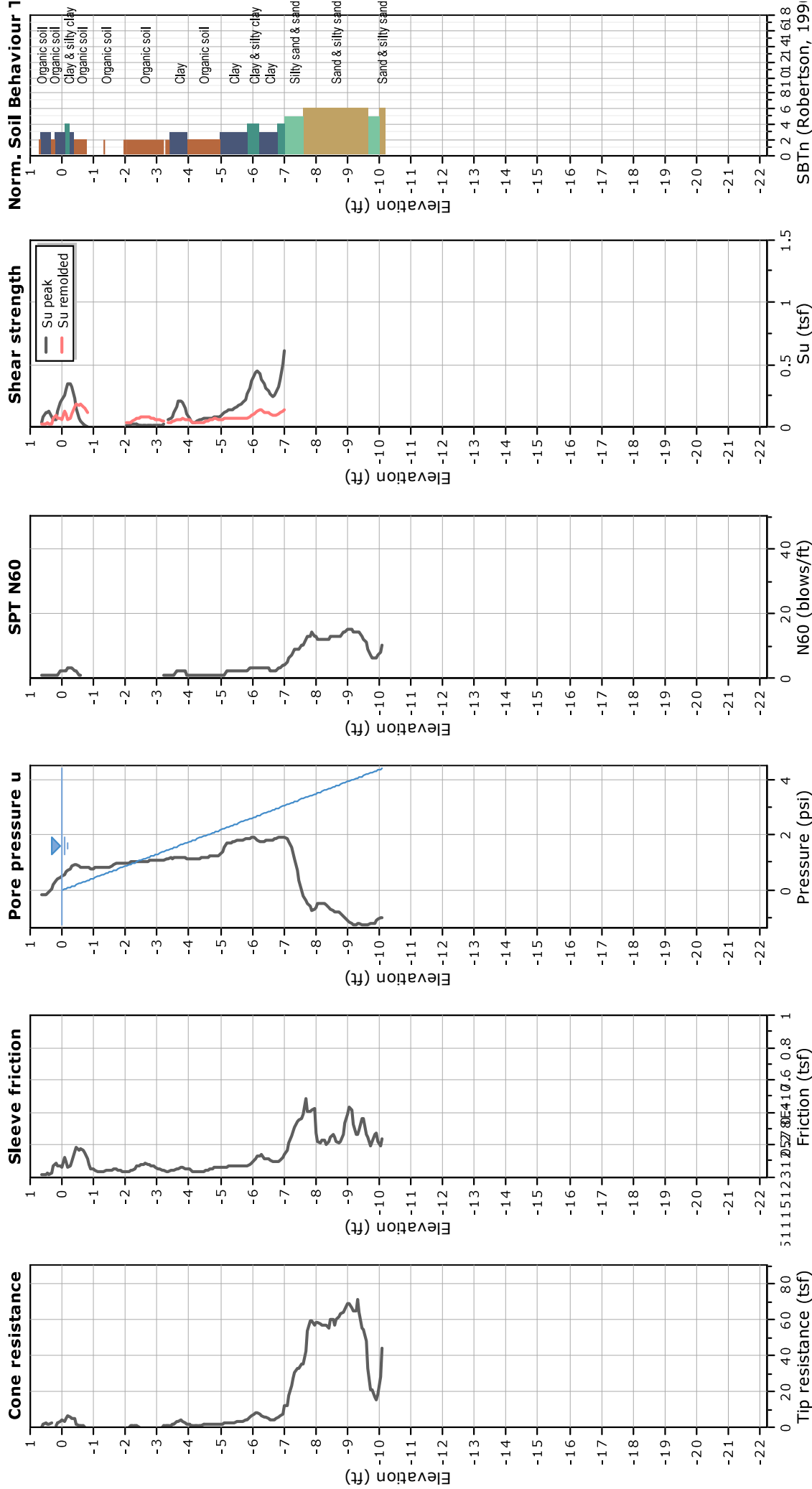
Appendix Figure No. G.1-73



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-10

Total depth: 10.83 ft, Date: 9/2/2020
Surface Elevation: 0.72 ft
Coords: lat 30.321383° lon -90.019933°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



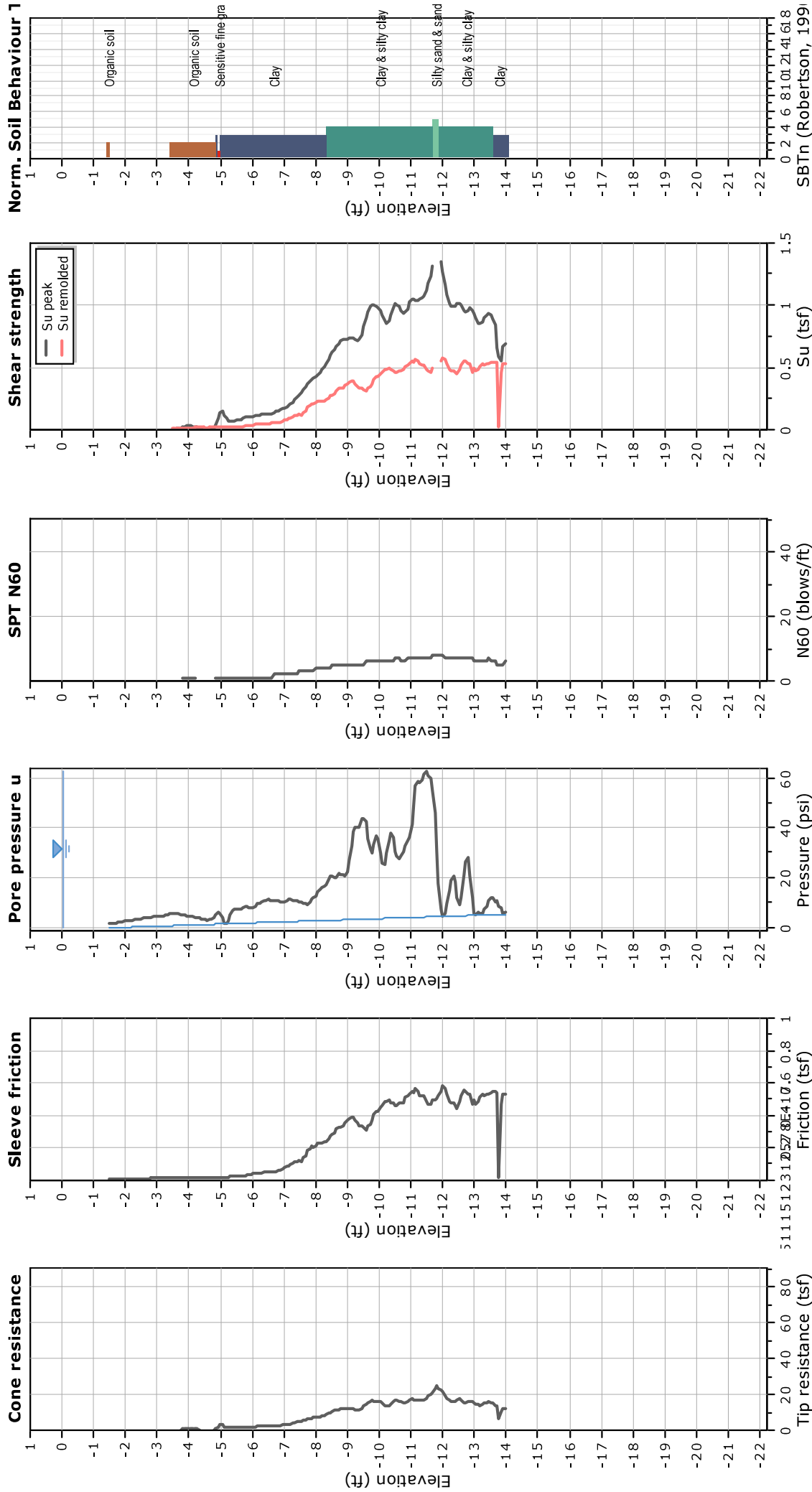
Appendix Figure No. G.1-74



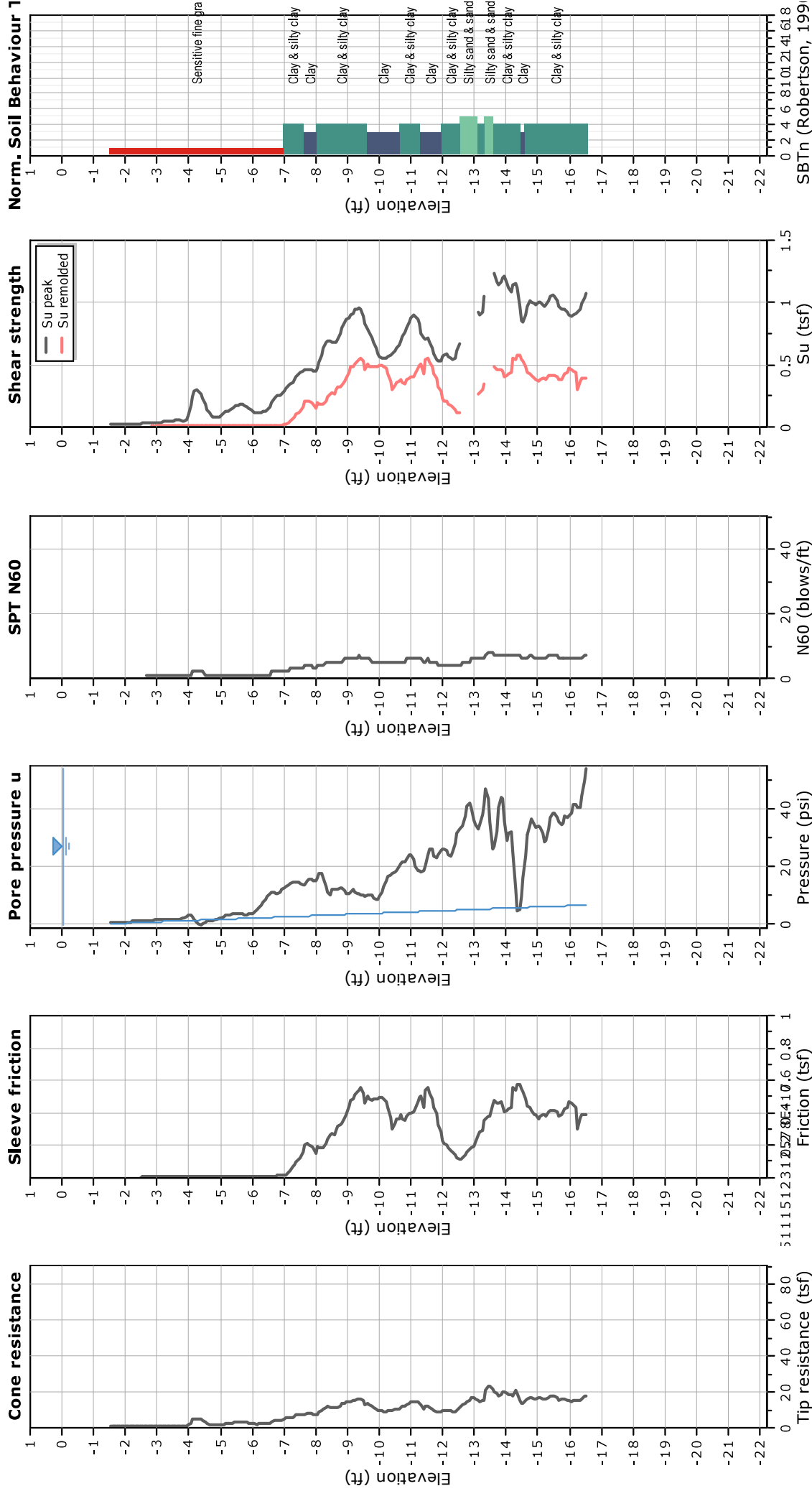
Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-11

Total depth: 12.53 ft, Date: 9/3/2020
Surface Elevation: -1.44 ft
Coords: lat 30.313372° lon -90.000653°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



Appendix Figure No. G.1-75



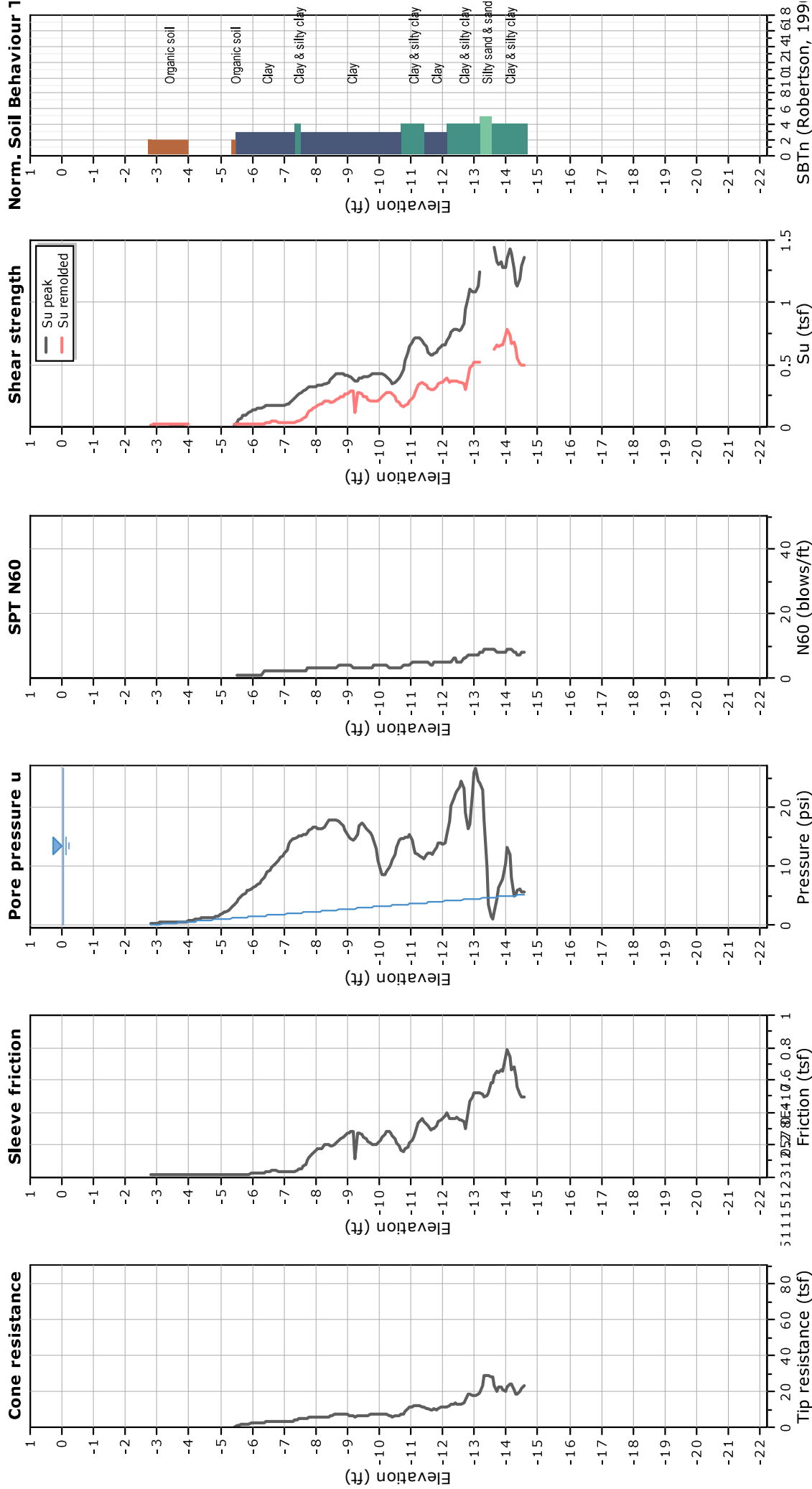
Appendix Figure No. G.1-76



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-13

Total depth: 11.81 ft, Date: 9/3/2020
Surface Elevation: -2.75 ft
Coords: lat 30.312283° lon -89.993433°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



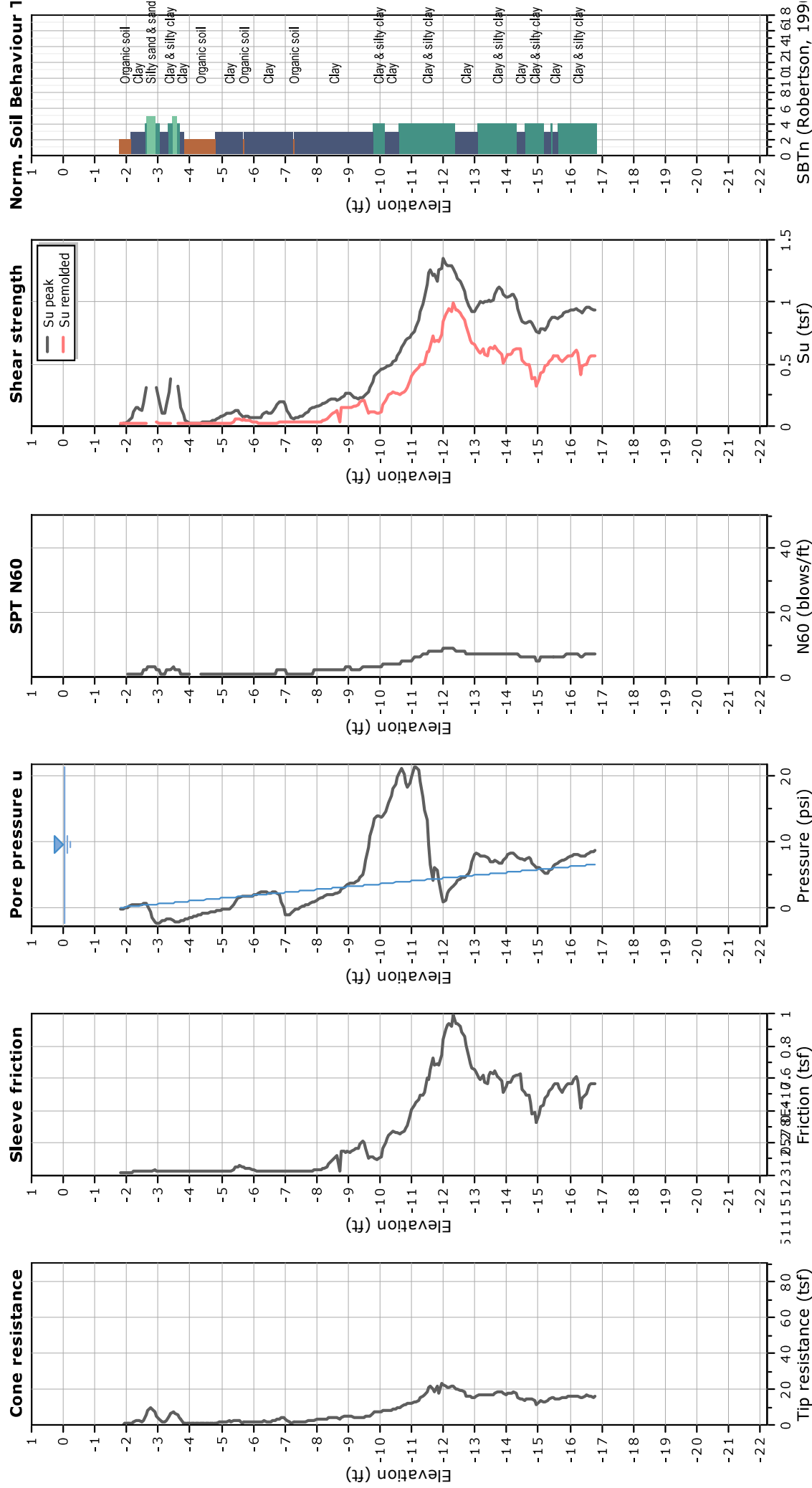
Appendix Figure No. G.1-77



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-14

Total depth: 15.03 ft, Date: 9/3/2020
Surface Elevation: -1.76 ft
Coords: lat 30.307683° lon -89.995378°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



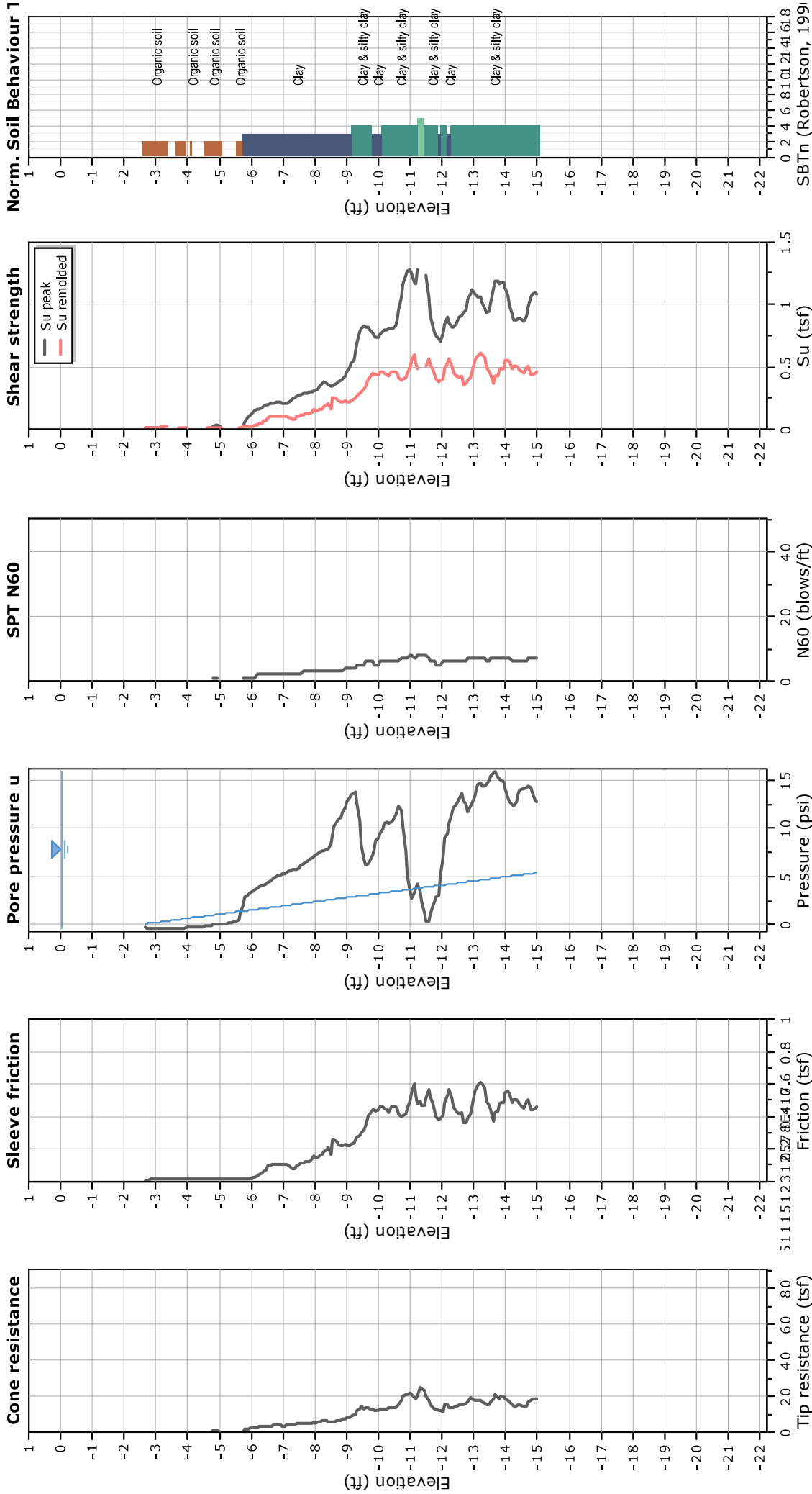
Appendix Figure No. G.1-78



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-15

Total depth: 12.40 ft, Date: 9/3/2020
Surface Elevation: -2.58 ft
Coords: lat 30.305728° lon -89.990425°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



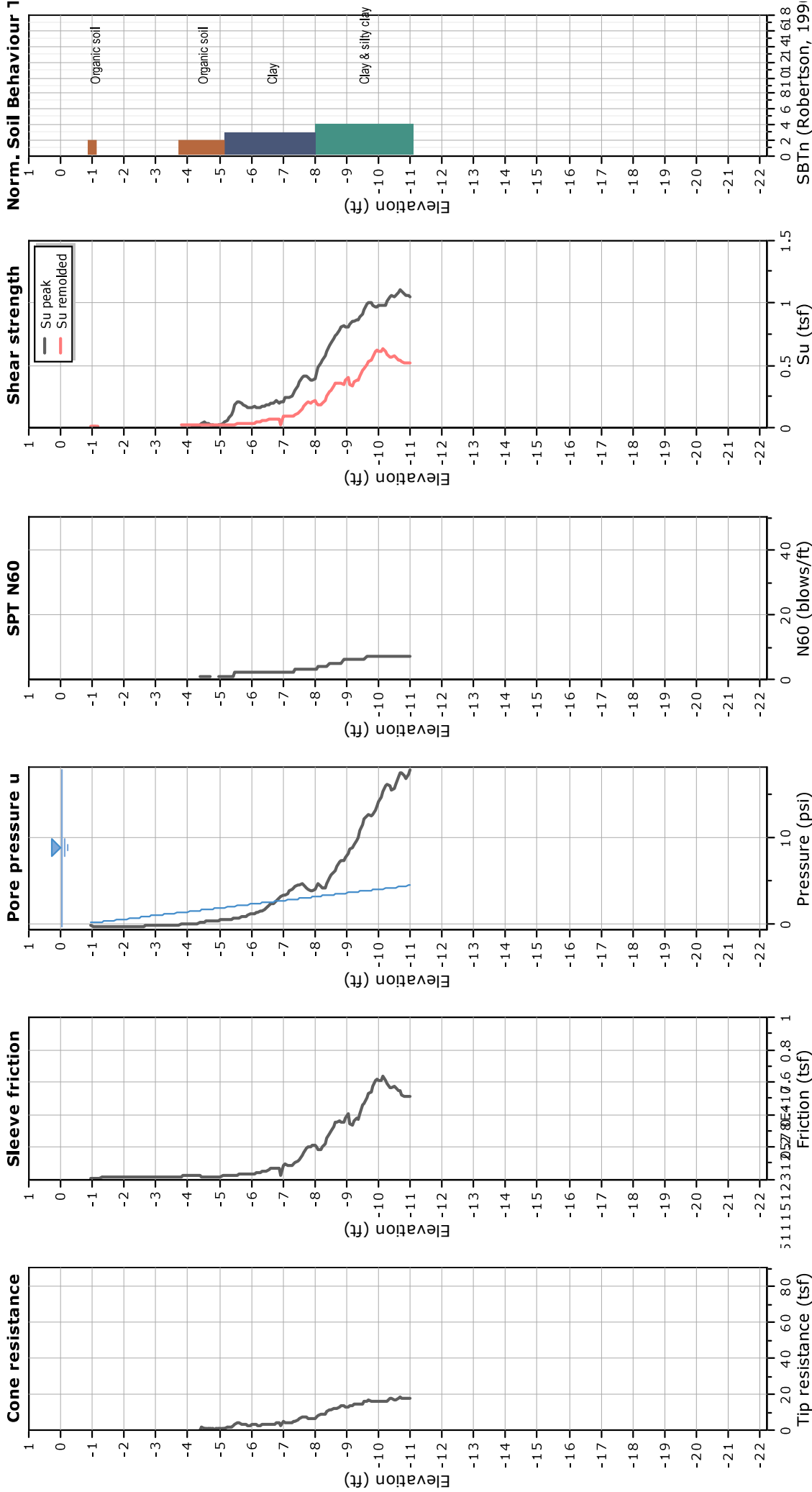
Appendix Figure No. G.1-79



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-16

Total depth: 10.10 ft, Date: 9/3/2020
Surface Elevation: -0.90 ft
Coords: lat 30.301747° lon -89.989286°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



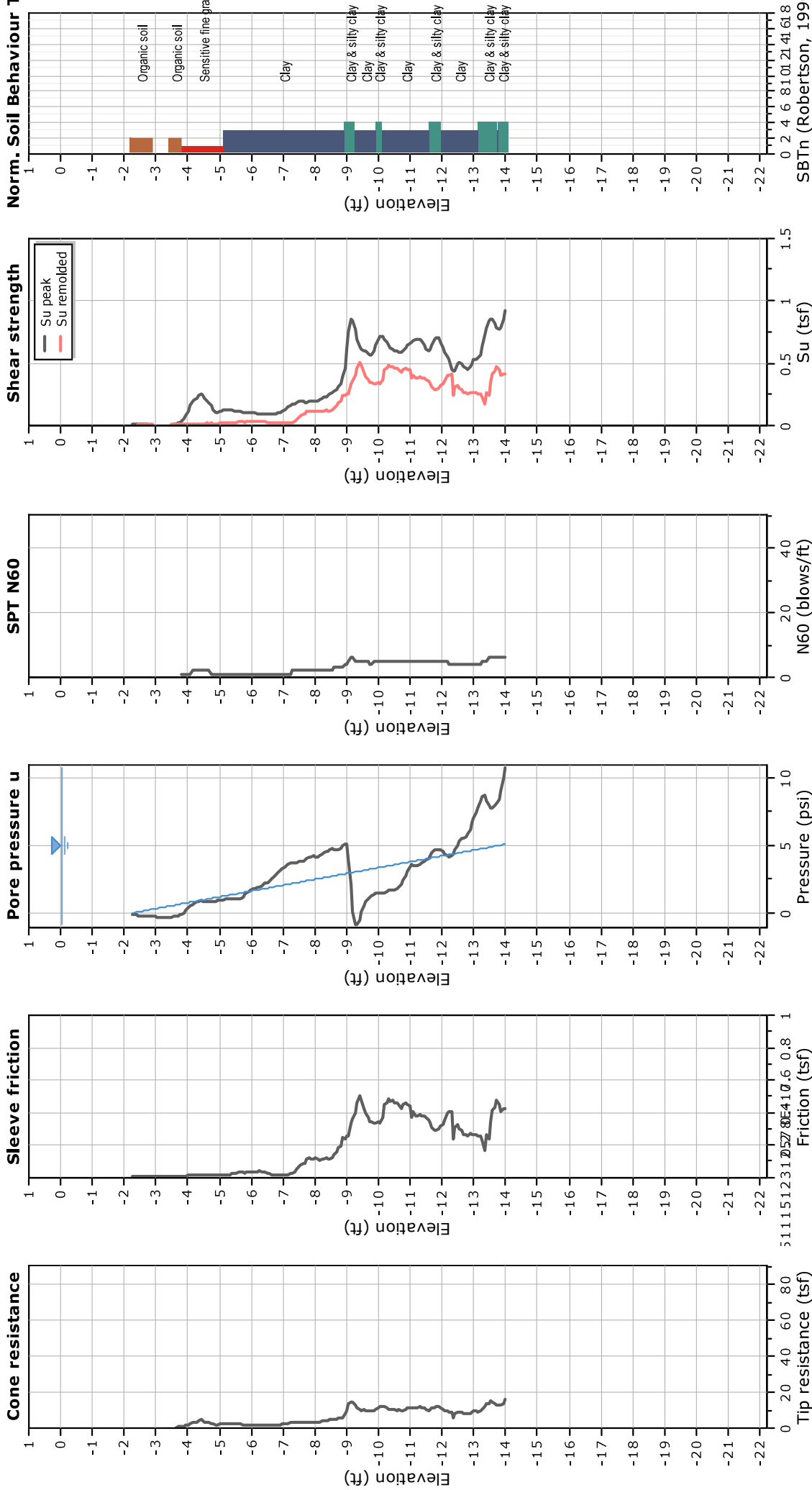
Appendix Figure No. G.1-80



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-17

Total depth: 11.81 ft, Date: 9/3/2020
Surface Elevation: -2.18 ft
Coords: lat 30.298814° lon -89.983239°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



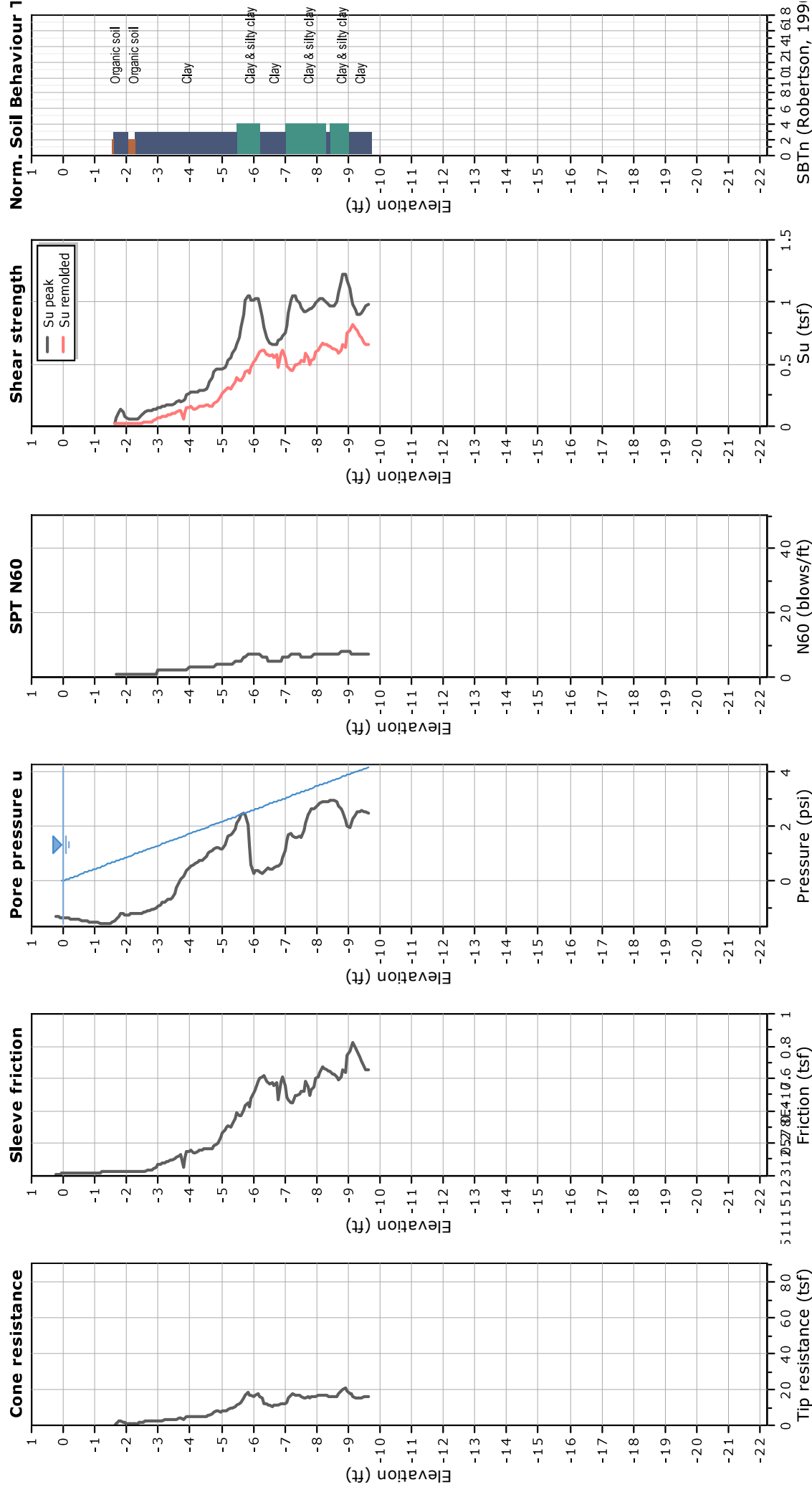
Appendix Figure No. G.1-81



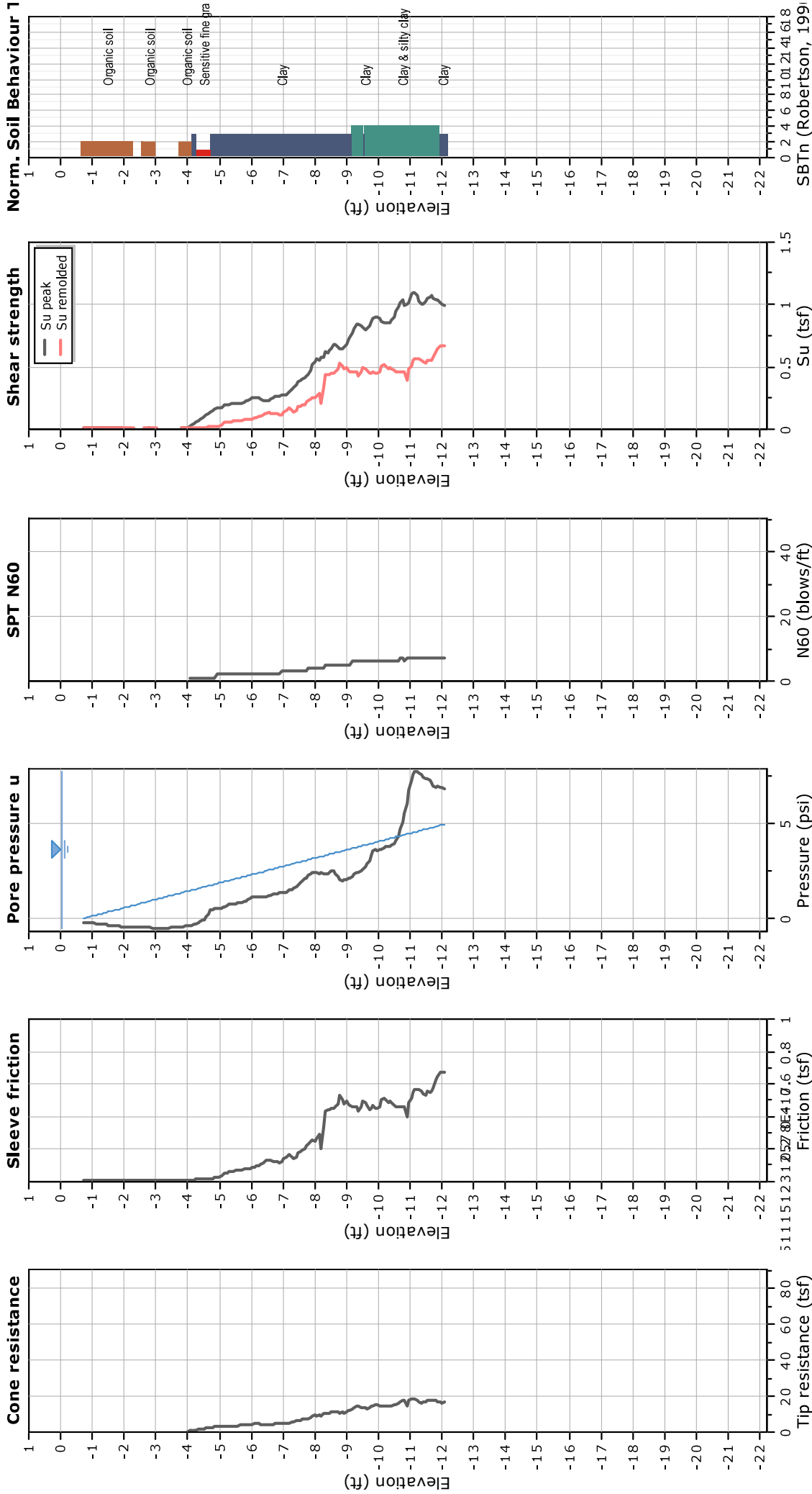
Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-18

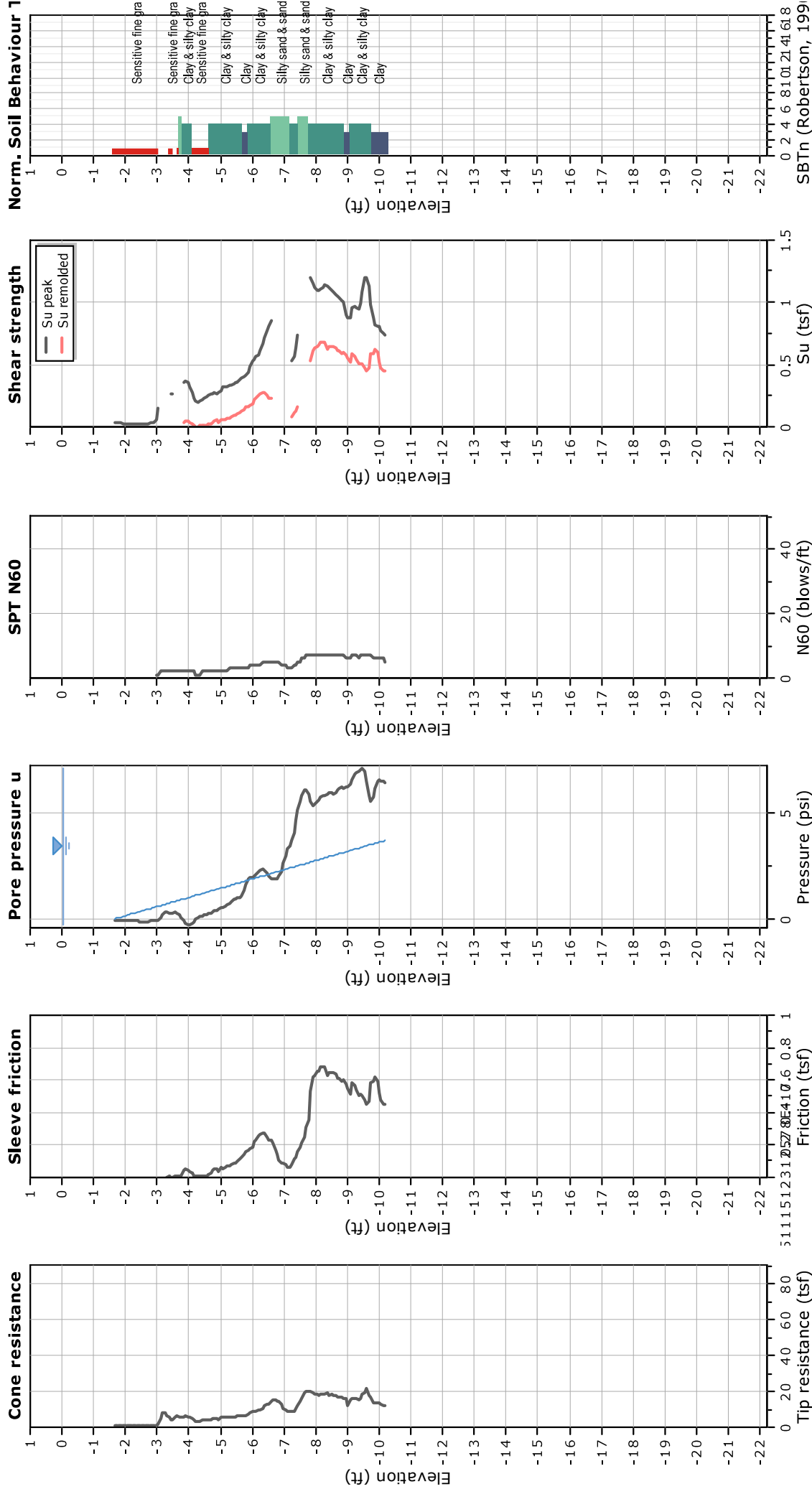
Total depth: 9.91 ft, Date: 9/3/2020
Surface Elevation: 0.29 ft
Coords: lat 30.291939° lon -89.982253°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



Appendix Figure No. G.1-82



Appendix Figure No. G.1-83



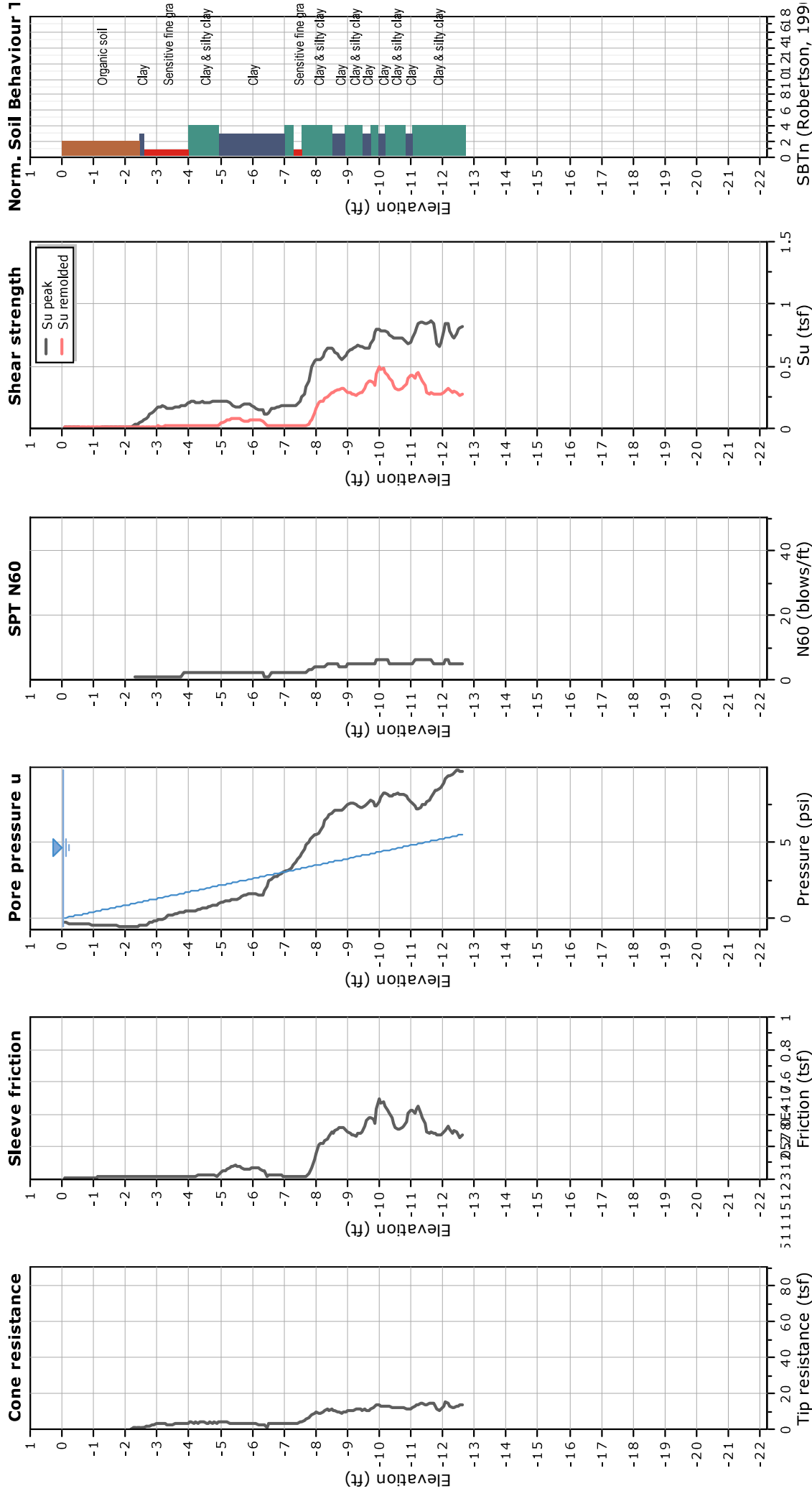
Appendix Figure No. G.1-84



Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-21

Total depth: 12.60 ft, Date: 9/3/2020
Surface Elevation: -0.02 ft
Coords: lat 30.284631° lon -89.975872°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



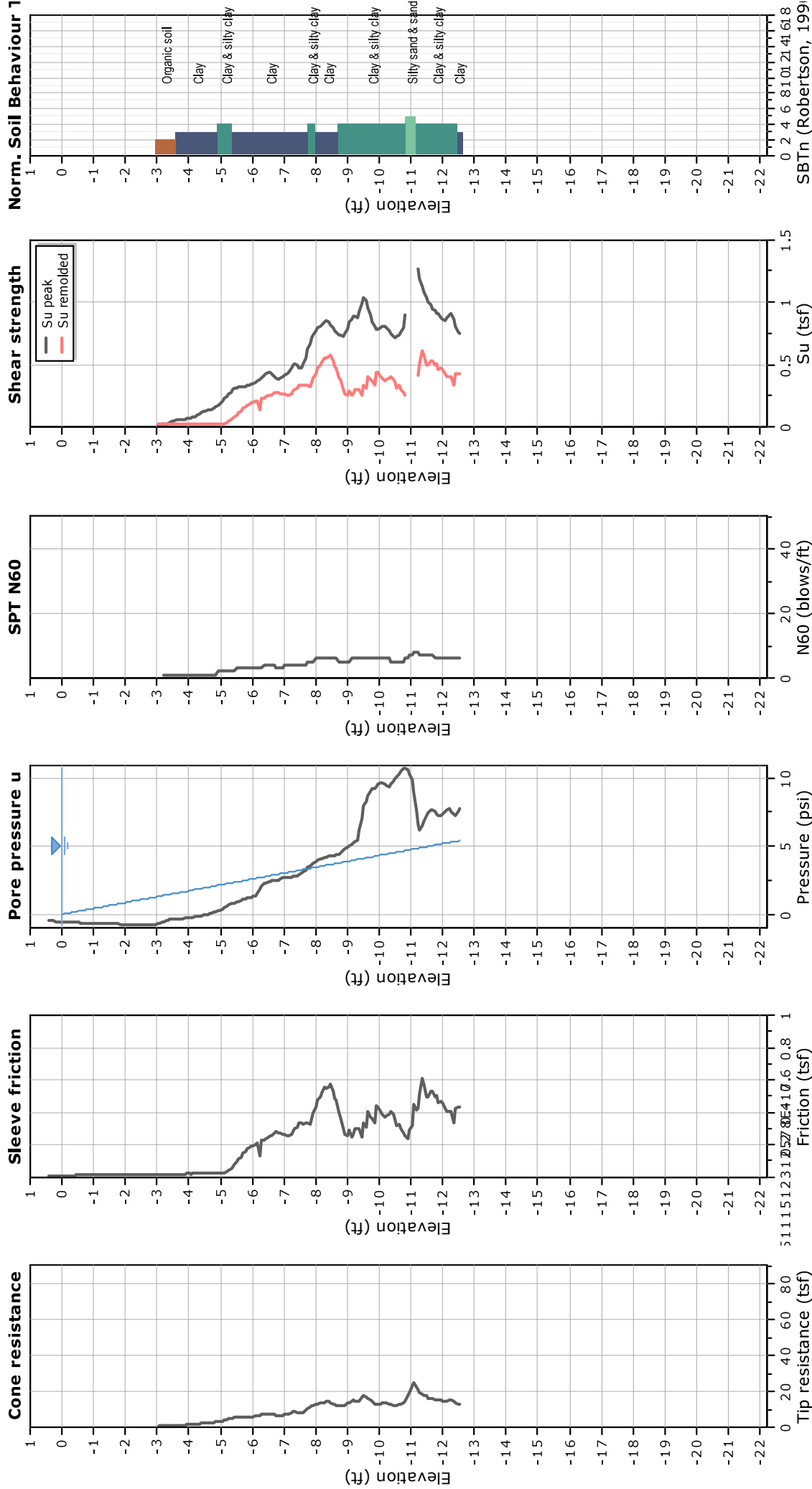
Appendix Figure No. G.1-85



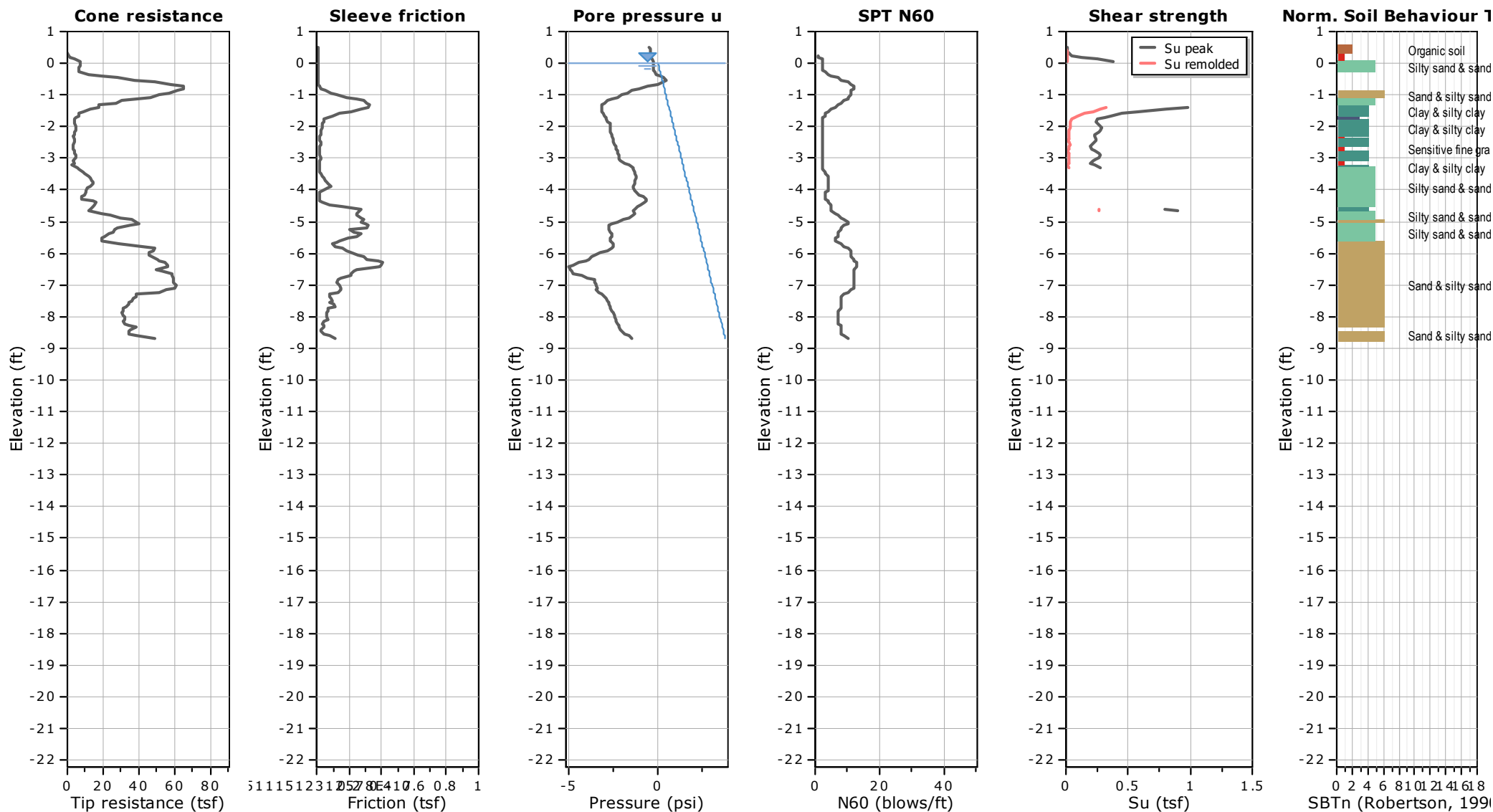
Project: 02541139 - Bayou Cane Marsh Creation
Location: St. Tammany Parish, Louisiana

CPT: CPT-22

Total depth: 12.99 ft, Date: 9/3/2020
Surface Elevation: 0.46 ft
Coords: lat 30.281125° lon -89.976447°
Cone Type: 60° 1.4" Diam. Type 2 Electric Cone Penetrometer
Cone Operator: Charley Baker



Appendix Figure No. G.1-86



KEY TO TERMS AND SYMBOLS USED ON LOGS

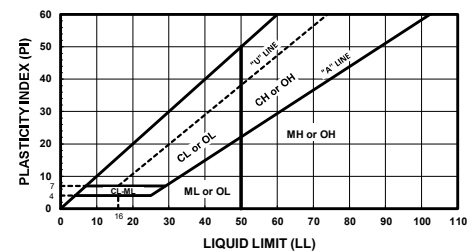
SOIL TYPE					
FAT CLAY	LEAN CLAY	ORGANIC CLAY	SAND	SILT	GRAVEL
SOIL TYPE		MODIFIERS			
TOPSOIL	FILL	CLAYEY	SANDY	SILTY	GRAVELY

SAMPLER TYPE			
NO RECOVERY	AUGER SAMPLE	SHELBY TUBE	SPLIT SPOON
GROUNDWATER DURING DRILLING GROUNDWATER UPON COMPLETION			

UNIFIED SOIL CLASSIFICATION SYSTEM - ASTM D 2487 (1980)					
MAJOR DIVISIONS			LETTER SYMBOL	TYPICAL DESCRIPTIONS	
COARSE-GRAINED SOILS LESS THAN 50% PASSING NO. 200 SIEVE	GRAVEL & GRAVELLY SOILS LESS THAN 50% PASSING NO. 4 SIEVE	CLEAN GRAVEL (LITTLE OR NO FINES)	GW	WELL-GRADED GRAVEL, GRAVEL-SAND MIXTURES WITH LITTLE OR NO FINES	
			GP	POORLY GRADED GRAVEL, GRAVEL-SAND MIXTURES WITH LITTLE OR NO FINES	
		WITH APPRECIABLE FINES	GM	SILTY GRAVEL, GRAVEL-SAND-SILT MIXTURES	
			GC	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES	
	SANDS MORE THAN 50% PASSING NO. 4 SIEVE	CLEAN SANDS (LITTLE OR NO FINES)	SW	WELL-GRADED SAND	
			SP	POORLY-GRADED SANDS	
		WITH APPRECIABLE FINES	SM	SILTY SANDS	
			SC	CLAYEY SANDS	
FINE-GRAINED SOILS MORE THAN 50% PASSING NO. 200 SIEVE	SILTS AND CLAYS LIQUID LIMIT < 50		ML	INORGANIC SILTS & VERY FINE SANDS, CLAYEY SILT W/ LOW PLASTICITY INDEX	
			CL	INORGANIC LEAN CLAYS GRAVELLY, SANDY, OR SILTY LEAN CLAYS	
			OL	ORGANIC SILTS & ORGANIC SILTY CLAYS W/LOW PLASTICITY INDEX	
	SILTS AND CLAYS LIQUID LIMIT ≥ 50		MH	INORGANIC SILTS W/ HIGH PLASTICITY INDEX, ELASTIC SILTS	
			CH	INORGANIC FAT CLAYS GRAVELLY, SANDY, OR SILTY FAT CLAYS	
			OH	ORGANIC CLAYS OF MED TO HIGH PLASTICITY, ORGANIC SILTS	
HIGHLY ORGANIC SOIL			PT	PEAT AND OTHER HIGHLY ORGANIC SOILS	
UNCLASSIFIED FILL MATERIALS			ARTIFICIALLY DEPOSITED AND OTHER UNCLASSIFIED SOILS AND MAN-MADE SOIL MIXTURES		

CONSISTENCY - COHESIVE SOILS	
CONSISTENCY	SHEAR STRENGTH IN TONS/FT ²
VERY SOFT	0 TO 0.125
SOFT	0.125 TO 0.25
FIRM	0.25 TO .50
STIFF	0.50 TO 1.00
VERY STIFF	1.00 TO 2.00
HARD	> 2.00 OR 2.00+

RELATIVE DENSITY - GRANULAR SOILS	
DENSITY	N-VALUE (BLOWS/FT)
VERY LOOSE	0-4
LOOSE	4-9
MEDIUM DENSE	10-29
DENSE	30-49
VERY DENSE	> 50 OR 50+



ABBREVIATIONS

HP - HAND PENETROMETER

UC - UNCONFINED COMPRESSION TEST

TV - MINIATURE TORVANE

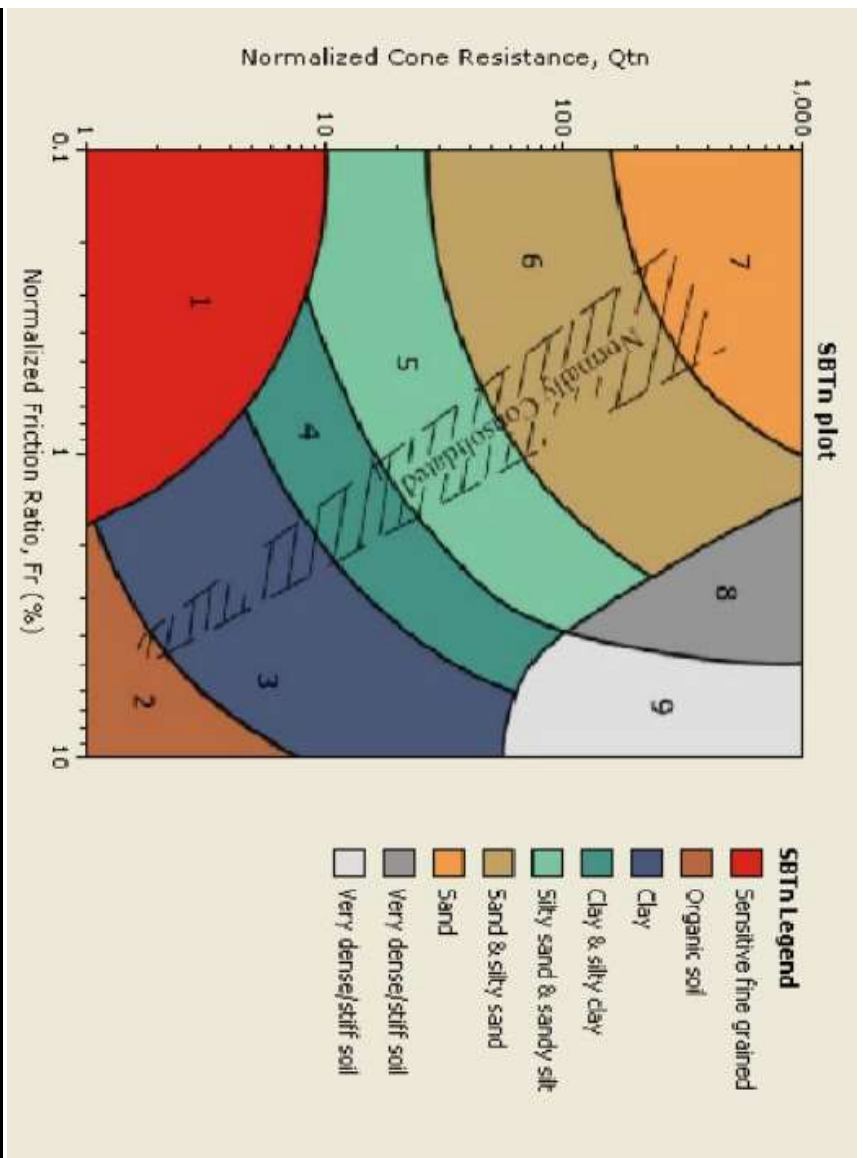
UU - UNCONSOLIDATED UNDRAINED TRIAXIAL

NOTE: BORING LOGS INDICATE SHEAR STRENGTH AS OBTAINED BY ABOVE TESTS

CLASSIFICATION OF GRANULAR SOILS

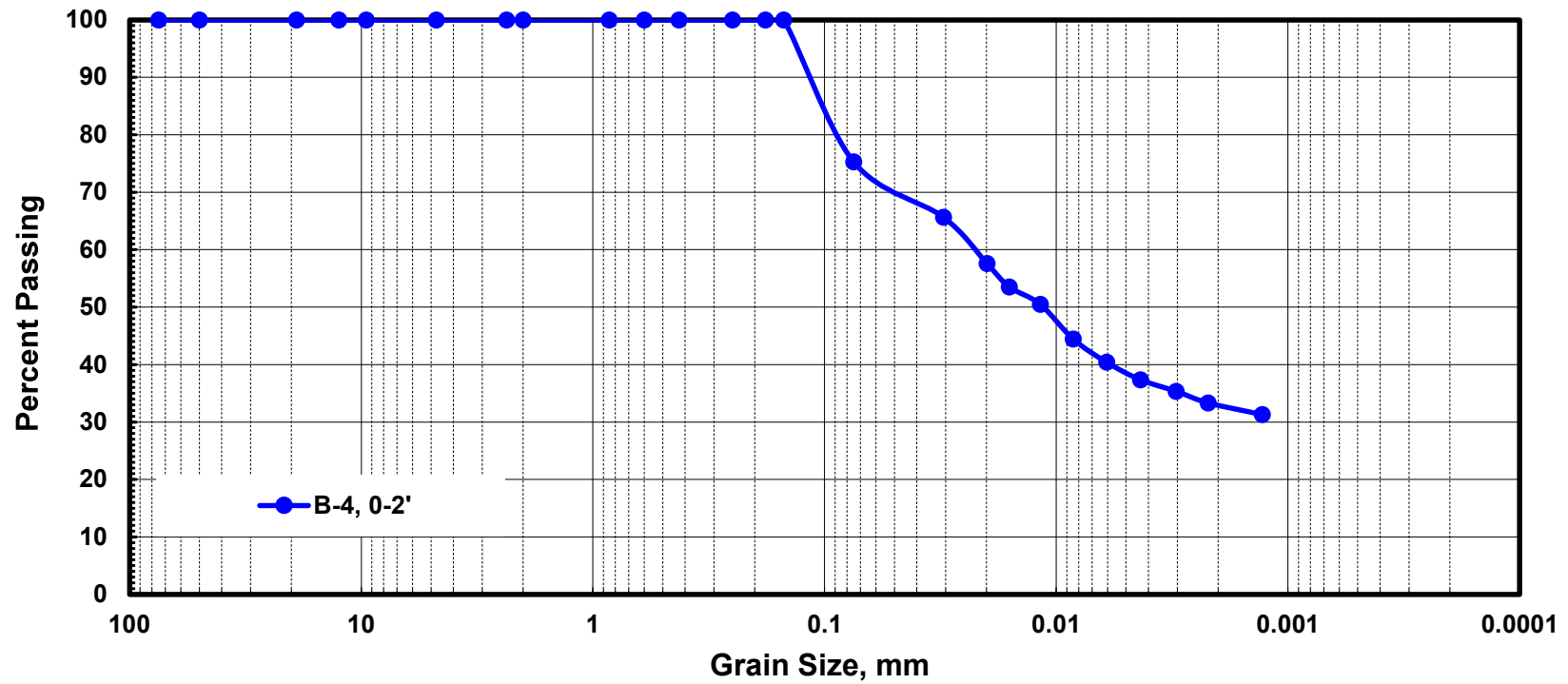
U.S. STANDARD SIEVE SIZE(S)															
12"		3"		3/4"		4		10		40		200			
BOULDERS	COBBLES	GRAVEL				SAND				SILT OR CLAY				CLAY	
		COARSE		FINE		COARSE		MEDIUM							FINE
300		75		19		4.75		2.0		0.42		0.075		0.005	
GRAIN SIZE IN MM															

CPT: NORMALIZED SOIL BEHAVIOUR TYPE LEGEND



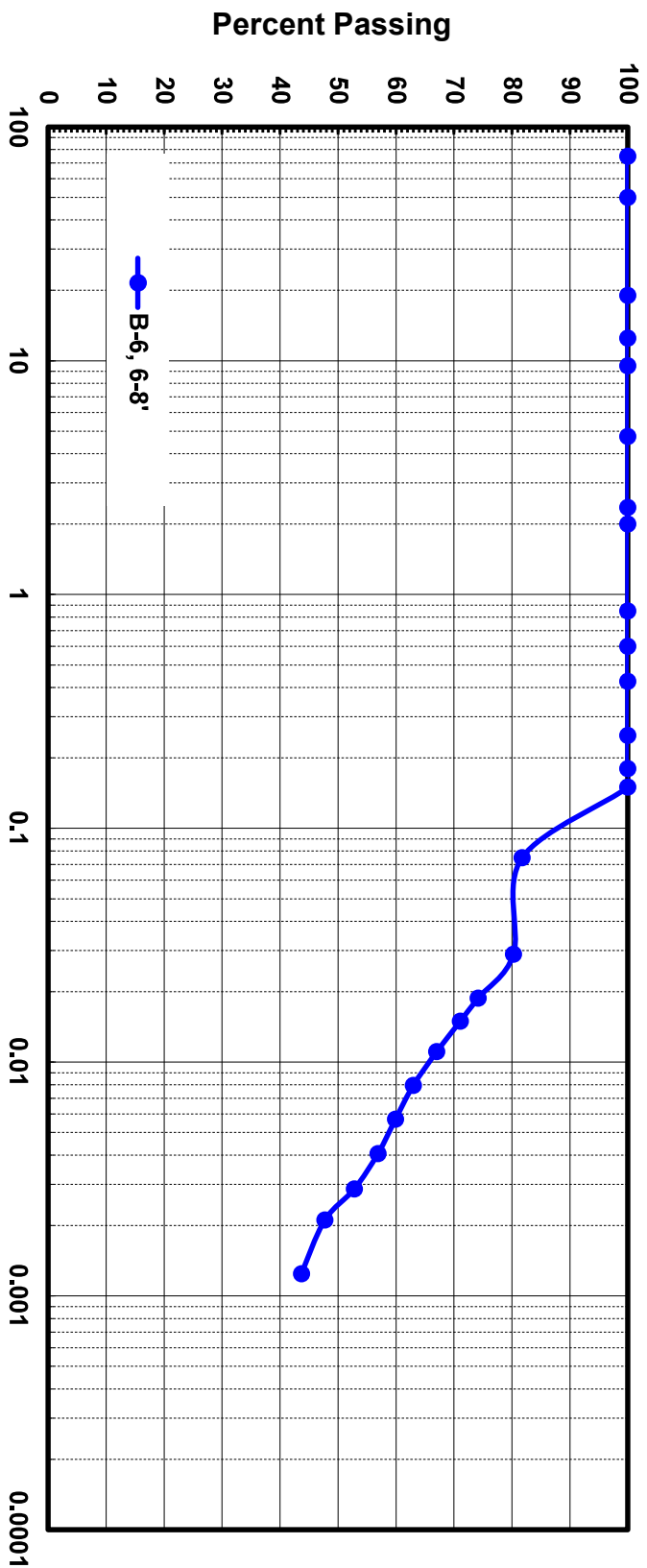
APPENDIX C

HYDROMETER TEST RESULTS
SPECIFIC GRAVITY TEST RESULTS
ORGANIC CONTENT RESULTS



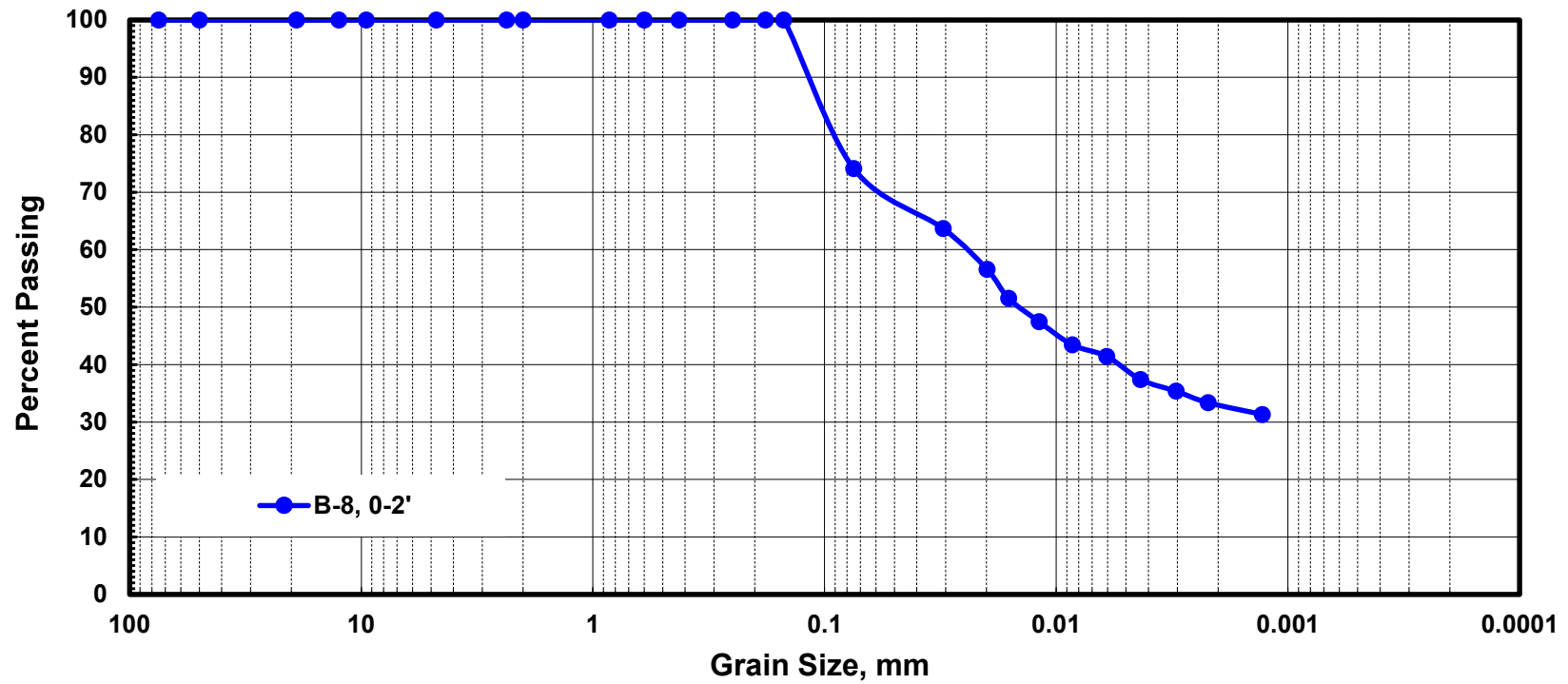
US STANDARD SIEVE SIZE									
6"	3"	3/4"	#4	#10	#40	#200			
BOULDERS	COBBLES	GRAVEL		SAND			SILT OR CLAY	CLAY	
		COARSE	FINE	COARSE	MEDIUM	FINE			
152	75	19	4.75	2.0	0.425	0.075			0.002

GRADATION INFORMATION		PARTICLE SIZE IN MM						%Gravel	%Sand	%Fines	%Clay
		D ₁₀ mm	D ₃₀ mm	D ₅₀ mm	D ₆₀ mm	C _u (D ₆₀ /D ₁₀)	C _c (D ₃₀ ² /(D ₆₀ *D ₁₀))	Ret. #4	<#4 & >#200	Pass #200	< 0.002 mm
CPRA PO 181		NA	NA	0.0114	0.0227	NA	NA	0.00	24.73	75.27	32.86
B-4, 0-2'											
FAT CLAY WITH SAND, GRAY											



US STANDARD SIEVE SIZE									
6"	3"	3/4"	#4	#10	#40	#200			
BOULDERS	COBBLES	GRAVEL		SAND			SILT OR CLAY		
		COARSE	FINE	COARSE	MEDIUM	FINE			
152	75	19	4.75	2.0	0.425	0.075	0.002		

GRADATION INFORMATION		D ₁₀ mm	D ₃₀ mm	D ₅₀ mm	D ₆₀ mm	C _u (D ₆₀ /D ₁₀)	C _c D ₅₀ ² /(D ₆₀ ·D ₁₀)	%Gravel Ret. #4	%Sand <#4 & >#200	%Fines Pass #200	%Clay < 0.002 mm
CPRA PO 181		NA	NA	0.0024	0.0057	NA	NA	0.00	18.24	81.76	47.21
B-6, 6-8'											
FAT CLAY WITH SAND, GRAY											

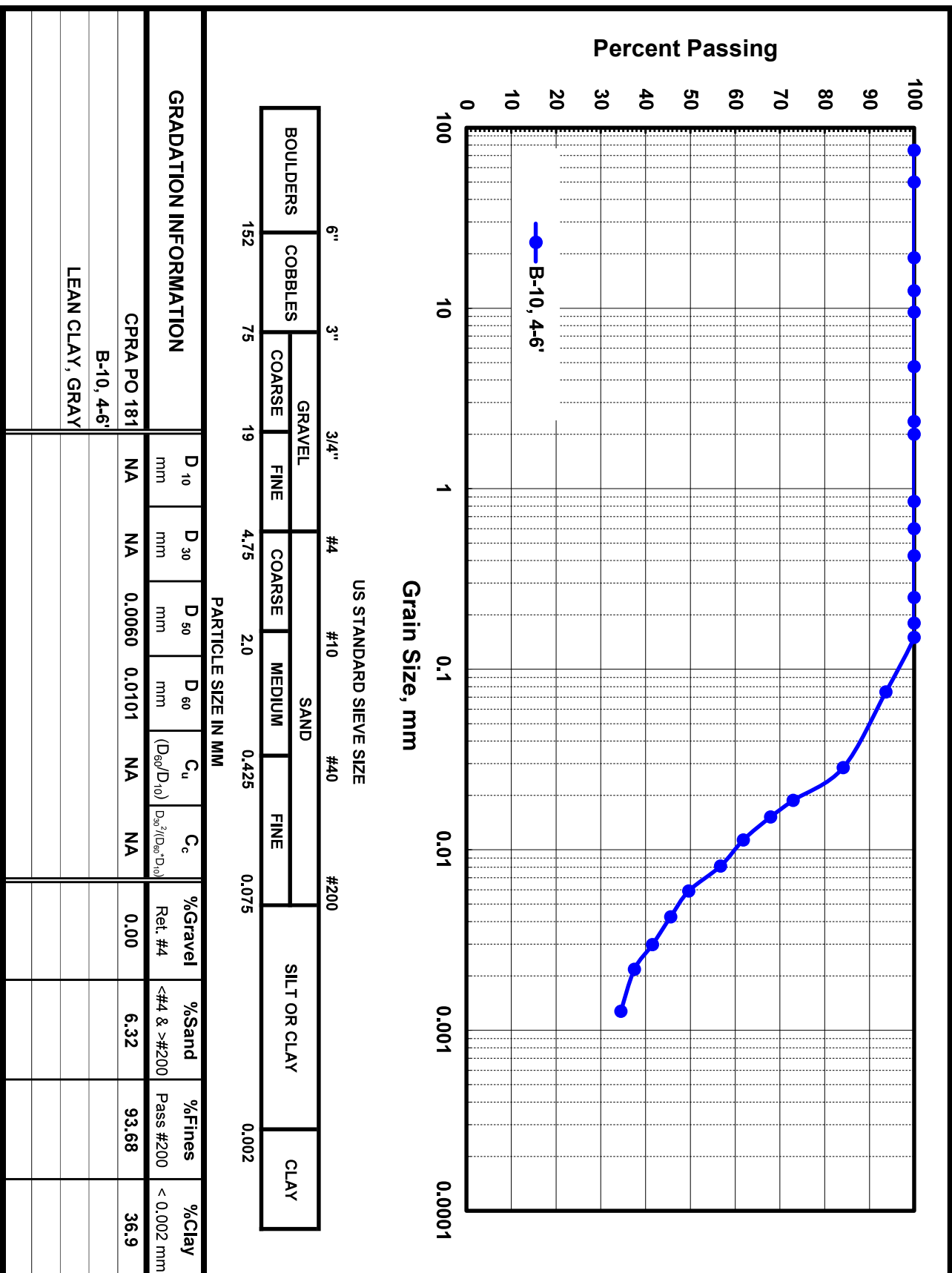


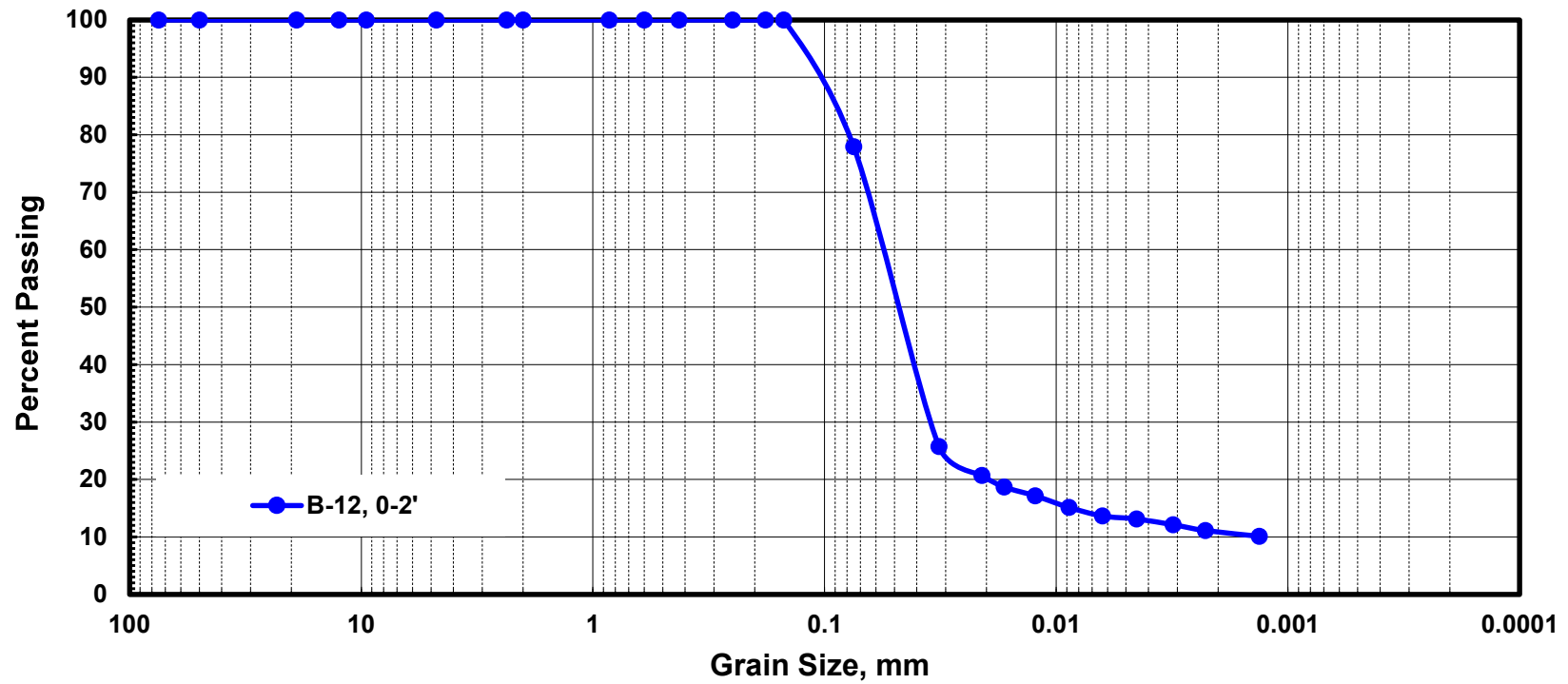
US STANDARD SIEVE SIZE									
6"	3"	3/4"	#4	#10	#40	#200			
BOULDERS	COBBLES	GRAVEL		SAND			SILT OR CLAY	CLAY	
		COARSE	FINE	COARSE	MEDIUM	FINE			
152	75	19	4.75	2.0	0.425	0.075			0.002

GRADATION INFORMATION		PARTICLE SIZE IN MM						%Gravel	%Sand	%Fines	%Clay
		D ₁₀ mm	D ₃₀ mm	D ₅₀ mm	D ₆₀ mm	C _u (D ₆₀ /D ₁₀)	C _c (D ₃₀ ² /(D ₆₀ *D ₁₀))	Ret. #4	<#4 & >#200	Pass #200	< 0.002 mm
CPRA PO 181		NA	NA	0.0143	0.0246	NA	NA	0.00	25.89	74.11	32.88
B-8, 0-2'											
LEAN CLAY WITH SAND, GRAY											



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US STANDARD SIEVE SIZE									
6"	3"	3/4"	#4	#10	#40	#200			
BOULDERS	COBBLES	GRAVEL		SAND			SILT OR CLAY	CLAY	
		COARSE	FINE	COARSE	MEDIUM	FINE			
152	75	19	4.75	2.0	0.425	0.075			0.002

GRADATION INFORMATION		PARTICLE SIZE IN MM						%Gravel	%Sand	%Fines	%Clay
		D ₁₀ mm	D ₃₀ mm	D ₅₀ mm	D ₆₀ mm	C _u (D ₆₀ /D ₁₀)	C _c (D ₃₀ ² /(D ₆₀ *D ₁₀))	Ret. #4	<#4 & >#200	Pass #200	< 0.002 mm
CPRA PO 181		NA	0.0344	0.0476	0.0560	NA	NA	0.00	22.06	77.94	10.81
B-12, 0-2'											
LEAN CLAY WITH SAND, GRAY											



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 Information <i>To Build On</i>		Specific Gravity ASTM (D 854)	
Project Name: CPRA PO 181	Checked By:	Computed By:	
Project No: 02541139	Date tested: 9/2-4/20		
Date Prepared:	Scales Control Number: 03ES254		
Tested by:	Oven Control Number: 02OV251		
Boring Number :	M-1		
Sample Number:	S-3		
Depth, feet:	4-6'		
Description of Material:			
Pycometer Calibration Information			
Flask ID.	17PYC254		
Calibrated Mass of Flask (Mpc), gms:	162.170		
Calibrated Volume of the Flask (Vpc), mL:	499.100		
Check for Pycometer Calibration			
Mass of Flask, gms(Mp):	162.10	Is Mp within 0.06g of Mpc (Y/N):	Y
Mass of Flask + Water, (Mpw,t), gms:	659.75		
Temperature of Water (Ct), deg. C:	25.4	Temperature of Water After Cooling	
Density of Water at Tt, (pw,ct), g/mL:	0.99694	(From Table 1 of ASTM D 854)	
Volume of the Flask (Vp), mL:	499.177	Vp = (Mpwct - Mp)/pw,ct	
Test Data (Method B)			
Mass of Flask + Water at Test (Mpw, t), gms:	659.805	Mpw = Mp + (Vp * pw,t)	
Mass of Dry Soil (Ms), gms:	65.20		
Mass of Flask + Water + Soil, (Mpws, t), gms:	700.43		
Temperature of Water (Tt), deg. C:	25.0	Temperature of Water+Soil After Cooling	
Density of Water at Tt, (pw,t), g/mL:	0.99705	(From Table 1 of ASTM D 854)	
Specific gravity of Water at Tt, (Gt):	2.653	Gt = Ms /{Mpw,t - (Mpwst - Ms)}	
Temperature Coefficient, K:	0.99884	(From Table 1 of ASTM D 854)	
Specific gravity of soil, Gs (20 deg. C):	2.650	Gs (20C) = Gt * K	
Remarks:			

		Specific Gravity ASTM (D 854)	
Project Name: CPRA PO 181		Checked By: _____ Computed By: _____	
Project No: 02541139		Date tested: 9/2-4/20	
Date Prepared: 9/1/20		Scales Control Number: 03ES254	
Tested by: RB / DD		Oven Control Number: 02OV251	
Boring Number :	M-2		
Sample Number:	S-4 (M)		
Depth, feet:	6-8'		
Description of Material:	LT GR CL W/ SA		
Pycnometer Calibration Information			
Flask ID.	17PYC254		
Calibrated Mass of Flask (Mpc), gms:	162.170		
Calibrated Volume of the Flask (Vpc), mL:	499.100		
Check for Pycnometer Calibration			
Mass of Flask, gms(Mp):	162.12	Is Mp within 0.06g of Mpc (Y/N):	Y
Mass of Flask + Water, (Mpw,t), gms:	659.75		
Temperature of Water (Ct), deg. C:	25.4	(Temperature of Water After Cooling)	
Density of Water at Tt, (pw,ct), g/mL:	0.99694	(From Table 1 of ASTM D 854)	
Volume of the Flask (Vp), mL:	499.157	$V_p = (M_{pwct} - M_p) / p_{w,ct}$	
Test Data (Method B)			
Mass of Flask + Water at Test (Mpw, t), gms:	659.675	$(M_{pw} = M_p + (V_p * p_{w,t}))$	
Mass of Dry Soil (Ms), gms:	68.46		
Mass of Flask + Water + Soil, (Mpws, t), gms:	702.68		
Temperature of Water (Tt), deg. C:	26.0	(Temperature of Water+Soil After Cooling)	
Density of Water at Tt, (pw,t), g/mL:	0.99679	(From Table 1 of ASTM D 854)	
Specific gravity of Water at T, (Gt):	2.689	$G_t = M_s / \{M_{pw,t} - (M_{pwst} - M_s)\}$	
Temperature Coefficient, K:	0.99858	(From Table 1 of ASTM D 854)	
Specific gravity of soil, Gs (20 deg. C):	2.686	$G_s (20C) = G_t * K$	
Remarks:			

 Information <i>To Build On</i>		Specific Gravity ASTM (D 854)	
Project Name: CPRA PO 181	Checked By:	Computed By:	
Project No: 02541139	Date tested: 9/2-4/20		
Date Prepared: 9/1/20	Scales Control Number: 03ES254		
Tested by: RB / DD	Oven Control Number: 02OV251		
Boring Number :	M-4		
Sample Number:	S-3 (M)		
Depth, feet:	4-6'		
Description of Material:	LT GR CL W/ SA		
Pycometer Calibration Information			
Flask ID.	19PYC254		
Calibrated Mass of Flask (Mpc), gms:	168.230		
Calibrated Volume of the Flask (Vpc), mL:	499.090		
Check for Pycometer Calibration			
Mass of Flask, gms(Mp):	168.20	Is Mp within 0.06g of Mpc (Y/N):	Y
Mass of Flask + Water, (Mpw,t), gms:	665.79		
Temperature of Water (Ct), deg. C:	25.5	Temperature of Water After Cooling	
Density of Water at Tt, (pw,ct), g/mL:	0.99692	(From Table 1 of ASTM D 854)	
Volume of the Flask (Vp), mL:	499.127	Vp = (Mpwct - Mp)/pw,ct	
Test Data (Method B)			
Mass of Flask + Water at Test (Mpw, t), gms:	665.725	Mpw = Mp + (Vp * pw,t)	
Mass of Dry Soil (Ms), gms:	63.71		
Mass of Flask + Water + Soil, (Mpw, t), gms:	705.71		
Temperature of Water (Tt), deg. C:	26.0	Temperature of Water+Soil After Cooling	
Density of Water at Tt, (pw,t), g/mL:	0.99679	(From Table 1 of ASTM D 854)	
Specific gravity of Water at Tt, (Gt):	2.685	Gt = Ms /{Mpw,t - (Mpwst - Ms)}	
Temperature Coefficient, K:	0.99858	(From Table 1 of ASTM D 854)	
Specific gravity of soil, Gs (20 deg. C):	2.682	Gs (20C) = Gt * K	
Remarks:			

		Specific Gravity ASTM (D 854)	
Project Name: CPRA PO 181		Checked By: _____ Computed By: _____	
Project No: 02541139		Date tested: 9/2-4/20	
Date Prepared:		Scales Control Number: 03ES254	
Tested by:		Oven Control Number: 02OV251	
Boring Number :	M-5		
Sample Number:	S-2		
Depth, feet:	2-4'		
Description of Material:			
Pyconmeter Calibration Information			
Flask ID.	19PYC254		
Calibrated Mass of Flask (Mpc), gms:	168.230		
Calibrated Volume of the Flask (Vpc), mL:	499.090		
Check for Pyconmeter Calibration			
Mass of Flask, gms(Mp):	168.18	Is Mp within 0.06g of Mpc (Y/N):	Y
Mass of Flask + Water, (Mpw,t), gms:	665.79		
Temperature of Water (Ct), deg. C:	25.5	(Temperature of Water After Cooling)	
Density of Water at Tt, (pw,ct), g/mL:	0.99692	(From Table 1 of ASTM D 854)	
Volume of the Flask (Vp), mL:	499.147	$Vp = (Mpwct - Mp)/pw,ct$	
Test Data (Method B)			
Mass of Flask + Water at Test (Mpw, t), gms:	665.855	$(Mpw = Mp + (Vp * pw,t))$	
Mass of Dry Soil (Ms), gms:	66.13		
Mass of Flask + Water + Soil, (Mpws, t), gms:	707.09		
Temperature of Water (Tt), deg. C:	25.0	(Temperature of Water+Soil After Cooling)	
Density of Water at Tt, (pw,t), g/mL:	0.99705	(From Table 1 of ASTM D 854)	
Specific gravity of Water at T, (Gt):	2.656	$Gt = Ms / \{Mpw,t - (Mpwst - Ms)\}$	
Temperature Coefficient, K:	0.99884	(From Table 1 of ASTM D 854)	
Specific gravity of soil, Gs (20 deg. C):	2.653	$Gs (20C) = Gt * K$	
Remarks:			

 Information <i>To Build On</i>		Specific Gravity ASTM (D 854)	
Project Name: CPRA PO 181	Checked By:	Computed By:	
Project No: 02541139	Date tested: 9/2-4/20		
Date Prepared: 9/1/20	Scales Control Number: 03ES254		
Tested by: DD	Oven Control Number: 02OV251		
Boring Number :	M-6		
Sample Number:	S-2		
Depth, feet:	2-4'		
Description of Material:			
Pycometer Calibration Information			
Flask ID.	23PYC254		
Calibrated Mass of Flask (Mpc), gms:	170.250		
Calibrated Volume of the Flask (Vpc), mL:	499.070		
Check for Pycometer Calibration			
Mass of Flask, gms(Mp):	170.25	Is Mp within 0.06g of Mpc (Y/N):	Y
Mass of Flask + Water, (Mpw,t), gms:	667.79		
Temperature of Water (Ct), deg. C:	25.4	Temperature of Water After Cooling)	
Density of Water at Tt, (pw,ct), g/mL:	0.99694	(From Table 1 of ASTM D 854)	
Volume of the Flask (Vp), mL:	499.067	Vp = (Mpwct - Mp)/pw,ct	
Test Data (Method B)			
Mass of Flask + Water at Test (Mpw, t), gms:	667.780	(Mpw = Mp + (Vp * pw,t))	
Mass of Dry Soil (Ms), gms:	60.14		
Mass of Flask + Water + Soil, (Mpws, t), gms:	705.37		
Temperature of Water (Tt), deg. C:	25.5	Temperature of Water+Soil After Cooling)	
Density of Water at Tt, (pw,t), g/mL:	0.99692	(From Table 1 of ASTM D 854)	
Specific gravity of Water at T, (Gt):	2.667	Gt = Ms /{Mpw,t - (Mpwst - Ms)}	
Temperature Coefficient, K:	0.99871	(From Table 1 of ASTM D 854)	
Specific gravity of soil, Gs (20 deg. C):	2.664	Gs (20C) = Gt * K	
Remarks:			

		Specific Gravity ASTM (D 854)	
Project Name: CPRA PO 181		Checked By: _____ Computed By: _____	
Project No: 02541139		Date tested: 9/2-4/20	
Date Prepared:		Scales Control Number: 03ES254	
Tested by:		Oven Control Number: 02OV251	
Boring Number :	M-7		
Sample Number:	S-3 (B)		
Depth, feet:	4-6'		
Description of Material:			
Pyconmeter Calibration Information			
Flask ID.	22PYC254		
Calibrated Mass of Flask (Mpc), gms:	164.840		
Calibrated Volume of the Flask (Vpc), mL:	499.140		
Check for Pyconmeter Calibration			
Mass of Flask, gms(Mp):	164.84	Is Mp within 0.06g of Mpc (Y/N):	Y
Mass of Flask + Water, (Mpw,t), gms:	662.45		
Temperature of Water (Ct), deg. C:	25.4	(Temperature of Water After Cooling)	
Density of Water at Tt, (pw,ct), g/mL:	0.99694	(From Table 1 of ASTM D 854)	
Volume of the Flask (Vp), mL:	499.137	$Vp = (Mpwct - Mp)/pw,ct$	
Test Data (Method B)			
Mass of Flask + Water at Test (Mpw, t), gms:	662.749	$(Mpw = Mp + (Vp * pw,t))$	
Mass of Dry Soil (Ms), gms:	61.72		
Mass of Flask + Water + Soil, (Mpws, t), gms:	701.42		
Temperature of Water (Tt), deg. C:	23.0	(Temperature of Water+Soil After Cooling)	
Density of Water at Tt, (pw,t), g/mL:	0.99754	(From Table 1 of ASTM D 854)	
Specific gravity of Water at T, (Gt):	2.678	$Gt = Ms / \{Mpw,t - (Mpwst - Ms)\}$	
Temperature Coefficient, K:	0.99933	(From Table 1 of ASTM D 854)	
Specific gravity of soil, Gs (20 deg. C):	2.676	$Gs (20C) = Gt * K$	
Remarks:			

 Information <i>To Build On</i>		Specific Gravity ASTM (D 854)	
Project Name: CPRA PO 181	Checked By:	Computed By:	
Project No: 02541139	Date tested: 9/2-4/20		
Date Prepared:	Scales Control Number: 03ES254		
Tested by:	Oven Control Number: 02OV251		
Boring Number :	M-8		
Sample Number:	S-8		
Depth, feet:	14-16'		
Description of Material:			
Pycometer Calibration Information			
Flask ID.	22PYC254		
Calibrated Mass of Flask (Mpc), gms:	164.840		
Calibrated Volume of the Flask (Vpc), mL:	499.140		
Check for Pycometer Calibration			
Mass of Flask, gms(Mp):	164.84	Is Mp within 0.06g of Mpc (Y/N):	Y
Mass of Flask + Water, (Mpw,t), gms:	662.45		
Temperature of Water (Ct), deg. C:	25.4	(Temperature of Water After Cooling)	
Density of Water at Tt, (pw,ct), g/mL:	0.99694	(From Table 1 of ASTM D 854)	
Volume of the Flask (Vp), mL:	499.137	Vp = (Mpwct - Mp)/pw,ct	
Test Data (Method B)			
Mass of Flask + Water at Test (Mpw, t), gms:	662.505	(Mpw = Mp + (Vp * pw,t))	
Mass of Dry Soil (Ms), gms:	65.56		
Mass of Flask + Water + Soil, (Mpw, t), gms:	703.66		
Temperature of Water (Tt), deg. C:	25.0	(Temperature of Water+Soil After Cooling)	
Density of Water at Tt, (pw,t), g/mL:	0.99705	(From Table 1 of ASTM D 854)	
Specific gravity of Water at Tt, (Gt):	2.686	Gt = Ms /{Mpw,t - (Mpwst - Ms)}	
Temperature Coefficient, K:	0.99884	(From Table 1 of ASTM D 854)	
Specific gravity of soil, Gs (20 deg. C):	2.683	Gs (20C) = Gt * K	
Remarks:			

ORGANIC CONTENT

Boring No.	Depth (feet)	Organic Content (%)
M-1	0-2	1.3
M-2	0-2	31
M-3	0-2	50.3
M-4	0-2	66
M-5	0-2	61.2
M-6	0-2	8.6
M-7	0-2	5.7
M-8	2-4	0.9
M-9	0-2	47.3

APPENDIX D

UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST RESULTS

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name: CPRA PO 181	Date Tested: 9/4/2020
Project No.: 02541139	Scale: 03ES254
Boring No.: M-1	Calipers: 01CAL254
Samp. and Spec. No.: S-1 (B)	Oven: 100V251
Sample Depth: 0-2'	GeoJac Station No.: 02GJ254
Qp / Torvane (tsf): TV = 0.10	Loadcell: 17COM254
Sample Description: GR CL W/ SA	Air Pressure (psi): 0.4

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.860	2.873	2.912	2.882	2.02
Height (in)	5.819	5.828	5.826	5.824	
	Area, ft ² :	0.045	Volume, ft ³ :	0.022	

Moisture Content (%)		Unit Weight	
Can Number:	60H	Wet Weight of Sample (g):	1226.77
Wt. of Can (g):	31.33	Dry Weight of Sample (g):	1010.35
Wt. of Wet Soil + Can (g):	86.03	Wet Unit Weight (pcf):	123.0
Wt. of Dry Soil + Can (g):	76.38	Dry Unit Weight (pcf):	101.3
Wt. of Dry Soil (g):	45.05		
Wt. of Water (g):	9.65		
Moisture Content (%):	21.4		

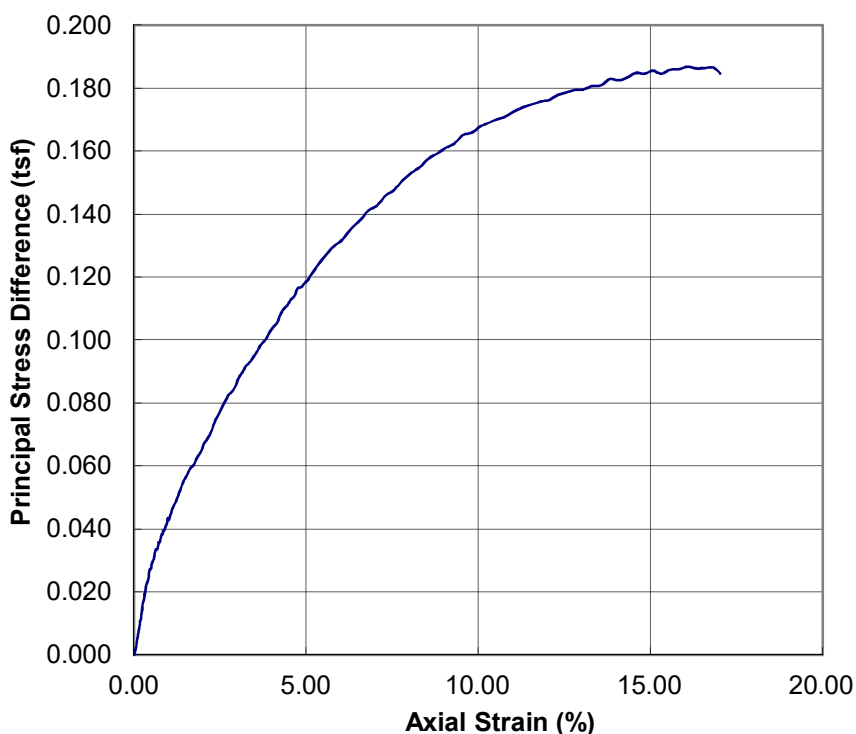
	Max Strain (≤ 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.071	0.19	0.09

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: RB
 Computed By: RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	9/4/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-1	Calipers:	01CAL254
Samp. and Spec. No.:	S-3 (B)	Oven:	100OV251
Sample Depth:	4-6'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	0.75 / 3 TV = 0.20	Loadcell:	17COM254
Sample Description:	GR CL W/ SA & SA POCKETS	Air Pressure (psi):	2.0

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.746	2.804	2.794	2.781	2.05
Height (in)	5.690	5.693	5.706	5.696	
Area, ft²:	0.042		Volume, ft³:		0.020

Moisture Content (%)		Unit Weight	
Can Number:	DB-13	Wet Weight of Sample (g):	1181.11
Wt. of Can (g):	31.45	Dry Weight of Sample (g):	970.48
Wt. of Wet Soil + Can (g):	83.88	Wet Unit Weight (pcf):	130.0
Wt. of Dry Soil + Can (g):	74.53	Dry Unit Weight (pcf):	106.8
Wt. of Dry Soil (g):	43.08		
Wt. of Water (g):	9.35		
Moisture Content (%):	21.7		

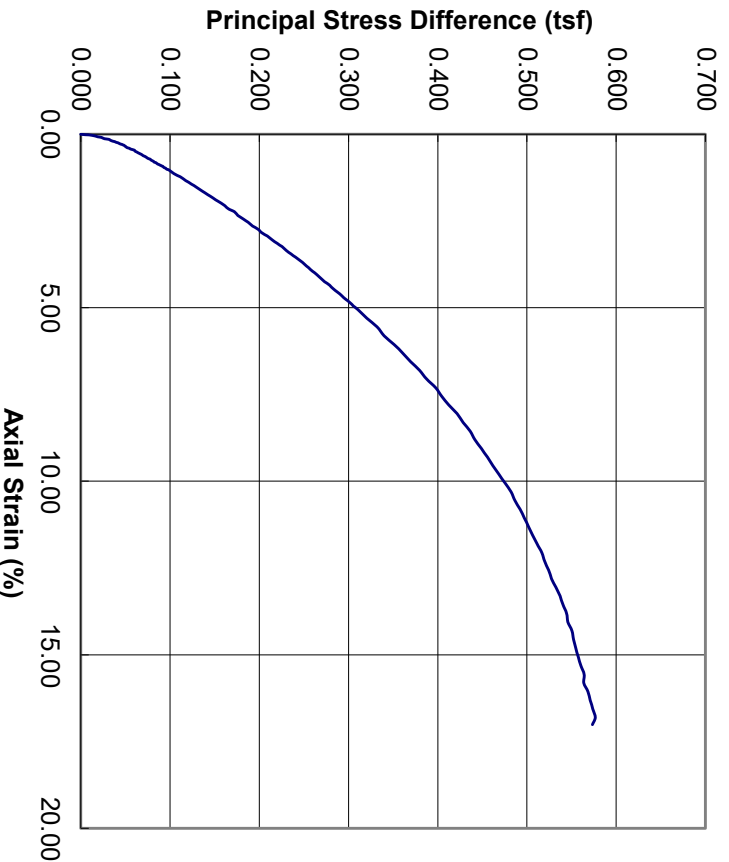
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.068	0.56
		0.28

FAILURE TYPE:



NOTE:

Stress-Strain



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	9/4/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-1	Calipers:	01CAL254
Samp. and Spec. No.:	S-5 (B)	Oven:	10OV251
Sample Depth:	8-10'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	1.75 / 3	Loadcell:	12COM254
Sample Description:	LTGR/TN CLS	Air Pressure (psi):	3.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.841	2.816	2.856	2.838	2.06
Height (in)	5.851	5.855	5.850	5.852	
	Area, ft ² :	0.044	Volume, ft ³ :	0.021	

Moisture Content (%)		Unit Weight	
Can Number:	DB-1	Wet Weight of Sample (g):	1209.94
Wt. of Can (g):	31.26	Dry Weight of Sample (g):	979.66
Wt. of Wet Soil + Can (g):	83.75	Wet Unit Weight (pcf):	124.5
Wt. of Dry Soil + Can (g):	73.76	Dry Unit Weight (pcf):	100.8
Wt. of Dry Soil (g):	42.50		
Wt. of Water (g):	9.99		
Moisture Content (%):	23.5		

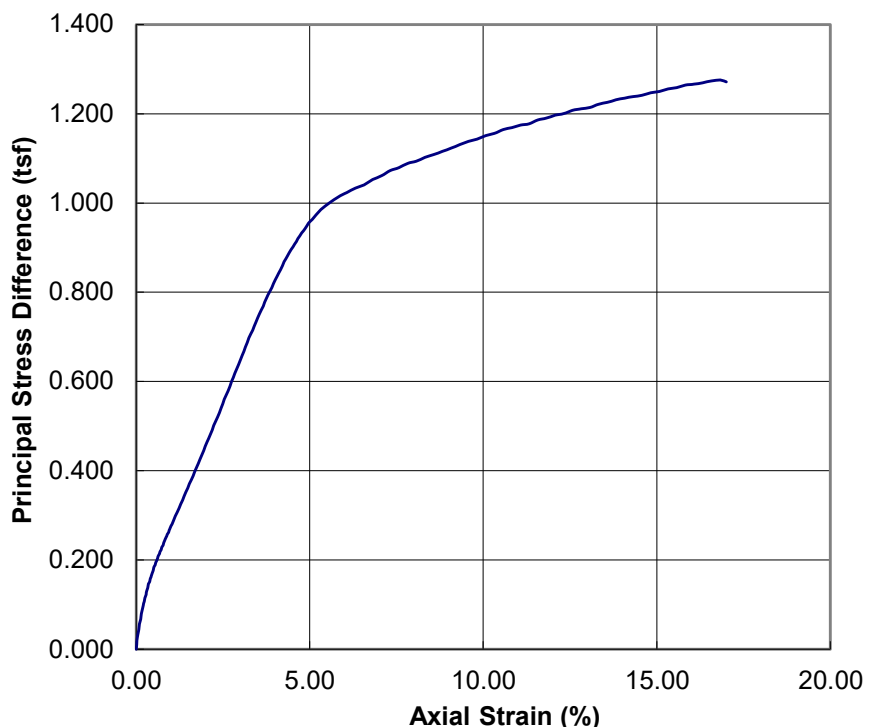
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.075	1.25	0.63

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name: CPRA PO 181	Date Tested: 9/4/2020
Project No.: 02541139	Scale: 03ES254
Boring No.: M-1	Calipers: 01CAL254
Samp. and Spec. No.: S-8 (B)	Oven: 100OV251
Sample Depth: 14-16'	Geolac Station No.: 02GJ254
Qp / Torvane (tsf): 2.0 / 3	Loadcell: 12COM254
Sample Description: TN CL-ML	Air Pressure (psi): 6.0

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.856	2.830	2.852	2.846	2.03
Height (in)	5.776	5.784	5.776	5.779	
Area, ft²:	0.044		Volume, ft³:	0.021	

Moisture Content (%)		Unit Weight	
Can Number:	59	Wet Weight of Sample (g):	1153.04
Wt. of Can (g):	31.11	Dry Weight of Sample (g):	889.54
Wt. of Wet Soil + Can (g):	84.19	Wet Unit Weight (pcf):	119.5
Wt. of Dry Soil + Can (g):	72.06	Dry Unit Weight (pcf):	92.2
Wt. of Dry Soil (g):	40.95		
Wt. of Water (g):	12.13		
Moisture Content (%):	29.6		

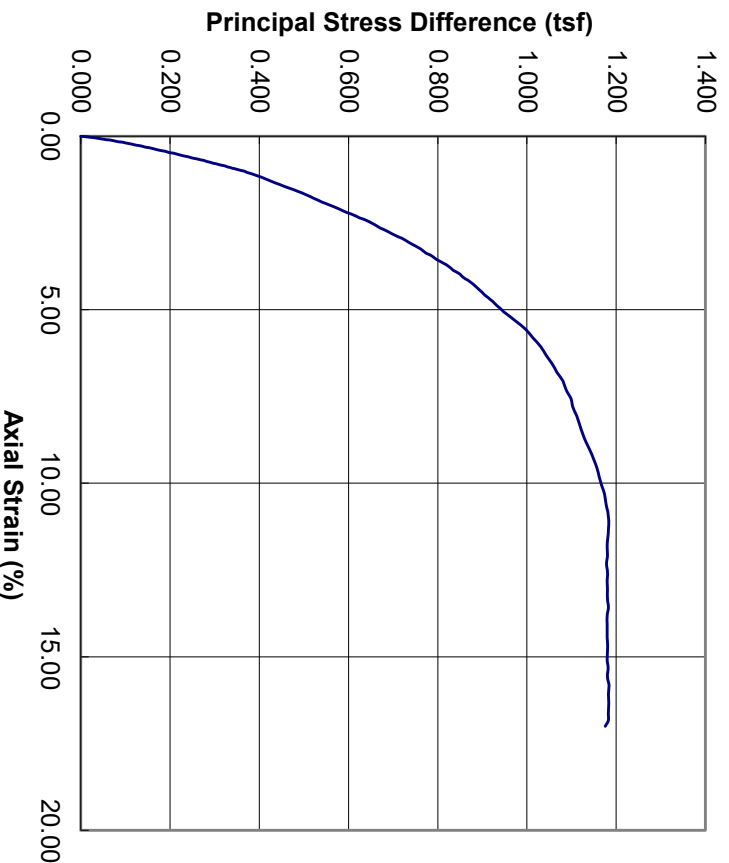
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS: 15.076	1.18	0.59

FAILURE TYPE:



NOTE:

Stress-Strain



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	9/25/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-1	Calipers:	01CAL254
Samp. and Spec. No.:	S-11 (B)	Oven:	10OV251
Sample Depth:	23-25'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	2.25 / 3	Loadcell:	12COM254
Sample Description:	LTGR/TN CL W/ SA	Air Pressure (psi):	9.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.872	2.862	2.868	2.867	2.01
Height (in)	5.748	5.756	5.760	5.755	
	Area, ft ² :	0.045	Volume, ft ³ :	0.022	

Moisture Content (%)		Unit Weight	
Can Number:	L-21	Wet Weight of Sample (g):	1158.91
Wt. of Can (g):	31.33	Dry Weight of Sample (g):	876.45
Wt. of Wet Soil + Can (g):	83.97	Wet Unit Weight (pcf):	118.8
Wt. of Dry Soil + Can (g):	71.14	Dry Unit Weight (pcf):	89.9
Wt. of Dry Soil (g):	39.81		
Wt. of Water (g):	12.83		
Moisture Content (%):	32.2		

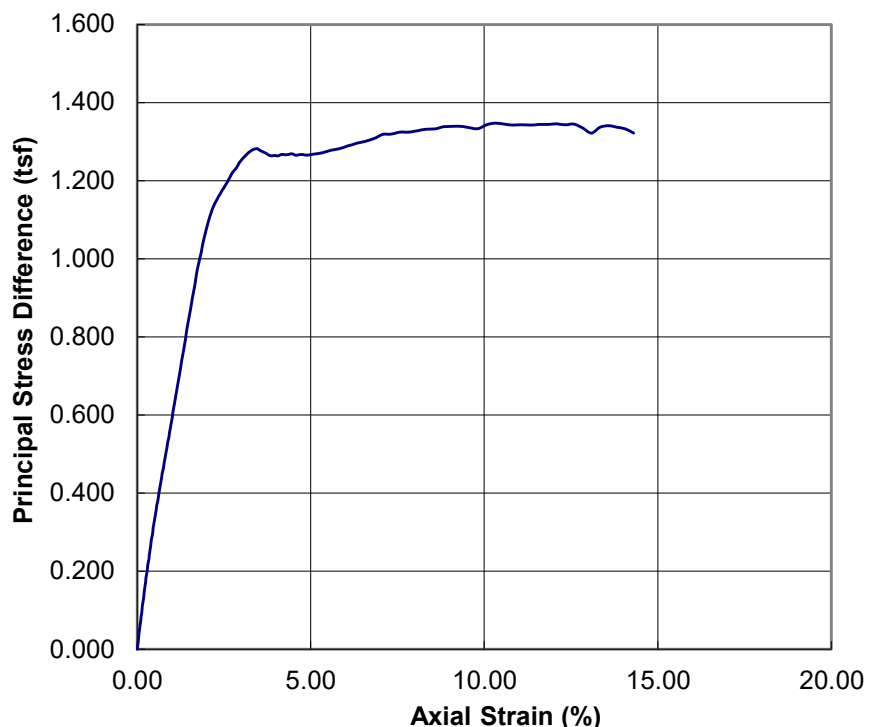
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	3.457	1.28	0.64

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: RB
 Computed By: RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	8/28/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-2	Calipers:	01CAL254
Samp. and Spec. No.:	S-1 (B)	Oven:	100V251
Sample Depth:	0-2'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	TV = 0.05	Loadcell:	16COM254
Sample Description:	DKGR/BK OH W/ SAND	Air Pressure (psi):	0.4

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.766	2.824	2.811	2.800	2.02
Height (in)	5.635	5.666	5.653	5.651	
Area, ft²:	0.043	Volume, ft³:		0.020	

Moisture Content (%)		Unit Weight	
Can Number:	246-10	Wet Weight of Sample (g):	1019.68
Wt. of Can (g):	30.09	Dry Weight of Sample (g):	726.29
Wt. of Wet Soil + Can (g):	82.64	Wet Unit Weight (pcf):	111.6
Wt. of Dry Soil + Can (g):	67.52	Dry Unit Weight (pcf):	79.5
Wt. of Dry Soil (g):	37.43		
Wt. of Water (g):	15.12		
Moisture Content (%):	40.4		

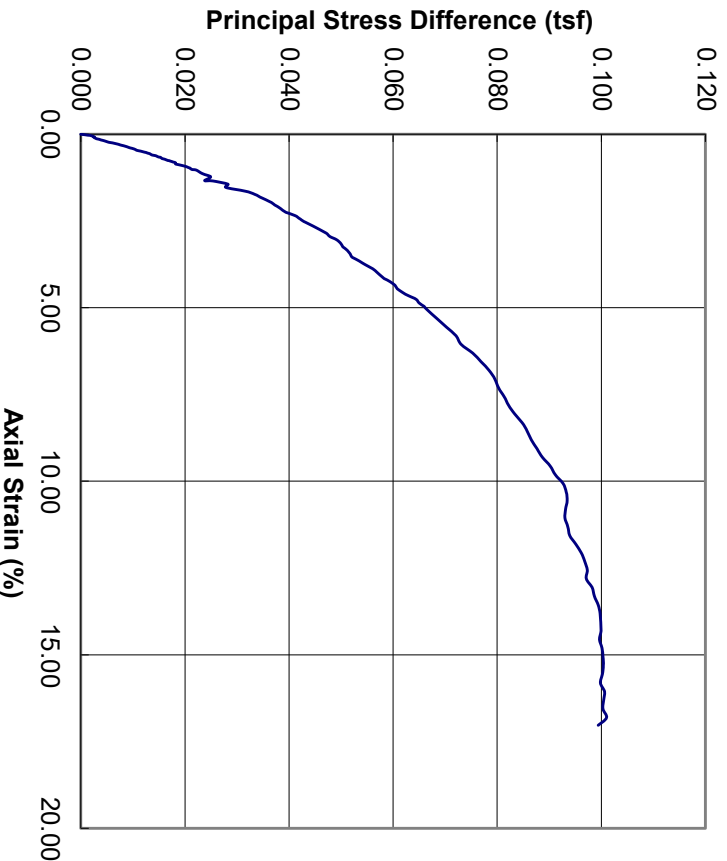
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.063	0.10
		0.05

FAILURE TYPE:



NOTE:

Stress-Strain



Tested By: RB

Computed By: RB

Reviewed By: _____

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UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	8/28/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-2	Calipers:	01CAL254
Samp. and Spec. No.:	S-3 (B)	Oven:	10OV251
Sample Depth:	4-6'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	TV = 0.50	Loadcell:	17COM254
Sample Description:	LTGR CH W/ SAND	Air Pressure (psi):	2.0

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.870	2.854	2.838	2.854	2.05
Height (in)	5.861	5.833	5.850	5.848	
	Area, ft ² :	0.044	Volume, ft ³ :	0.022	

Moisture Content (%)		Unit Weight	
Can Number:	246-14	Wet Weight of Sample (g):	1175.65
Wt. of Can (g):	29.83	Dry Weight of Sample (g):	883.37
Wt. of Wet Soil + Can (g):	83.93	Wet Unit Weight (pcf):	119.7
Wt. of Dry Soil + Can (g):	70.48	Dry Unit Weight (pcf):	90.0
Wt. of Dry Soil (g):	40.65		
Wt. of Water (g):	13.45		
Moisture Content (%):	33.1		

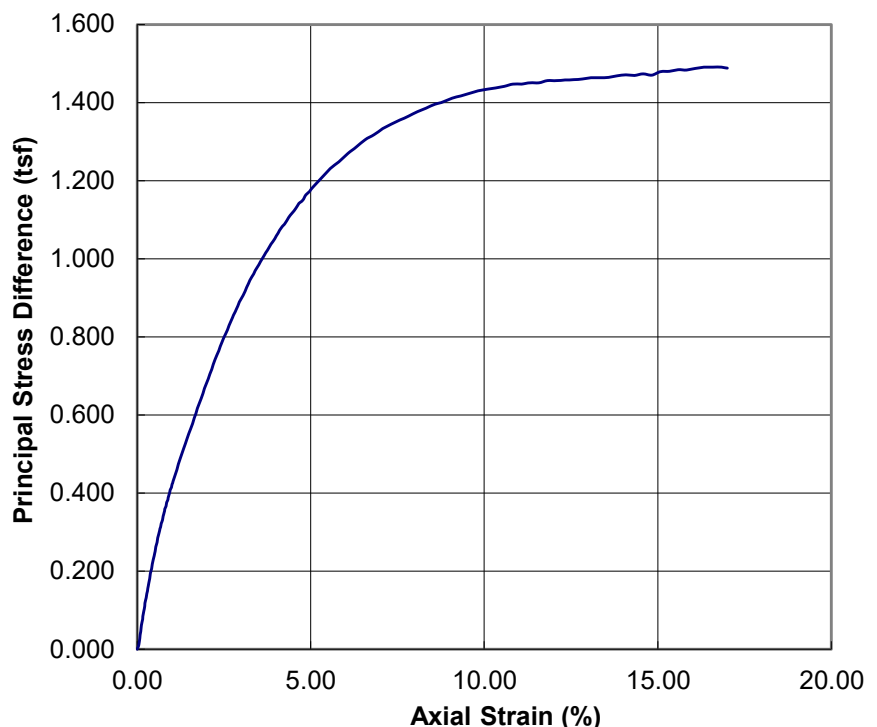
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.083	1.48	0.74

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: DD
 Computed By: RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	8/28/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-2	Calipers:	01CAL254
Samp. and Spec. No.:	S-6 (M)	Oven:	100OV251
Sample Depth:	10-12'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	1.5 / 3	Loadcell:	17COM254
Sample Description:	GR/OR CLS	Air Pressure (psi):	4.4

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.826	2.832	2.826	2.828	2.05
Height (in)	5.806	5.806	5.804	5.805	
Area, ft²:	0.044	Volume, ft³:		0.021	

Moisture Content (%)		Unit Weight	
Can Number:	83	Wet Weight of Sample (g):	1178.35
Wt. of Can (g):	31.28	Dry Weight of Sample (g):	921.82
Wt. of Wet Soil + Can (g):	84.15	Wet Unit Weight (pcf):	123.1
Wt. of Dry Soil + Can (g):	72.64	Dry Unit Weight (pcf):	96.3
Wt. of Dry Soil (g):	41.36		
Wt. of Water (g):	11.51		
Moisture Content (%):	27.8		

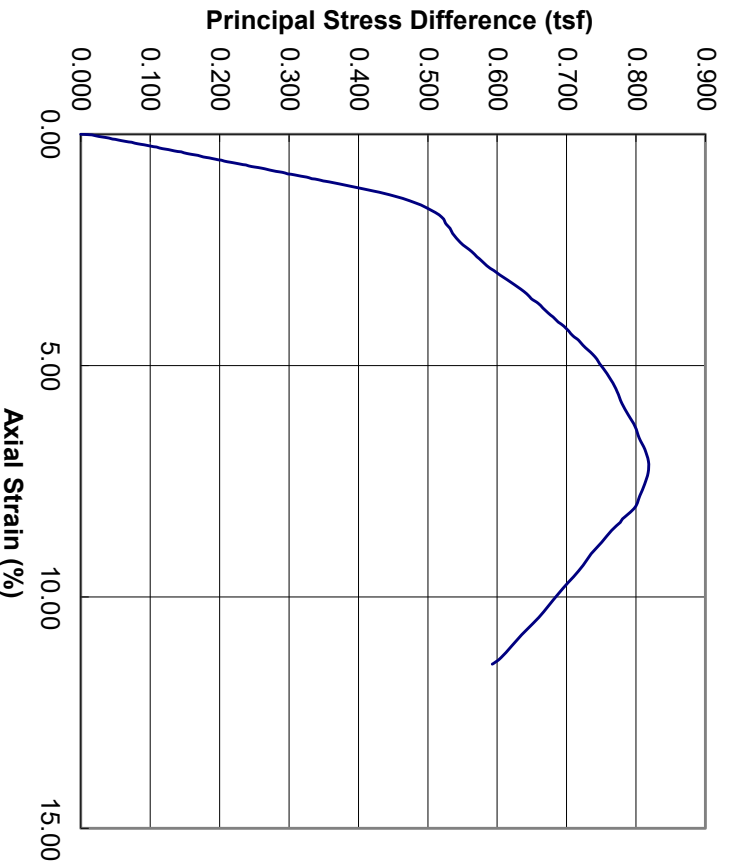
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	7.066	0.82
		0.41

FAILURE TYPE:



NOTE:

Stress-Strain



Tested By: DD
 Computed By: RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	8/28/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-2	Calipers:	01CAL254
Samp. and Spec. No.:	S-10 (B)	Oven:	10OV251
Sample Depth:	18-20'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	1.25 / 3	Loadcell:	17COM254
Sample Description:	LTGR/BR CH W/ SI PARTINGS	Air Pressure (psi):	7.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.866	2.875	2.859	2.867	2.05
Height (in)	5.881	5.915	5.863	5.886	
	Area, ft ² :	0.045	Volume, ft ³ :	0.022	

Moisture Content (%)		Unit Weight	
Can Number:	K-19	Wet Weight of Sample (g):	1179.35
Wt. of Can (g):	31.64	Dry Weight of Sample (g):	896.49
Wt. of Wet Soil + Can (g):	86.30	Wet Unit Weight (pcf):	118.3
Wt. of Dry Soil + Can (g):	73.19	Dry Unit Weight (pcf):	89.9
Wt. of Dry Soil (g):	41.55		
Wt. of Water (g):	13.11		
Moisture Content (%):	31.6		

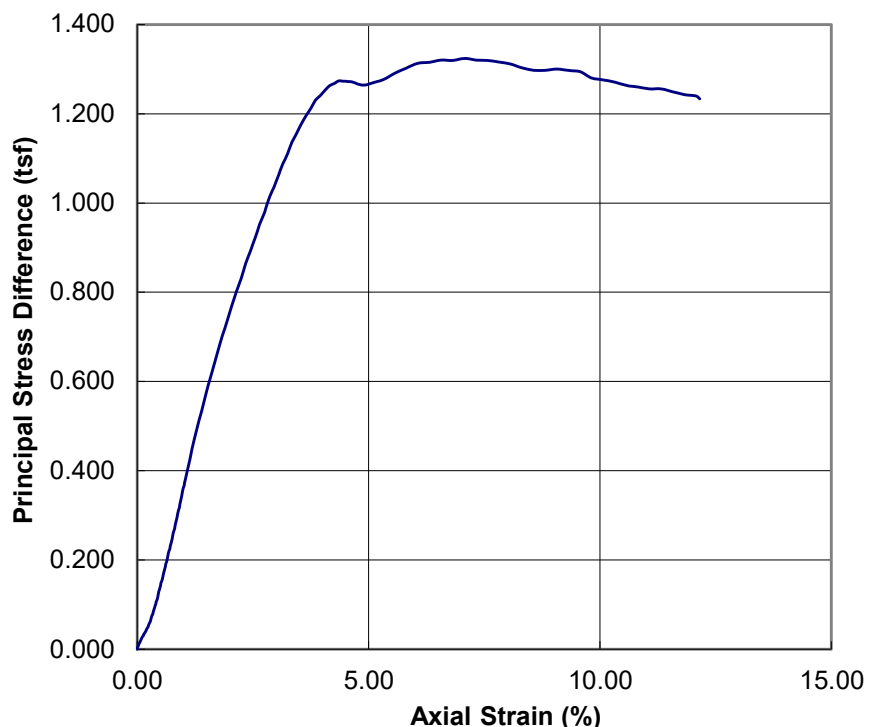
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	7.067	1.32	0.66

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: DD
 Computed By: RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name: CPRA PO 181

Project No.: 02541139

Boring No.: M-3

Samp. and Spec. No.: S-1 (B)

Sample Depth: 0-2'

Qp / Torvane (tsf): TV = 0.025

Sample Description: BK OH W/ ROOTS

Date Tested: 9/3/2020

Scale: 03ES254

Calipers: 01CAL254

Oven: 100OV251

Geolac Station No.: 02GJ254

Loadcell: 16COM254

Air Pressure (psi): 0.4

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.801	2.779	2.800	2.793	2.07
Height (in)	5.775	5.772	5.776	5.774	
Area, ft²:	0.043	Volume, ft³:		0.020	

Moisture Content (%)		Unit Weight	
Can Number:	L-21	Wet Weight of Sample (g):	625.29
Wt. of Can (g):	31.32	Dry Weight of Sample (g):	114.14
Wt. of Wet Soil + Can (g):	81.83	Wet Unit Weight (pcf):	67.3
Wt. of Dry Soil + Can (g):	40.54	Dry Unit Weight (pcf):	12.3
Wt. of Dry Soil (g):	9.22		
Wt. of Water (g):	41.29		
Moisture Content (%):	447.8		

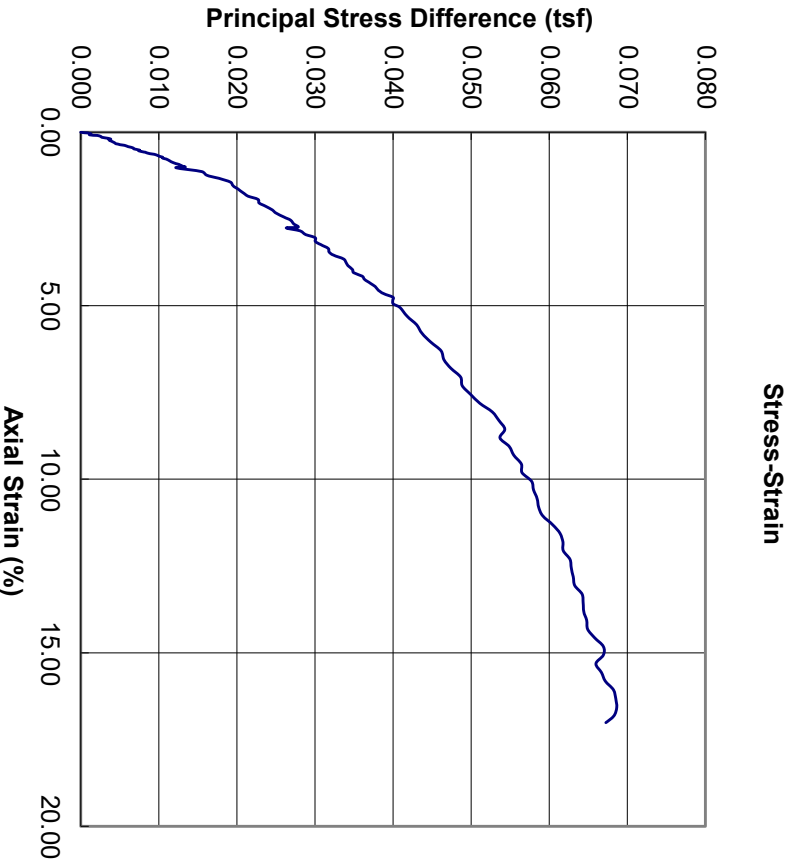
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS: 15.070	0.07	0.03

FAILURE TYPE:



NOTE: _____

Tested By: RB
 Computed By: RB
 Reviewed By: _____



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	8/31/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-3	Calipers:	01CAL254
Samp. and Spec. No.:	S-3 (B)	Oven:	10OV251
Sample Depth:	4-6'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	TV = 0.025	Loadcell:	16COM254
Sample Description:	BK OH W/ ROOTS	Air Pressure (psi):	2.0

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.781	2.751	2.788	2.773	2.08
Height (in)	5.784	5.776	5.780	5.780	
	Area, ft ² :	0.042	Volume, ft ³ :	0.020	

Moisture Content (%)		Unit Weight	
Can Number:	L-27	Wet Weight of Sample (g):	643.34
Wt. of Can (g):	31.23	Dry Weight of Sample (g):	150.14
Wt. of Wet Soil + Can (g):	82.35	Wet Unit Weight (pcf):	70.2
Wt. of Dry Soil + Can (g):	43.16	Dry Unit Weight (pcf):	16.4
Wt. of Dry Soil (g):	11.93		
Wt. of Water (g):	39.19		
Moisture Content (%):	328.5		

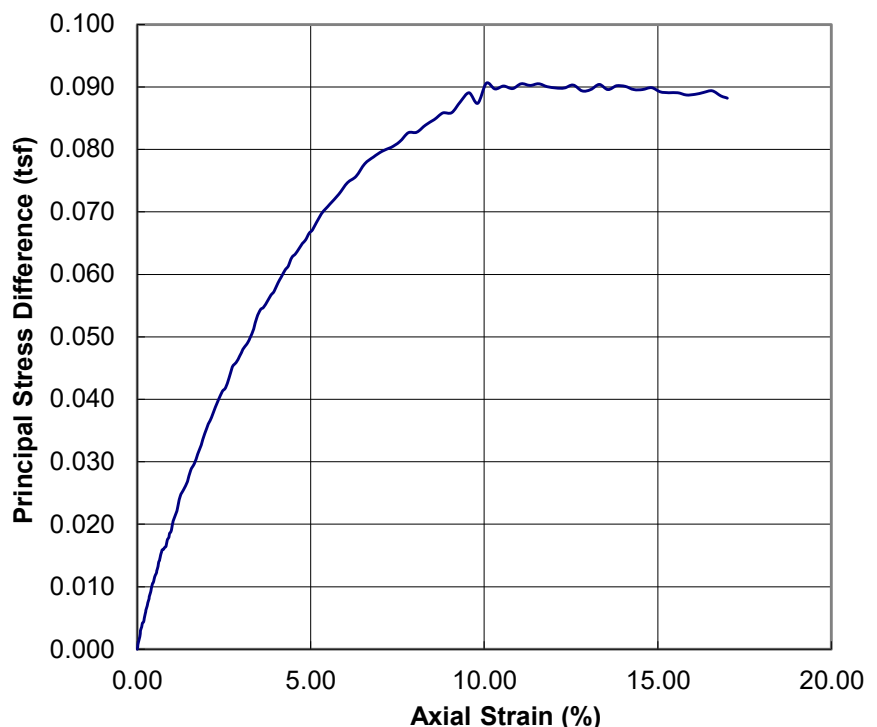
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	10.062	0.09	0.05

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: RB
 Computed By: RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	8/31/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-3	Calipers:	01CAL254
Samp. and Spec. No.:	S-6 (B)	Oven:	100V251
Sample Depth:	10-12'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	1.5 / 3 TV = 0.50	Loadcell:	12COM254
Sample Description:	LT GR CL W/ SA	Air Pressure (psi):	4.4

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.832	2.834	2.857	2.841	2.07
Height (in)	5.887	5.877	5.888	5.884	
Area, ft²:	0.044	Volume, ft³:		0.022	

Moisture Content (%)		Unit Weight	
Can Number:	V-13	Wet Weight of Sample (g):	1178.40
Wt. of Can (g):	31.32	Dry Weight of Sample (g):	916.64
Wt. of Wet Soil + Can (g):	82.10	Wet Unit Weight (pcf):	120.4
Wt. of Dry Soil + Can (g):	70.82	Dry Unit Weight (pcf):	93.6
Wt. of Dry Soil (g):	39.50		
Wt. of Water (g):	11.28		
Moisture Content (%):	28.6		

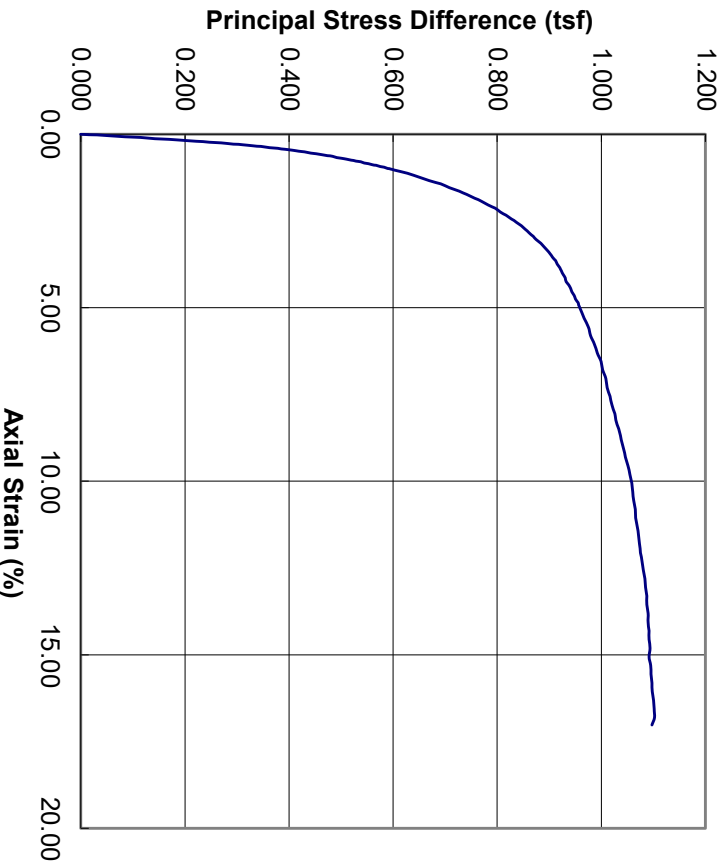
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.060	1.09
		0.55

FAILURE TYPE:

NOTE:



Stress-Strain



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	9/25/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-3	Calipers:	01CAL254
Samp. and Spec. No.:	S-12 (T)	Oven:	10OV251
Sample Depth:	28-30'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	2.25 / 3	Loadcell:	12COM254
Sample Description:	GR/BR CL W/ SI PARTINGS	Air Pressure (psi):	11.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.877	2.876	2.871	2.875	1.93
Height (in)	5.548	5.552	5.540	5.547	
	Area, ft ² :	0.045	Volume, ft ³ :	0.021	

Moisture Content (%)		Unit Weight	
Can Number:	K-5	Wet Weight of Sample (g):	1083.48
Wt. of Can (g):	31.44	Dry Weight of Sample (g):	745.48
Wt. of Wet Soil + Can (g):	81.96	Wet Unit Weight (pcf):	114.7
Wt. of Dry Soil + Can (g):	66.20	Dry Unit Weight (pcf):	78.9
Wt. of Dry Soil (g):	34.76		
Wt. of Water (g):	15.76		
Moisture Content (%):	45.3		

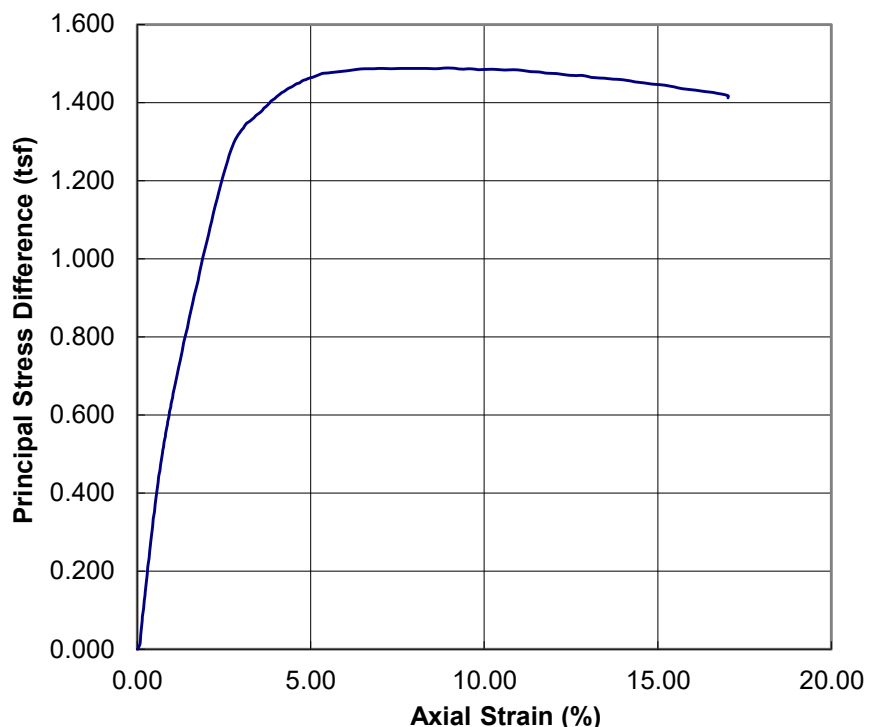
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	9.080	1.49	0.74

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	8/31/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-3	Calipers:	01CAL254
Samp. and Spec. No.:	S-14	Oven:	100V251
Sample Depth:	38.40'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	1.25 / 3 TV = 0.45	Loadcell:	12COM254
Sample Description:	GR CH	Air Pressure (psi):	15.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.826	2.846	2.819	2.830	
Height (in)	5.734	5.742	5.735	5.737	2.03
Area, ft²:	0.044	Volume, ft³:		0.021	

Moisture Content (%)		Unit Weight	
Can Number:	DB-2	Wet Weight of Sample (g):	1014.02
Wt. of Can (g):	31.41	Dry Weight of Sample (g):	656.78
Wt. of Wet Soil + Can (g):	84.12	Wet Unit Weight (pcf):	107.0
Wt. of Dry Soil + Can (g):	65.55	Dry Unit Weight (pcf):	69.3
Wt. of Dry Soil (g):	34.14		
Wt. of Water (g):	18.57		
Moisture Content (%):	54.4		

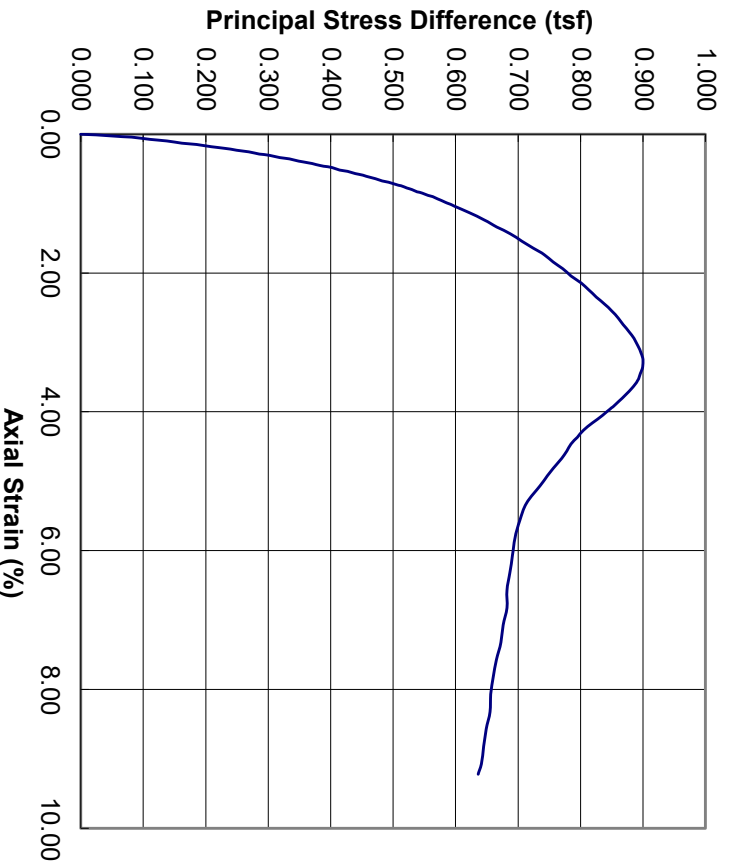
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	3.247	0.90
		0.45

FAILURE TYPE:



NOTE:

Stress-Strain



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	8/26/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-4	Calipers:	01CAL254
Samp. and Spec. No.:	S-2 (B)	Oven:	10OV251
Sample Depth:	2-4'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	TV = 0.15	Loadcell:	16COM254
Sample Description:	GR CH W/ OH LAYER & SA	Air Pressure (psi):	1.2

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.883	2.840	2.867	2.863	1.96
Height (in)	5.624	5.570	5.653	5.616	
	Area, ft ² :	0.045	Volume, ft ³ :	0.021	

Moisture Content (%)		Unit Weight	
Can Number:	K-16	Wet Weight of Sample (g):	1142.57
Wt. of Can (g):	31.31	Dry Weight of Sample (g):	890.86
Wt. of Wet Soil + Can (g):	84.01	Wet Unit Weight (pcf):	120.4
Wt. of Dry Soil + Can (g):	72.40	Dry Unit Weight (pcf):	93.9
Wt. of Dry Soil (g):	41.09		
Wt. of Water (g):	11.61		
Moisture Content (%):	28.3		

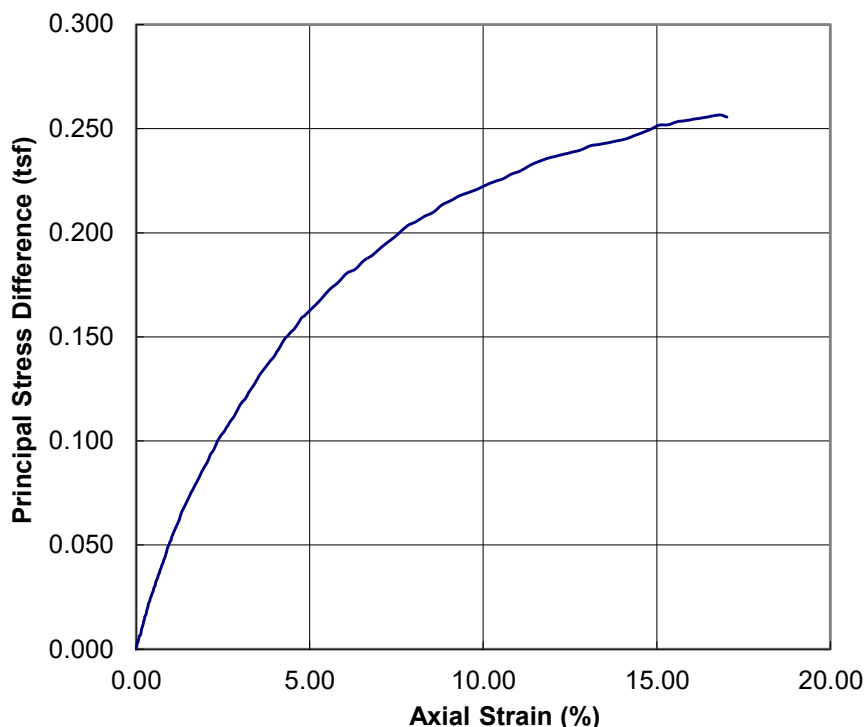
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.080	0.25	0.13

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: DD
 Computed By: RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	8/27/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-4	Calipers:	01CAL254
Samp. and Spec. No.:	S-5	Oven:	100OV251
Sample Depth:	8-10'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	1.75 / 3	Loadcell:	12COM254
Sample Description:	BLU-GR CL W/ SA	Air Pressure (psi):	3.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.868	2.844	2.878	2.863	2.04
Height (in)	5.843	5.835	5.831	5.836	
Area, ft²:	0.045		Volume, ft³:		0.022

Moisture Content (%)		Unit Weight	
Can Number:	31	Wet Weight of Sample (g):	1192.89
Wt. of Can (g):	31.16	Dry Weight of Sample (g):	928.59
Wt. of Wet Soil + Can (g):	81.89	Wet Unit Weight (pcf):	120.9
Wt. of Dry Soil + Can (g):	70.65	Dry Unit Weight (pcf):	94.1
Wt. of Dry Soil (g):	39.49		
Wt. of Water (g):	11.24		
Moisture Content (%):	28.5		

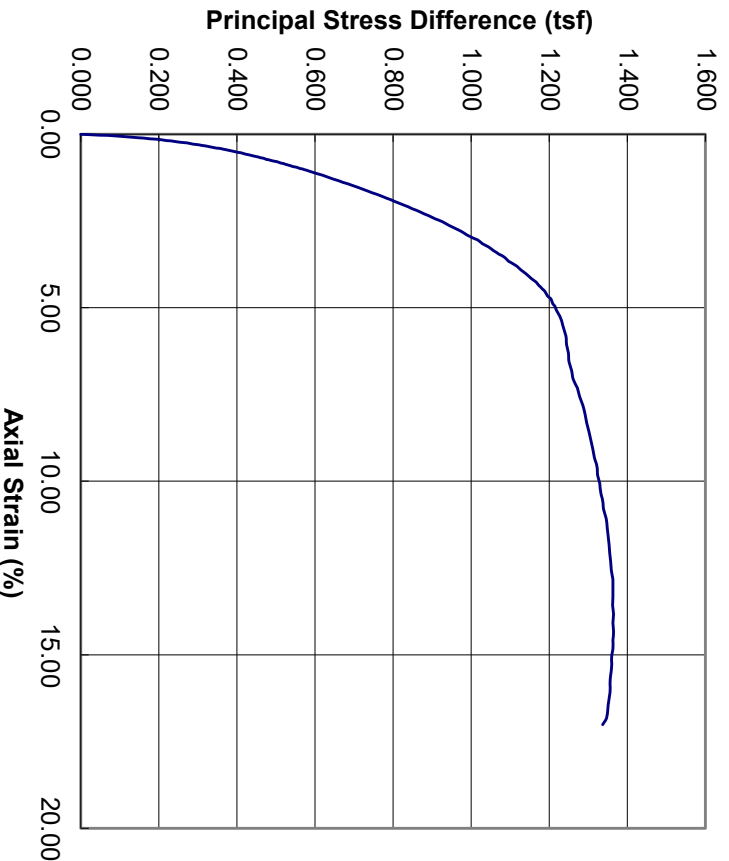
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	14.327	1.36
		0.68

FAILURE TYPE:

NOTE:



Stress-Strain



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name: CPRA PO 181	Date Tested: 8/27/2020
Project No.: 02541139	Scale: 03ES254
Boring No.: M-4	Calipers: 01CAL254
Samp. and Spec. No.: S-9 (B)	Oven: 10OV251
Sample Depth: 16-18'	GeoJac Station No.: 02GJ254
Qp / Torvane (tsf): 1.5 / 3	Loadcell: 17COM254
Sample Description: GR CL W/ SA	Air Pressure (psi): 6.8

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.765	2.766	2.819	2.783	2.03
Height (in)	5.651	5.655	5.648	5.651	
Area, ft ² :		0.042	Volume, ft ³ :		0.020

Moisture Content (%)		Unit Weight	
Can Number:	71-H	Wet Weight of Sample (g):	1114.72
Wt. of Can (g):	31.22	Dry Weight of Sample (g):	845.06
Wt. of Wet Soil + Can (g):	83.72	Wet Unit Weight (pcf):	123.5
Wt. of Dry Soil + Can (g):	71.02	Dry Unit Weight (pcf):	93.6
Wt. of Dry Soil (g):	39.80		
Wt. of Water (g):	12.70		
Moisture Content (%):	31.9		

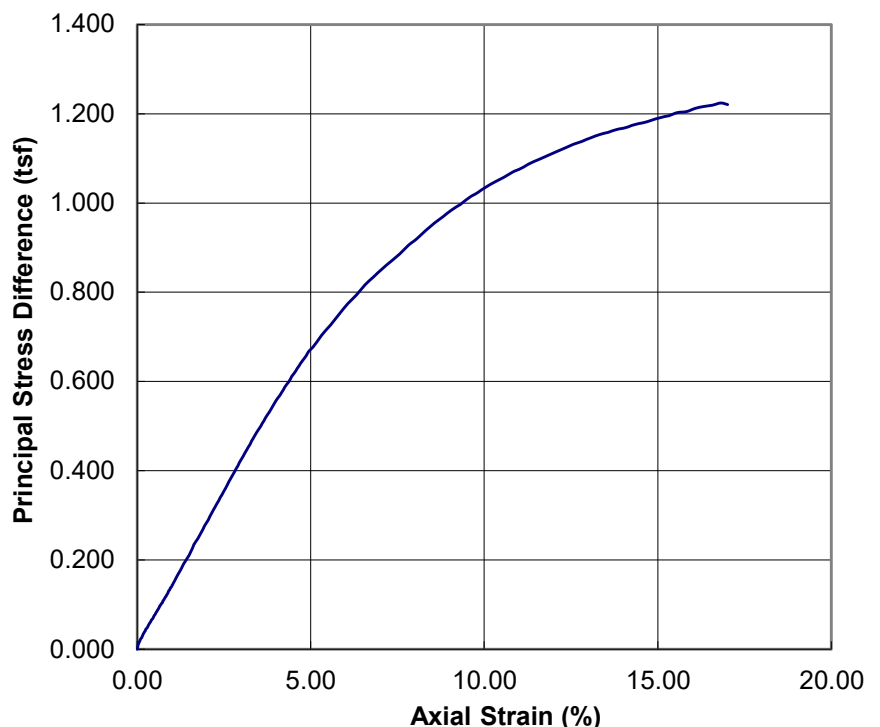
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.069	1.19	0.60

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: RB
 Computed By: RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	8/26/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-4	Calipers:	01CAL254
Samp. and Spec. No.:	S-14	Oven:	100V251
Sample Depth:	38'-40'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	TV = 0.50	Loadcell:	17COM254
Sample Description:	GR CH W/ SILT PARTING	Air Pressure (psi):	15.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.886	2.850	2.806	2.847	2.06
Height (in)	5.880	5.870	5.876	5.875	
Area, ft²:	0.044		Volume, ft³:		0.022

Moisture Content (%)		Unit Weight	
Can Number:	40H	Wet Weight of Sample (g):	1059.43
Wt. of Can (g):	30.93	Dry Weight of Sample (g):	689.65
Wt. of Wet Soil + Can (g):	82.73	Wet Unit Weight (pcf):	107.9
Wt. of Dry Soil + Can (g):	64.65	Dry Unit Weight (pcf):	70.2
Wt. of Dry Soil (g):	33.72		
Wt. of Water (g):	18.08		
Moisture Content (%):	53.6		

Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS: 2.242	1.14	0.57

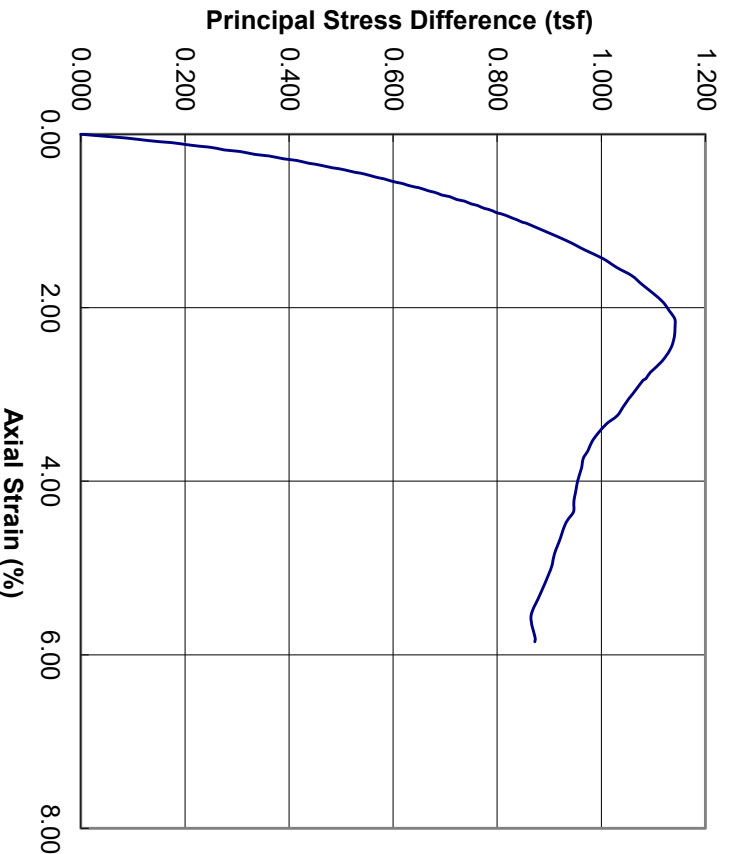
FAILURE TYPE:



NOTE:

Tested By: DD
 Computed By: RB
 Reviewed By:

Stress-Strain



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	8/27/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-5	Calipers:	01CAL254
Samp. and Spec. No.:	S-1 (B)	Oven:	10OV251
Sample Depth:	0-2'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	TV = 0.05	Loadcell:	16COM254
Sample Description:	DKGR/BK OH W/ PEAT	Air Pressure (psi):	0.4

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.714	2.734	2.782	2.743	2.10
Height (in)	5.768	5.773	5.778	5.773	
	Area, ft ² :	0.041	Volume, ft ³ :	0.020	

Moisture Content (%)		Unit Weight	
Can Number:	22	Wet Weight of Sample (g):	705.94
Wt. of Can (g):	30.77	Dry Weight of Sample (g):	272.88
Wt. of Wet Soil + Can (g):	85.02	Wet Unit Weight (pcf):	78.8
Wt. of Dry Soil + Can (g):	51.74	Dry Unit Weight (pcf):	30.5
Wt. of Dry Soil (g):	20.97		
Wt. of Water (g):	33.28		
Moisture Content (%):	158.7		

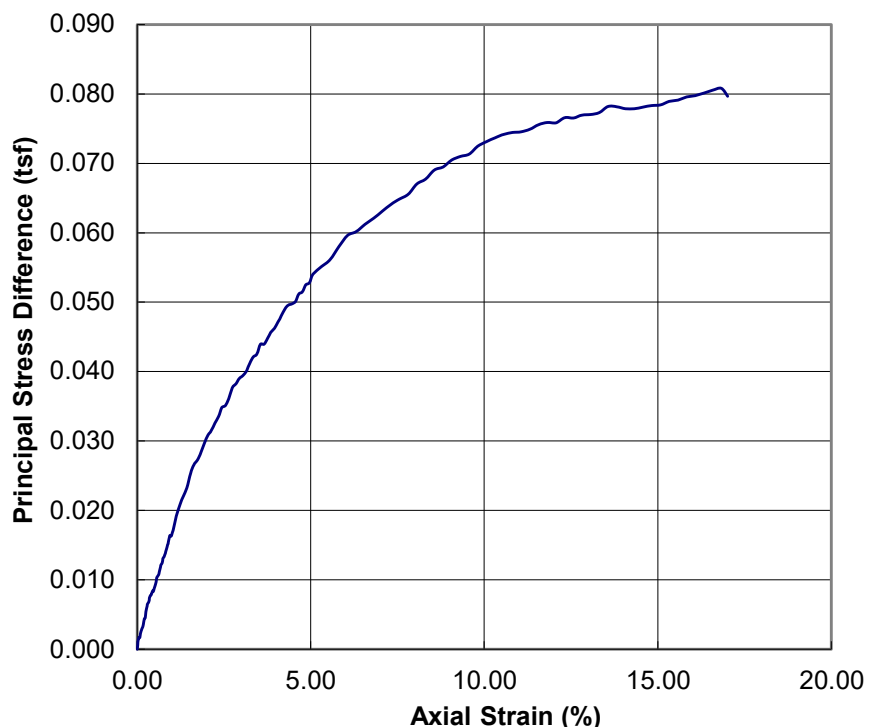
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.081	0.08	0.04

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	8/27/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-5	Calipers:	01CAL254
Samp. and Spec. No.:	S-5 (B)	Oven:	100OV251
Sample Depth:	8-10'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	2.25 / 3	Loadcell:	12COM254
Sample Description:	LT GR CL W/ SA POCKETS	Air Pressure (psi):	3.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.859	2.848	2.843	2.850	2.04
Height (in)	5.807	5.797	5.799	5.801	
Area, ft²:	0.044		Volume, ft³:	0.021	

Moisture Content (%)		Unit Weight	
Can Number:	25L	Wet Weight of Sample (g):	1231.16
Wt. of Can (g):	31.28	Dry Weight of Sample (g):	1026.04
Wt. of Wet Soil + Can (g):	84.76	Wet Unit Weight (pcf):	126.7
Wt. of Dry Soil + Can (g):	75.85	Dry Unit Weight (pcf):	105.6
Wt. of Dry Soil (g):	44.57		
Wt. of Water (g):	8.91		
Moisture Content (%):	20.0		

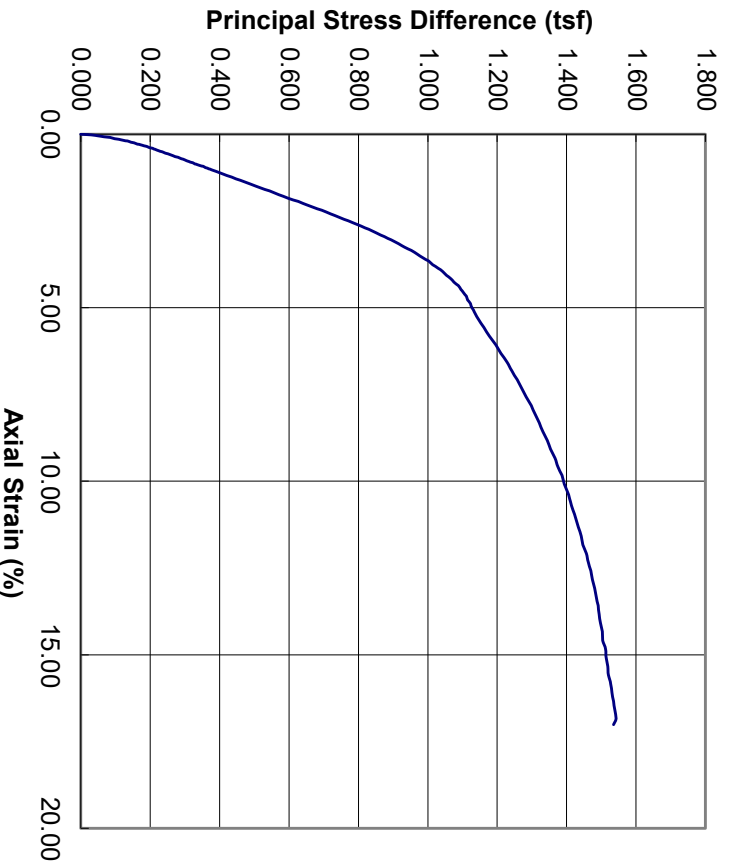
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.084	1.51
		0.76

FAILURE TYPE:

NOTE:



Stress-Strain



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	8/27/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-5	Calipers:	01CAL254
Samp. and Spec. No.:	S-8 (B)	Oven:	10OV251
Sample Depth:	14-16'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	1.5 / 3	Loadcell:	12COM254
Sample Description:	LTGR/BR CL W/ SA	Air Pressure (psi):	6.0

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.896	2.864	2.868	2.876	2.01
Height (in)	5.787	5.789	5.784	5.787	
	Area, ft ² :	0.045	Volume, ft ³ :	0.022	

Moisture Content (%)		Unit Weight	
Can Number:	L-12	Wet Weight of Sample (g):	1143.31
Wt. of Can (g):	31.57	Dry Weight of Sample (g):	813.79
Wt. of Wet Soil + Can (g):	82.47	Wet Unit Weight (pcf):	115.9
Wt. of Dry Soil + Can (g):	67.80	Dry Unit Weight (pcf):	82.5
Wt. of Dry Soil (g):	36.23		
Wt. of Water (g):	14.67		
Moisture Content (%):	40.5		

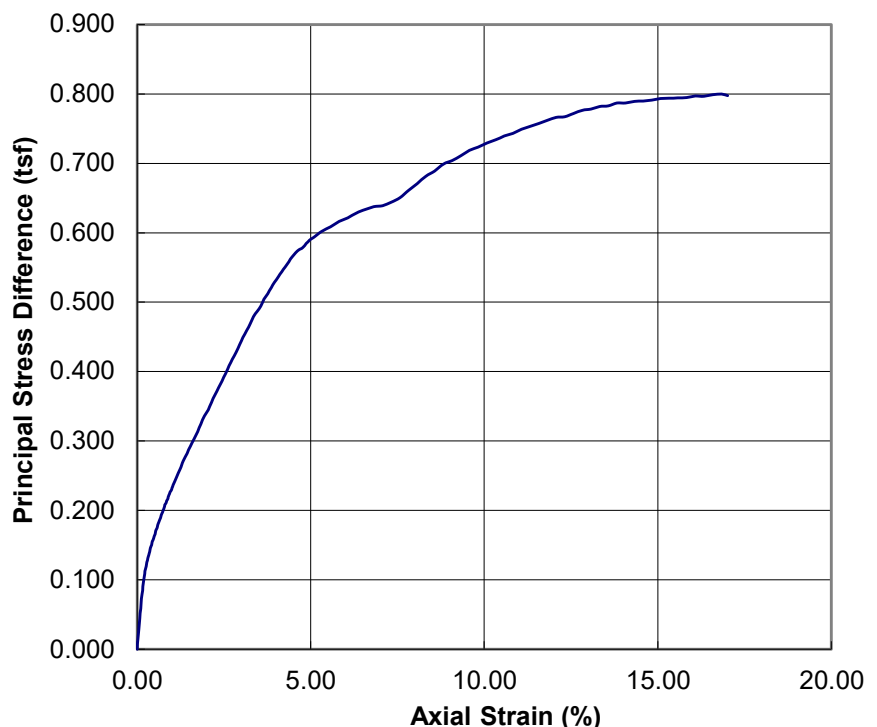
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.076	0.79	0.40

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	8/27/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-5	Calipers:	01CAL254
Samp. and Spec. No.:	S-12	Oven:	100OV251
Sample Depth:	28-30'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	0.75 / 3 TV = 0.55	Loadcell:	12COM254
Sample Description:	GR/BR CH W/ TR SA	Air Pressure (psi):	11.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.846	2.868	2.862	2.859	2.00
Height (in)	5.718	5.733	5.737	5.729	
Area, ft²:	0.045	Volume, ft³:		0.021	

Moisture Content (%)		Unit Weight	
Can Number:	L-31	Wet Weight of Sample (g):	1018.87
Wt. of Can (g):	31.34	Dry Weight of Sample (g):	672.26
Wt. of Wet Soil + Can (g):	81.90	Wet Unit Weight (pcf):	105.6
Wt. of Dry Soil + Can (g):	64.70	Dry Unit Weight (pcf):	69.6
Wt. of Dry Soil (g):	33.36		
Wt. of Water (g):	17.20		
Moisture Content (%):	51.6		

Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS: 4.461	0.66	0.33

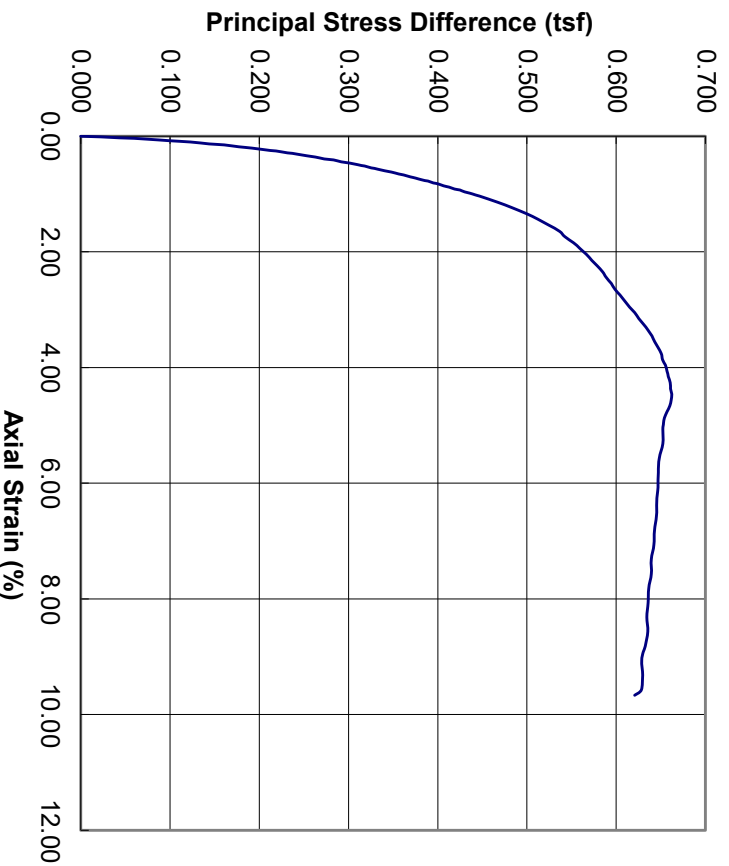
FAILURE TYPE:



NOTE: Slickenside

Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

Stress-Strain



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	9/8/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-6	Calipers:	01CAL254
Samp. and Spec. No.:	S-2 (B)	Oven:	10OV251
Sample Depth:	2-4'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	0.25 / 3 TV = 0.15	Loadcell:	17COM254
Sample Description:	GR CL W/ SA	Air Pressure (psi):	1.2

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.799	2.855	2.794	2.816	2.00
Height (in)	5.638	5.630	5.628	5.632	
	Area, ft ² :	0.043	Volume, ft ³ :	0.020	

Moisture Content (%)		Unit Weight	
Can Number:	33H	Wet Weight of Sample (g):	1199.82
Wt. of Can (g):	30.84	Dry Weight of Sample (g):	974.26
Wt. of Wet Soil + Can (g):	83.82	Wet Unit Weight (pcf):	130.3
Wt. of Dry Soil + Can (g):	73.86	Dry Unit Weight (pcf):	105.8
Wt. of Dry Soil (g):	43.02		
Wt. of Water (g):	9.96		
Moisture Content (%):	23.2		

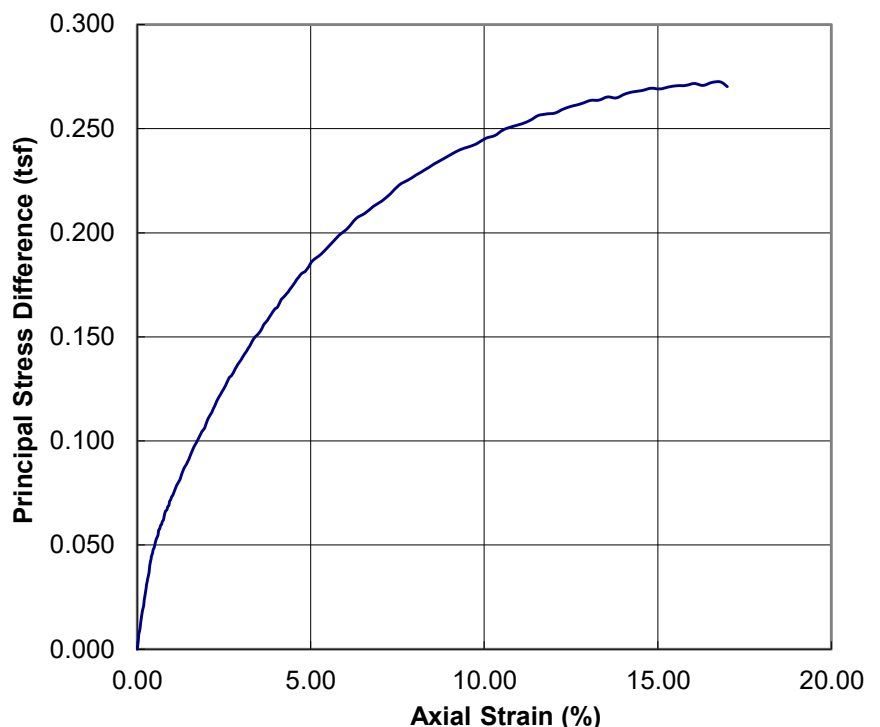
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.056	0.27	0.13

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: RB
 Computed By: RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name: CPRA PO 181
Project No.: 02541139
Boring No.: M-6
Samp. and Spec. No.: S-5 (B)
Sample Depth: 8-10'
Qp / Torvane (tsf): 1.25 / 3
Sample Description: GR/BR CL W/ SA & SA PARTINGS

Date Tested: 9/8/2020
Scale: 03ES254
Calipers: 01CAL254
Oven: 100V251
Geolac Station No.: 02GJ254
Loadcell: 12COM254
Air Pressure (psi): 3.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.846	2.828	2.824	2.833	1.90
Height (in)	5.364	5.373	5.375	5.371	
Area, ft²:	0.044	Volume, ft³:	0.020		

Moisture Content (%)		Unit Weight	
Can Number:	S-36	Wet Weight of Sample (g):	1059.44
Wt. of Can (g):	31.70	Dry Weight of Sample (g):	791.84
Wt. of Wet Soil + Can (g):	83.92	Wet Unit Weight (pcf):	119.2
Wt. of Dry Soil + Can (g):	70.73	Dry Unit Weight (pcf):	89.1
Wt. of Dry Soil (g):	39.03		
Wt. of Water (g):	13.19		
Moisture Content (%):	33.8		

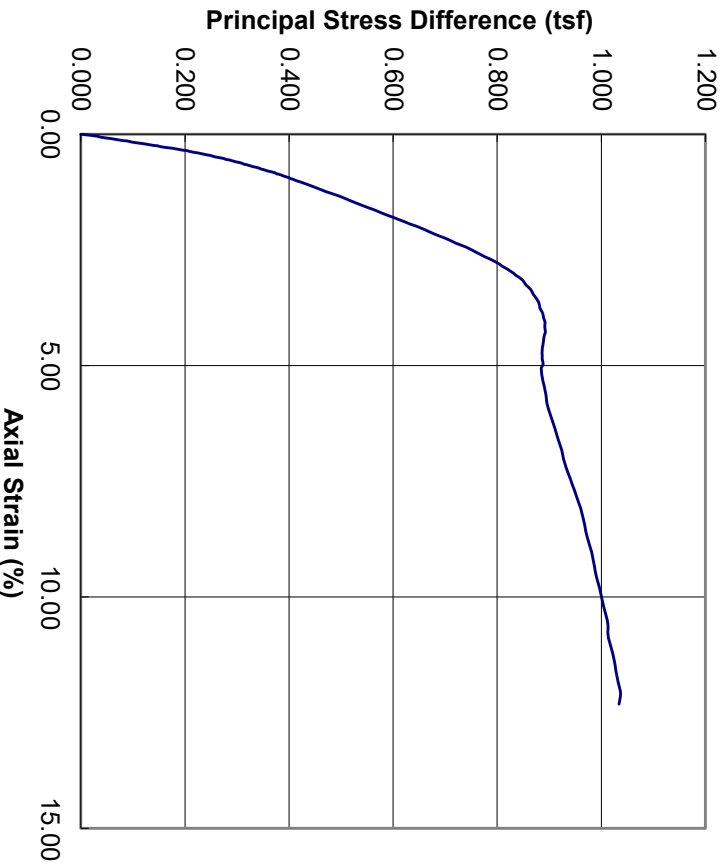
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS: 4.269	0.89	0.45

FAILURE TYPE:



NOTE:

Stress-Strain



Tested By: RB
Computed By: RB
Reviewed By:

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	9/8/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-6	Calipers:	01CAL254
Samp. and Spec. No.:	S-7 (B)	Oven:	10OV251
Sample Depth:	12-14'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	1.5 / 3	Loadcell:	12COM254
Sample Description:	LT GR/BR CL W/ SA & SA PARTINGS	Air Pressure (psi):	5.2

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.867	2.839	2.877	2.861	2.03
Height (in)	5.812	5.796	5.798	5.802	
	Area, ft ² :	0.045	Volume, ft ³ :	0.022	

Moisture Content (%)		Unit Weight	
Can Number:	246-10	Wet Weight of Sample (g):	1097.92
Wt. of Can (g):	30.08	Dry Weight of Sample (g):	803.51
Wt. of Wet Soil + Can (g):	81.58	Wet Unit Weight (pcf):	112.1
Wt. of Dry Soil + Can (g):	67.77	Dry Unit Weight (pcf):	82.1
Wt. of Dry Soil (g):	37.69		
Wt. of Water (g):	13.81		
Moisture Content (%):	36.6		

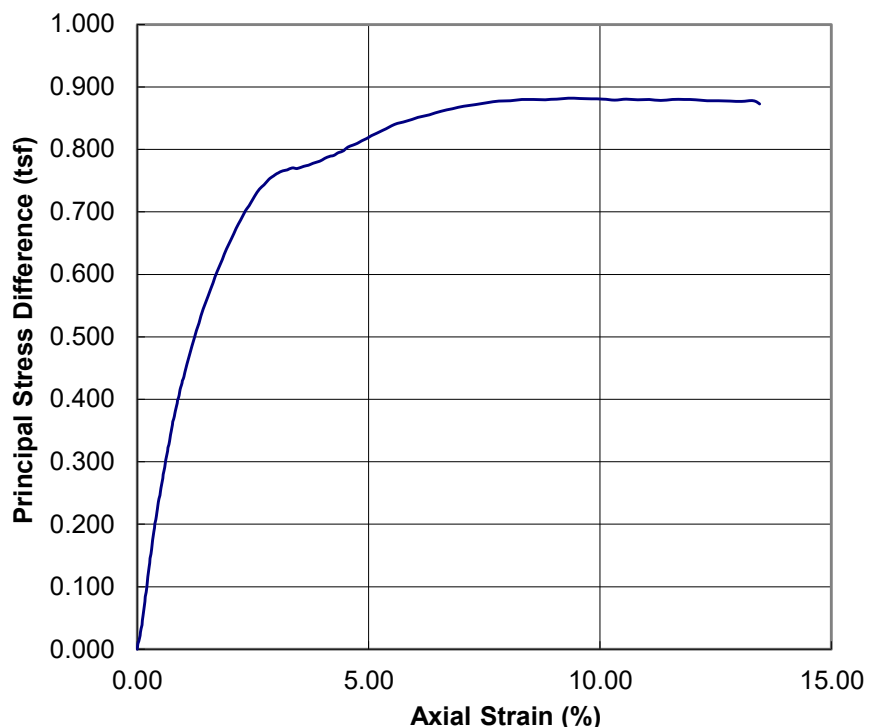
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	9.317	0.88	0.44

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	9/8/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-6	Calipers:	01CAL254
Samp. and Spec. No.:	S-10 (B)	Oven:	100OV251
Sample Depth:	18-20'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	1.25 / 3	Loadcell:	17COM254
Sample Description:	GR CH W/ SI PARTINGS	Air Pressure (psi):	7.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.869	2.850	2.824	2.848	2.00
Height (in)	5.700	5.712	5.703	5.705	
Area, ft²:	0.044	Volume, ft³:		0.021	

Moisture Content (%)		Unit Weight	
Can Number:	S-2	Wet Weight of Sample (g):	1064.99
Wt. of Can (g):	30.79	Dry Weight of Sample (g):	740.74
Wt. of Wet Soil + Can (g):	81.60	Wet Unit Weight (pcf):	111.7
Wt. of Dry Soil + Can (g):	66.13	Dry Unit Weight (pcf):	77.7
Wt. of Dry Soil (g):	35.34		
Wt. of Water (g):	15.47		
Moisture Content (%):	43.8		

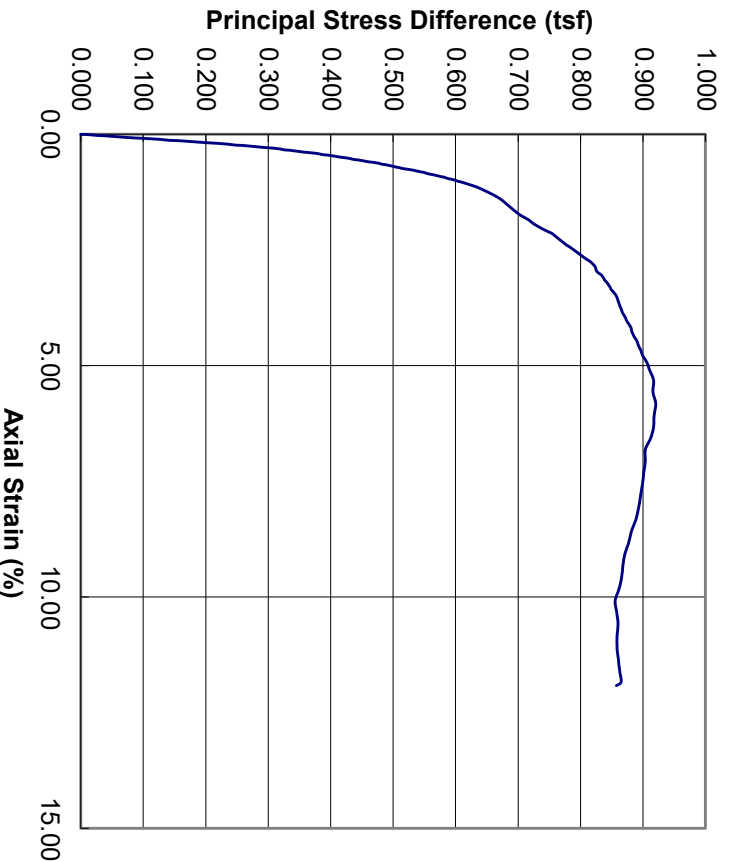
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	5.823	0.92
		0.46

FAILURE TYPE:



NOTE:

Stress-Strain



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	9/8/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-7	Calipers:	01CAL254
Samp. and Spec. No.:	S-1 (M)	Oven:	10OV251
Sample Depth:	0-2'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	TV = 0.05	Loadcell:	16COM254
Sample Description:	DKGR OH W/ SA	Air Pressure (psi):	0.4

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.795	2.812	2.842	2.816	2.03
Height (in)	5.713	5.708	5.716	5.712	
	Area, ft ² :	0.043	Volume, ft ³ :	0.021	

Moisture Content (%)		Unit Weight	
Can Number:	246-2	Wet Weight of Sample (g):	1018.98
Wt. of Can (g):	29.94	Dry Weight of Sample (g):	768.20
Wt. of Wet Soil + Can (g):	81.99	Wet Unit Weight (pcf):	109.1
Wt. of Dry Soil + Can (g):	69.18	Dry Unit Weight (pcf):	82.2
Wt. of Dry Soil (g):	39.24		
Wt. of Water (g):	12.81		
Moisture Content (%):	32.6		

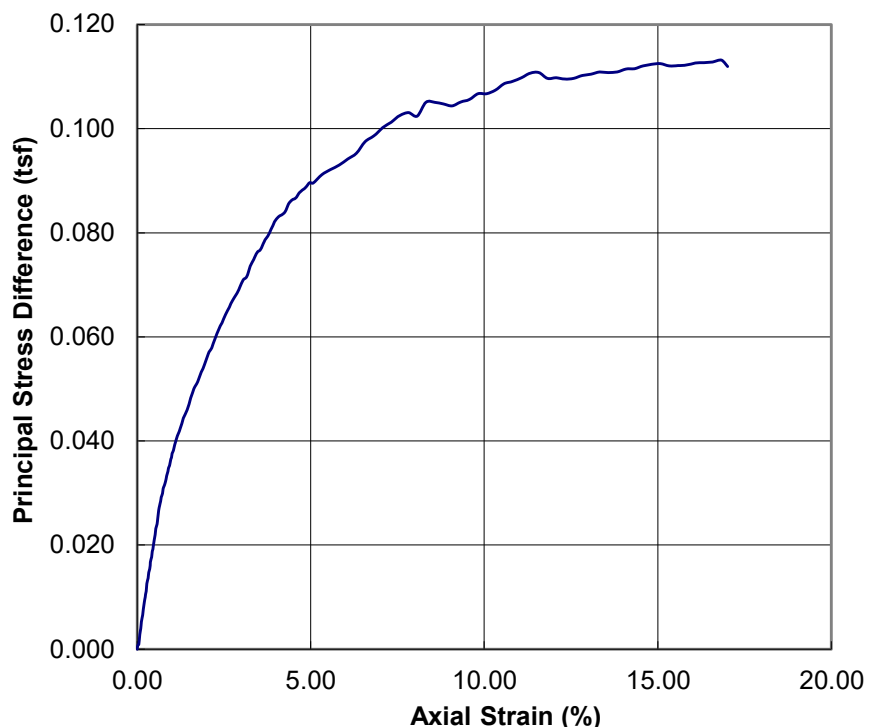
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.081	0.11	0.06

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	9/9/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-7	Calipers:	01CAL254
Samp. and Spec. No.:	S-4 (B)	Oven:	100OV251
Sample Depth:	6-8'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	TV = 0.975	Loadcell:	12COM254
Sample Description:	LTGR CL W/ SA	Air Pressure (psi):	2.8

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.838	2.851	2.845	2.845	2.03
Height (in)	5.788	5.788	5.783	5.786	
Area, ft²:	0.044	Volume, ft³:		0.021	

Moisture Content (%)		Unit Weight	
Can Number:	L-27	Wet Weight of Sample (g):	1197.79
Wt. of Can (g):	31.23	Dry Weight of Sample (g):	963.01
Wt. of Wet Soil + Can (g):	83.88	Wet Unit Weight (pcf):	124.1
Wt. of Dry Soil + Can (g):	73.56	Dry Unit Weight (pcf):	99.8
Wt. of Dry Soil (g):	42.33		
Wt. of Water (g):	10.32		
Moisture Content (%):	24.4		

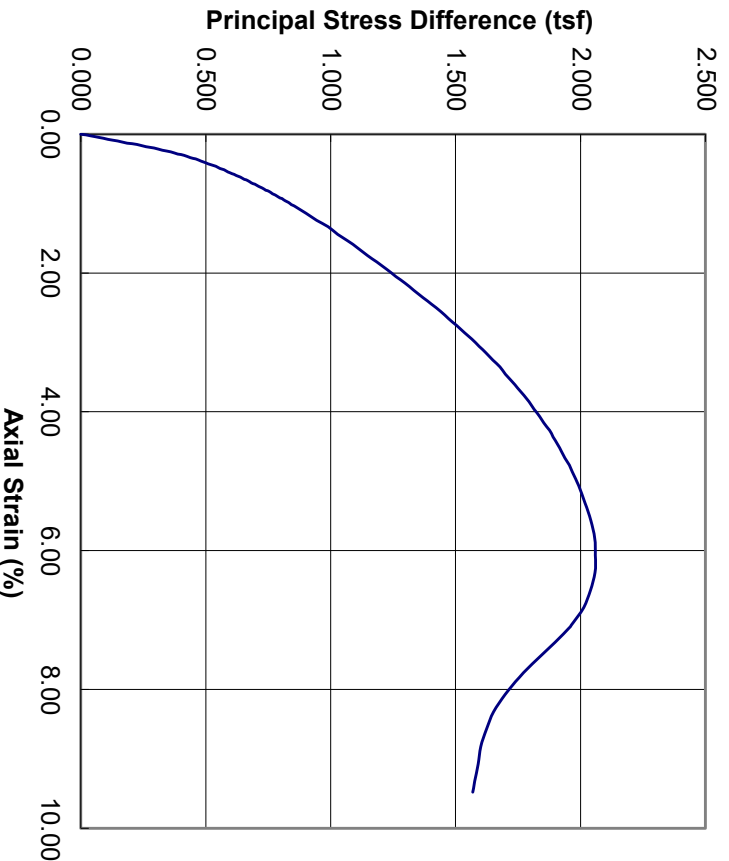
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	6.069	2.06
		1.03

FAILURE TYPE:



NOTE:

Stress-Strain



Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	9/9/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-7	Calipers:	01CAL254
Samp. and Spec. No.:	S-6 (B)	Oven:	10OV251
Sample Depth:	10-12'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	1.75 / 3 TV = 0.50	Loadcell:	12COM254
Sample Description:	LTGR CL W/ SA & SA PARTINGS	Air Pressure (psi):	4.4

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.876	2.854	2.875	2.868	1.88
Height (in)	5.390	5.385	5.380	5.385	
	Area, ft ² :	0.045	Volume, ft ³ :	0.020	

Moisture Content (%)		Unit Weight	
Can Number:	71	Wet Weight of Sample (g):	1090.34
Wt. of Can (g):	31.03	Dry Weight of Sample (g):	842.49
Wt. of Wet Soil + Can (g):	84.92	Wet Unit Weight (pcf):	119.4
Wt. of Dry Soil + Can (g):	72.67	Dry Unit Weight (pcf):	92.2
Wt. of Dry Soil (g):	41.64		
Wt. of Water (g):	12.25		
Moisture Content (%):	29.4		

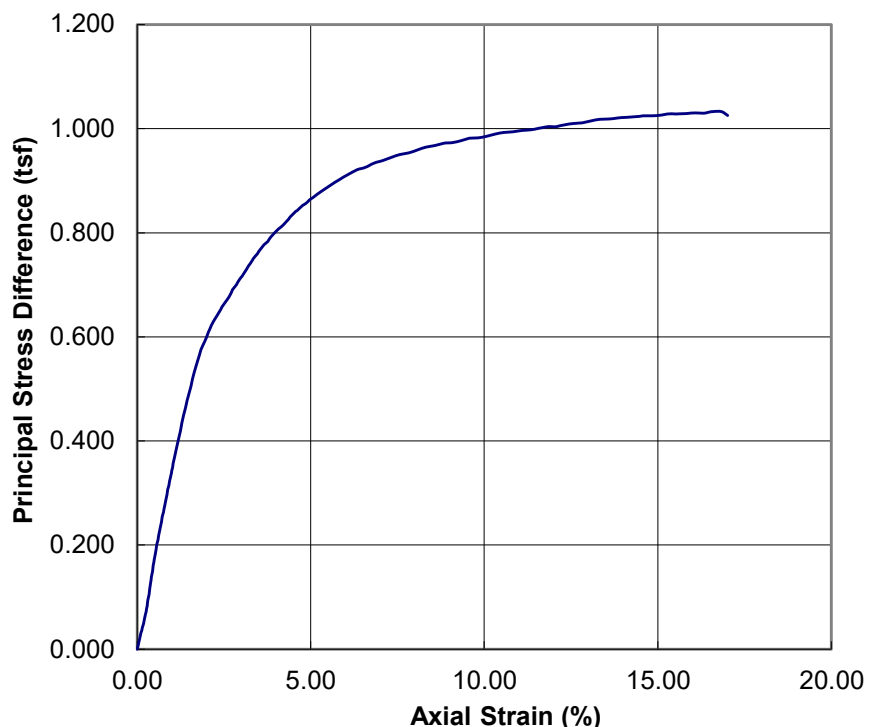
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	15.084	1.03	0.51

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	9/9/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-7	Calipers:	01CAL254
Samp. and Spec. No.:	S-9 (B)	Oven:	10OV251
Sample Depth:	16-18'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	TV = 0.875	Loadcell:	12COM254
Sample Description:	GR/BR CH W/ SI PARTINGS	Air Pressure (psi):	6.8

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.846	2.813	2.844	2.834	2.03
Height (in)	5.749	5.740	5.738	5.742	
	Area, ft ² :	0.044	Volume, ft ³ :	0.021	

Moisture Content (%)		Unit Weight	
Can Number:	DB-18	Wet Weight of Sample (g):	1078.72
Wt. of Can (g):	31.39	Dry Weight of Sample (g):	767.36
Wt. of Wet Soil + Can (g):	83.60	Wet Unit Weight (pcf):	113.4
Wt. of Dry Soil + Can (g):	68.53	Dry Unit Weight (pcf):	80.7
Wt. of Dry Soil (g):	37.14		
Wt. of Water (g):	15.07		
Moisture Content (%):	40.6		

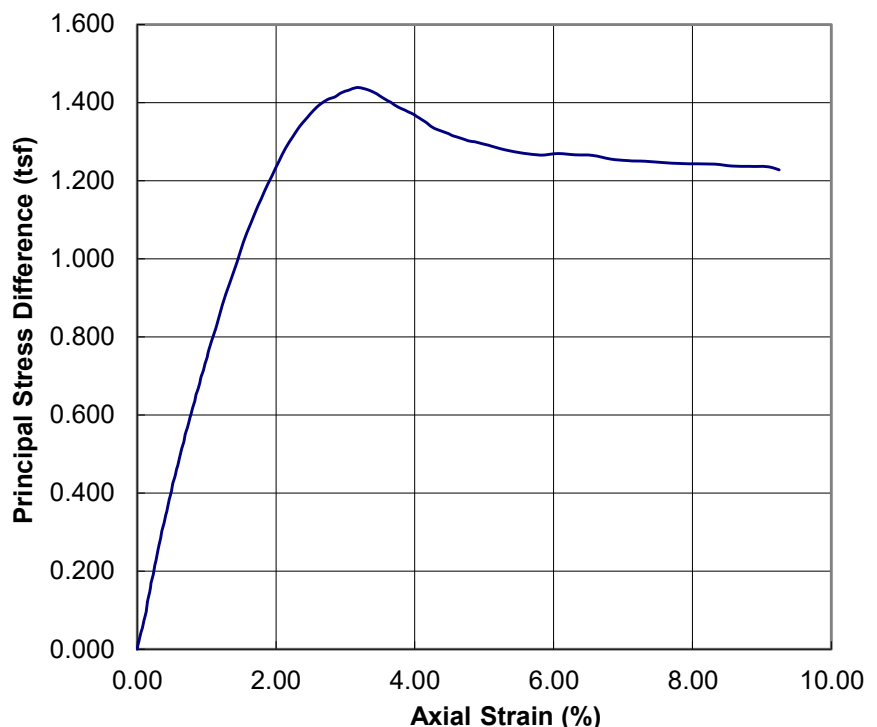
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	3.154	1.44	0.72

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST **ASTM D 2850**

Project Name:	CPRA PO 181	Date Tested:	9/25/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-7	Calipers:	01CAL254
Samp. and Spec. No.:	S-13 (M)	Oven:	100V251
Sample Depth:	33-35'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	TV = 0.275	Loadcell:	17COM254
Sample Description:	GR CH	Air Pressure (psi):	13.6

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.817	2.865	2.838	2.840	2.01
Height (in)	5.690	5.696	5.701	5.696	
Area, ft²:	0.044		Volume, ft³:		0.021

Moisture Content (%)		Unit Weight	
Can Number:	31H	Wet Weight of Sample (g):	969.55
Wt. of Can (g):	31.03	Dry Weight of Sample (g):	595.03
Wt. of Wet Soil + Can (g):	83.53	Wet Unit Weight (pcf):	102.4
Wt. of Dry Soil + Can (g):	63.25	Dry Unit Weight (pcf):	62.8
Wt. of Dry Soil (g):	32.22		
Wt. of Water (g):	20.28		
Moisture Content (%):	62.9		

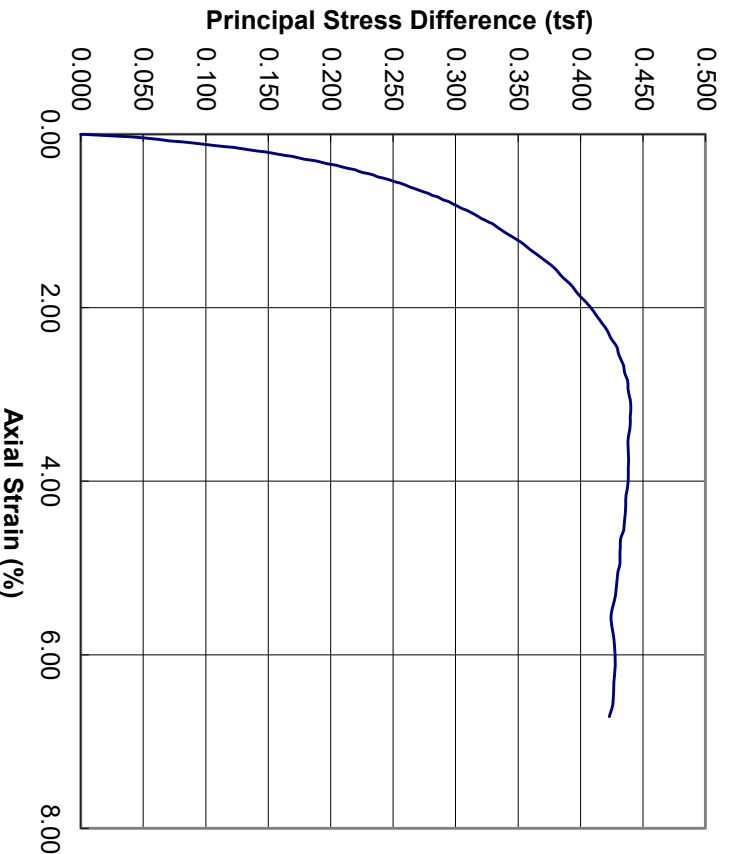
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	3.153	0.44
		0.22

FAILURE TYPE:



NOTE:

Stress-Strain



UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	9/9/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-8	Calipers:	01CAL254
Samp. and Spec. No.:	S-7 (M)	Oven:	10OV251
Sample Depth:	12-14'	GeoJac Station No.:	02GJ254
Qp / Torvane (tsf):	1.75 / 3	Loadcell:	12COM254
Sample Description:	LTGR CL W/ SA	Air Pressure (psi):	5.2

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.844	2.865	2.835	2.848	2.03
Height (in)	5.773	5.769	5.769	5.770	
	Area, ft ² :	0.044	Volume, ft ³ :	0.021	

Moisture Content (%)		Unit Weight	
Can Number:	36	Wet Weight of Sample (g):	1184.56
Wt. of Can (g):	30.92	Dry Weight of Sample (g):	915.86
Wt. of Wet Soil + Can (g):	82.19	Wet Unit Weight (pcf):	122.8
Wt. of Dry Soil + Can (g):	70.56	Dry Unit Weight (pcf):	94.9
Wt. of Dry Soil (g):	39.64		
Wt. of Water (g):	11.63		
Moisture Content (%):	29.3		

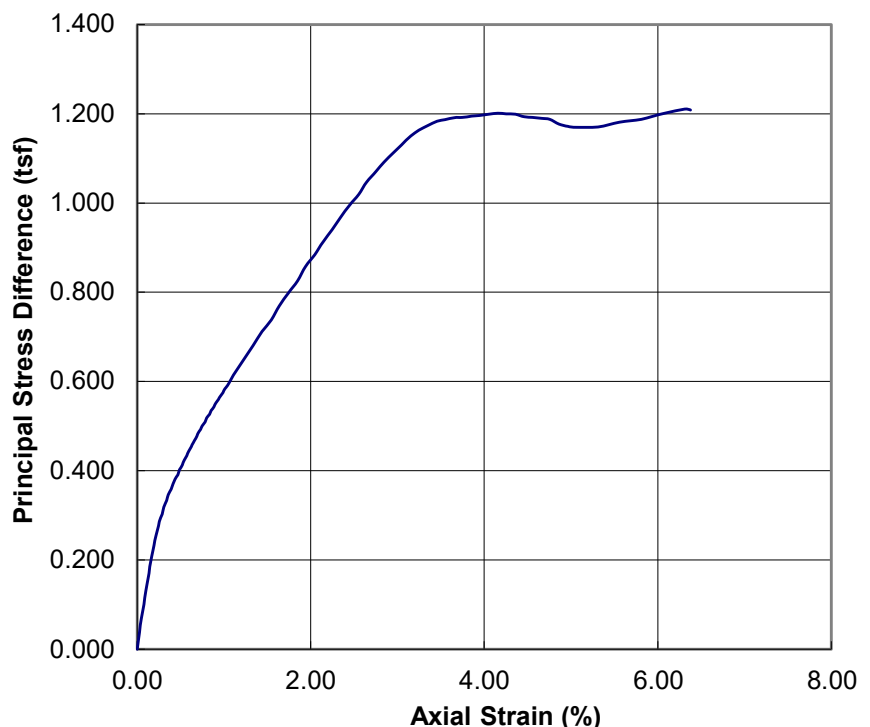
	Max Strain ($\leq 15\%$)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	4.359	1.20	0.60

FAILURE TYPE:



NOTE: _____

Stress-Strain



Tested By: RB
 Computed By: RB
 Reviewed By: _____

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

ASTM D 2850

Project Name:	CPRA PO 181	Date Tested:	9/9/2020
Project No.:	02541139	Scale:	03ES254
Boring No.:	M-8	Calipers:	01CAL254
Samp. and Spec. No.:	S-9 (B)	Oven:	100OV251
Sample Depth:	16-18'	Geolac Station No.:	02GJ254
Qp / Torvane (tsf):	1.25 / 3	Loadcell:	12COM254
Sample Description:	LTGR CH W/ SI PARTINGS	Air Pressure (psi):	6.8

Sample Data:	1	2	3	Average	Height / Diameter Ratio (2 to 2.5)
Diameter (in)	2.831	2.852	2.806	2.830	
Height (in)	5.778	5.784	5.781	5.781	2.04
Area, ft²:	0.044		Volume, ft³:	0.021	

Moisture Content (%)		Unit Weight	
Can Number:	S-23	Wet Weight of Sample (g):	1104.90
Wt. of Can (g):	30.91	Dry Weight of Sample (g):	769.98
Wt. of Wet Soil + Can (g):	81.55	Wet Unit Weight (pcf):	115.8
Wt. of Dry Soil + Can (g):	66.20	Dry Unit Weight (pcf):	80.7
Wt. of Dry Soil (g):	35.29		
Wt. of Water (g):	15.35		
Moisture Content (%):	43.5		

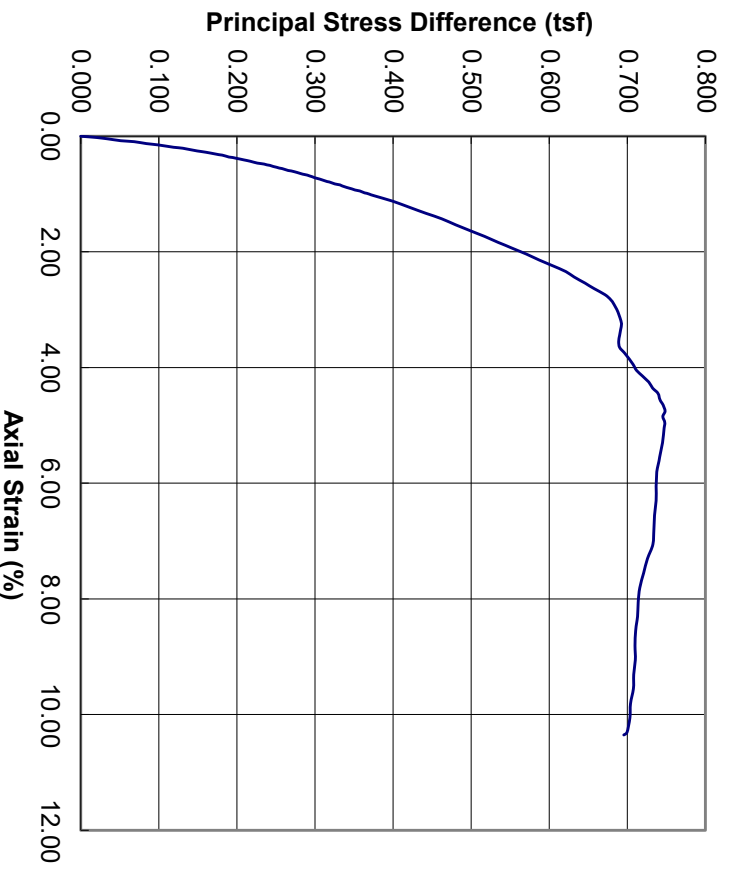
Max Strain (< 15%)	Compressive Strength, tsf	Shear Strength, tsf
RESULTS:	3.244	0.69
		0.35

FAILURE TYPE:



NOTE:

Stress-Strain

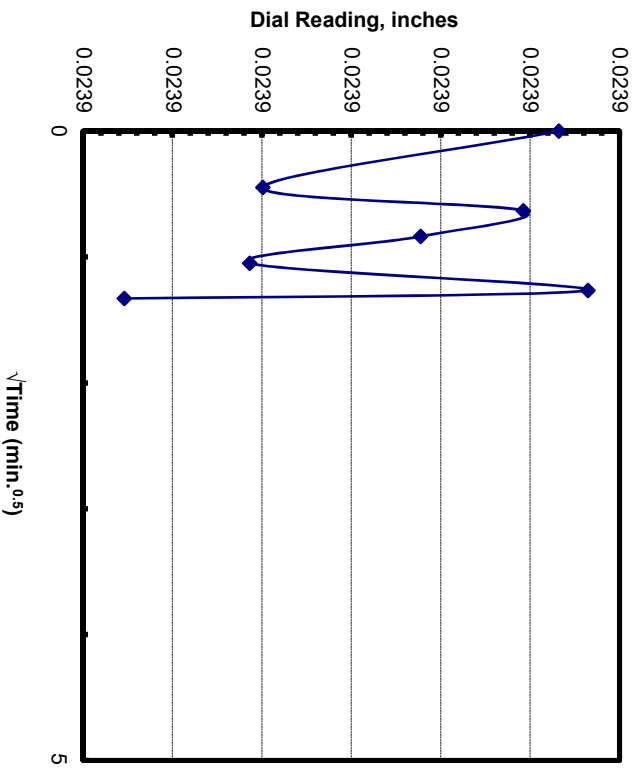
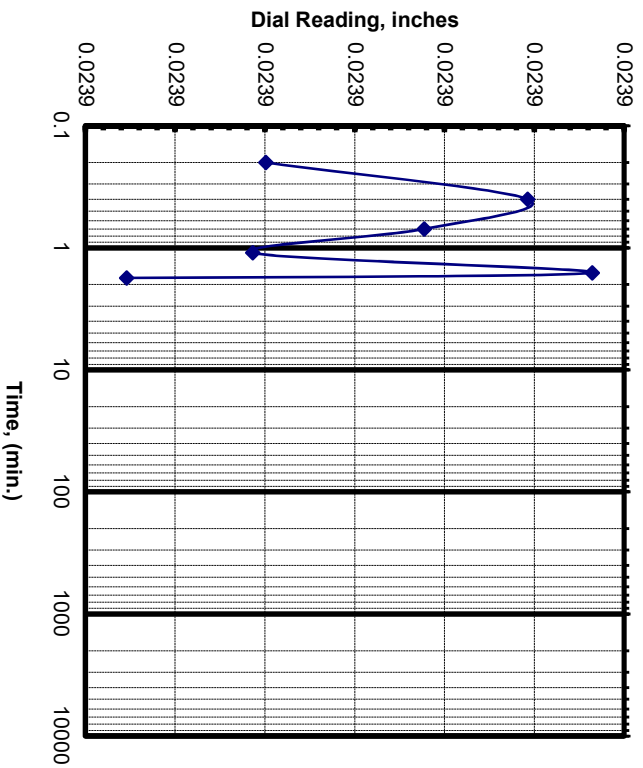


Tested By: _____ RB
 Computed By: _____ RB
 Reviewed By: _____

APPENDIX E

CONSOLIDATION TEST RESULTS SETTLING COLUMN TEST AND SLURRY CONSOLIDATION TEST RESULTS

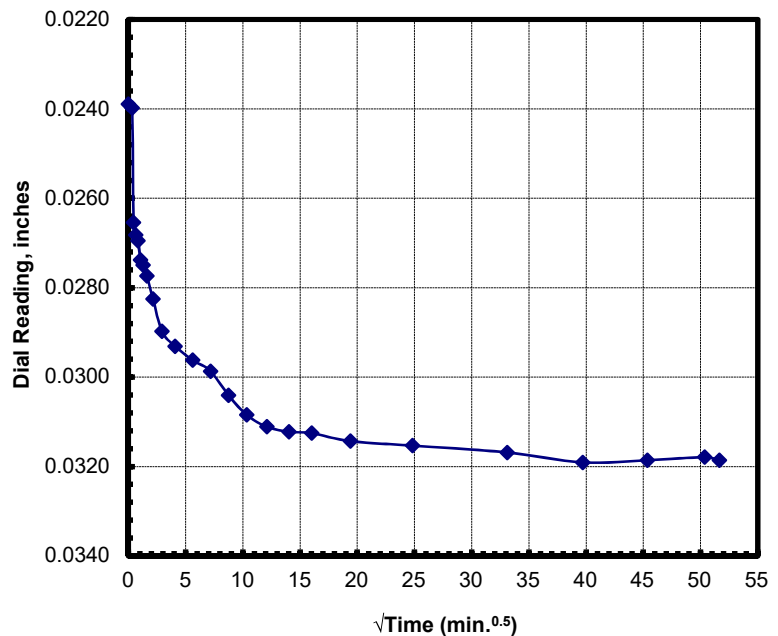
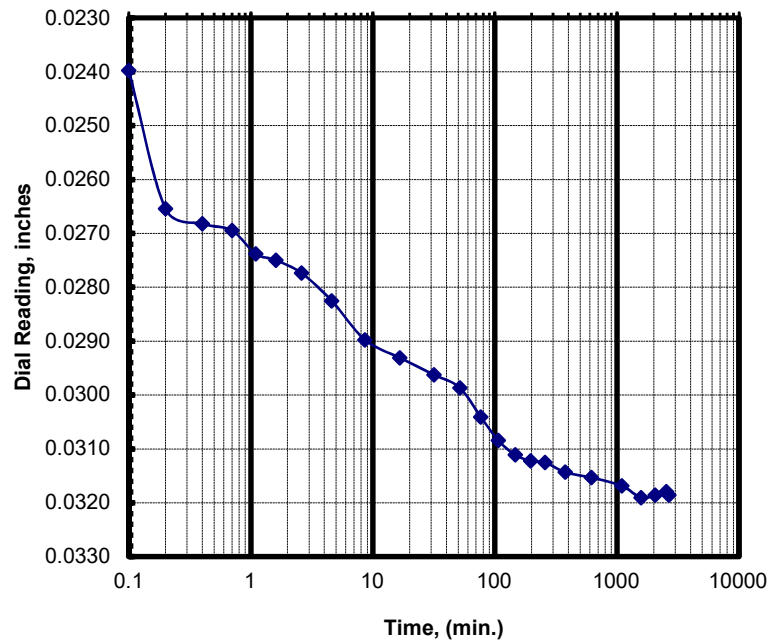
**CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.05 tsf**

[illegible]

Project Number:	02541139	LL: 33	D _{60mm} (in): 0.0239	d ₀ (in): 0.0239
Project Name:	CPRA PO 181	PL: 13	D _{end} (in): 0.0239	d ₁₀₀ (in): 0.0239
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δ _h (in.): 0.0000	δ _{exp} (in.): 0.0000
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min):	t ₅₀ (min):

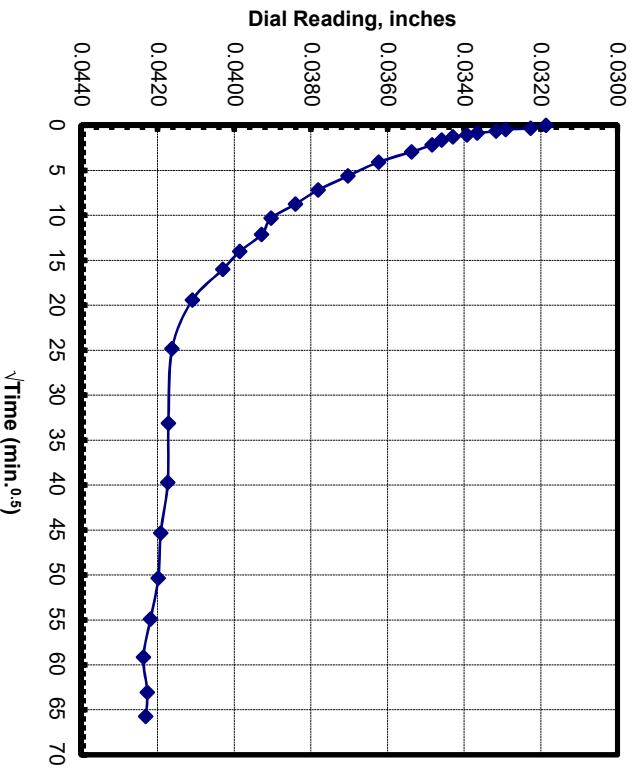
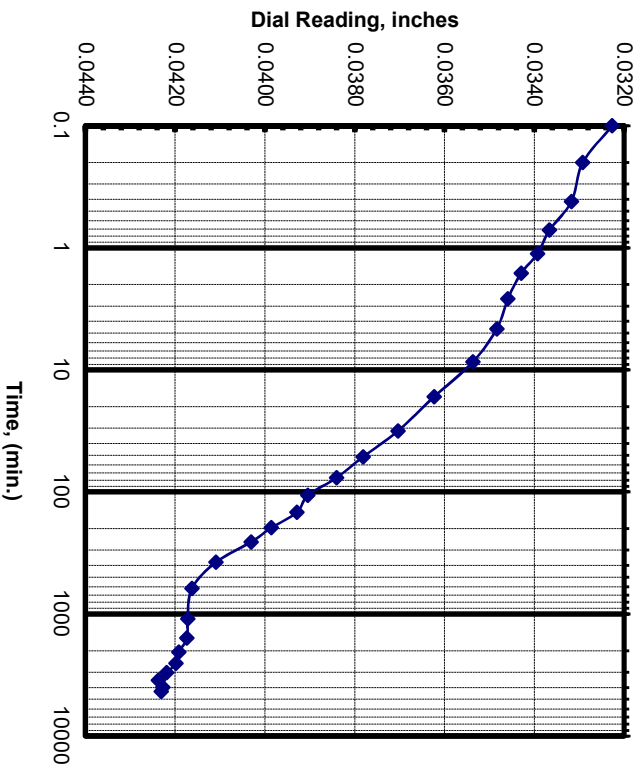
t50 & t90 Curves for the Vertical Stress of 0.125 tsf

Project Number:	02541139
Project Name:	CPRA PO 181
Boring & Depth:	M-1, S-3, 4-6 ft
Sample:	Light Gray SANDY LEAN CLAY



Project Number:	02541139	LL: 33	D _{0(min)} (in): 0.0239	d ₀ (in): 0.0239
Project Name:	CPRA PO 181	PL: 13	D _{end} (in): 0.0319	d ₁₀₀ (in): 0.0291
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δ _{hl} (in.): 0.0080	δ _{exp} (in.): 0.0052
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 0.2	t ₉₀ (min): 6

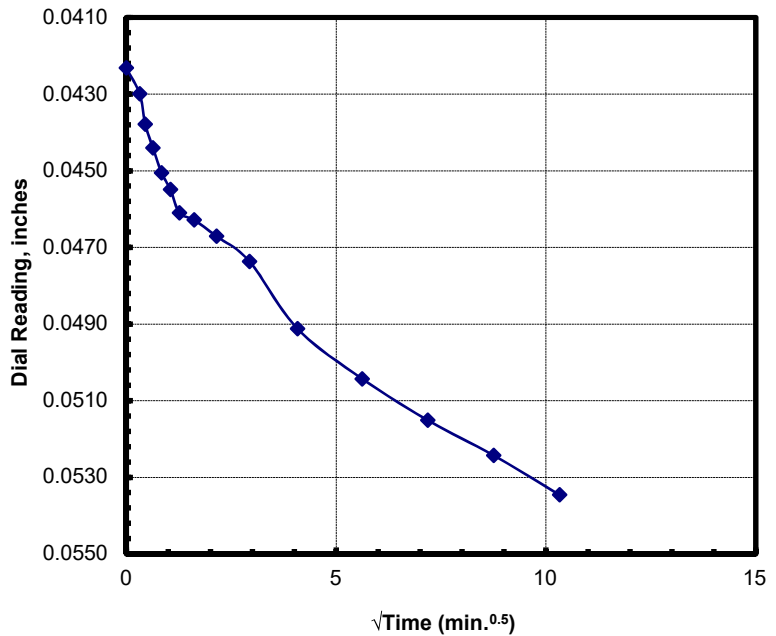
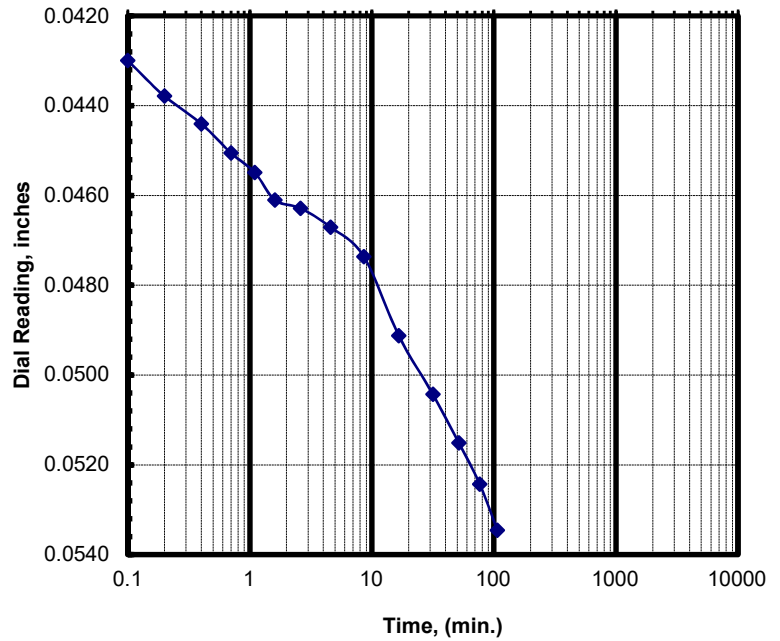
CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.25 tsf

[illegible]

Project Number:	02541139	LL: 33	D _{liquid} (in): 0.0319	d ₀ (in): 0.0319
Project Name:	CPRA PO 181	PL: 13	D _{end} (in): 0.0423	d ₁₀₀ (in): 0.0388
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δ _h (in.): 0.0104	δ _{deep} (in.): 0.0069
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 8.5	t ₉₀ (min): 61

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.5 tsf

Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
11/10/20 8:30:17 AM	0.00	0.00	0.0423
11/10/20 8:30:23 AM	0.10	0.32	0.0430
11/10/20 8:30:29 AM	0.20	0.45	0.0438
11/10/20 8:30:41 AM	0.40	0.63	0.0444
11/10/20 8:30:59 AM	0.70	0.84	0.0451
11/10/20 8:31:23 AM	1.10	1.05	0.0455
11/10/20 8:31:53 AM	1.60	1.26	0.0461
11/10/20 8:32:53 AM	2.60	1.61	0.0463
11/10/20 8:34:53 AM	4.60	2.14	0.0467
11/10/20 8:38:53 AM	8.60	2.93	0.0474
11/10/20 8:46:53 AM	16.60	4.07	0.0491
11/10/20 9:01:53 AM	31.60	5.62	0.0504
11/10/20 9:21:54 AM	51.62	7.18	0.0515
11/10/20 9:46:54 AM	76.62	8.75	0.0524
11/10/20 10:16:54 AM	106.62	10.33	0.0535

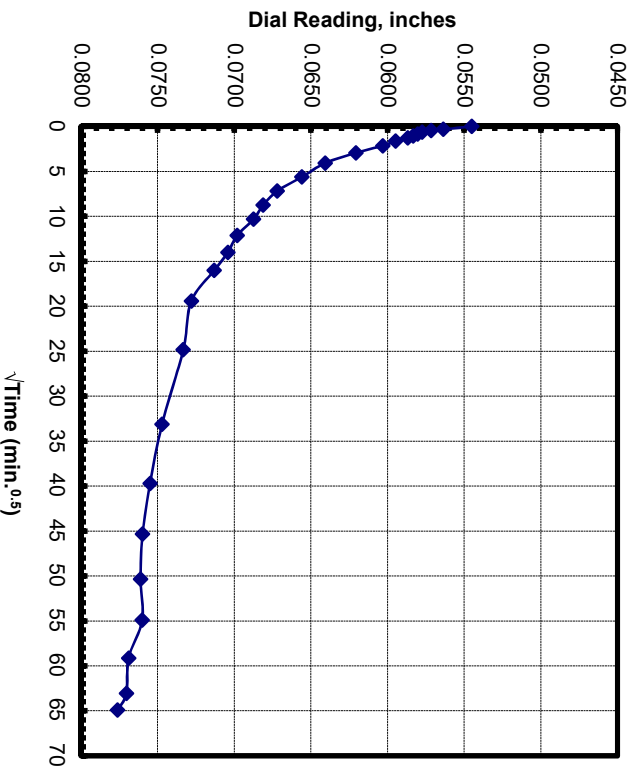
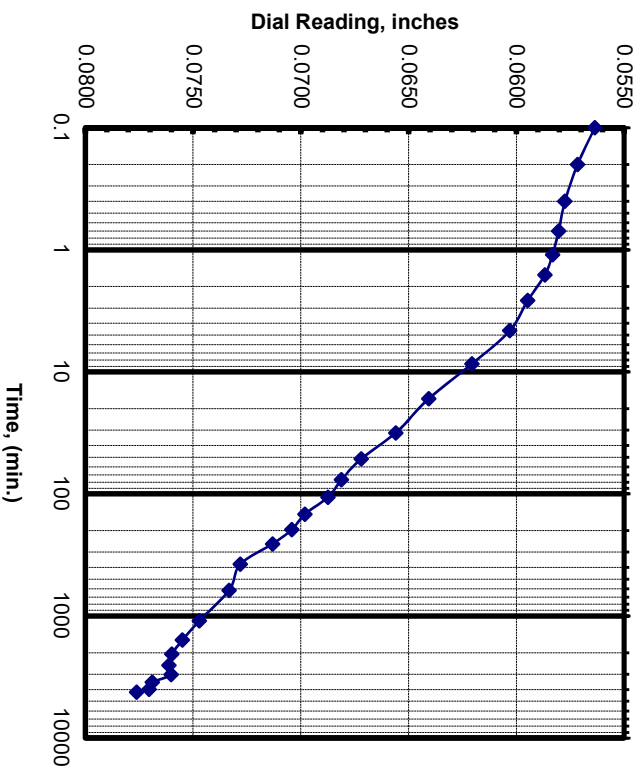


Project Number:	02541139	LL: 33	D _{0(min)} (in): 0.0423	d ₀ (in): 0.0423
Project Name:	CPRA PO 181	PL: 13	D _{end} (in): 0.0535	d ₁₀₀ (in): 0.0496
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δ _{hi} (in.): 0.0111	δ _{exp} (in.): 0.0073
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 1.5	t ₉₀ (min): 16

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02541139 M-1 S-3 (M) 4-6'

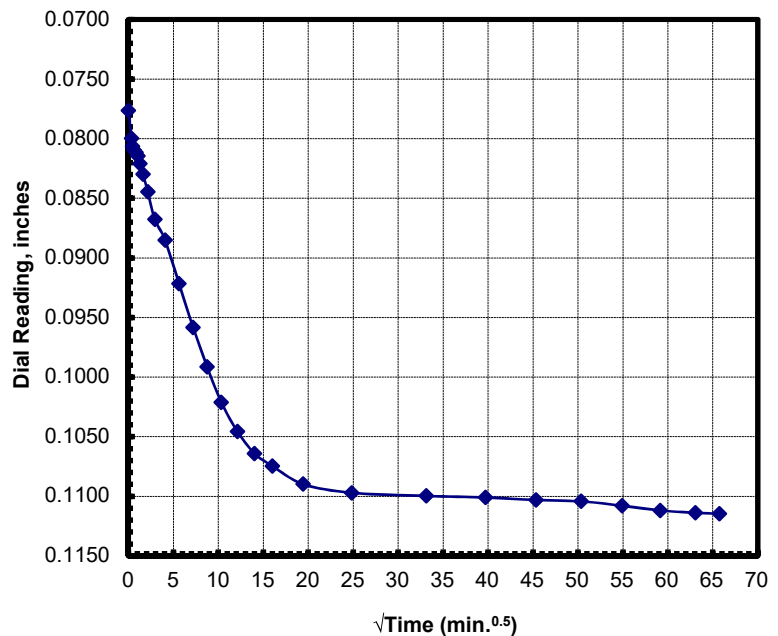
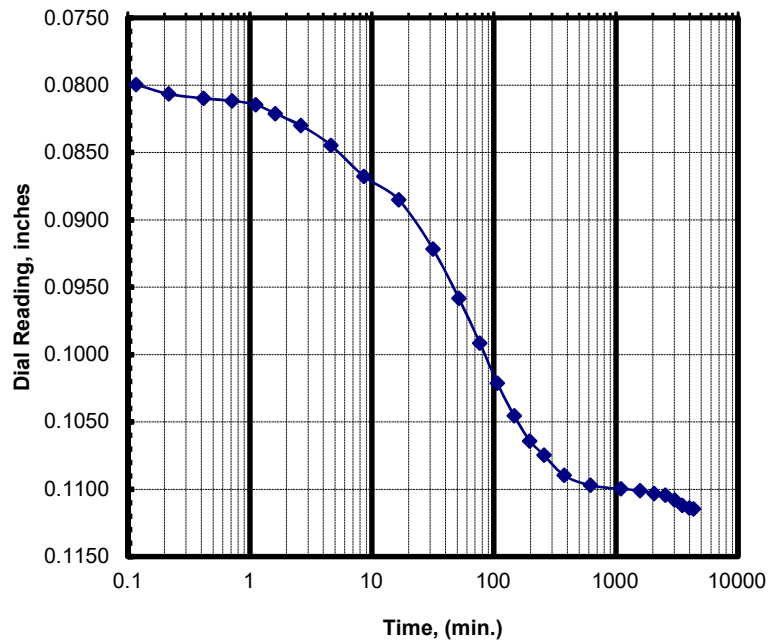
t50 & t90 Curves for the Vertical Stress of 1 tsf

[illegible]

Project Number:	02541139	LL: 33	D _{60(mm)} (in): 0.0545	d ₆ (in): 0.0545
Project Name:	CPRA PO 181	PL: 13	D _{and} (in): 0.0776	d ₁₀₀ (in): 0.0730
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δ _{hi} (in.): 0.0231	δ _{deep} (in.): 0.0185
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 15	t ₅₀ (min): 260

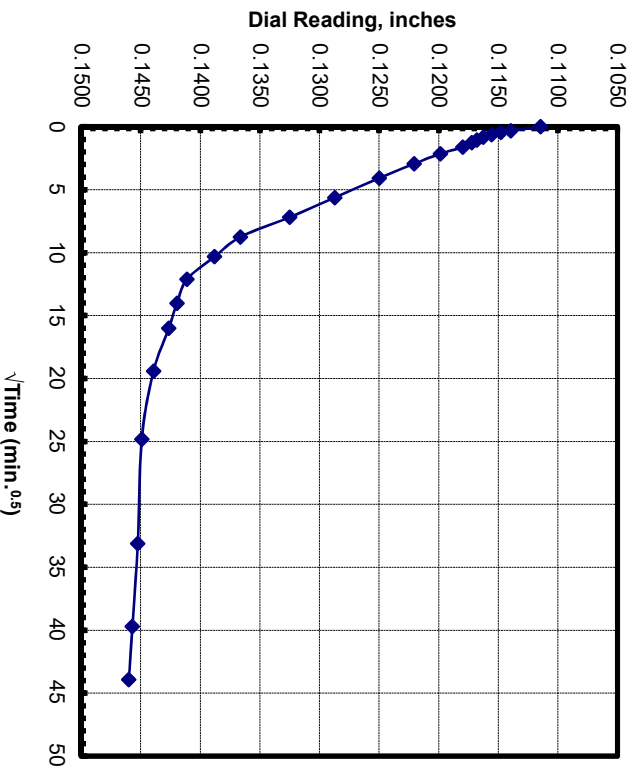
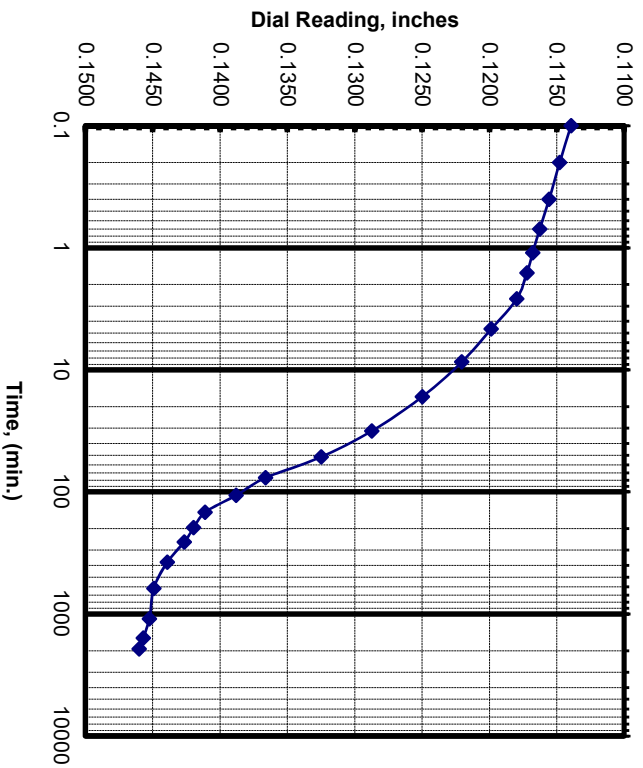
CONSOLIDATION TEST RESULTS - ASTM D 2435 ***t₅₀ & t₉₀ Curves for the Vertical Stress of 2 tsf***

Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
11/13/20 8:42:42 AM	0.00	0.00	0.0776
11/13/20 8:42:49 AM	0.12	0.34	0.0800
11/13/20 8:42:55 AM	0.22	0.47	0.0807
11/13/20 8:43:07 AM	0.42	0.65	0.0810
11/13/20 8:43:25 AM	0.72	0.85	0.0812
11/13/20 8:43:49 AM	1.12	1.06	0.0814
11/13/20 8:44:19 AM	1.62	1.27	0.0821
11/13/20 8:45:19 AM	2.62	1.62	0.0830
11/13/20 8:47:19 AM	4.62	2.15	0.0845
11/13/20 8:51:19 AM	8.62	2.94	0.0868
11/13/20 8:59:19 AM	16.62	4.08	0.0885
11/13/20 9:14:19 AM	31.62	5.62	0.0922
11/13/20 9:34:19 AM	51.62	7.18	0.0958
11/13/20 9:59:19 AM	76.62	8.75	0.0991
11/13/20 10:29:19 AM	106.62	10.33	0.1021
11/13/20 11:09:20 AM	146.63	12.11	0.1046
11/13/20 11:59:20 AM	196.63	14.02	0.1064
11/13/20 12:59:20 PM	256.63	16.02	0.1075
11/13/20 2:59:20 PM	376.63	19.41	0.1090
11/13/20 6:59:20 PM	616.63	24.83	0.1097
11/14/20 2:59:21 AM	1096.65	33.12	0.1100
11/14/20 10:59:21 AM	1576.65	39.71	0.1101
11/14/20 6:59:22 PM	2056.67	45.35	0.1103
11/15/20 2:59:22 AM	2536.67	50.37	0.1104
11/15/20 10:59:23 AM	3016.68	54.92	0.1108
11/15/20 6:59:23 PM	3496.68	59.13	0.1112
11/16/20 2:59:24 AM	3976.70	63.06	0.1114
11/16/20 8:42:43 AM	4320.02	65.73	0.1115



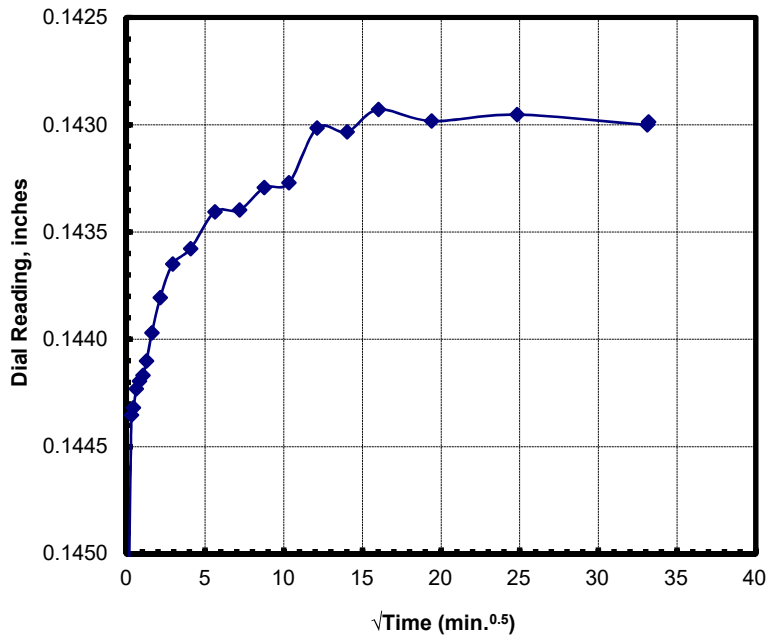
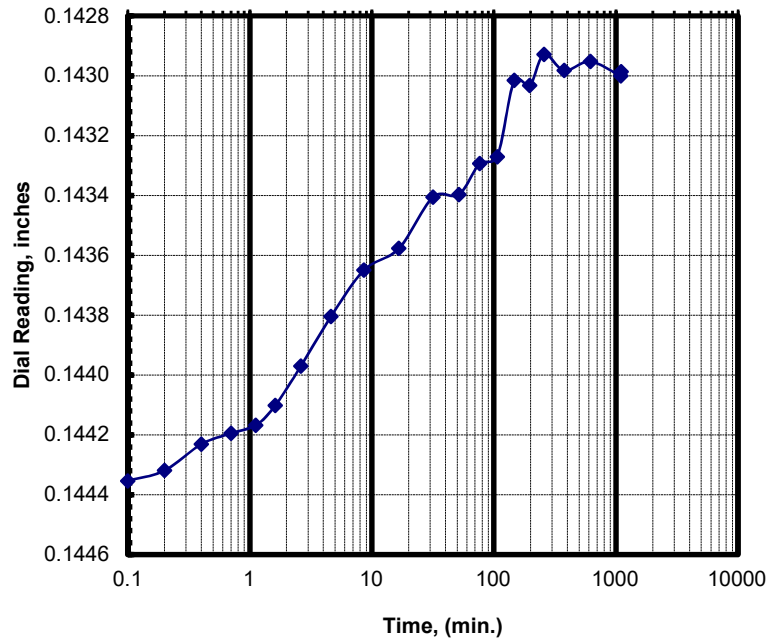
Project Number:	02541139	LL: 33	D_{0(min)} (in): 0.0776	d₀ (in): 0.0776
Project Name:	CPRA PO 181	PL: 13	D_{end} (in): 0.1115	d₁₀₀ (in): 0.1091
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δ_h (in.): 0.0338	δ_{eop} (in.): 0.0315
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t₅₀ (min): 39	t₉₀ (min): 185

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

Project Number:	02541139	LL: 33	D _{90(gm)} (in): 0.1115	d ₀ (in): 0.1115
Project Name:	CPRA PO 181	PL: 13	D _{end} (in): 0.1460	d ₁₀₀ (in): 0.1438
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δ _{hi} (in.): 0.0345	δ _{deep} (in.): 0.0323
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 27	t ₉₀ (min): 130

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

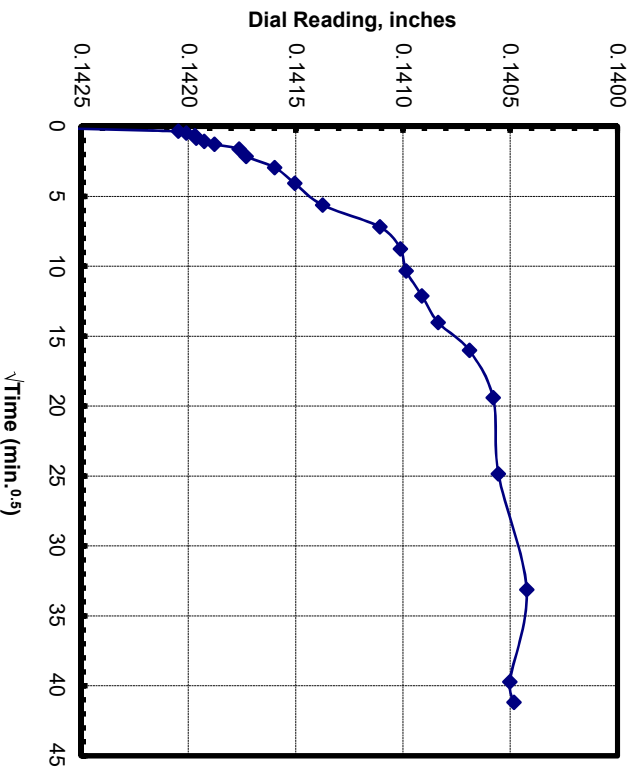
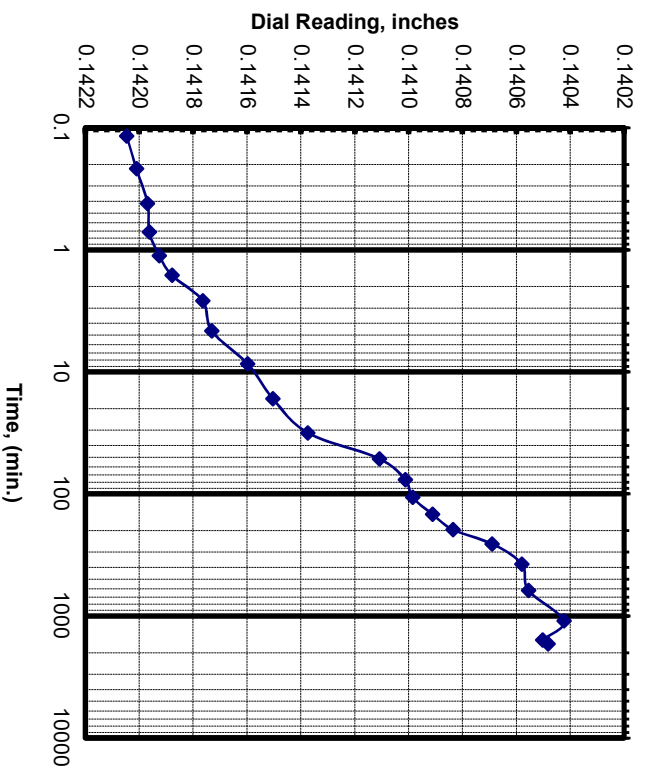
[illegible]

Project Number: 02541139	LL: 33	D _{0(min)} (in): 0.1460	d ₀ (in): 0.1460
Project Name: CPRA PO 181	PL: 13	D _{end} (in): 0.1430	d ₁₀₀ (in): 0.1430
Boring & Depth: M-1, S-3, 4-6 ft	PL: 20	Δ _{hl} (in.): -0.0030	δ _{exp} (in.): -0.0030
Sample: Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

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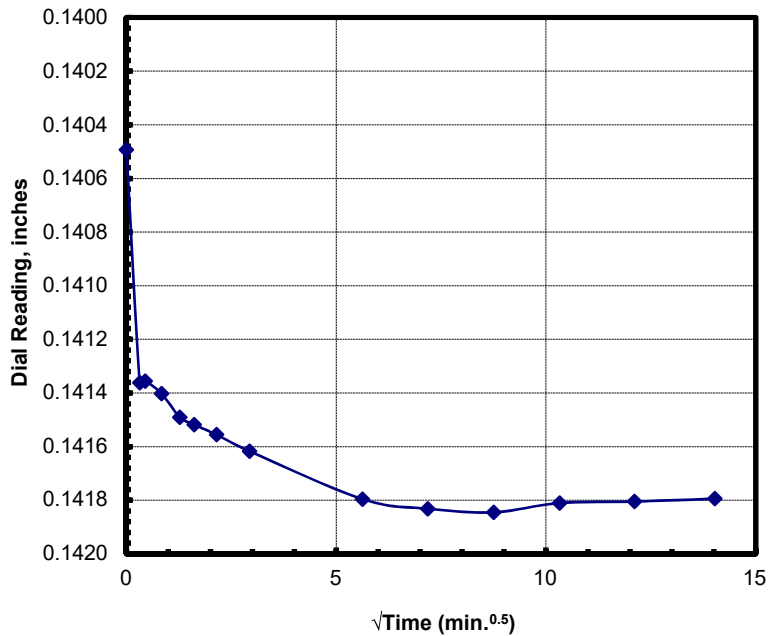
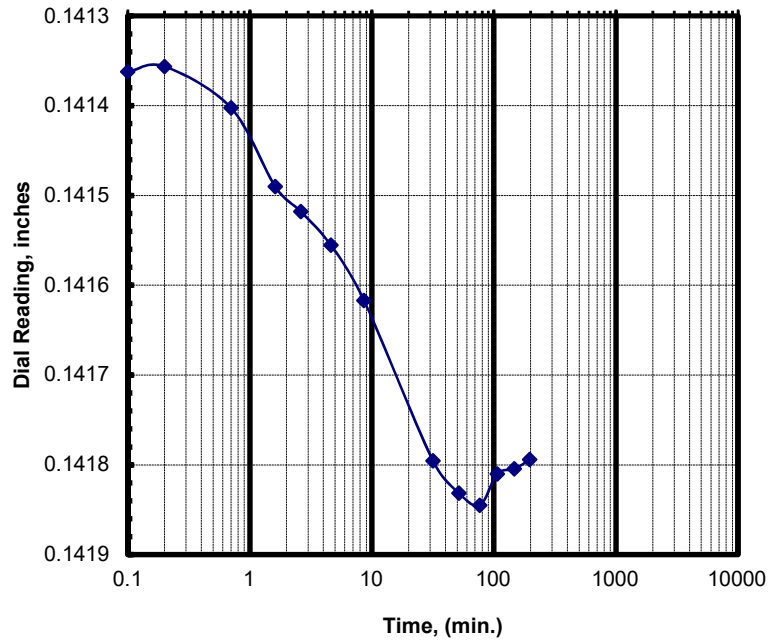
02541139 M-1 S-3 (M) 4-6'

t50 & t90 Curves for the Vertical Stress of 1 tsf

[illegible]

Project Number:	02541139	LL: 33	D _{60(mm)} (in): 0.1430	d ₆ (in): 0.1430
Project Name:	CPRA PO 181	PL: 13	D ₁₀₀ (in): 0.1405	d ₁₀₀ (in): 0.1405
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δh ₁ (in.): -0.0025	Δ _{deep} (in.): -0.0025
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min):	t ₅₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 2 tsf

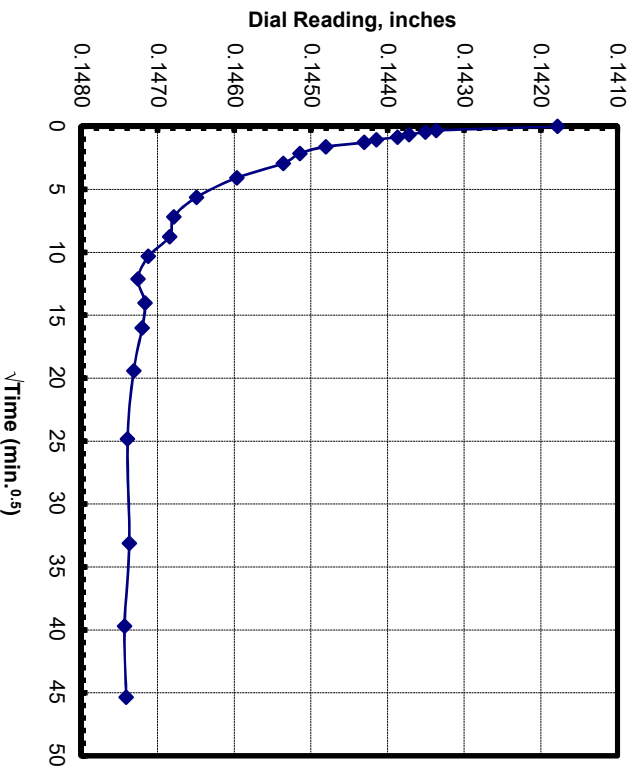
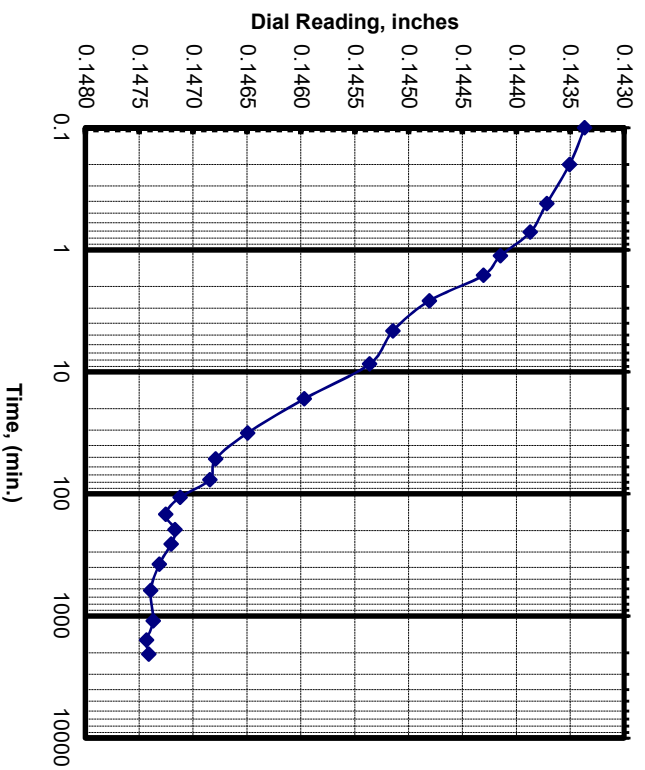
[illegible]

Project Number:	02541139	LL: 33	D _{0(min)} (in): 0.1405	d ₀ (in): 0.1405
Project Name:	CPRA PO 181	PL: 13	D _{end} (in): 0.1418	d ₁₀₀ (in): 0.1418
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δ _{hi} (in.): 0.0013	δ _{eop} (in.): 0.0013
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

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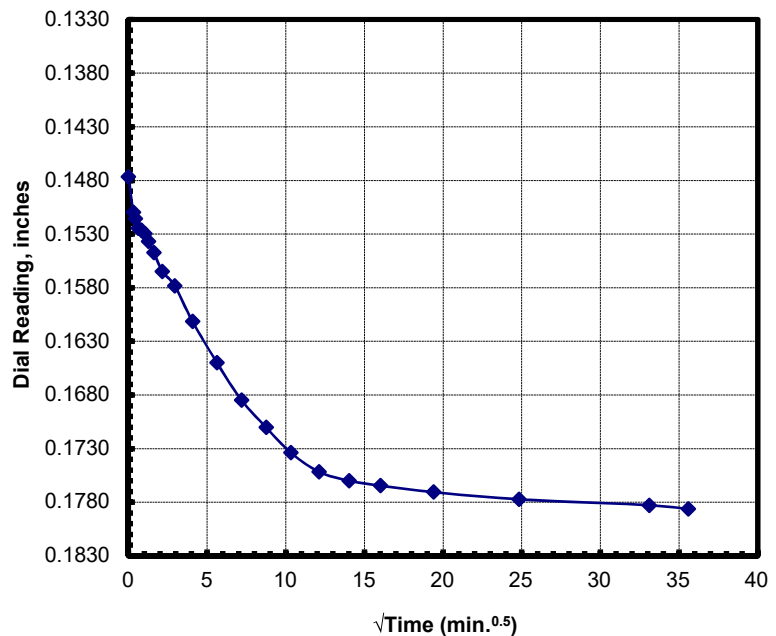
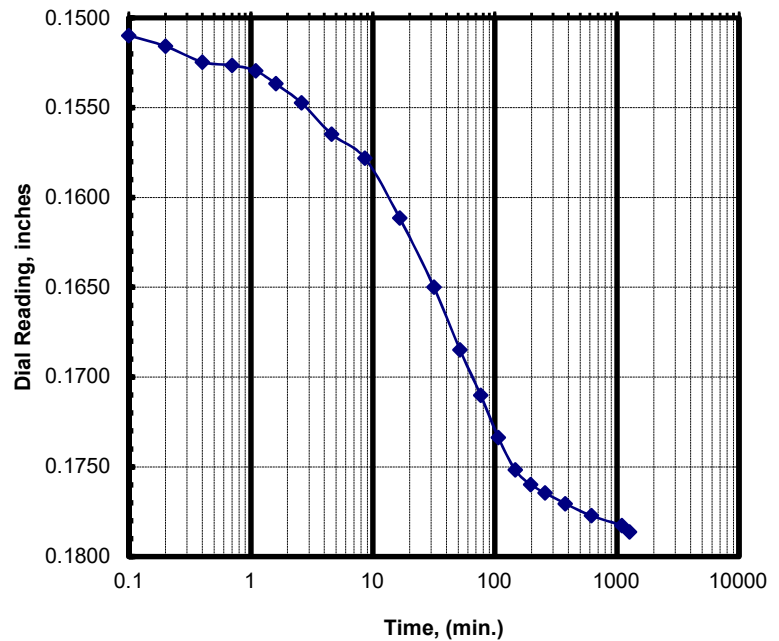
02541139 M-1 S-3 (M) 4-6'

t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

Project Number:	02541139	LL: 33	D _{60(mm)} (in): 0.1418	d ₆ (in): 0.1418
Project Name:	CPRA PO 181	PL: 13	D _{and} (in): 0.1476	d ₁₀₀ (in): 0.1472
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δh ₁ (in.): 0.0059	δ _{deep} (in.): 0.0054
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min):	t ₅₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 8 tsf

[illegible]

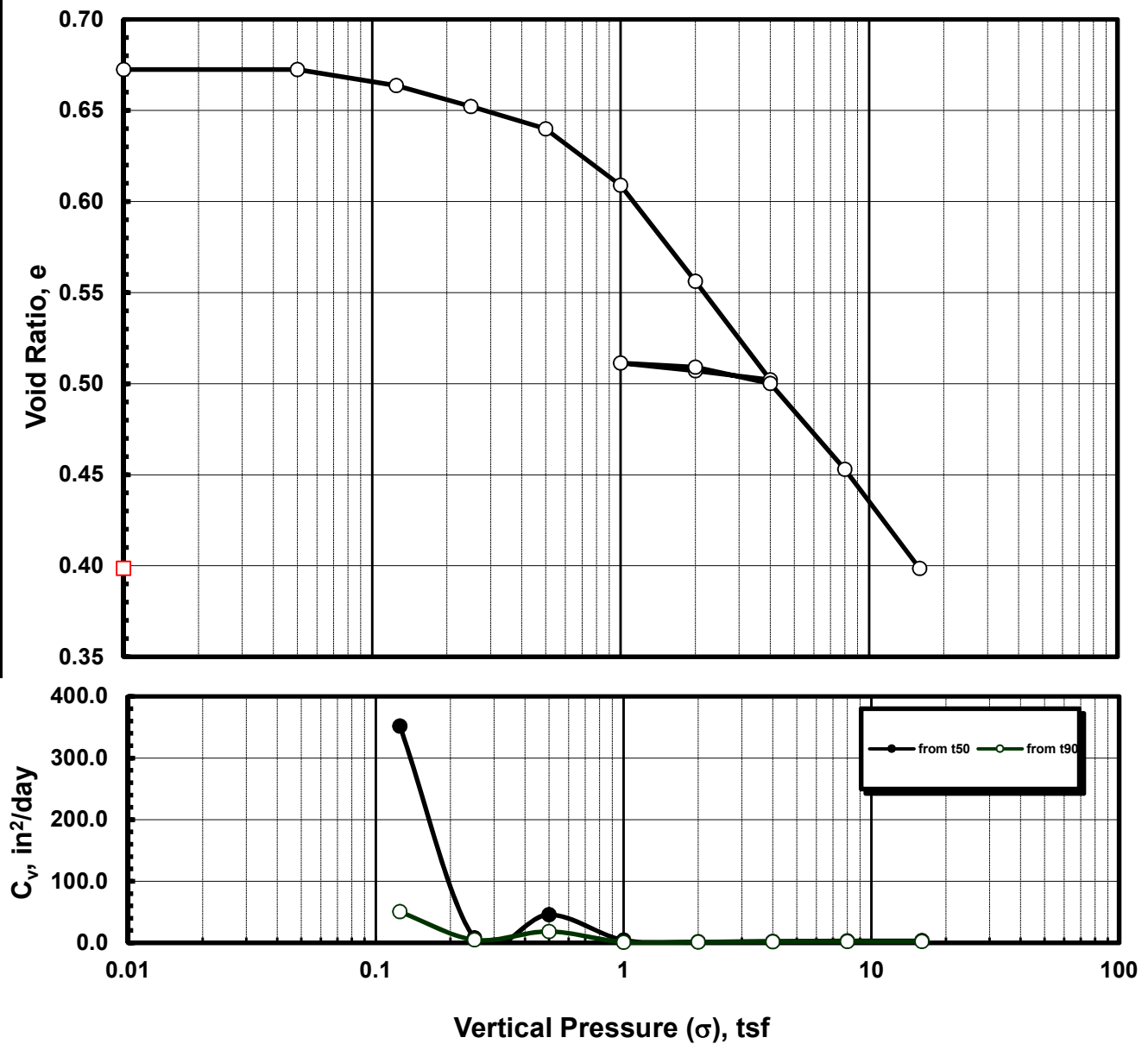
Project Number: 02541139	LL: 33	D _{0(min)} (in): 0.1476	d ₀ (in): 0.1476
Project Name: CPRA PO 181	PL: 13	D _{end} (in): 0.1786	d ₁₀₀ (in): 0.1758
Boring & Depth: M-1, S-3, 4-6 ft	PL: 20	Δ _{hi} (in.): 0.0310	δ _{oop} (in.): 0.0282
Sample: Light Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 18	t ₉₀ (min): 100

**CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 16 tsf**

[illegible]

Project Number:	02541139	LL: 33	$D_{0(\min)}$ (in): 0.1786	d_0 (in): 0.1786
Project Name:	CPRA PO 181	PL: 13	D_{end} (in): 0.2123	d_{100} (in): 0.2111
Boring & Depth:	M-1, S-3, 4-6 ft	PL: 20	Δ_{11} (in.): 0.0337	δ_{eop} (in.): 0.0325
Sample:	Light Gray SANDY LEAN CLAY	- 200 (%): NA	t_{50} (min): 15	t_{90} (min): 95

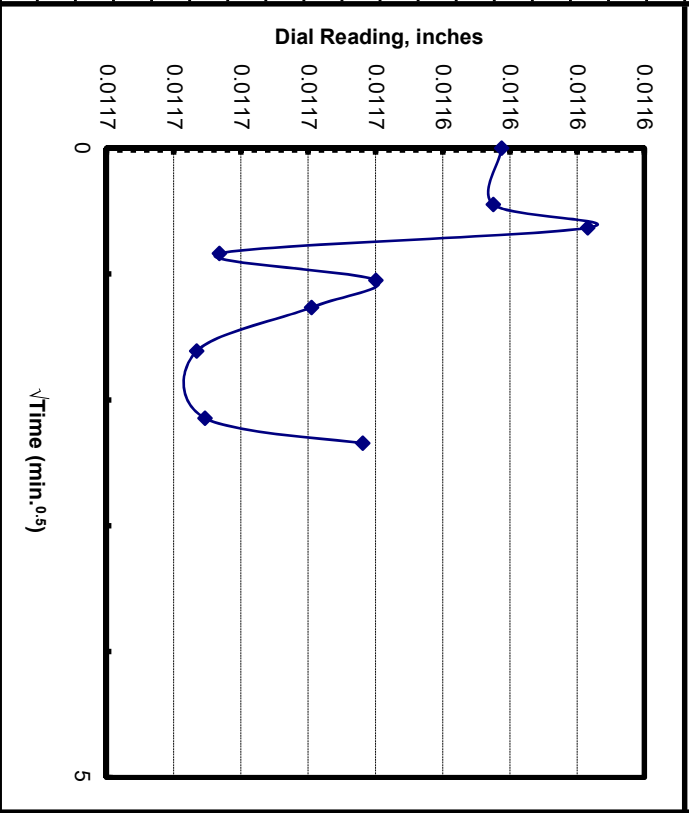
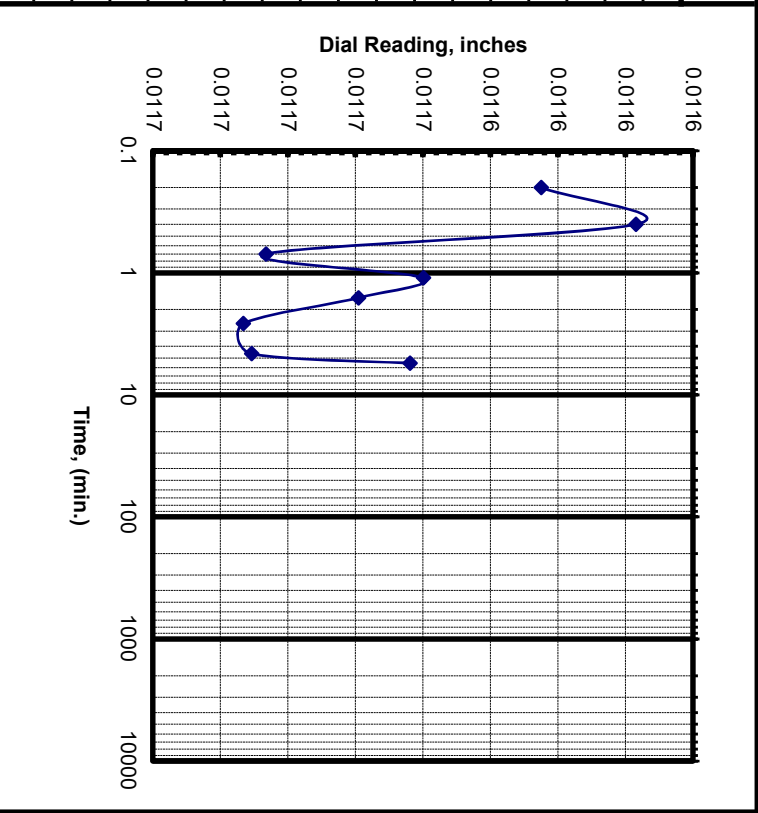
CONSOLIDATION TEST RESULTS - ASTM D 2435



Sample Condition	Before	After	Consolidation Parameters	
Moisture Content, %:	24.9%	15.0%	Overburden Pressure, tsf:	0.2
Sample Height, in.:	0.9990	0.8353	Preconsolidation Pressure, tsf:	0.8
Void Ratio, e:	0.6725	0.3985	Compression Index, (Cc):	0.181
Unit Weight, pcf:	98.9	118.2	Re-Compression Index, (Cr)	0.019
Degree of Saturation:	0.98	1.00	Swell Index, Cs:	-
Project Number: 02541139			Liquid Limit:	33
Project Name: CPRA PO 181			Hand Pen:	0.15 tsf
Boring No.: M-1			Plastic Limit:	13
Sample ID, Depth: S-3, 4-6 ft			-200 %:	NA
			Plasticity Index:	20
			SPG:	2.65
			Classification:	Light Gray SANDY LEAN CLAY

**CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.05 tsf**

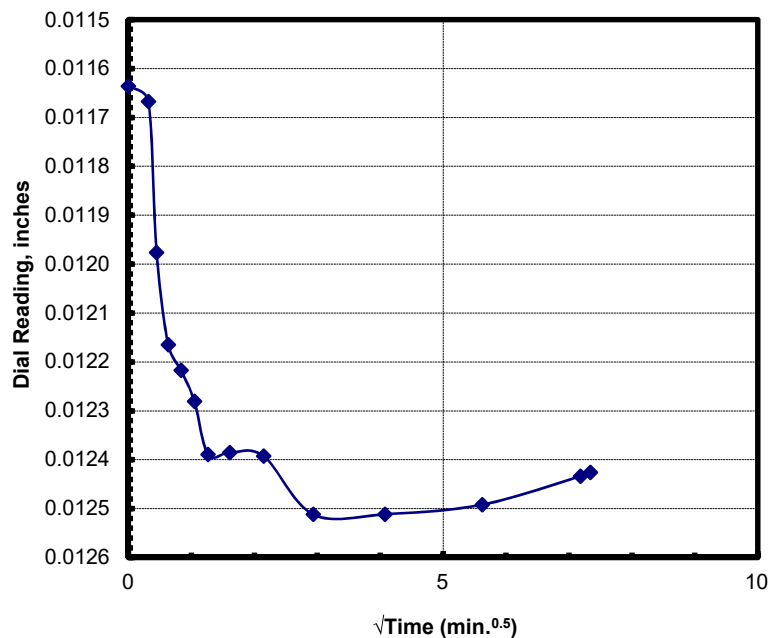
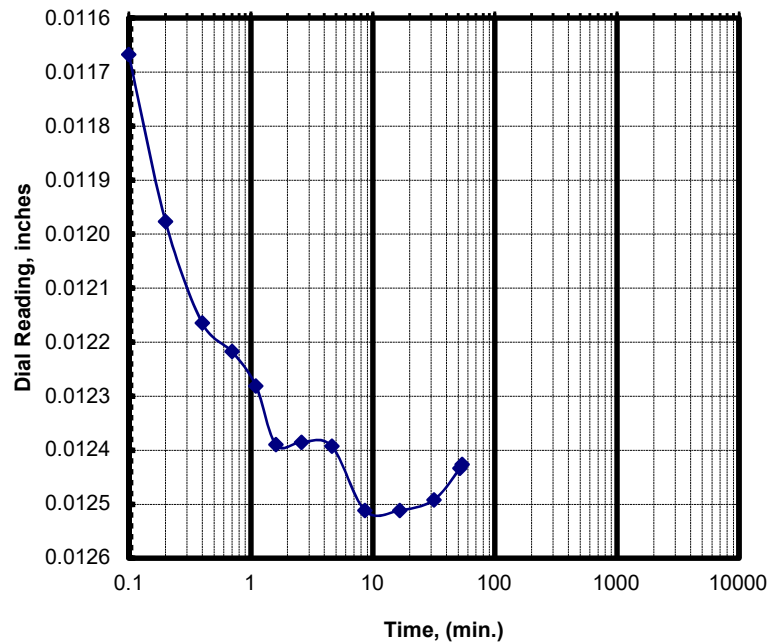
Date & Time	Elasped Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
12/2/20 3:17:51 PM	0.00	0.00	0.0116
12/2/20 3:18:03 PM	0.20	0.45	0.0116
12/2/20 3:18:15 PM	0.40	0.63	0.0116
12/2/20 3:18:33 PM	0.70	0.84	0.0117
12/2/20 3:18:57 PM	1.10	1.05	0.0117
12/2/20 3:19:27 PM	1.60	1.26	0.0117
12/2/20 3:20:27 PM	2.60	1.61	0.0117
12/2/20 3:22:27 PM	4.60	2.14	0.0117
12/2/20 3:23:21 PM	5.50	2.35	0.0117



Project Number:	02541139	LL: 58	D _{orgm} (in): 0.0116	d ₀ (in): 0.0116
Project Name:	CPRA PO 181	PL: 16	D _{end} (in): 0.0117	d ₁₀₀ (in): 0.0117
Boring & Depth:	M-2, S-4, 6-8 ft	PL: 42	Δ _{hi} (in.): 0.0000	δ _{exp} (in.): 0.0000
Sample:	Gray SANDY FAT CLAY (CH)	- 200 (%): 61	t ₅₀ (min):	t ₉₀ (min):

t50 & t90 Curves for the Vertical Stress of 0.125 tsf

Project Number:	02541139
Project Name:	CPRA PO 181
Boring & Depth:	M-2, S-4, 6-8 ft
Sample:	Gray SANDY FAT CLAY (CH)



Project Number:	02541139	LL: 58	D _{0(min)} (in): 0.0116	d ₀ (in): 0.0116
Project Name:	CPRA PO 181	PL: 16	D _{end} (in): 0.0124	d ₁₀₀ (in): 0.0124
Boring & Depth:	M-2, S-4, 6-8 ft	PL: 42	Δ _{hi} (in.): 0.0008	δ _{exp} (in.): 0.0008
Sample:	Gray SANDY FAT CLAY (CH)	- 200 (%): 61	t ₅₀ (min): 0.21	t ₉₀ (min): 1.2

**CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.25 tsf**

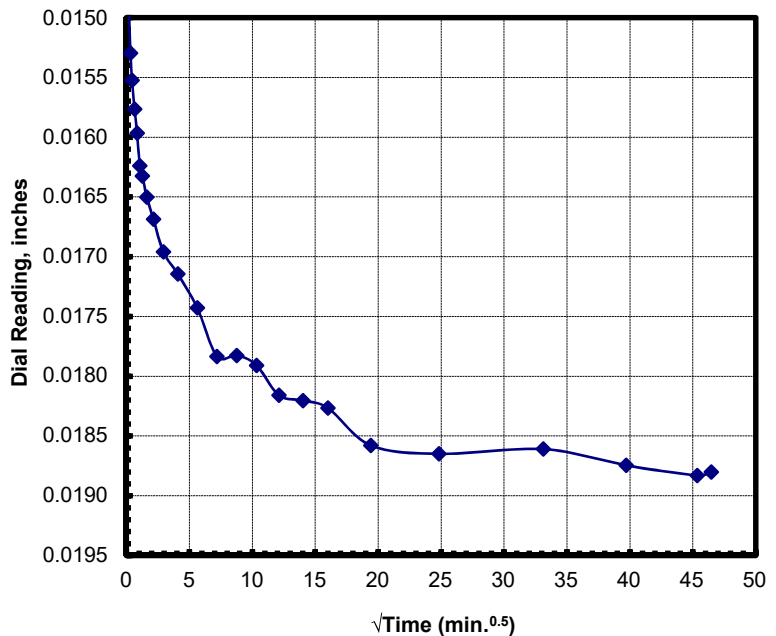
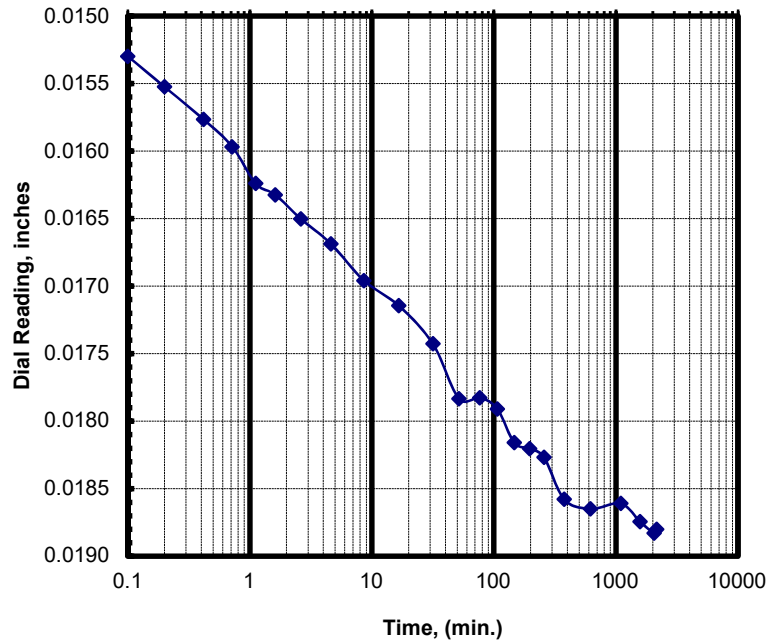
Figure 1 consists of two plots showing the effect of time on dial reading. The top plot shows Dial Reading (inches) on the y-axis (ranging from 0.0120 to 0.0145) versus Time (min.) on the x-axis (ranging from 0 to 35). The bottom plot shows Dial Reading (inches) on the y-axis (ranging from 0.0139 to 0.0145) versus Time (min.) on a logarithmic x-axis (ranging from 0.1 to 10000). Both plots show a sharp initial increase in dial reading followed by a plateau.

Time (min.)	Dial Reading (inches)
0	0.0125
1	0.0125
2	0.0125
3	0.0125
4	0.0125
5	0.0125
6	0.0135
7	0.0136
8	0.0137
9	0.0138
10	0.0139
11	0.0139
12	0.0139
13	0.0139
14	0.0139
15	0.0139
16	0.0139
17	0.0139
18	0.0139
19	0.0139
20	0.0139
21	0.0139
22	0.0139
23	0.0139
24	0.0139
25	0.0139
26	0.0139
27	0.0139
28	0.0139
29	0.0139
30	0.0139
31	0.0139
32	0.0139
33	0.0139
34	0.0139
35	0.0139

Time (min.)	Dial Reading (inches)
0.1	0.0125
0.2	0.0125
0.5	0.0125
1	0.0125
2	0.0125
5	0.0125
10	0.0125
20	0.0125
50	0.0125
100	0.0125
200	0.0125
500	0.0125
1000	0.0125
2000	0.0125
5000	0.0125
10000	0.0125

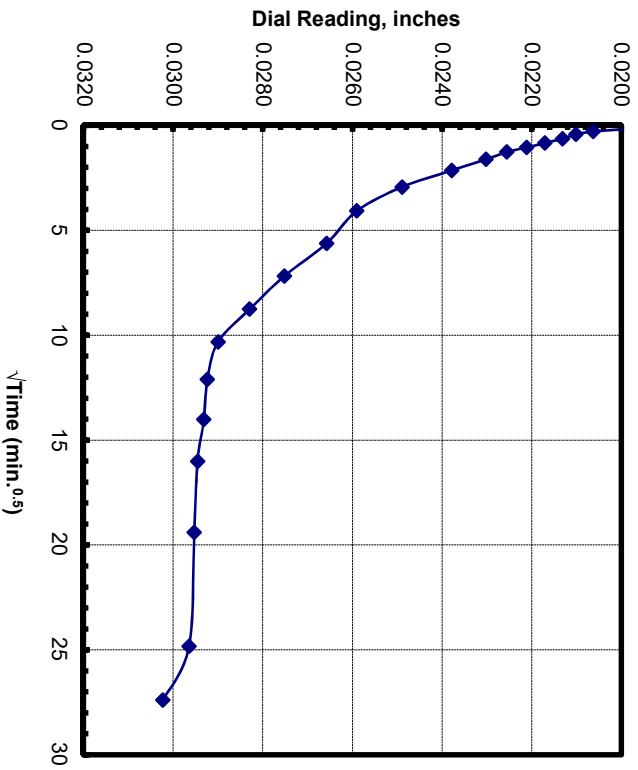
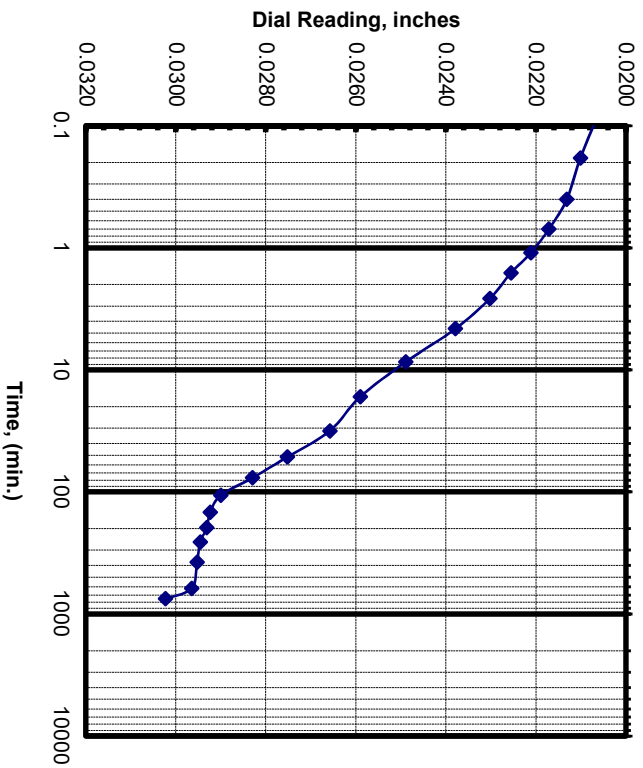
LL: 58	$D_{0(min)}$ (in): 0.0125	d_0 (in): 0.0125
PL: 16	D_{end} (in): 0.0144	d_{100} (in): 0.0136
PL: 42	Δh_i (in.): 0.0020	δ_{exp} (in.): 0.0011
-200 (%): 61	t_{50} (min): 0.79	t_{90} (min): 7.9

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 0.5 tsf

[illegible]

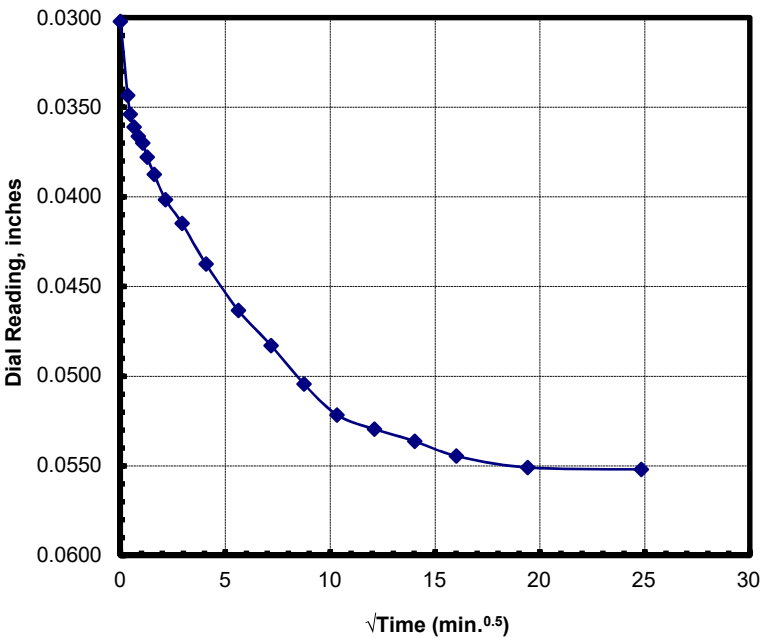
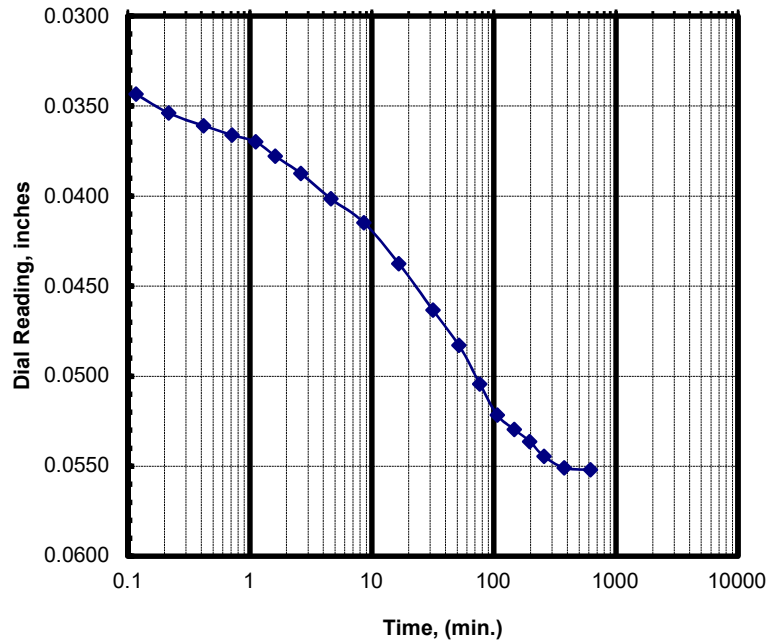
Project Number: 02541139	LL: 58	D _{0(min)} (in): 0.0144	d ₀ (in): 0.0144
Project Name: CPRA PO 181	PL: 16	D _{end} (in): 0.0188	d ₁₀₀ (in): 0.0178
Boring & Depth: M-2, S-4, 6-8 ft	PL: 42	Δ _{hi} (in.): 0.0044	δ _{exp} (in.): 0.0034
Sample: Gray SANDY FAT CLAY (CH)	- 200 (%): 61	t ₅₀ (min): 0.9	t ₉₀ (min): 35

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 1 tsf

[illegible]

Project Number:	02541139	LL: 58	D _{orgm} (in): 0.0188	d ₀ (in): 0.0188
Project Name:	CPRA PO 181	PL: 16	D _{end} (in): 0.0302	d ₁₀₀ (in): 0.0292
Boring & Depth:	M-2, S-4, 6-8 ft	PL: 42	Δ _{hi} (in.): 0.0115	δ _{exp} (in.): 0.0104
Sample:	Gray SANDY FAT CLAY (CH)	- 200 (%): 61	t ₅₀ (min): 5.2	t ₉₀ (min): 72

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

[illegible]

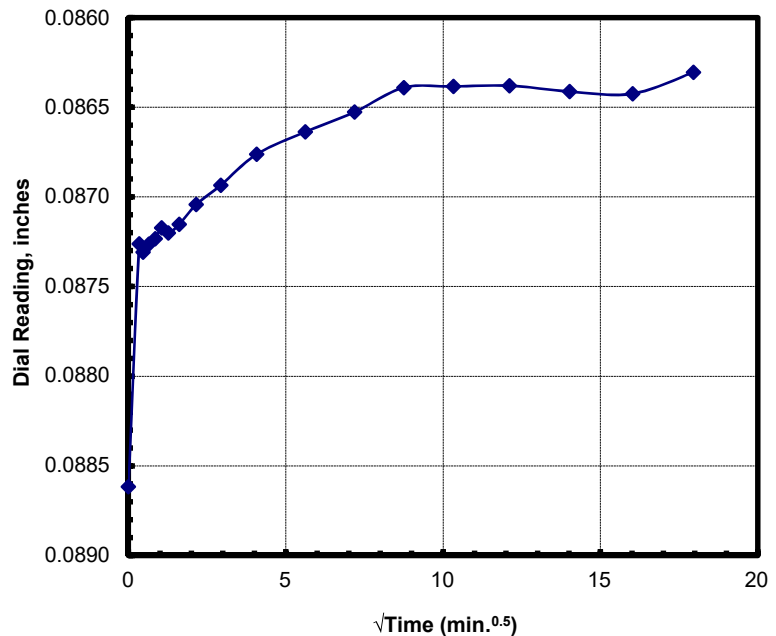
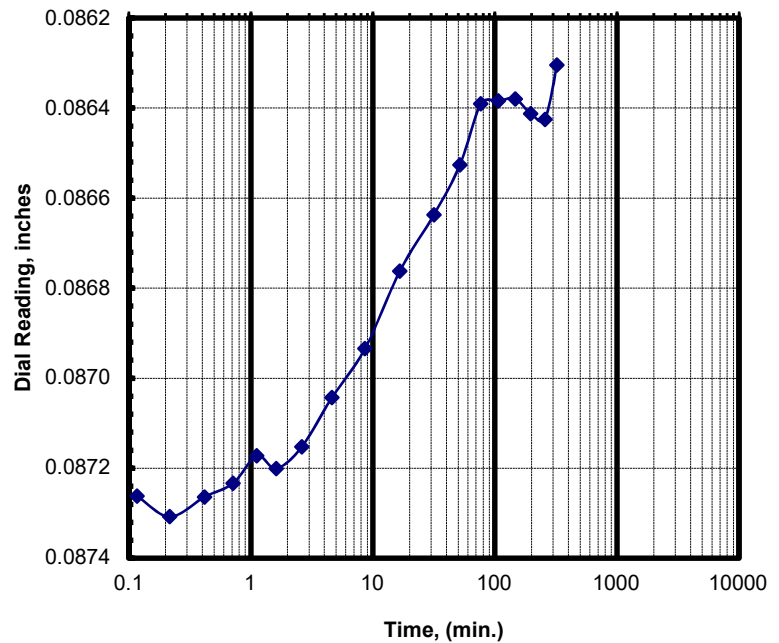
Project Number:	02541139	LL: 58	D _{0(min)} (in): 0.0302	d ₀ (in): 0.0302
Project Name:	CPRA PO 181	PL: 16	D _{end} (in): 0.0552	d ₁₀₀ (in): 0.0550
Boring & Depth:	M-2, S-4, 6-8 ft	PL: 42	Δ _{hi} (in.): 0.0250	δ _{exp} (in.): 0.0248
Sample:	Gray SANDY FAT CLAY (CH)	- 200 (%): 61	t ₅₀ (min): 11	t ₉₀ (min): 110

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

Appendix Figure No. G.1-160

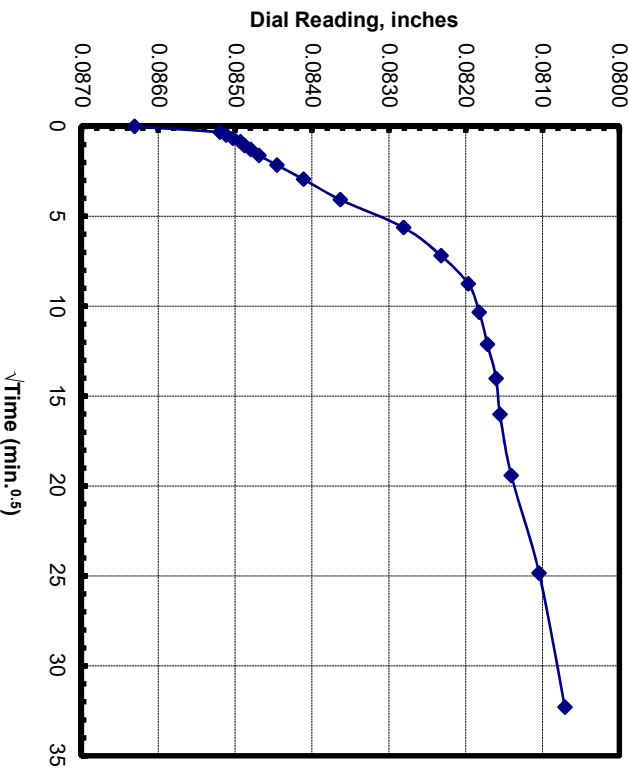
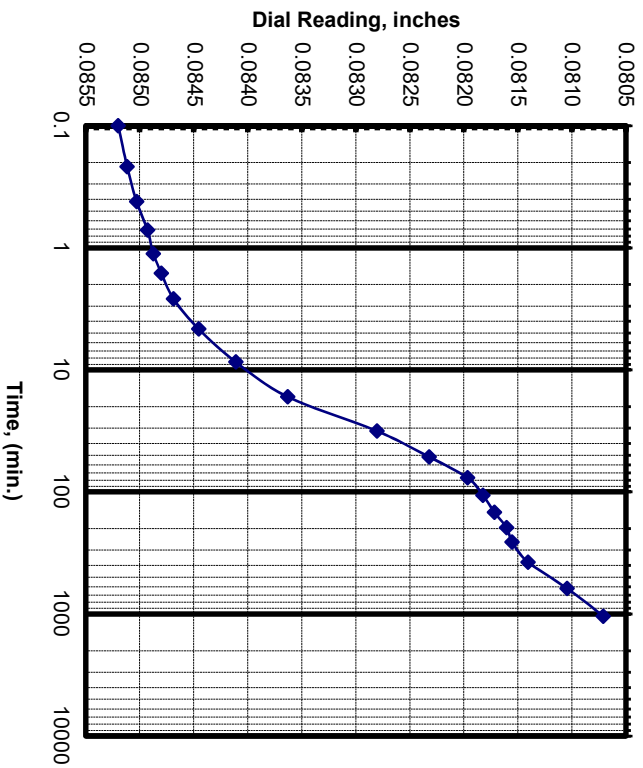
CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

[illegible]

Project Number: 02541139	LL: 58	D _{0(min)} (in): 0.0886	d ₀ (in): 0.0886
Project Name: CPRA PO 181	PL: 16	D _{end} (in): 0.0863	d ₁₀₀ (in): 0.0863
Boring & Depth: M-2, S-4, 6-8 ft	PL: 42	Δ _{hi} (in.): -0.0023	δ _{exp} (in.): -0.0023
Sample: Gray SANDY FAT CLAY (CH)	- 200 (%): 61	t ₅₀ (min):	t ₉₀ (min):

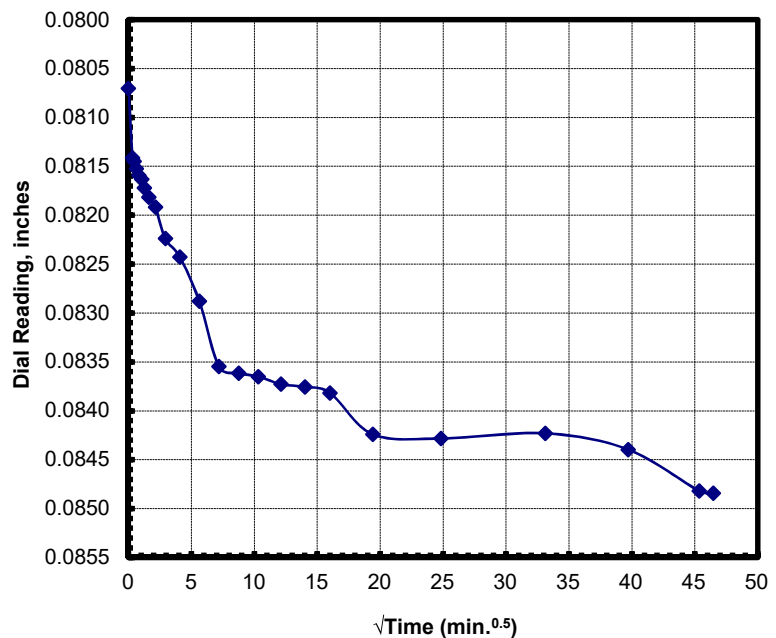
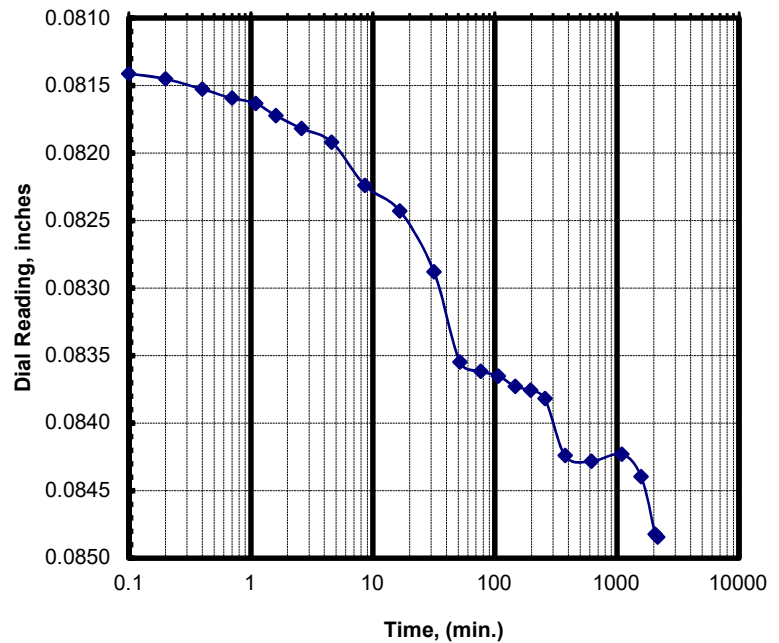
CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 1 tsf

Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
12/8/20 2:33:13 PM	0.00	0.00	0.0863
12/8/20 2:33:19 PM	0.10	0.32	0.0852
12/8/20 2:33:26 PM	0.22	0.47	0.0851
12/8/20 2:33:38 PM	0.42	0.65	0.0850
12/8/20 2:33:56 PM	0.72	0.85	0.0849
12/8/20 2:34:20 PM	1.12	1.06	0.0849
12/8/20 2:34:50 PM	1.62	1.27	0.0848
12/8/20 2:35:50 PM	2.62	1.62	0.0847
12/8/20 2:37:50 PM	4.62	2.15	0.0845
12/8/20 2:41:50 PM	8.62	2.94	0.0841
12/8/20 2:49:50 PM	16.62	4.08	0.0836
12/8/20 3:04:51 PM	31.63	5.62	0.0828
12/8/20 3:24:51 PM	51.63	7.19	0.0823
12/8/20 3:49:51 PM	76.63	8.75	0.0820
12/8/20 4:19:51 PM	106.63	10.33	0.0818
12/8/20 4:59:51 PM	146.63	12.11	0.0817
12/8/20 5:49:51 PM	196.63	14.02	0.0816
12/8/20 6:49:52 PM	256.65	16.02	0.0816
12/8/20 8:49:52 PM	376.65	19.41	0.0814
12/9/20 12:49:52 AM	616.65	24.83	0.0810
12/9/20 1:56:12 AM	1042.98	32.30	0.0807



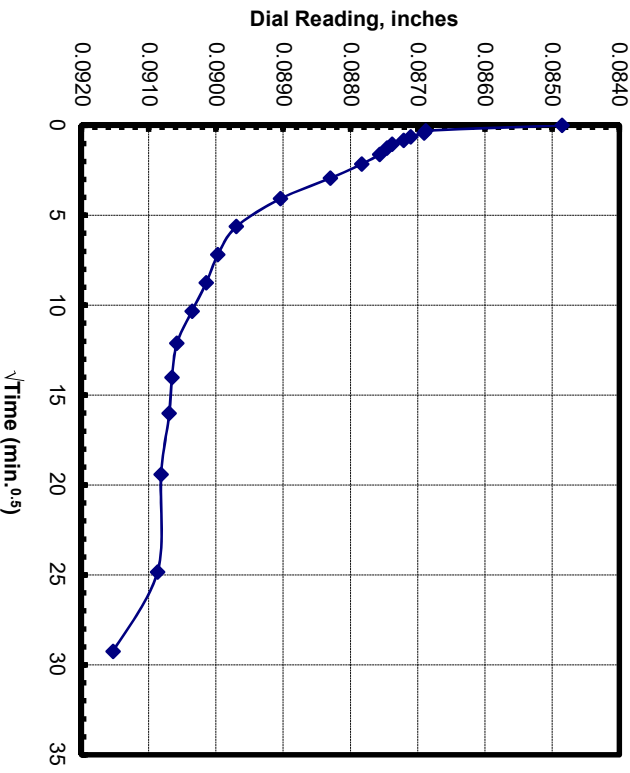
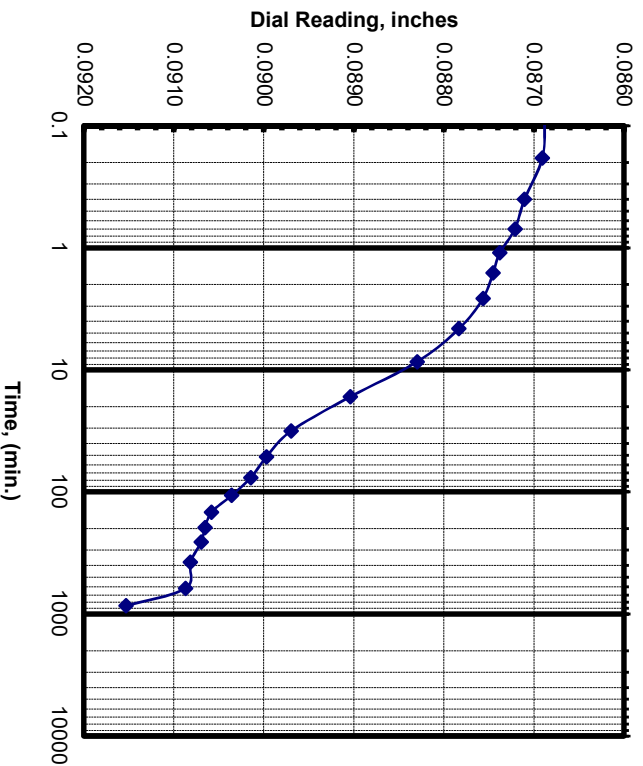
Project Number:	0254139	LL: 58	D _{0(min)} (in): 0.0863	d ₀ (in): 0.0863
Project Name:	CPRA PO 181	PL: 16	D _{end} (in): 0.0807	d ₁₀₀ (in): 0.0807
Boring & Depth:	M-2, S-4, 6-8 ft	PL: 42	Δ _{hi} (in.): -0.0056	δ _{exp} (in.): -0.0056
Sample:	Gray SANDY FAT CLAY (CH)	- 200 (%): 61	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

[illegible]

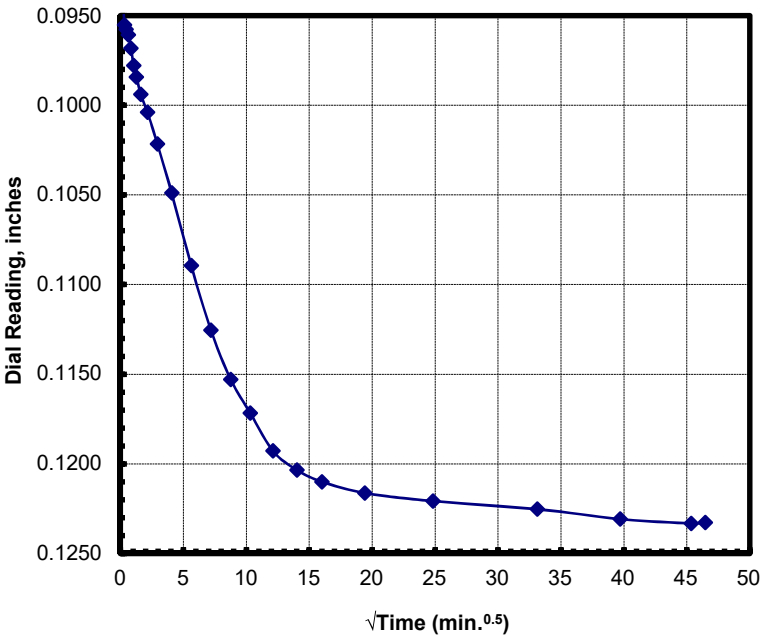
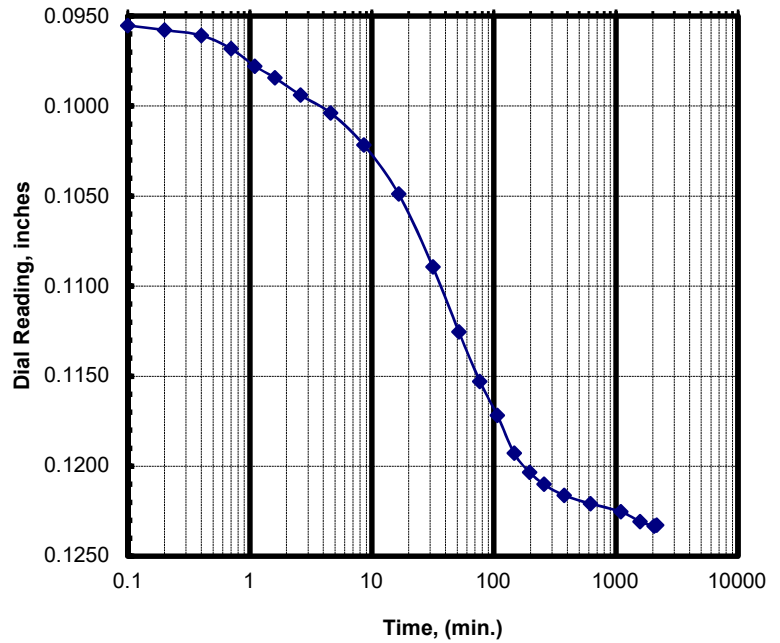
Project Number: 02541139	LL: 58	D _{0(min)} (in): 0.0807	d ₀ (in): 0.0807
Project Name: CPRA PO 181	PL: 16	D _{end} (in): 0.0848	d ₁₀₀ (in): 0.0836
Boring & Depth: M-2, S-4, 6-8 ft	PL: 42	Δ _{hi} (in.): 0.0041	δ _{oop} (in.): 0.0029
Sample: Gray SANDY FAT CLAY (CH)	- 200 (%): 61	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

Project Number:	02541139	LL: 58	D _{90(gm)} (in.): 0.0849	d ₀ (in.): 0.0849
Project Name:	CPRA PO 181	PL: 16	D _{end} (in.): 0.0915	d ₁₀₀ (in.): 0.0904
Boring & Depth:	M-2, S-4, 6-8 ft	PL: 42	Δ _{hi} (in.): 0.0067	δ _{deep} (in.): 0.0055
Sample:	Gray SANDY FAT CLAY (CH)	- 200 (%): 61	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 8 tsf

[illegible]

Project Number:	02541139	LL: 58	D _{0(min)} (in): 0.0915	d ₀ (in): 0.0915
Project Name:	CPRA PO 181	PL: 16	D _{end} (in): 0.1233	d ₁₀₀ (in): 0.1211
Boring & Depth:	M-2, S-4, 6-8 ft	PL: 42	Δ _{hi} (in.): 0.0317	δ _{exp} (in.): 0.0296
Sample:	Gray SANDY FAT CLAY (CH)	- 200 (%): 61	t ₅₀ (min): 21	t ₉₀ (min): 120

**CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 16 tsf**

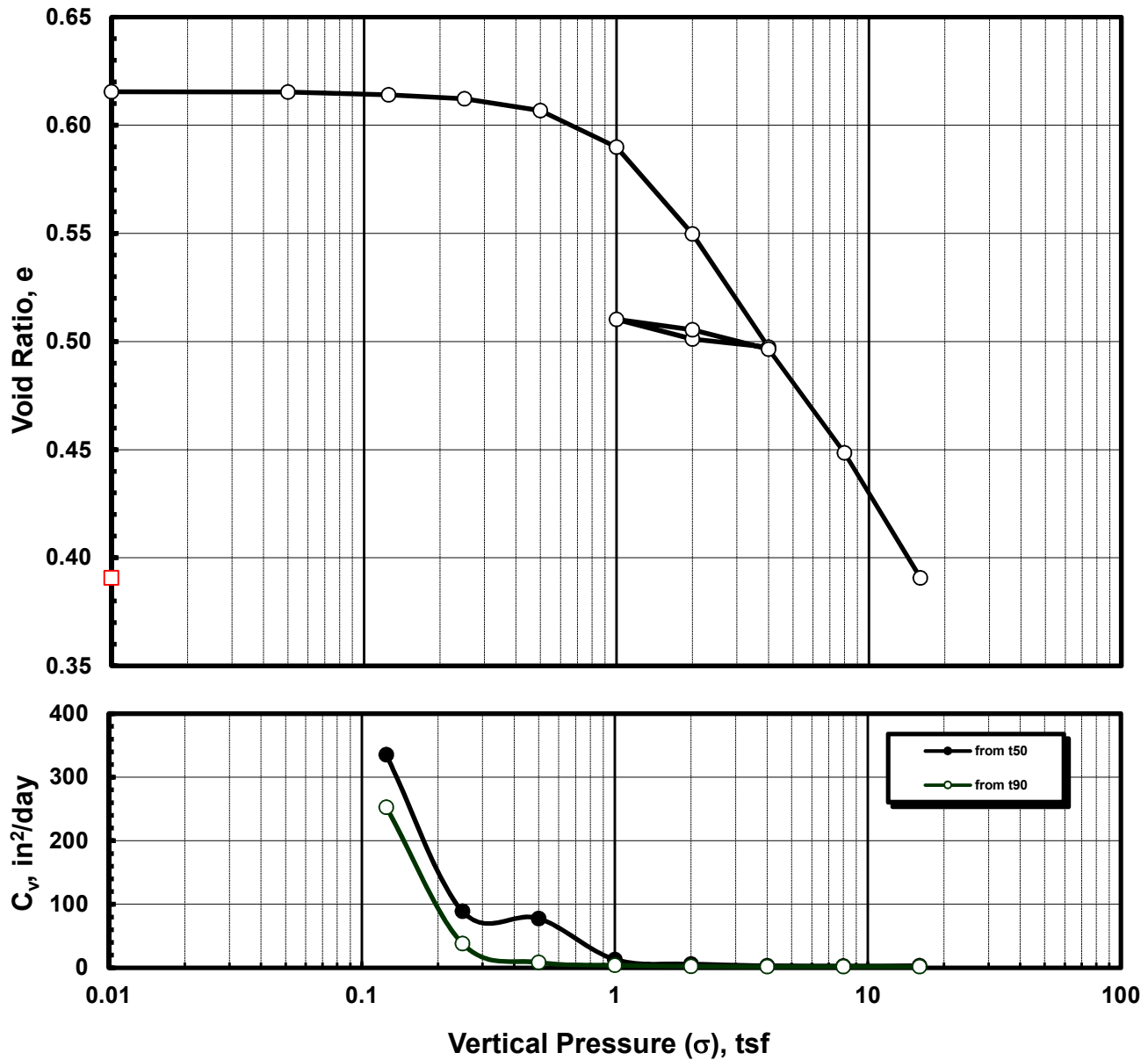
The figure consists of two plots side-by-side, both showing the relationship between Dial Reading (inches) on the y-axis and Time (min.) on the x-axis. The y-axis for both plots ranges from 0.1300 to 0.1600 inches, with major ticks every 0.0050 inches.

The left plot shows the relationship between Dial Reading and the square root of time ($\sqrt{t}$). The x-axis is labeled $\sqrt{t}$ (min.^{0.5}) and ranges from 0 to 35. The data points, represented by blue diamonds, form a nearly straight line with a positive slope, indicating a linear relationship between $\sqrt{t}$ and Dial Reading.

The right plot shows the relationship between Dial Reading and the logarithm of time ($\log(t)$). The x-axis is labeled Time, (min.) and is on a logarithmic scale ranging from 0.1 to 10000. The data points, represented by blue diamonds, form a nearly straight line with a positive slope, indicating a linear relationship between $\log(t)$ and Dial Reading.

LI: 58	$D_{0(gm)}$ (in.): 0.1233	d_0 (in.): 0.1233
PI: 16	D_{end} (in.): 0.1595	d_{100} (in.): 0.1590
PI: 42	Δ_{hl} (in.): 0.0465	δ_{sep} (in.): 0.0357
- 200 (%): 61	t_{50} (min): 17	t_{90} (min): 130

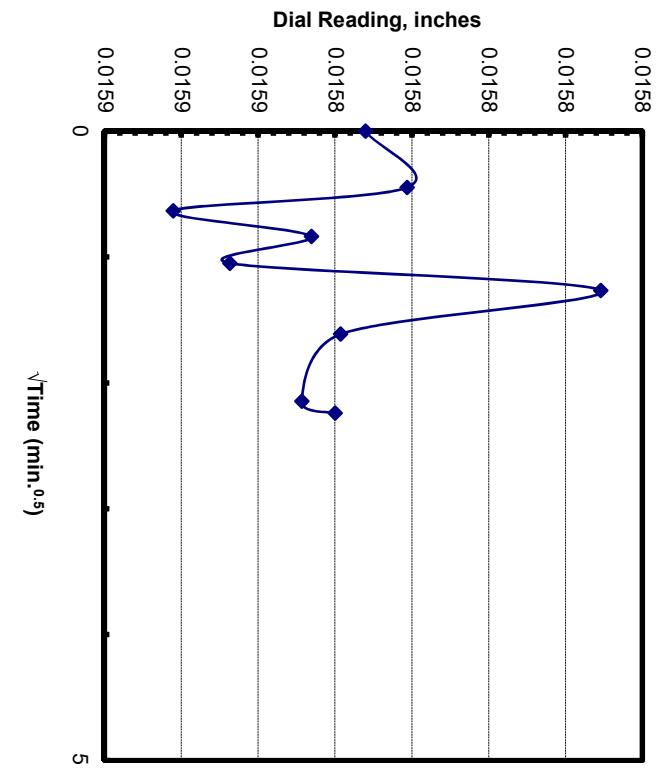
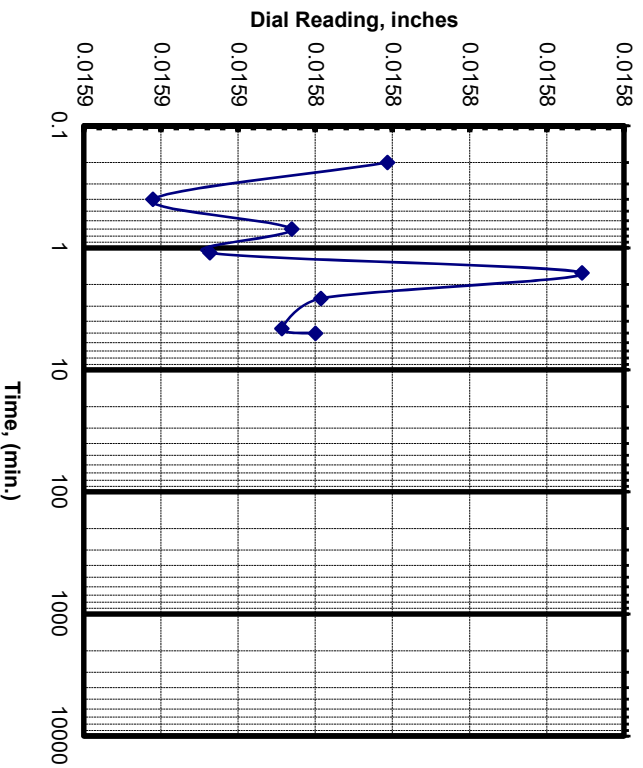
CONSOLIDATION TEST RESULTS - ASTM D 2435



Sample Condition	Before	After	Consolidation Parameters	
Moisture Content, %:	22.0%	14.5%	Overburden Pressure, tsf:	0.2
Sample Height, in.:	0.9970	0.8583	Preconsolidation Pressure, tsf:	1.3
Void Ratio, e:	0.6155	0.3907	Compression Index, (Cc):	0.192
Unit Weight, pcf:	103.8	120.5	Re-Compression Index, (Cr)	0.023
Degree of Saturation:	0.96	1.00	Swell Index, Cs:	-
Project Number: 02541139			Liquid Limit: 58	Hand Pen: 0.55 tsf
Project Name CPRA PO 181			Plastic Limit: 16	-200 %: 61
Boring No.: M-2			Plasticity Index: 42	SPG: 2.686
Sample ID, Depth: S-4, 6-8 ft			Classification:	Gray SANDY FAT CLAY (CH)

CONSOLIDATION TEST RESULTS - ASTM D 2435

Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
12/8/20 10:28:44 AM	0.00	0.00	0.0158
12/8/20 10:28:56 AM	0.20	0.45	0.0158
12/8/20 10:29:06 AM	0.40	0.63	0.0159
12/8/20 10:29:26 AM	0.70	0.84	0.0158
12/8/20 10:29:50 AM	1.10	1.05	0.0159
12/8/20 10:30:20 AM	1.60	1.26	0.0158
12/8/20 10:31:20 AM	2.60	1.61	0.0158
12/8/20 10:33:20 AM	4.60	2.14	0.0158
12/8/20 10:33:45 AM	5.02	2.24	0.0158

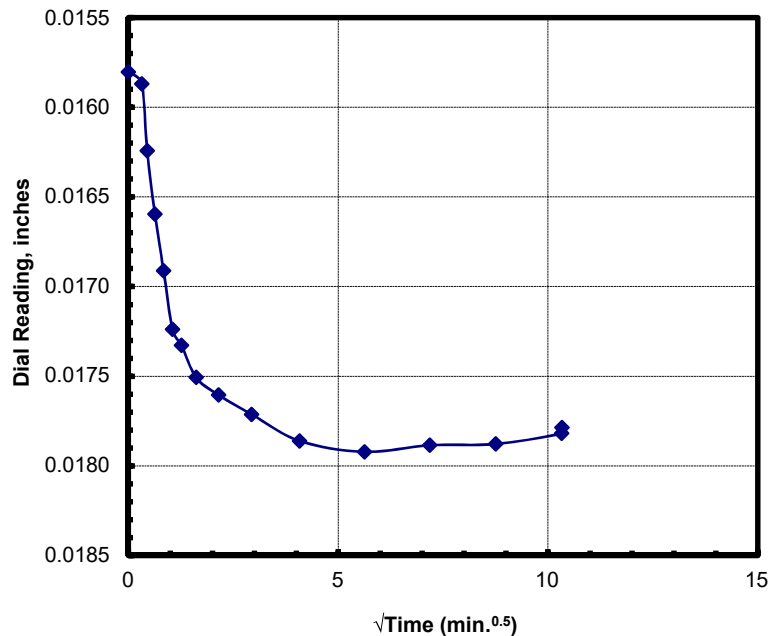
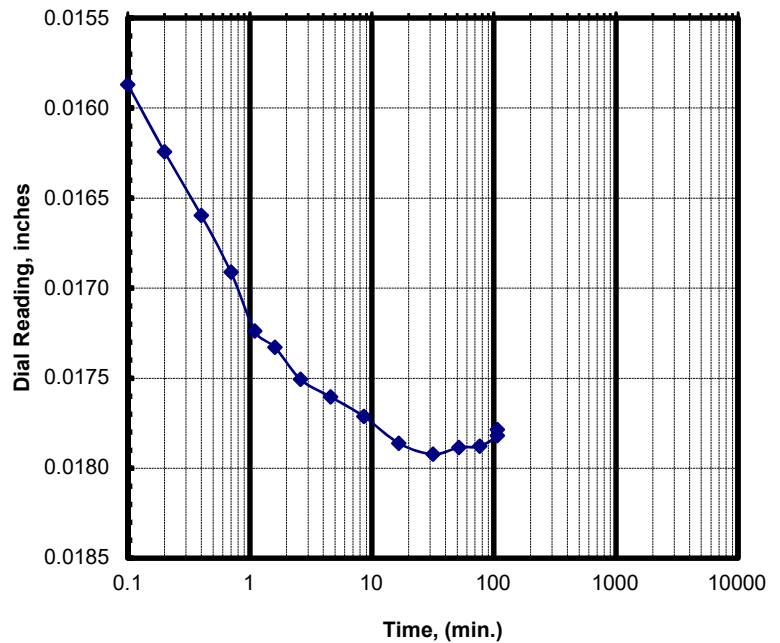


Project Number:	02541139	LL: NA	D _{o(min)} (in): 0.0158	d _o (in): 0.0158
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0158	d ₁₀₀ (in): 0.0158
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δh _i (in.): 0.0000	Δ _{deep} (in.): 0.0000
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min):	t ₅₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435

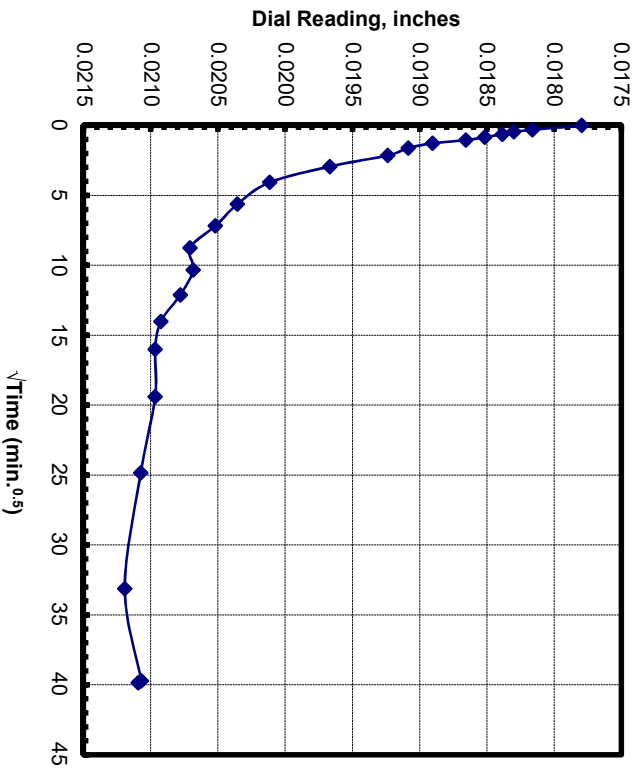
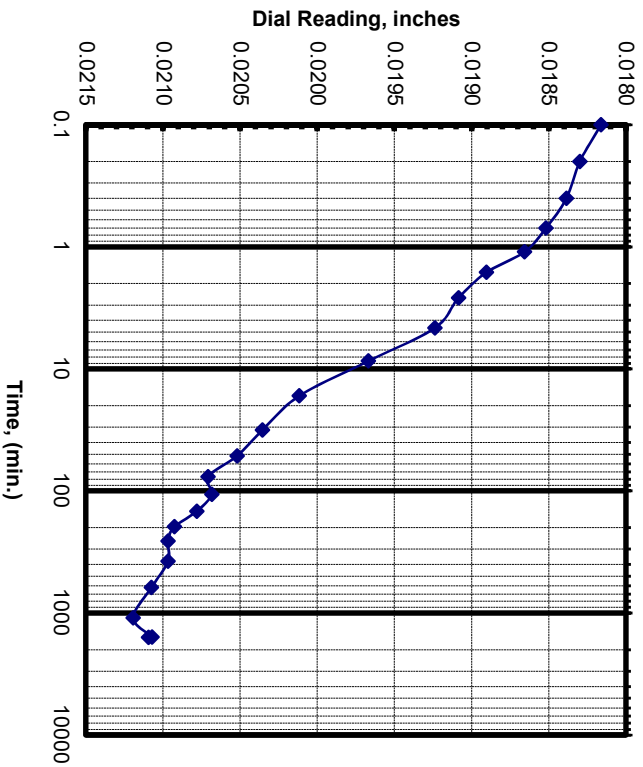
t₅₀ & t₉₀ Curves for the Vertical Stress of 0.125 tsf

Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
12/8/20 10:33:45 AM	0.00	0.00	0.0158
12/8/20 10:33:51 AM	0.10	0.32	0.0159
12/8/20 10:33:57 AM	0.20	0.45	0.0162
12/8/20 10:34:09 AM	0.40	0.63	0.0166
12/8/20 10:34:27 AM	0.70	0.84	0.0169
12/8/20 10:34:51 AM	1.10	1.05	0.0172
12/8/20 10:35:21 AM	1.60	1.26	0.0173
12/8/20 10:36:21 AM	2.60	1.61	0.0175
12/8/20 10:38:21 AM	4.60	2.14	0.0176
12/8/20 10:42:22 AM	8.62	2.94	0.0177
12/8/20 10:50:22 AM	16.62	4.08	0.0179
12/8/20 11:05:22 AM	31.62	5.62	0.0179
12/8/20 11:25:22 AM	51.62	7.18	0.0179
12/8/20 11:50:22 AM	76.62	8.75	0.0179
12/8/20 12:20:22 PM	106.62	10.33	0.0178
12/8/20 12:20:29 PM	106.73	10.33	0.0178



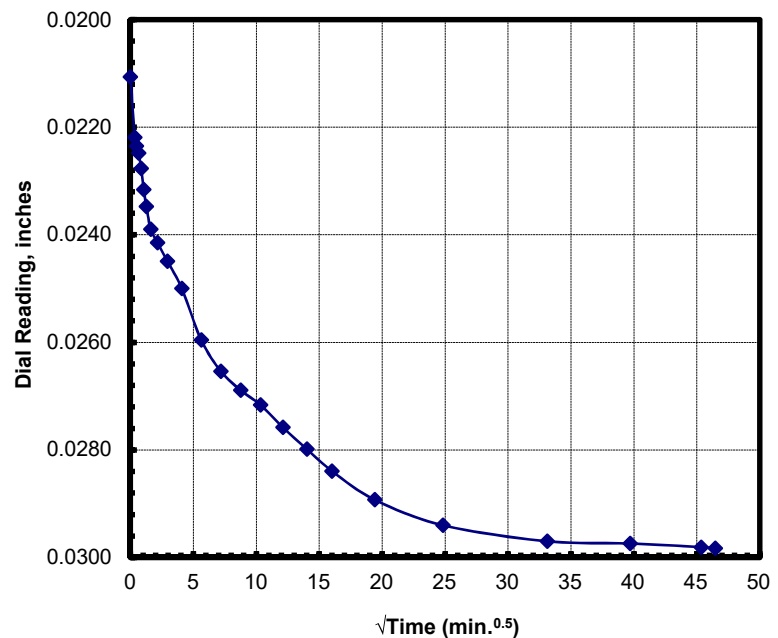
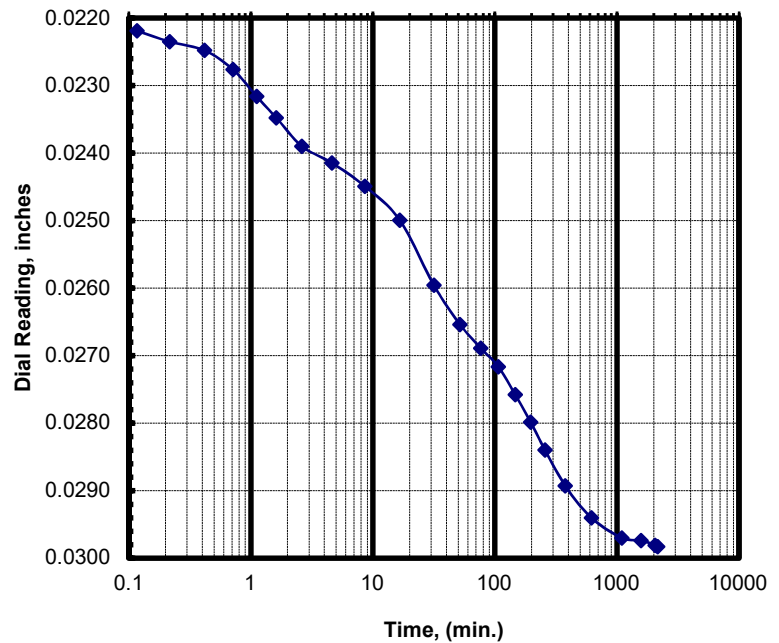
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0158	d ₀ (in): 0.0158
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0178	d ₁₀₀ (in): 0.0178
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δ _{hj} (in.): 0.0020	δ _{eop} (in.): 0.0020
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min): 0.59	t ₉₀ (min): 4.7

CONSOLIDATION TEST RESULTS - ASTM D 2435

[illegible]

Project Number:	02541139	LL: NA	D _{o(min)} (in): 0.0178	d _o (in): 0.0178
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0211	d ₁₀₀ (in): 0.0205
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δh _i (in.): 0.0033	Δe _{oep} (in.): 0.0027
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min): 2.7	t ₉₀ (min): 20

CONSOLIDATION TEST RESULTS - ASTM D 2435
***t*50 & *t*90 Curves for the Vertical Stress of 0.5 tsf**

[illegible]

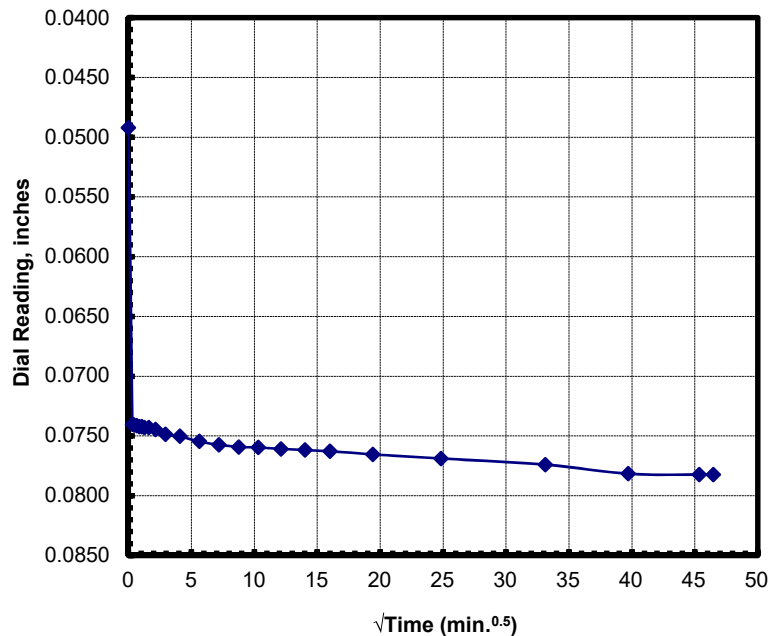
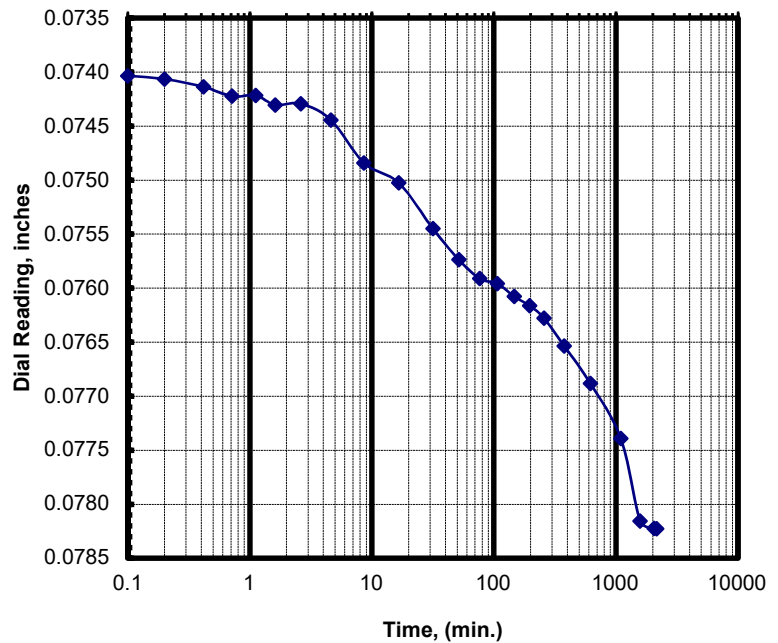
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0211	d ₀ (in): 0.0211
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0298	d ₁₀₀ (in): 0.0264
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0088	δ _{oop} (in.): 0.0053
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min): 2	t ₉₀ (min): 30

t50 & t90 Curves for the Vertical Stress of 1 tsf

[illegible]

Project Number:	02541139	LL: NA	D _{90mm} (in.): 0.0298	d ₀ (in.): 0.0298
Project Name:	CPRA PO 181	PL: NA	D _{end} (in.): 0.0492	d ₁₀₀ (in.): 0.0488
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0193	δ _{deep} (in.): 0.0190
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min): 35	t ₉₀ (min): 280

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

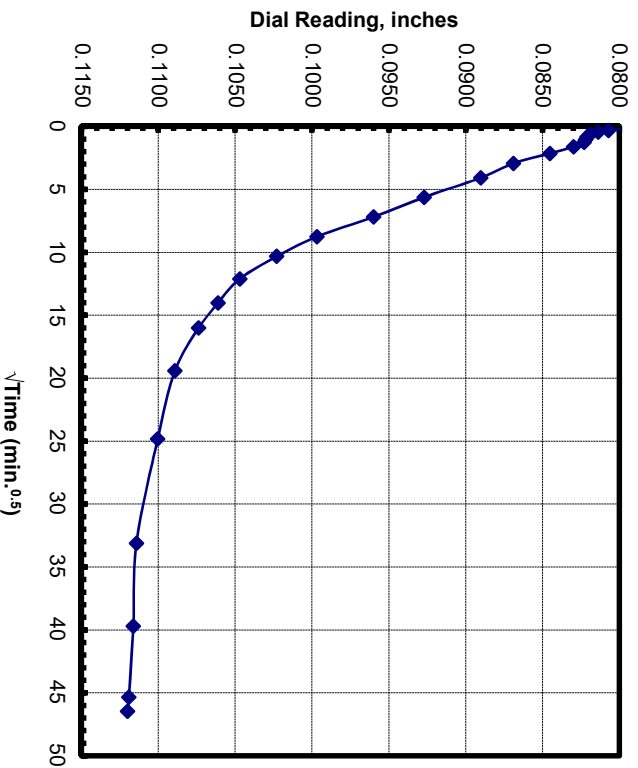
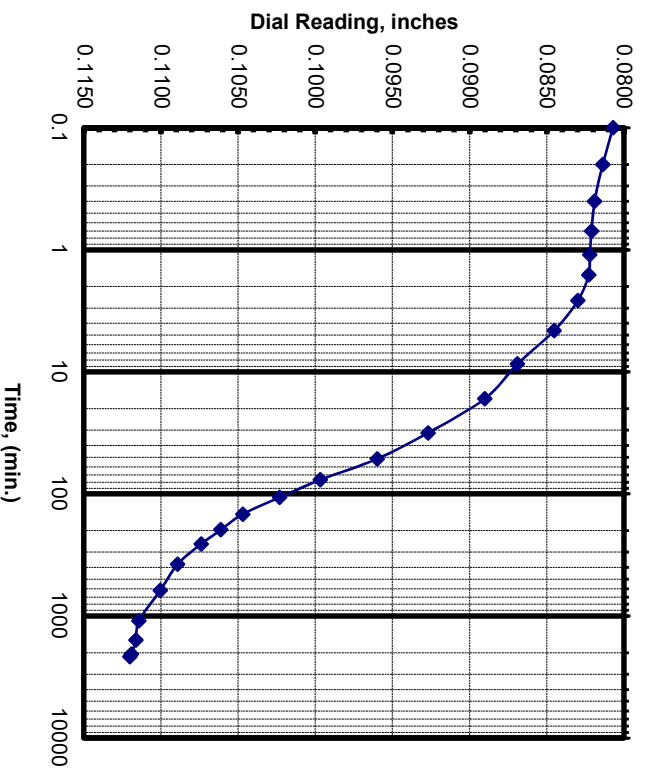
[illegible]

Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0492	d ₀ (in): 0.0492
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0782	d ₁₀₀ (in): 0.0759
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0076	δ _{exp} (in.): 0.0267
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min): 17	t ₉₀ (min): 50

intertek
psi

02541139 M-4 S-3 (M) 4-6' (NEW) (combined) RR

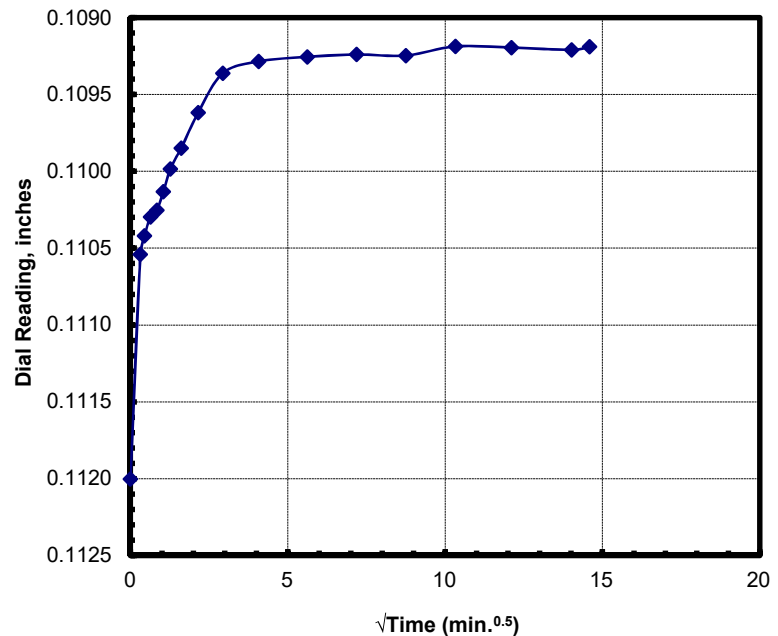
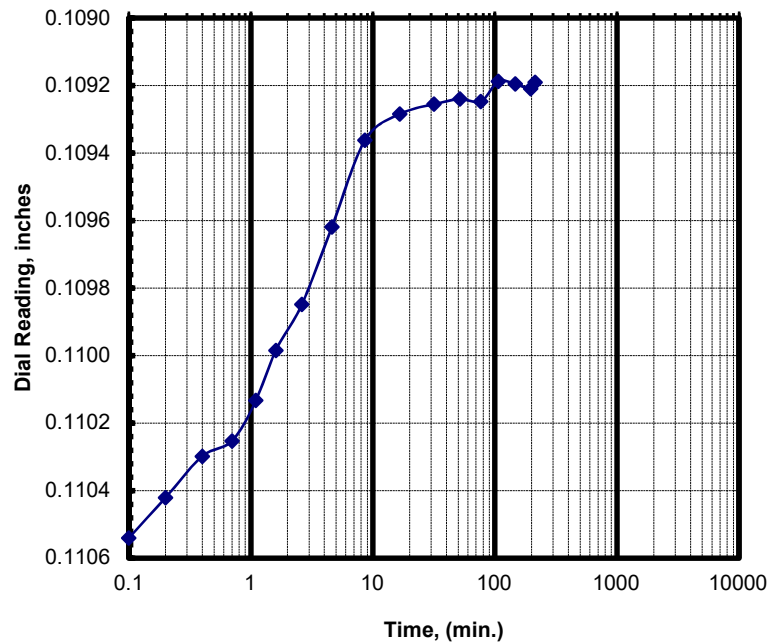
t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

Project Number:	02541139	LL: NA	$D_{(min)}$ (in): 0.0782	d_0 (in): 0.0782
Project Name:	CPRA PO 181	PL: NA	$D_{(end)}$ (in): 0.1120	$d_{(00)}$ (in): 0.1098
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δh_i (in.): 0.0338	$\delta_{(eep)}$ (in.): 0.0316
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t_{50} (min): 40	t_{50} (min): 210

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

Date & Time	Elaspsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
12/17/20 9:38:51 AM	0.00	0.00	0.1120
12/17/20 9:38:57 AM	0.10	0.32	0.1105
12/17/20 9:39:03 AM	0.20	0.45	0.1104
12/17/20 9:39:15 AM	0.40	0.63	0.1103
12/17/20 9:39:33 AM	0.70	0.84	0.1103
12/17/20 9:39:57 AM	1.10	1.05	0.1101
12/17/20 9:40:27 AM	1.60	1.26	0.1100
12/17/20 9:41:28 AM	2.62	1.62	0.1098
12/17/20 9:43:28 AM	4.62	2.15	0.1096
12/17/20 9:47:28 AM	8.62	2.94	0.1094
12/17/20 9:55:28 AM	16.62	4.08	0.1093
12/17/20 10:10:28 AM	31.62	5.62	0.1093
12/17/20 10:30:28 AM	51.62	7.18	0.1092
12/17/20 10:55:28 AM	76.62	8.75	0.1092
12/17/20 11:25:28 AM	106.62	10.33	0.1092
12/17/20 12:05:28 PM	146.62	12.11	0.1092
12/17/20 12:55:29 PM	196.63	14.02	0.1092
12/17/20 1:11:47 PM	212.93	14.59	0.1092

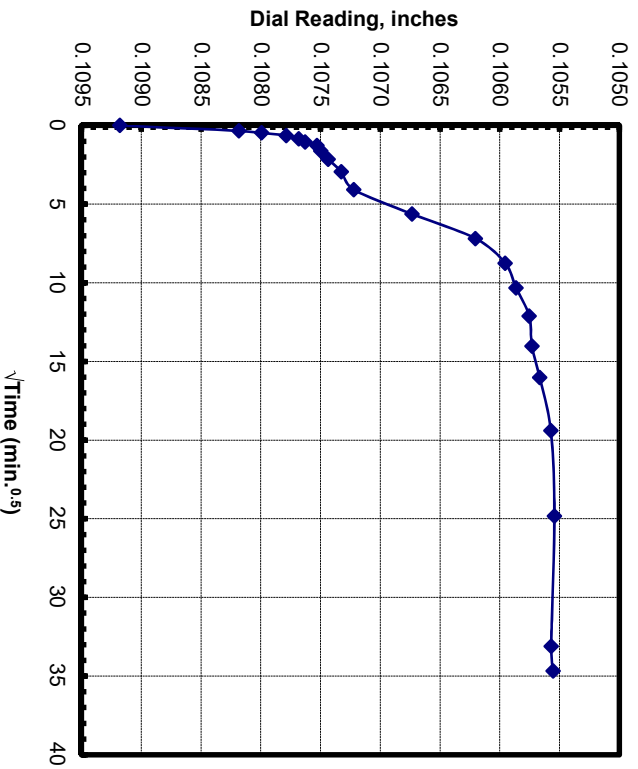
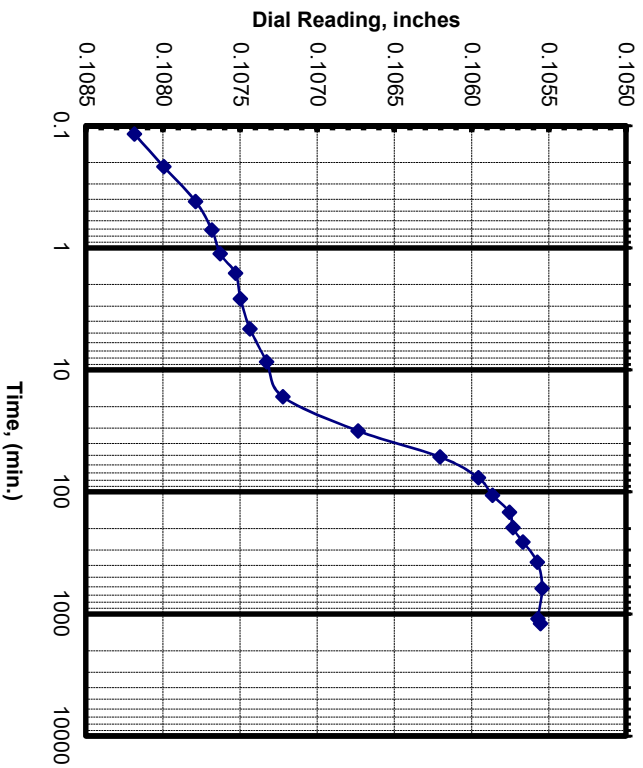


Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1120	d ₀ (in): 0.1120
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1092	d ₁₀₀ (in): 0.1092
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): -0.0028	δ _{oop} (in.): -0.0028
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min):	t ₉₀ (min):

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psi

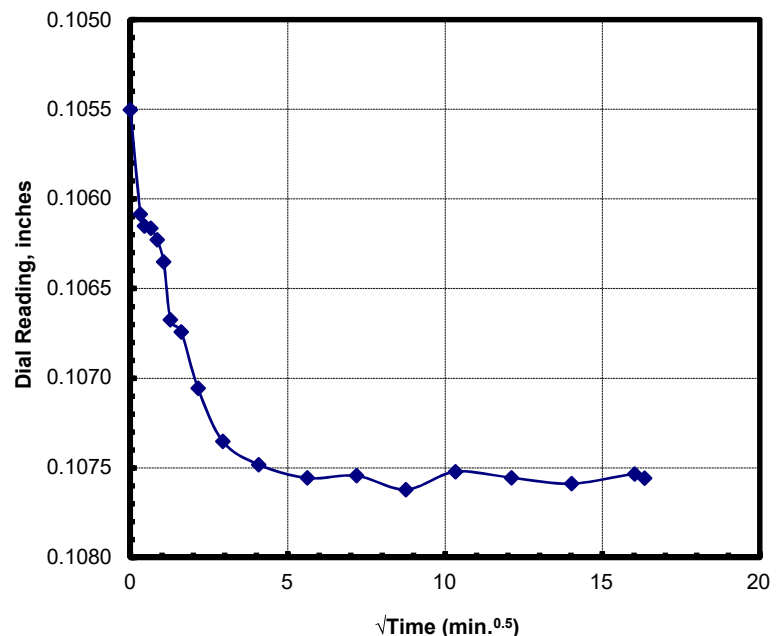
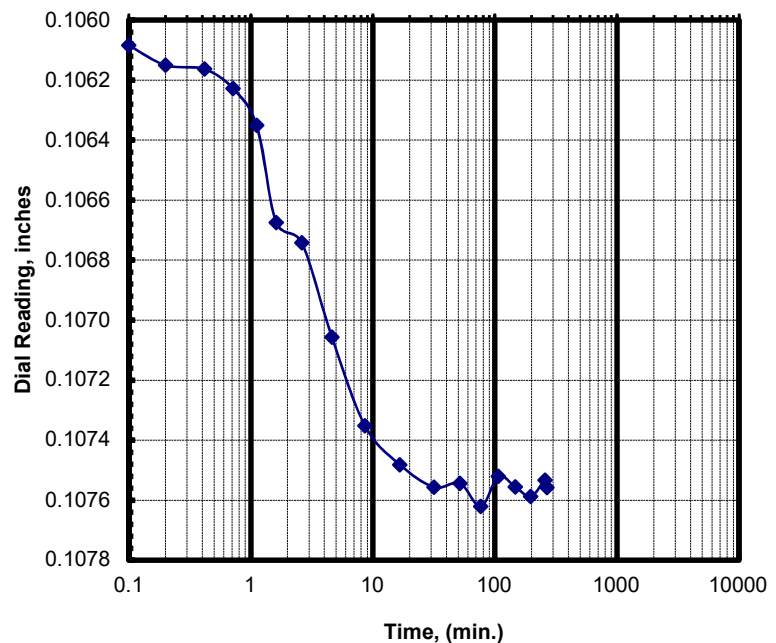
02541139 M-4 S-3 (M) 4-6' (NEW) (combined) RR

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 1 tsf

[illegible]

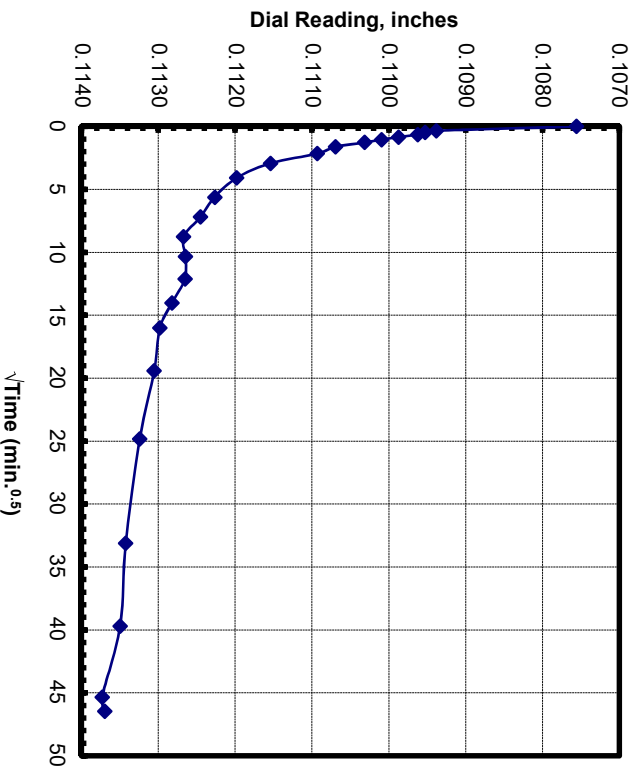
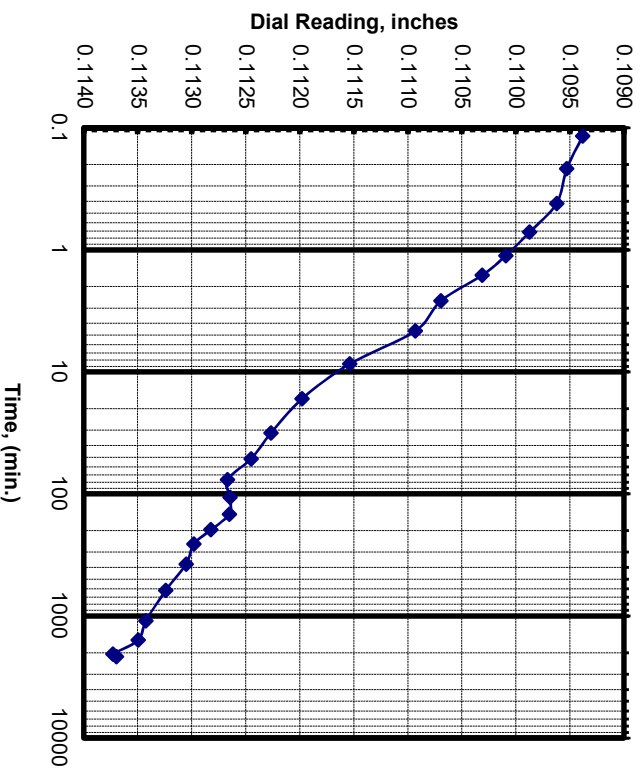
Project Number:	02541139	LL: NA	D _{90gm} (in): 0.1092	d ₀ (in): 0.1092
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1056	d ₁₀₀ (in): 0.1056
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): -0.0036	δ _{deep} (in.): -0.0036
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

[illegible]

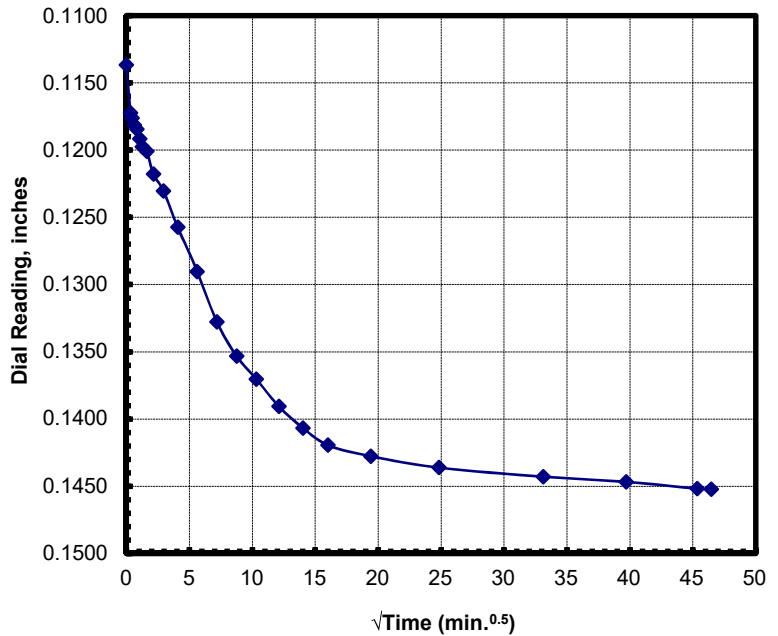
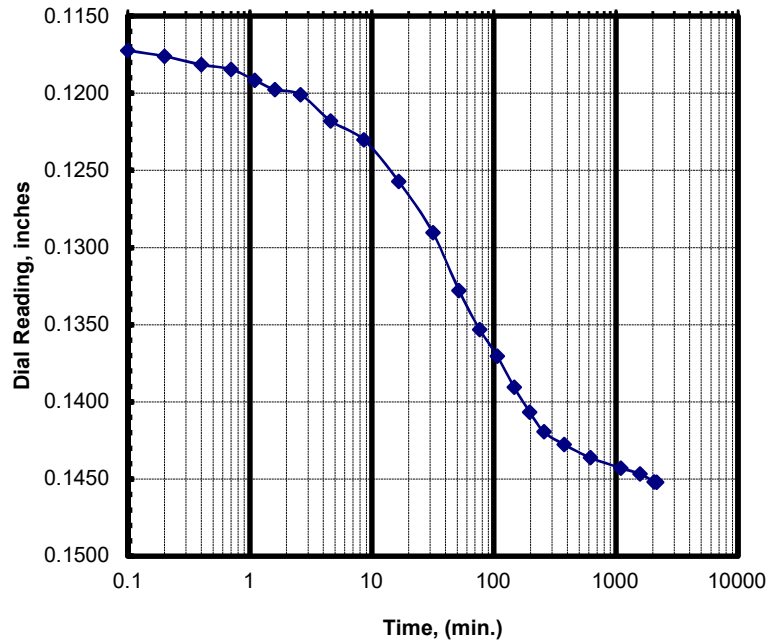
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1055	d ₀ (in): 0.1055
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1076	d ₁₀₀ (in): 0.1075
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0021	δ _{oop} (in.): 0.0020
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min):	t ₉₀ (min):

t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

Project Number:	02541139	LL: NA	D _{60(mm)} (in): 0.1076	d ₆ (in): 0.1076
Project Name:	CPRA PO 181	PL: NA	D _{and} (in): 0.1137	d ₁₀₀ (in): 0.1126
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δh _i (in.): 0.0061	δ _{deep} (in.): 0.0050
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min):	t ₅₀ (min):

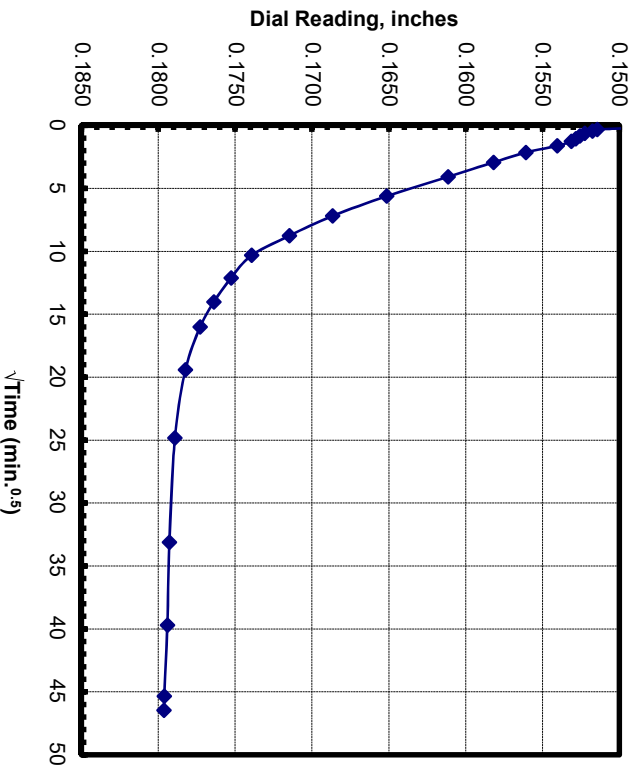
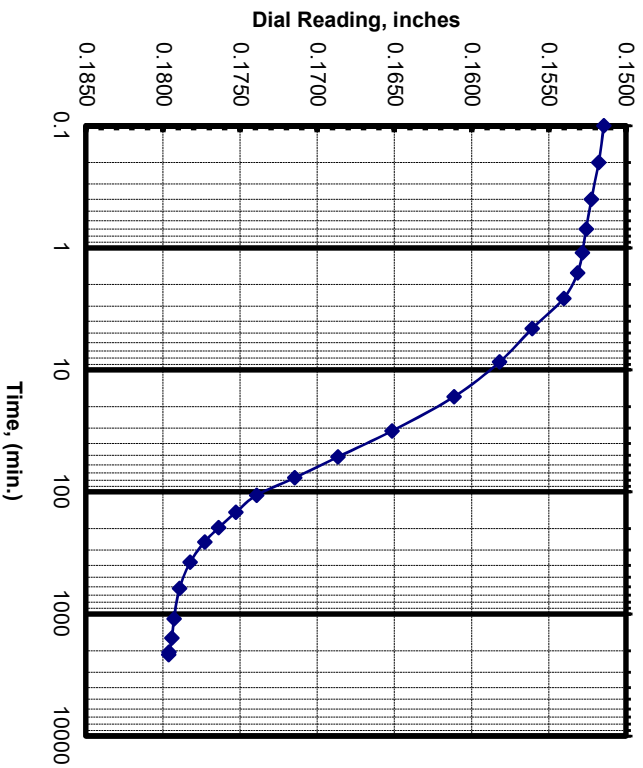
CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 8 tsf

[illegible]

Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1137	d ₀ (in): 0.1137
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1452	d ₁₀₀ (in): 0.1429
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0315	δ _{eop} (in.): 0.0292
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min): 29	t ₉₀ (min): 180

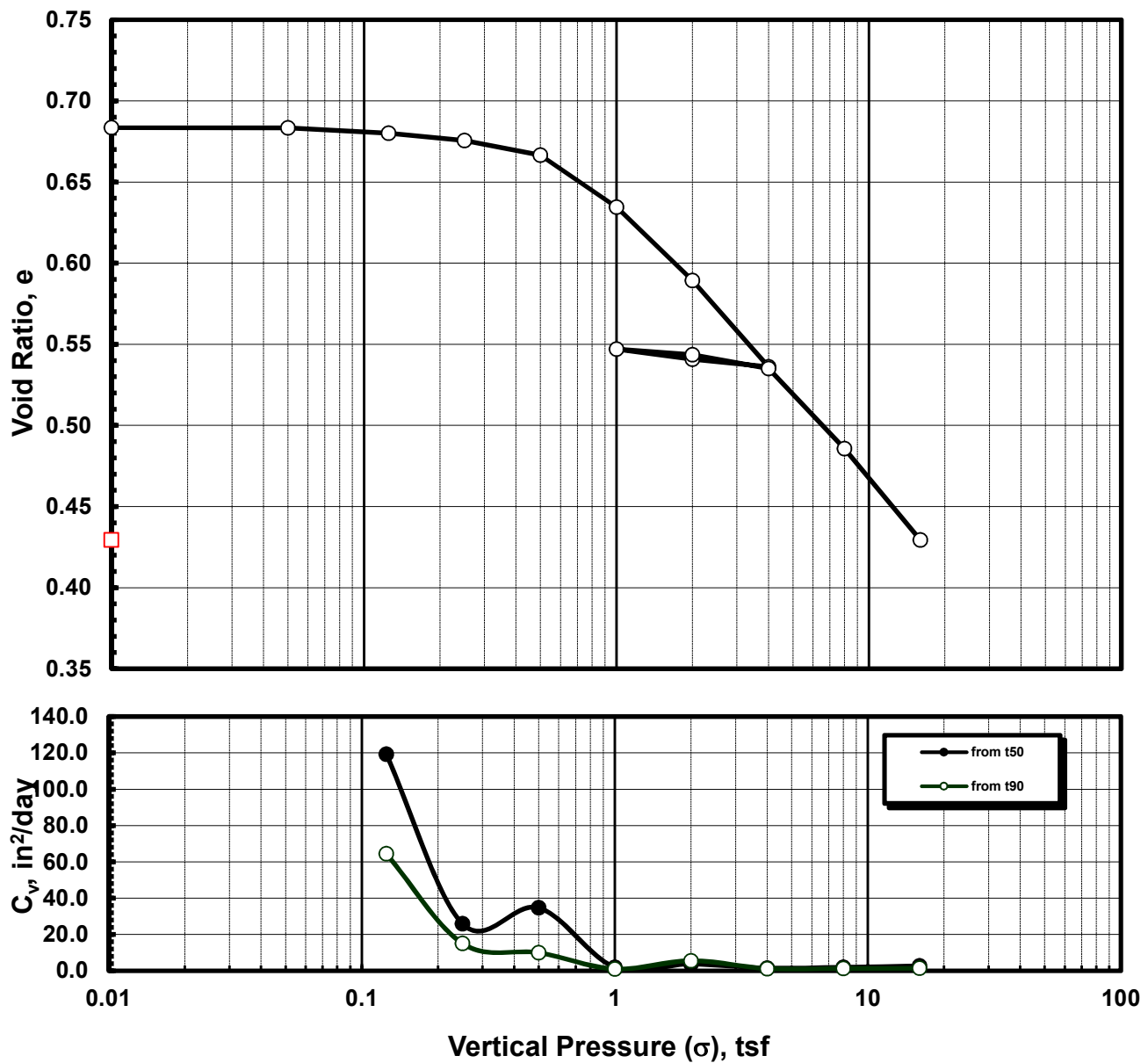
02541139 M-4 S-3 (M) 4-6' (NEW) (combined) RR

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 16 tsf

[illegible]

Project Number:	02541139	LL: NA	D _{90gm} (in.): 0.1452	d ₀ (in.): 0.1452
Project Name:	CPRA PO 181	PL: NA	D _{end} (in.): 0.1796	d ₁₀₀ (in.): 0.1786
Boring & Depth:	M-4, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0344	δ _{exp} (in.): 0.0334
Sample:	Gray LEAN CLAY WITH SAND	- 200 (%): 77	t ₅₀ (min): 19	t ₉₀ (min): 160

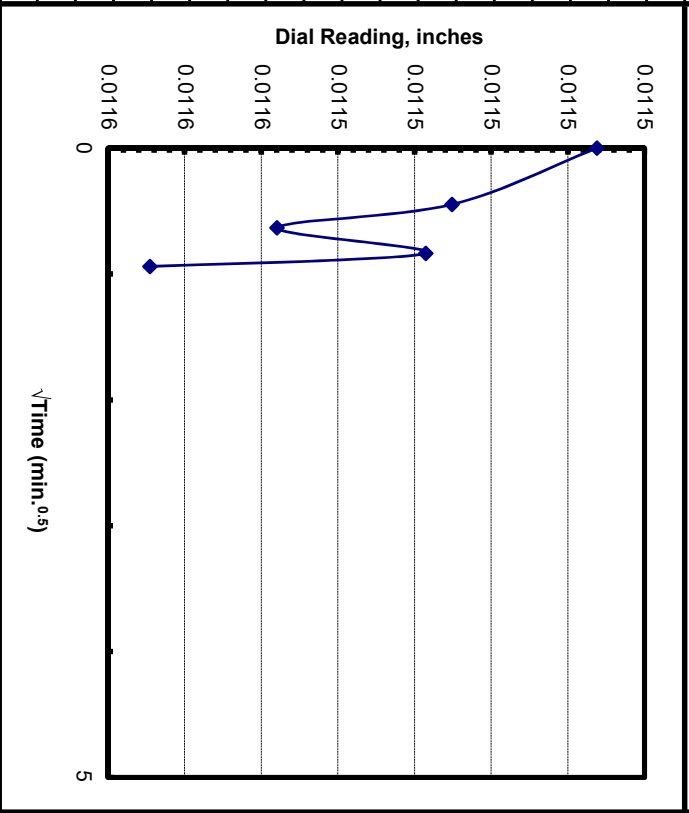
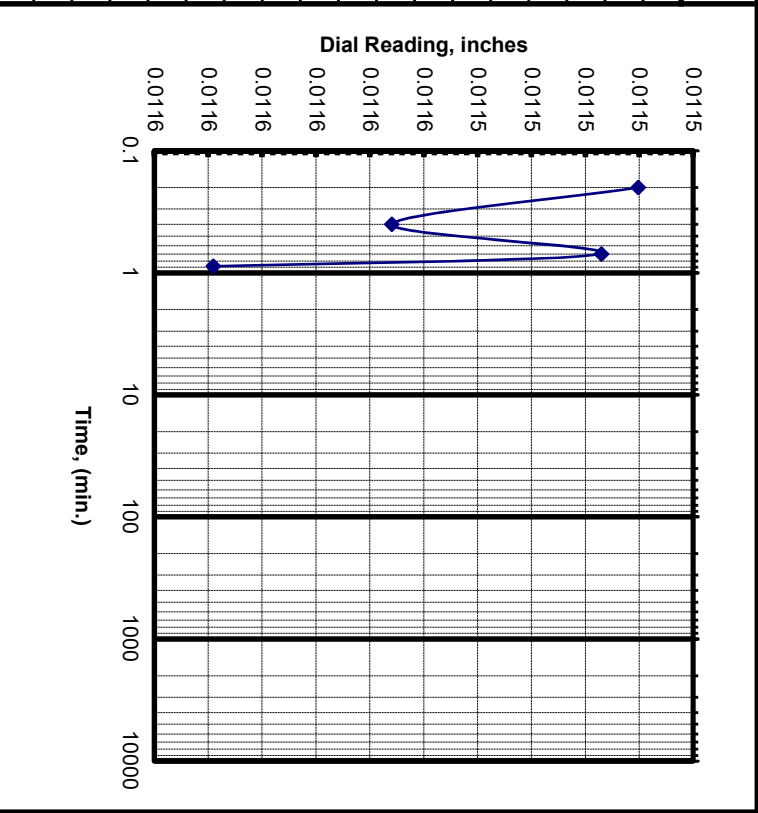
CONSOLIDATION TEST RESULTS - ASTM D 2435



Sample Condition	Before	After	Consolidation Parameters	
Moisture Content, %:	25.2%	16.0%	Overburden Pressure, tsf:	0.2
Sample Height, in.:	0.9970	0.8465	Preconsolidation Pressure, tsf:	0.9
Void Ratio, e:	0.6835	0.4293	Compression Index, (Cc):	0.187
Unit Weight, pcf:	99.4	117.1	Re-Compression Index, (Cr)	0.020
Degree of Saturation:	0.99	1.00	Swell Index, Cs:	-
Project Number: 02541139			Liquid Limit: NA	Hand Pen: 0.25 tsf
Project Name: CPRA PO 181			Plastic Limit: NA	-200 %: 77
Boring No.: M-4			Plasticity Index: NA	SPG: 2.682
Sample ID, Depth: S-3, 4-6 ft			Classification:	Gray LEAN CLAY WITH SAND

**CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.05 tsf**

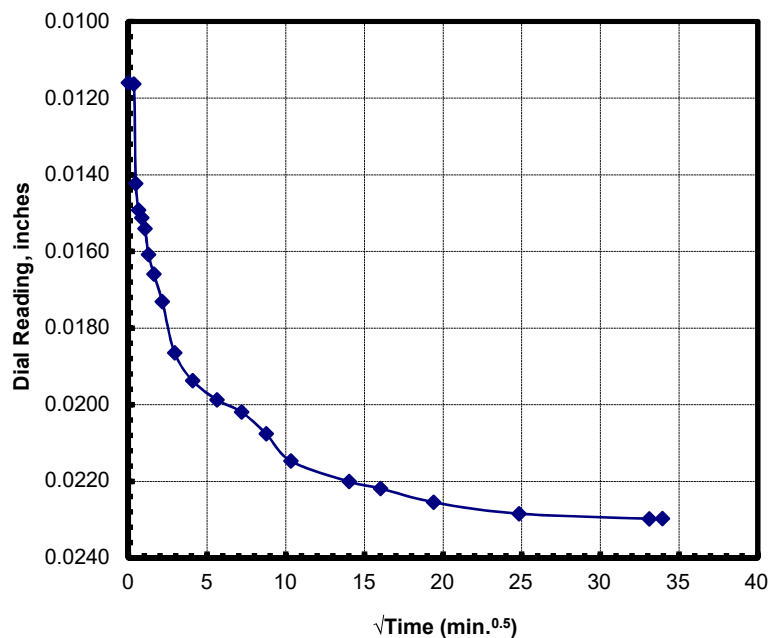
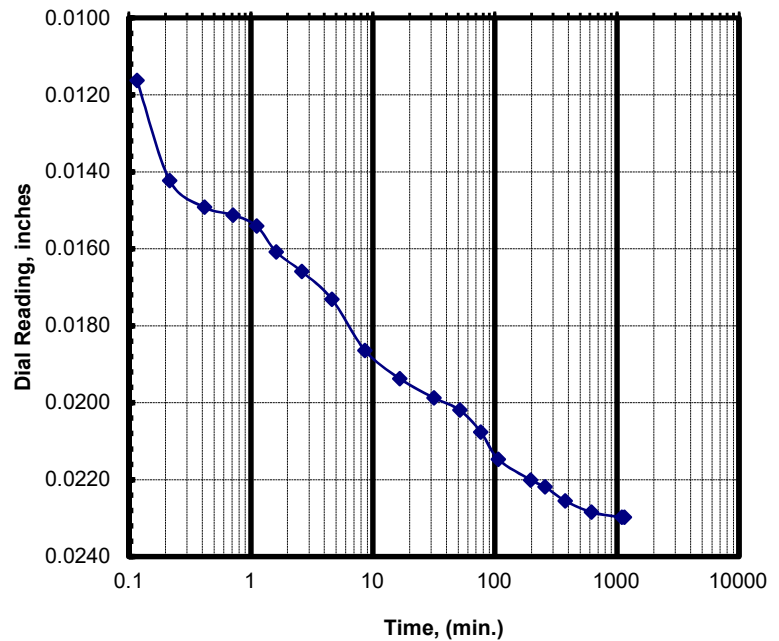
The graph displays the 'Yes' vote share over time. The Y-axis is labeled 'Yes' and has a scale from 0.0115 to 0.0115. The X-axis represents time from 1999 to 2005. The data shows a sharp increase in 'Yes' votes starting around 2002, reaching a peak of approximately 0.0115 in early 2005, and then a slight decline towards the end of 2005.



LL: NA	$D_{0(\text{mm})}$ (in.): 0.0115	d_0 (in.): 0.0115
PL: NA	D_{end} (in.): 0.0116	d_{100} (in.): 0.0116
PL: NA	Δh_1 (in.): 0.0001	δ_{exp} (in.): 0.0001
- 200 (%): NA	t_{50} (min):	t_{90} (min):

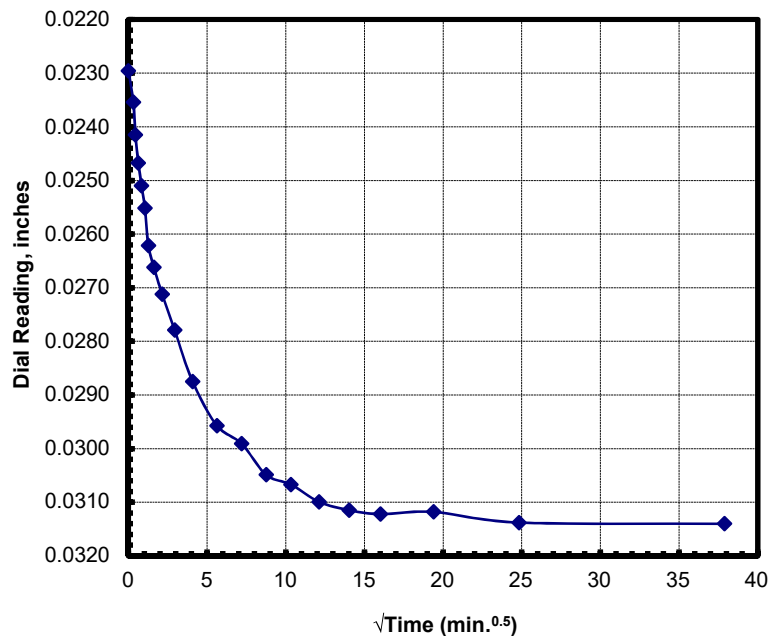
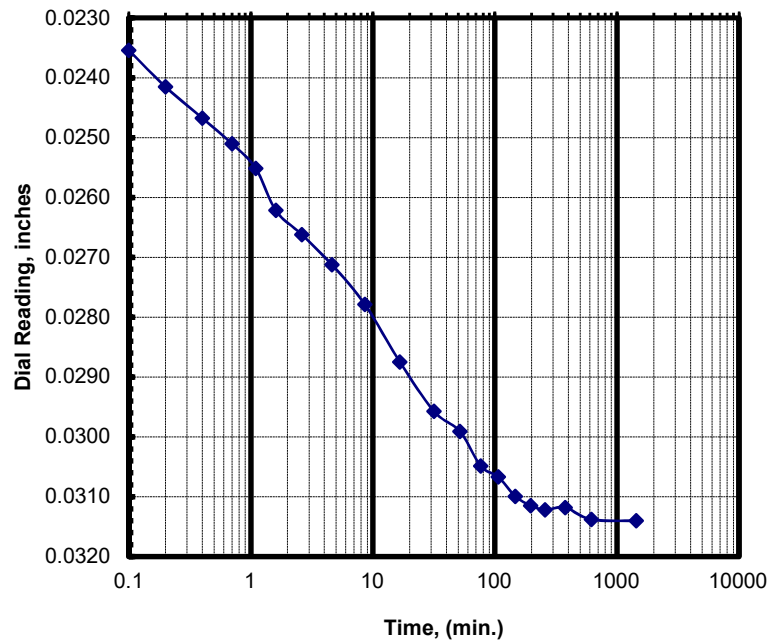
t50 & t90 Curves for the Vertical Stress of 0.125 tsf

Project Number:	02541139
Project Name:	CPRA PO 181
Boring & Depth:	M-5, S-2, 2-4 ft
Sample:	Sandy Lean Clay (CL), Light Gray



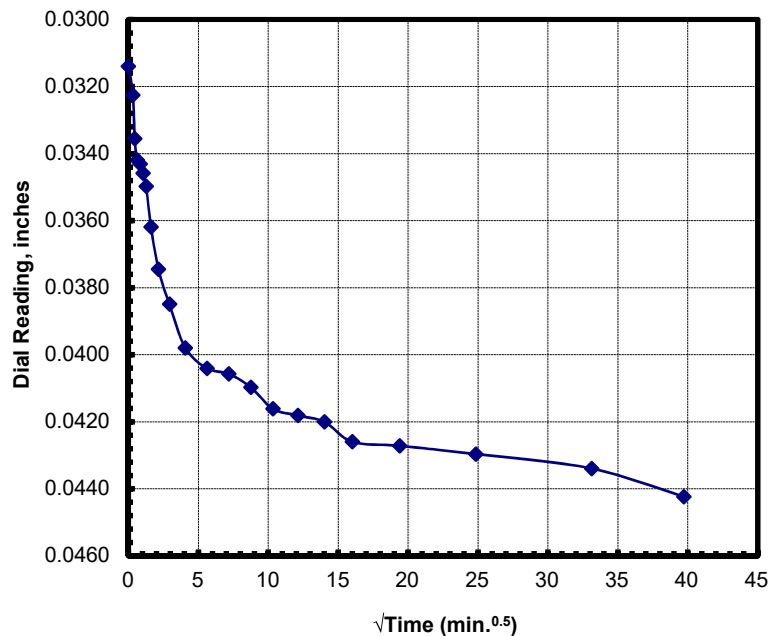
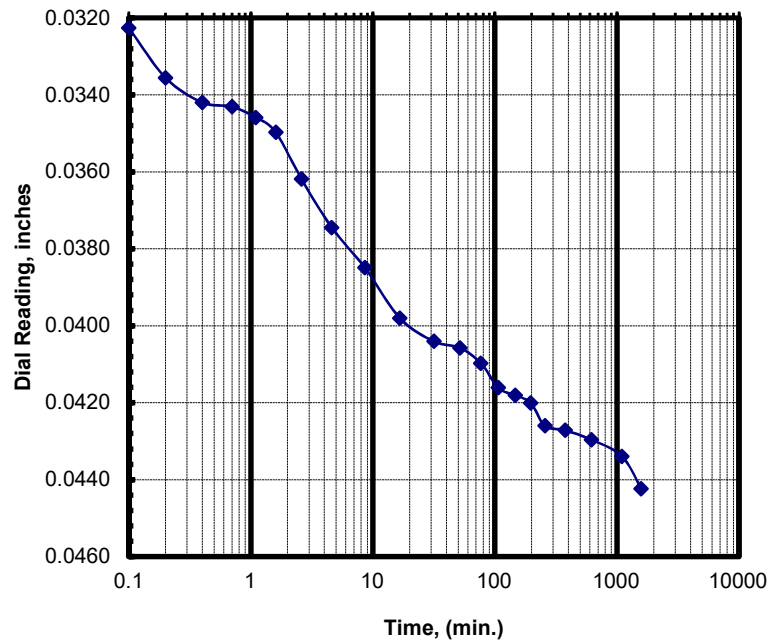
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0116	d ₀ (in): 0.0116
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0230	d ₁₀₀ (in): 0.0192
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): 0.0114	δ _{eop} (in.): 0.0076
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%): NA	t ₅₀ (min): 1	t ₉₀ (min): 7.7

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.25 tsf

[illegible]

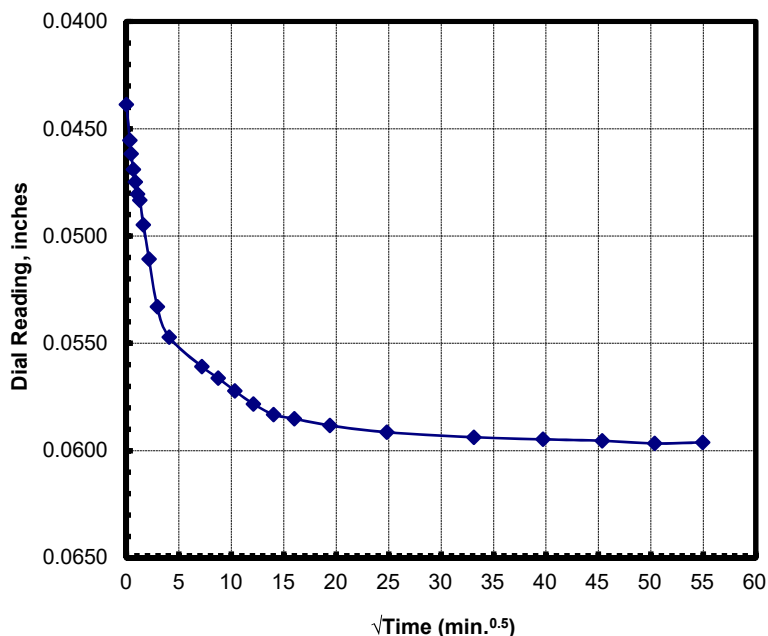
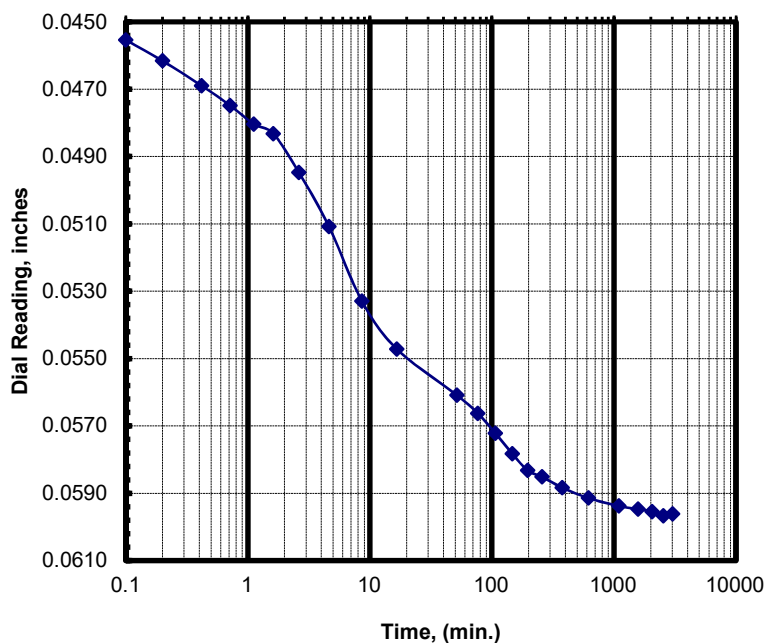
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0230	d ₀ (in): 0.0230
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0314	d ₁₀₀ (in): 0.0308
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): 0.0084	δ _{oop} (in.): 0.0078
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%): NA	t ₅₀ (min): 3.7	t ₉₀ (min): 57

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.5 tsf

[illegible]

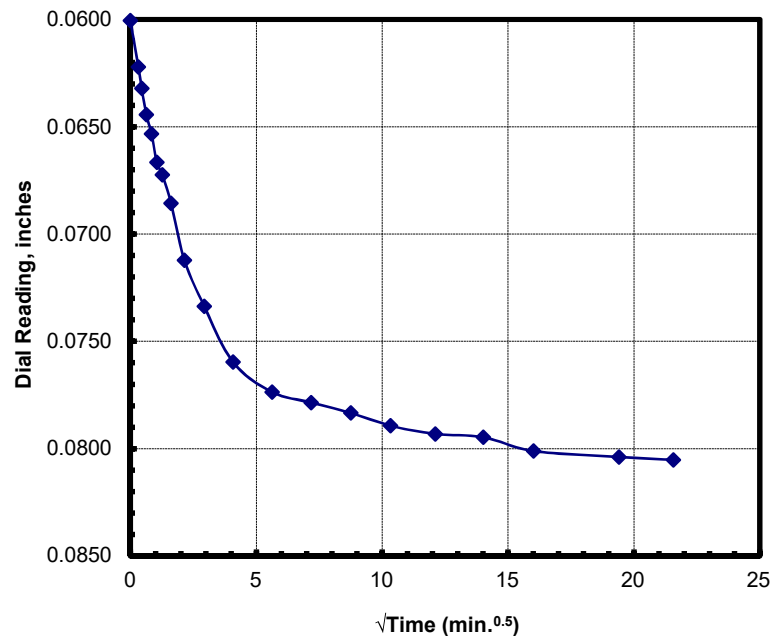
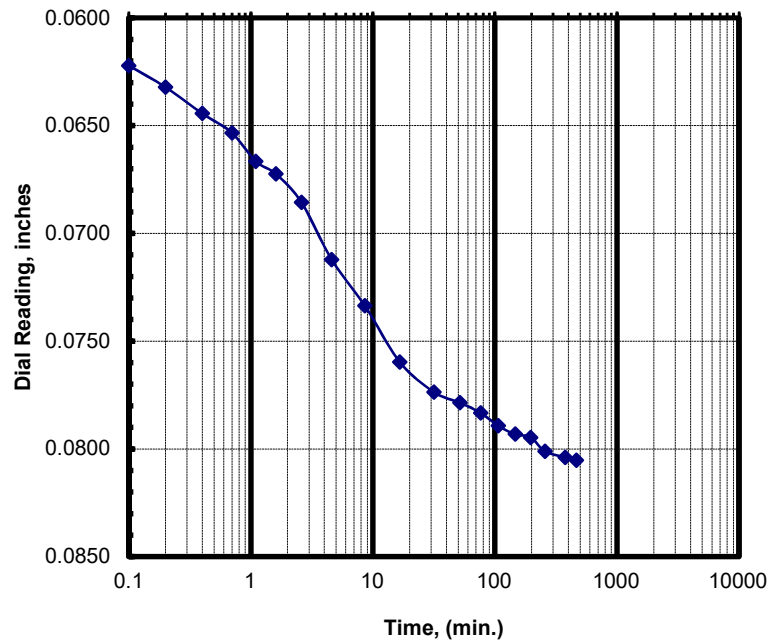
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0314	d ₀ (in): 0.0314
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0439	d ₁₀₀ (in): 0.0415
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): 0.0125	δ _{oop} (in.): 0.0101
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%): NA	t ₅₀ (min): 2.9	t ₉₀ (min): 32

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 1 tsf

[illegible]

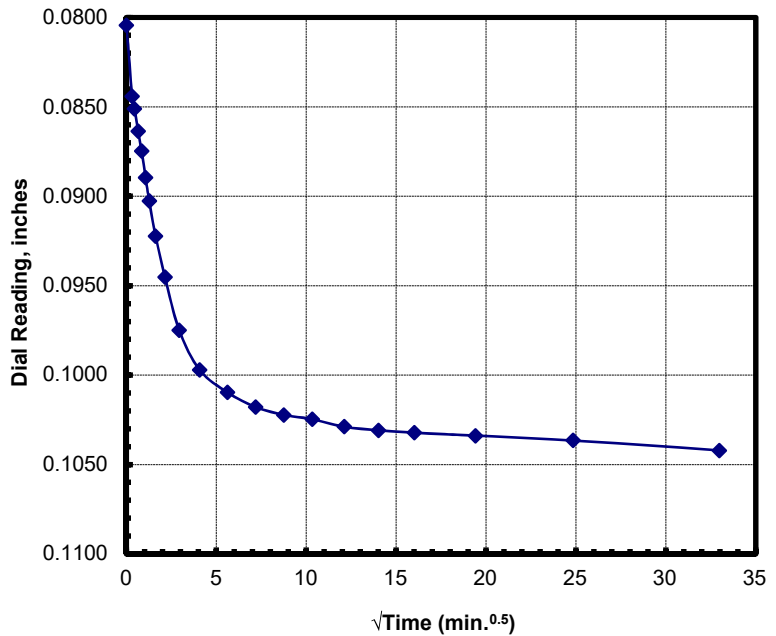
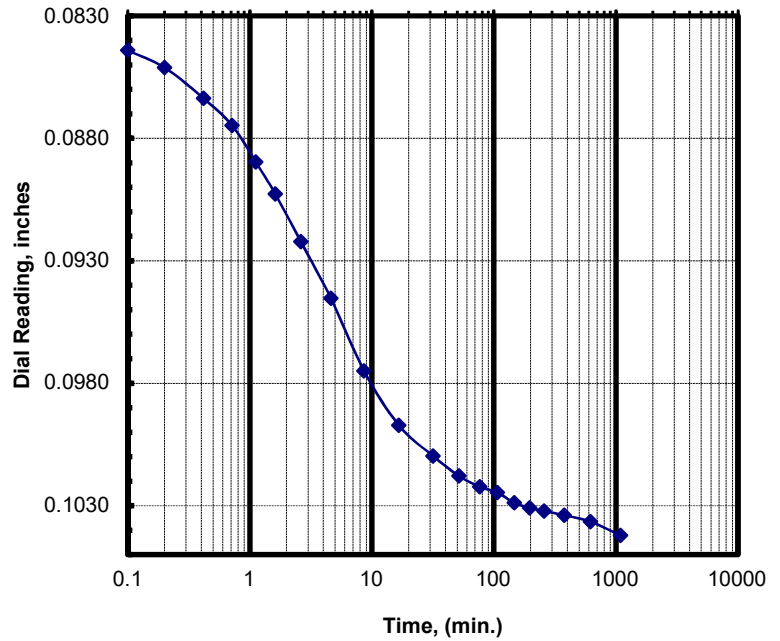
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0439	d ₀ (in): 0.0439
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0601	d ₁₀₀ (in): 0.0563
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hl} (in.): 0.0162	δ _{exp} (in.): 0.0124
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%) : NA	t ₅₀ (min): 3.2	t ₉₀ (min): 20

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

[illegible]

Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0601	d ₀ (in): 0.0601
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0805	d ₁₀₀ (in): 0.0771
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): 0.0205	δ _{eop} (in.): 0.0170
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%): NA	t ₅₀ (min): 2.6	t ₉₀ (min): 15

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 4 tsf

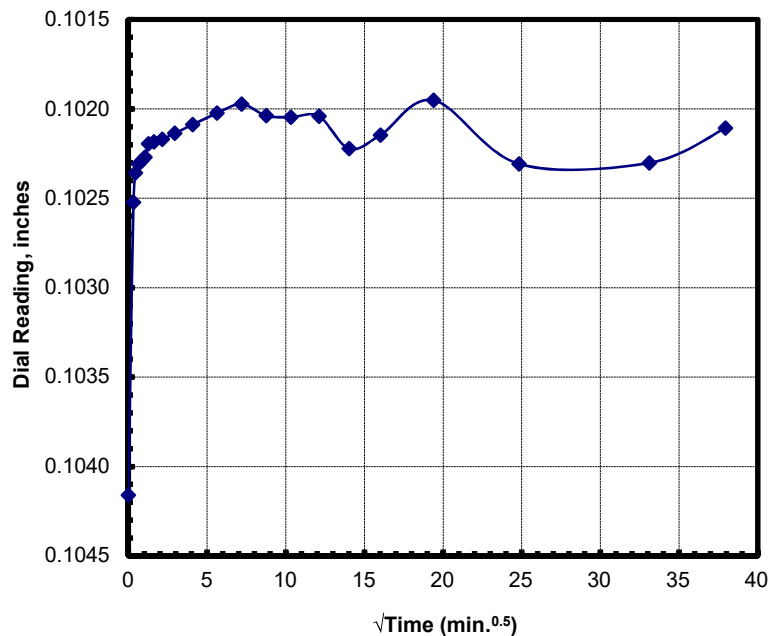
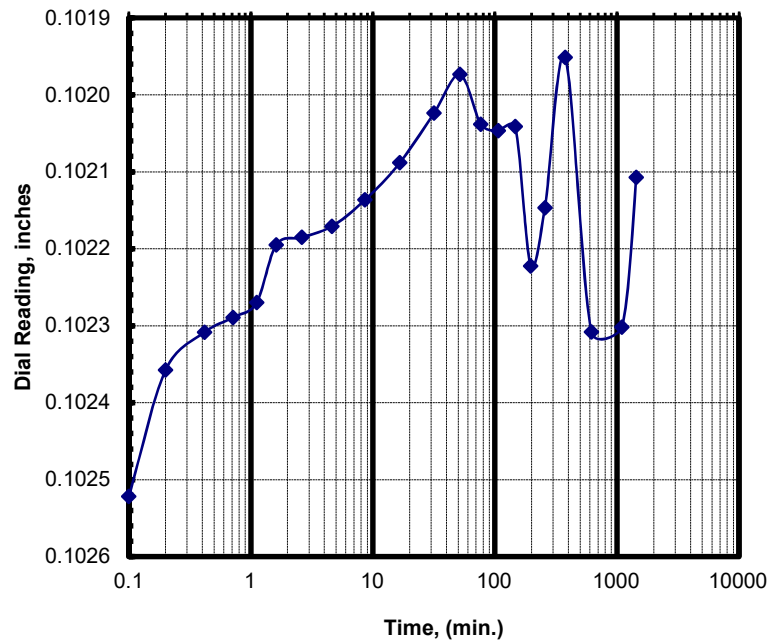
[illegible]

Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0804	d ₀ (in): 0.0804
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1042	d ₁₀₀ (in): 0.1012
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): 0.0238	δ _{exp} (in.): 0.0208
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%): NA	t ₅₀ (min): 1.8	t ₉₀ (min): 14

intertek
psi

02541139 M-5 S-2 (B) 2-4'

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 2 tsf

[illegible]

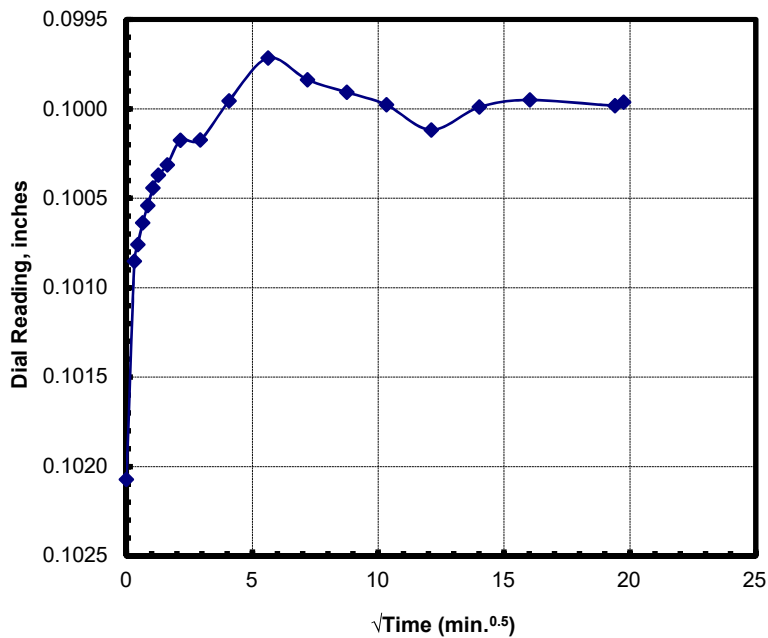
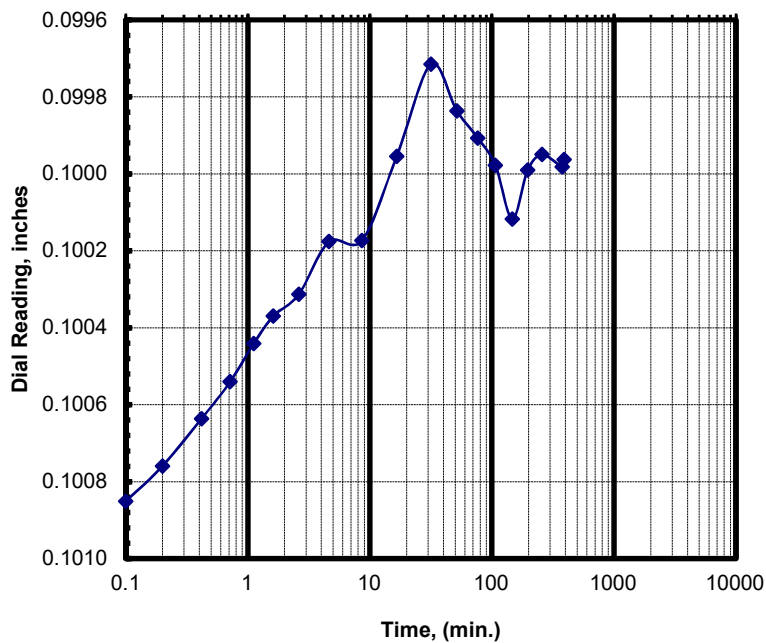
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1042	d ₀ (in): 0.1042
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1021	d ₁₀₀ (in): 0.1021
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): -0.0021	δ _{oop} (in.): -0.0021
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%) : NA	t ₅₀ (min):	t ₉₀ (min):

intertek
psi

02541139 M-5 S-2 (B) 2-4'

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 1 tsf

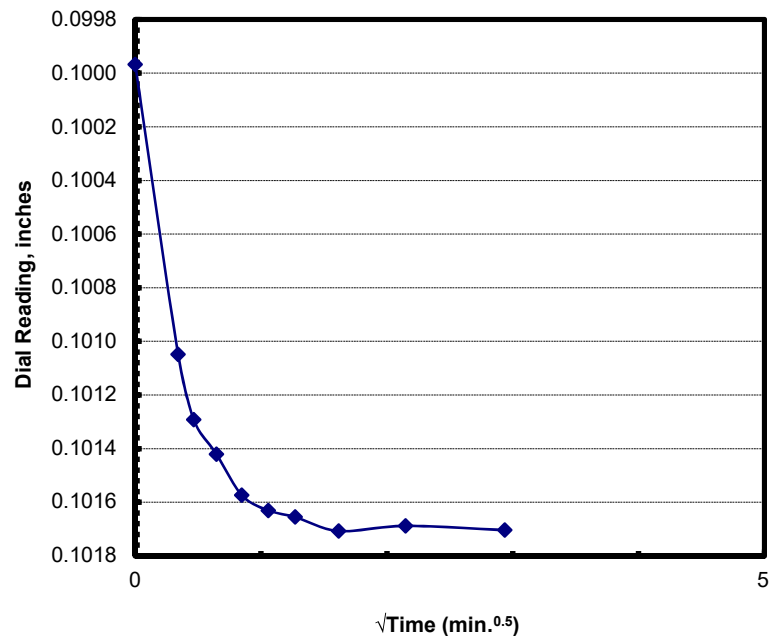
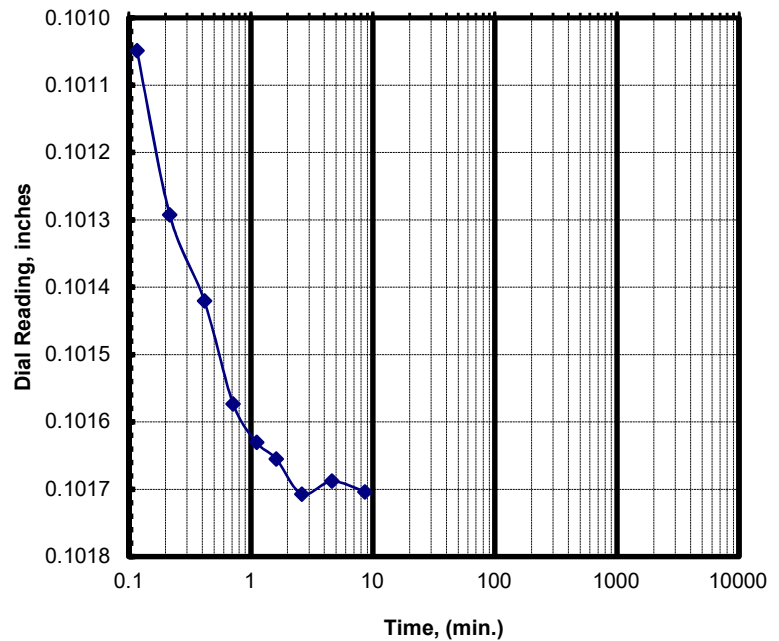
Date & Time	Elasped Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
10/21/20 8:52:42 AM	0.00	0.00	0.1021
10/21/20 8:52:48 AM	0.10	0.32	0.1009
10/21/20 8:52:54 AM	0.20	0.45	0.1008
10/21/20 8:53:07 AM	0.42	0.65	0.1006
10/21/20 8:53:25 AM	0.72	0.85	0.1005
10/21/20 8:53:49 AM	1.12	1.06	0.1004
10/21/20 8:54:19 AM	1.62	1.27	0.1004
10/21/20 8:55:19 AM	2.62	1.62	0.1003
10/21/20 8:57:19 AM	4.62	2.15	0.1002
10/21/20 9:01:19 AM	8.62	2.94	0.1002
10/21/20 9:09:19 AM	16.62	4.08	0.1000
10/21/20 9:24:19 AM	31.62	5.62	0.0997
10/21/20 9:44:19 AM	51.62	7.18	0.0998
10/21/20 10:09:19 AM	76.62	8.75	0.0999
10/21/20 10:39:20 AM	106.63	10.33	0.1000
10/21/20 11:19:20 AM	146.63	12.11	0.1001
10/21/20 12:09:20 PM	196.63	14.02	0.1000
10/21/20 1:09:20 PM	256.63	16.02	0.0999
10/21/20 3:09:20 PM	376.63	19.41	0.1000
10/21/20 3:22:42 PM	390.00	19.75	0.1000



Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1021	d ₀ (in): 0.1021
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1000	d ₁₀₀ (in): 0.1000
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): -0.0021	δ _{eop} (in.): -0.0021
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%) : NA	t ₅₀ (min):	t ₉₀ (min):

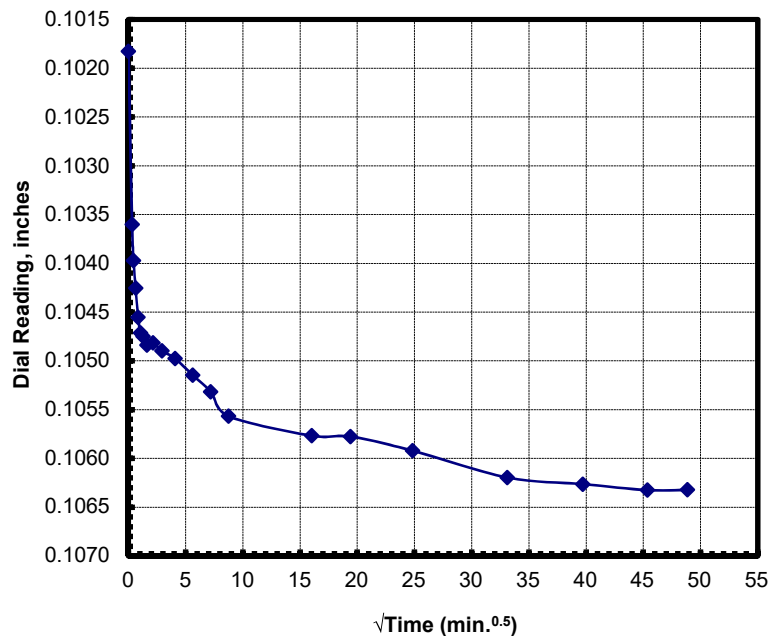
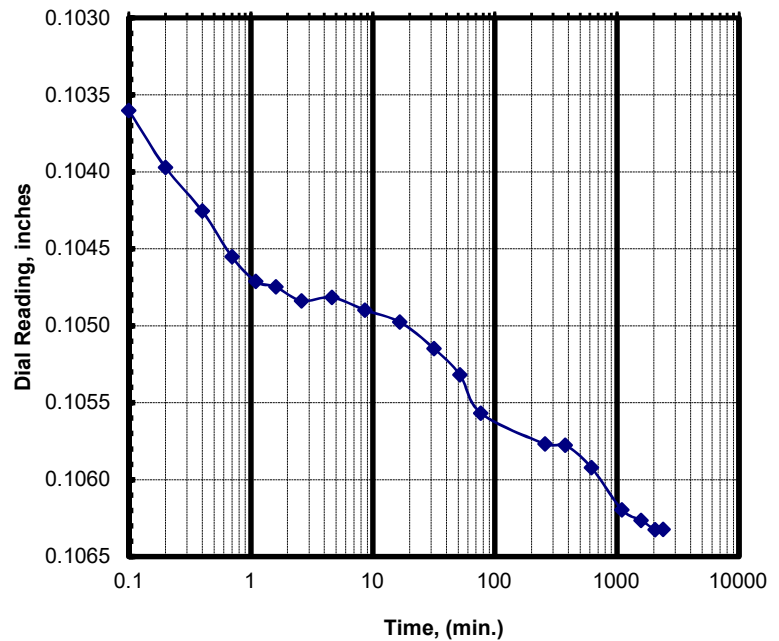
CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

Date & Time	Elaspsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
10/21/20 3:22:42 PM	0.00	0.00	0.1000
10/21/20 3:22:49 PM	0.12	0.34	0.1010
10/21/20 3:22:55 PM	0.22	0.47	0.1013
10/21/20 3:23:07 PM	0.42	0.65	0.1014
10/21/20 3:23:25 PM	0.72	0.85	0.1016
10/21/20 3:23:49 PM	1.12	1.06	0.1016
10/21/20 3:24:19 PM	1.62	1.27	0.1017
10/21/20 3:25:19 PM	2.62	1.62	0.1017
10/21/20 3:27:19 PM	4.62	2.15	0.1017
10/21/20 3:31:19 PM	8.62	2.94	0.1017
10/21/20 3:39:19 PM	16.62	4.08	0.1019
10/21/20 3:54:19 PM	31.62	5.62	0.1018
10/21/20 4:14:19 PM	51.62	7.18	0.1019
10/21/20 4:39:19 PM	76.62	8.75	0.1018
10/21/20 5:09:19 PM	106.62	10.33	0.1018
10/21/20 5:10:46 PM	108.07	10.40	0.1018



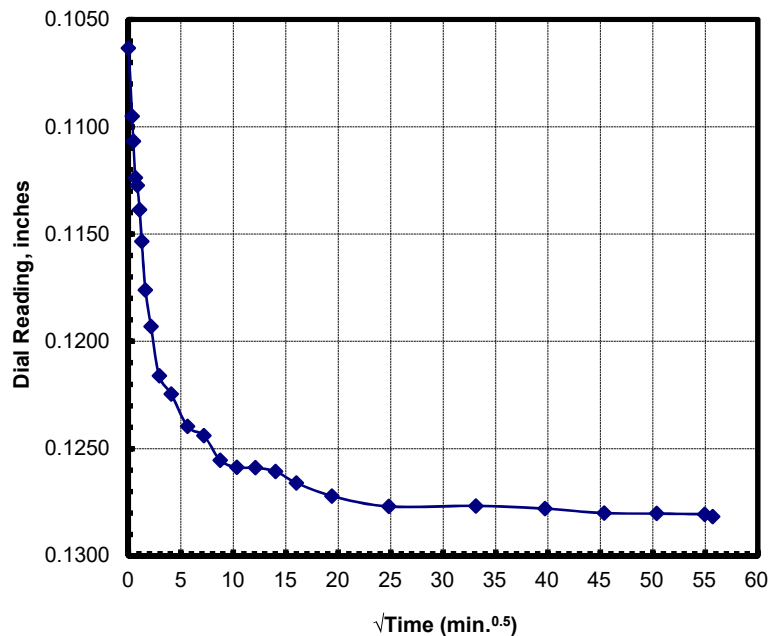
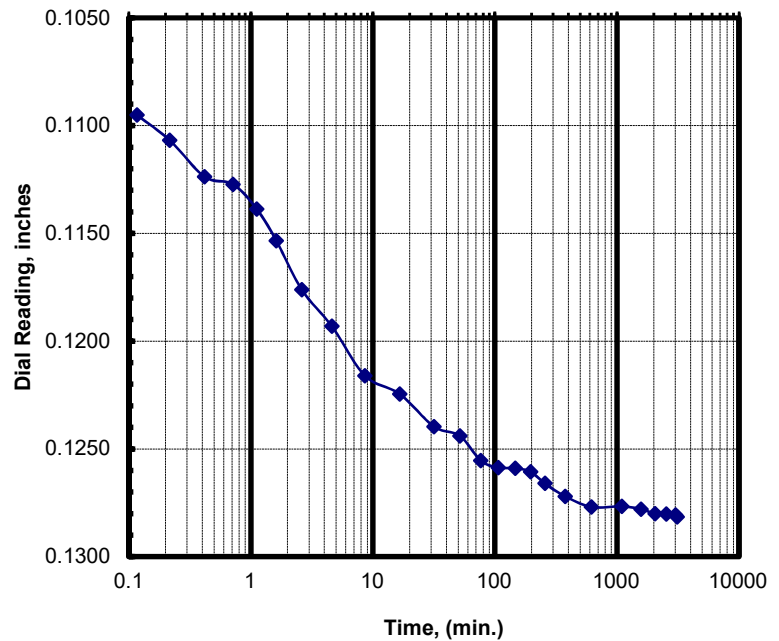
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1000	d ₀ (in): 0.1000
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1018	d ₁₀₀ (in): 0.1017
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): 0.0019	δ _{oop} (in.): 0.0018
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

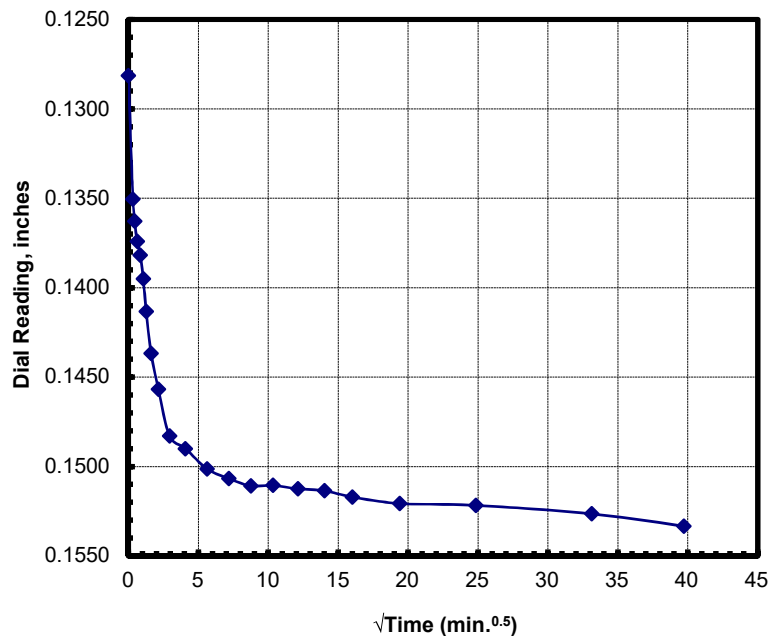
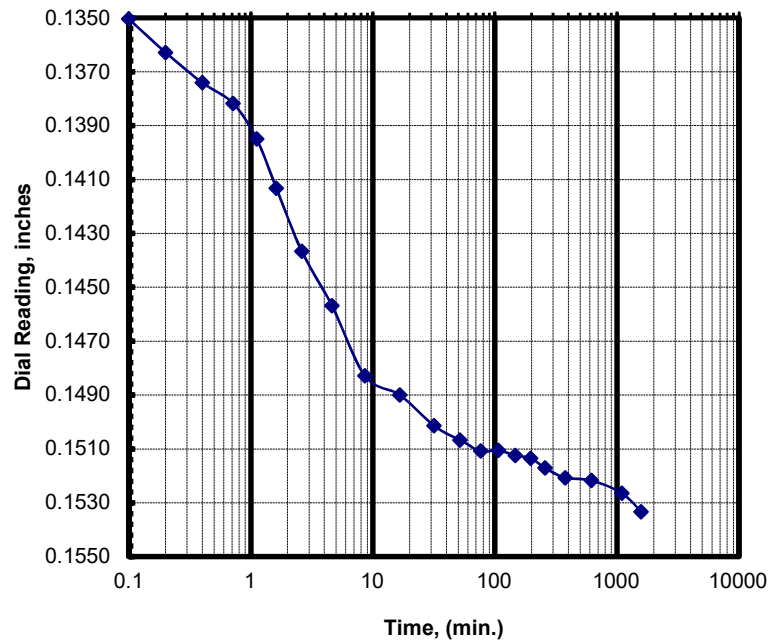
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1018	d ₀ (in): 0.1018
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1063	d ₁₀₀ (in): 0.1052
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): 0.0045	δ _{oop} (in.): 0.0034
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 8 tsf

[illegible]

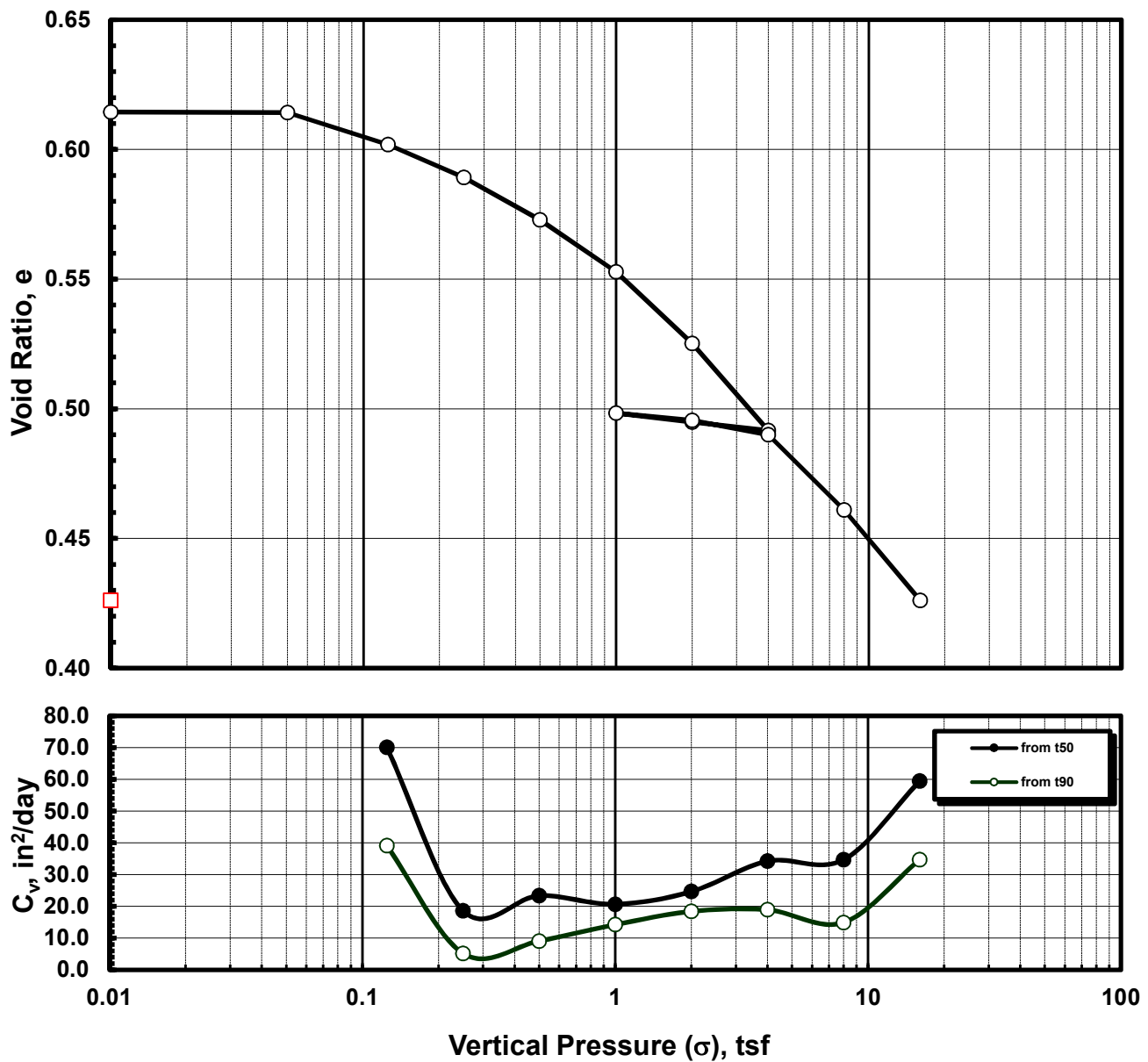
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1063	d ₀ (in): 0.1063
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1282	d ₁₀₀ (in): 0.1243
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): 0.0218	δ _{oop} (in.): 0.0180
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%): NA	t ₅₀ (min): 1.7	t ₉₀ (min): 17

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 16 tsf

[illegible]

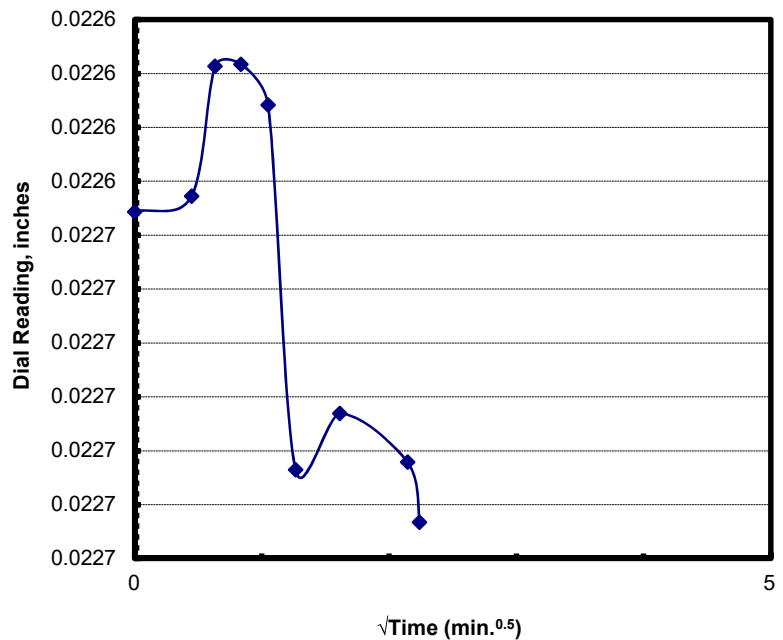
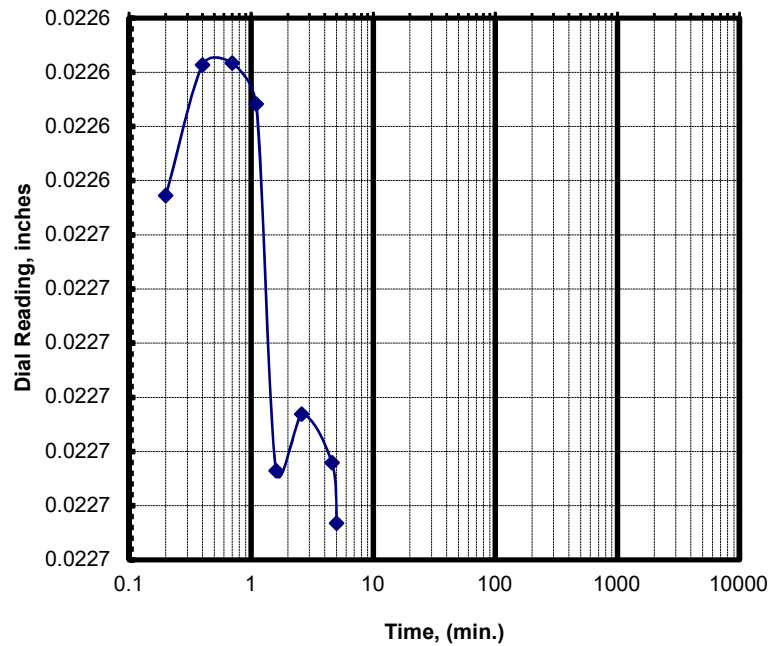
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1281	d ₀ (in): 0.1281
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1533	d ₁₀₀ (in): 0.1497
Boring & Depth:	M-5, S-2, 2-4 ft	PL: NA	Δ _{hi} (in.): 0.0252	δ _{oop} (in.): 0.0216
Sample:	Sandy Lean Clay (CL), Light Gray	- 200 (%): NA	t ₅₀ (min): 0.95	t ₉₀ (min): 7

CONSOLIDATION TEST RESULTS - ASTM D 2435



Sample Condition	Before	After	Consolidation Parameters		
Moisture Content, %:	23.1%	16.1%	Overburden Pressure, tsf:	0.2	
Sample Height, in.:	0.9980	0.8816	Preconsolidation Pressure, tsf:	0.8	
Void Ratio, e:	0.6144	0.4261	Compression Index, (Cc):	0.039	
Unit Weight, pcf:	102.5	116.1	Re-Compression Index, (Cr)	0.014	
Degree of Saturation:	1.00	1.00	Swell Index, Cs:	-	
Project Number:	02541139		Liquid Limit:	NA	Hand Pen: 0.20 tsf
Project Name	CPRA PO 181		Plastic Limit:	NA	-200 %: NA
Boring No.:	M-5		Plasticity Index:	NA	SPG: 2.653
Sample ID, Depth:	S-2, 2-4 ft		Classification:	Sandy Lean Clay (CL), Light Gray	

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.05 tsf

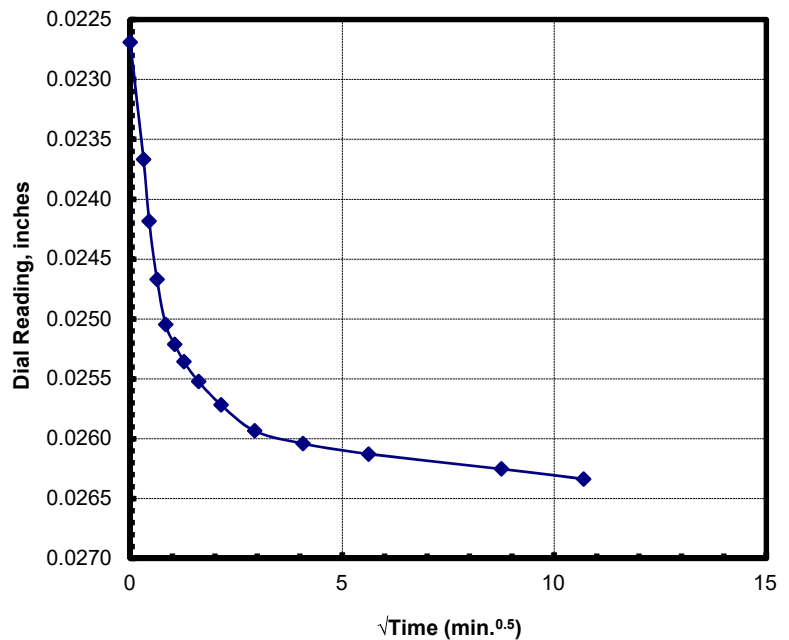
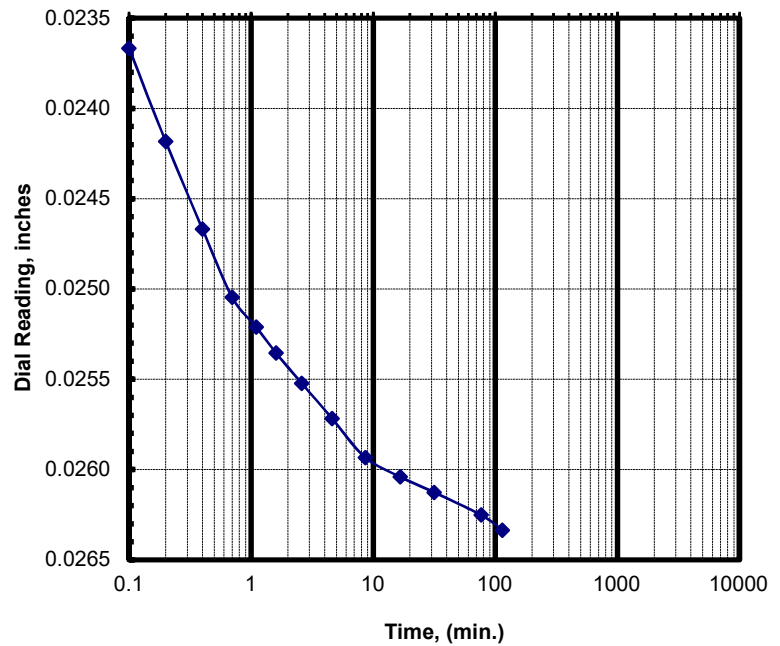
[illegible]

Project Number: 02541139	LL: 54	D _{0(min)} (in): 0.0226	d ₀ (in): 0.0226
Project Name: CPRA PO 181	PL: 22	D _{end} (in): 0.0227	d ₁₀₀ (in): 0.0227
Boring & Depth: M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): 0.0001	δ _{exp} (in.): 0.0001
Sample: Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435

t50 & t90 Curves for the Vertical Stress of 0.125 tsf

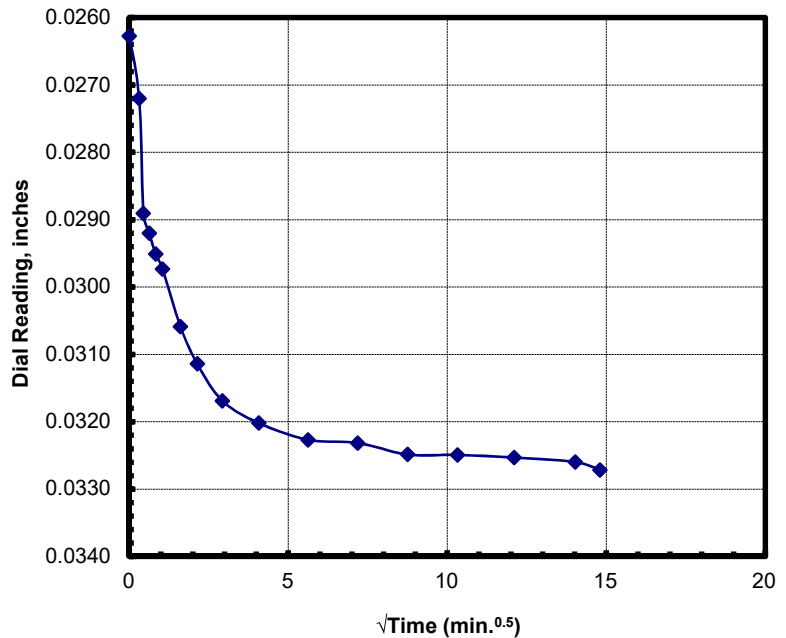
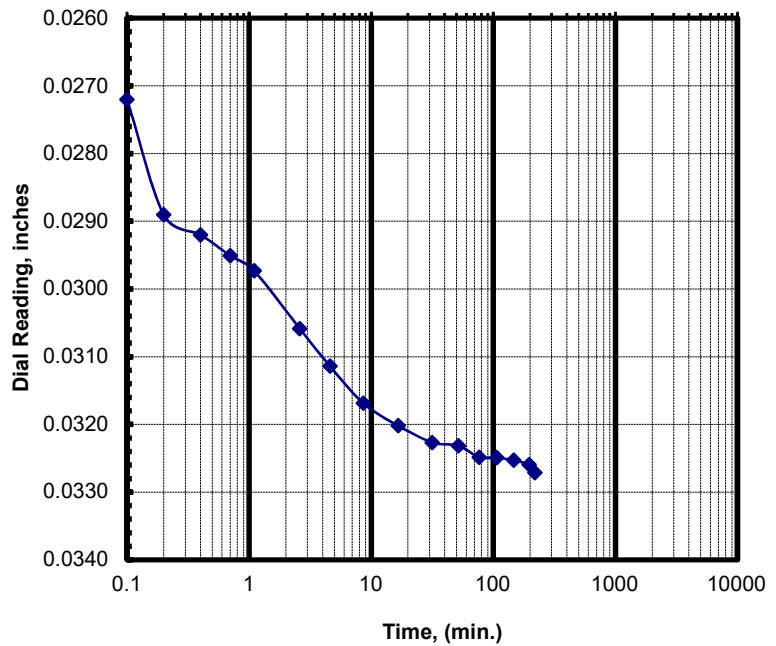
Date & Time	Elaspsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
11/24/20 10:54:17 AM	0.00	0.00	0.0227
11/24/20 10:54:23 AM	0.10	0.32	0.0237
11/24/20 10:54:29 AM	0.20	0.45	0.0242
11/24/20 10:54:41 AM	0.40	0.63	0.0247
11/24/20 10:54:59 AM	0.70	0.84	0.0250
11/24/20 10:55:23 AM	1.10	1.05	0.0252
11/24/20 10:55:53 AM	1.60	1.26	0.0254
11/24/20 10:56:53 AM	2.60	1.61	0.0255
11/24/20 10:58:53 AM	4.60	2.14	0.0257
11/24/20 11:02:53 AM	8.60	2.93	0.0259
11/24/20 11:10:53 AM	16.60	4.07	0.0260
11/24/20 11:25:53 AM	31.60	5.62	0.0261
11/24/20 11:45:53 AM	51.60	7.18	0.0263
11/24/20 12:10:53 PM	76.60	8.75	0.0263
11/24/20 12:40:53 PM	106.60	10.32	0.0262
11/24/20 12:48:36 PM	114.32	10.69	0.0263



Project Number: 02541139	LL: 54	D _{0(min)} (in): 0.0227	d ₀ (in): 0.0227
Project Name: CPRA PO 181	PL: 22	D _{end} (in): 0.0263	d ₁₀₀ (in): 0.0259
Boring & Depth: M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): 0.0036	δ _{oop} (in.): 0.0032
Sample: Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 0.24	t ₉₀ (min): 3.2

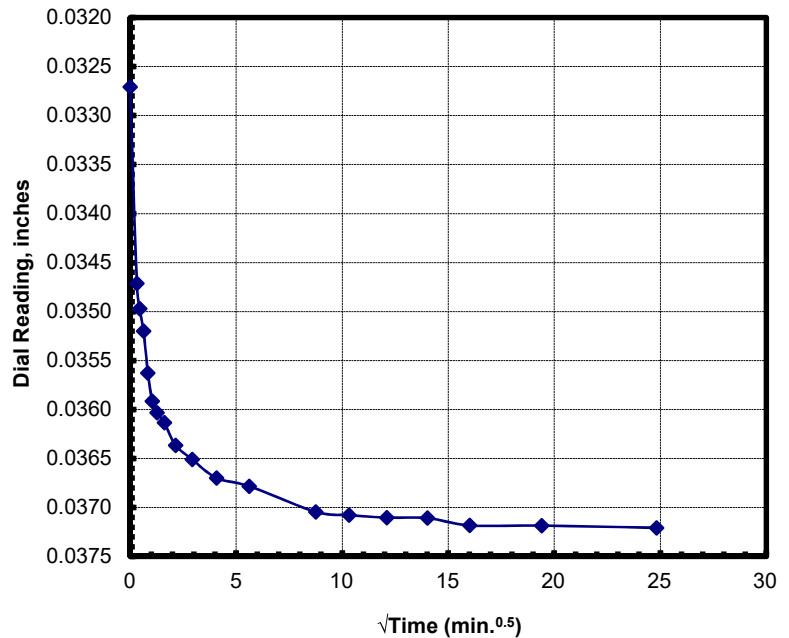
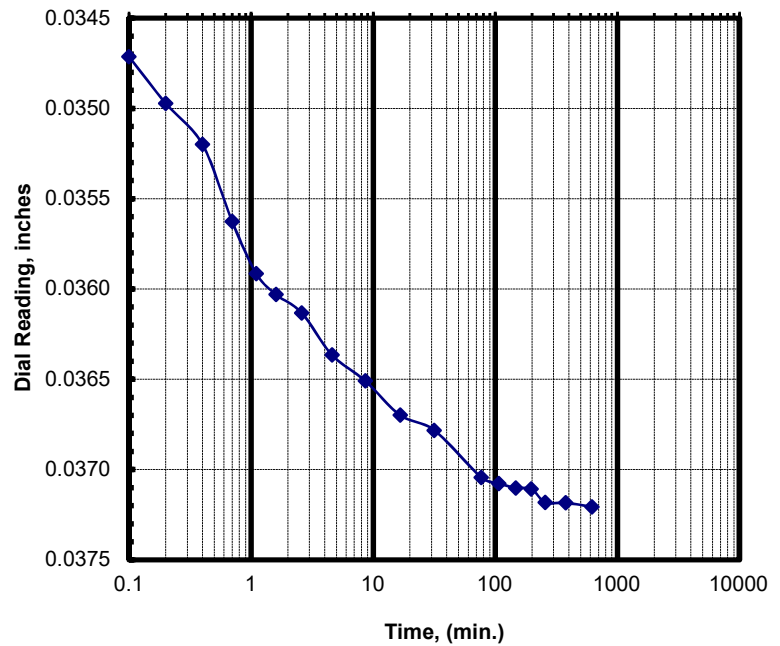
CONSOLIDATION TEST RESULTS - ASTM D 2435 ***t₅₀ & t₉₀ Curves for the Vertical Stress of 0.25 tsf***

Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
11/24/20 12:48:36 PM	0.00	0.00	0.0263
11/24/20 12:48:42 PM	0.10	0.32	0.0272
11/24/20 12:48:48 PM	0.20	0.45	0.0289
11/24/20 12:49:00 PM	0.40	0.63	0.0292
11/24/20 12:49:18 PM	0.70	0.84	0.0295
11/24/20 12:49:42 PM	1.10	1.05	0.0297
11/24/20 12:50:12 PM	1.60	1.26	0.0298
11/24/20 12:51:12 PM	2.60	1.61	0.0306
11/24/20 12:53:12 PM	4.60	2.14	0.0311
11/24/20 12:57:12 PM	8.60	2.93	0.0317
11/24/20 1:05:12 PM	16.60	4.07	0.0320
11/24/20 1:20:12 PM	31.60	5.62	0.0323
11/24/20 1:40:13 PM	51.62	7.18	0.0323
11/24/20 2:05:13 PM	76.62	8.75	0.0325
11/24/20 2:35:13 PM	106.62	10.33	0.0325
11/24/20 3:15:13 PM	146.62	12.11	0.0325
11/24/20 4:05:13 PM	196.62	14.02	0.0326
11/24/20 4:27:37 PM	219.02	14.80	0.0327



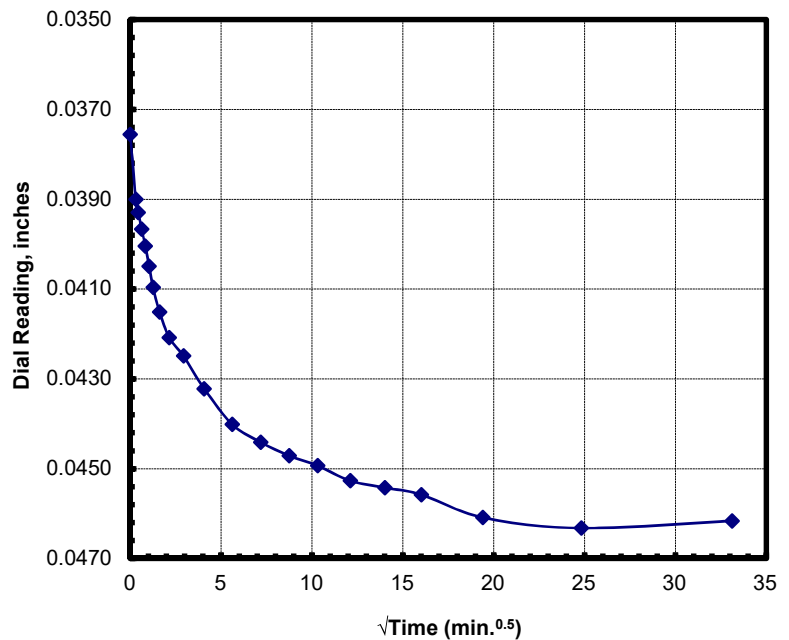
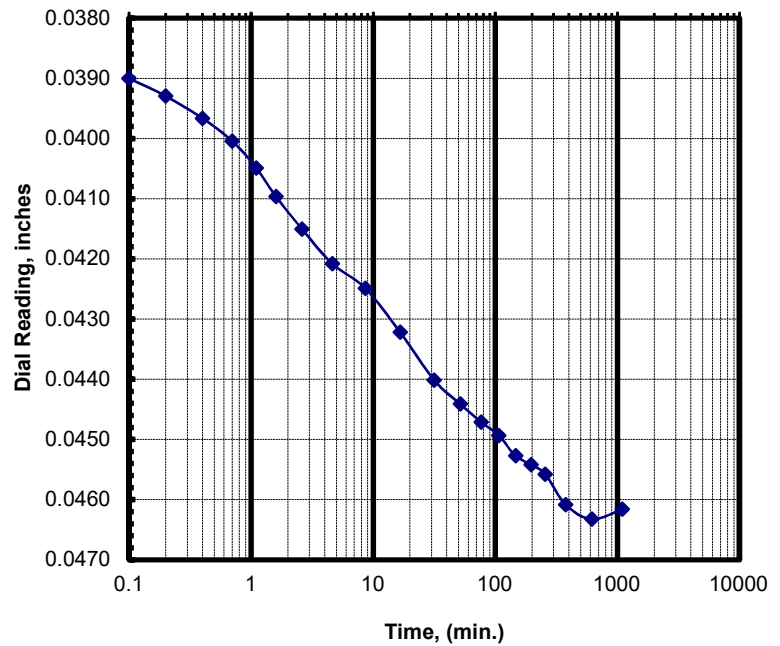
Project Number:	02541139	LL: 54	D _{0(min)} (in): 0.0263	d ₀ (in): 0.0263
Project Name:	CPRA PO 181	PL: 22	D _{end} (in): 0.0327	d ₁₀₀ (in): 0.0320
Boring & Depth:	M-6, S-5, 8-10 ft	PL: 32	Δ _h (in.): 0.0064	δ _{eop} (in.): 0.0057
Sample:	Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 0.27	t ₉₀ (min): 5.9

CONSOLIDATION TEST RESULTS - ASTM D 2435
***t*50 & *t*90 Curves for the Vertical Stress of 0.5 tsf**

[illegible]

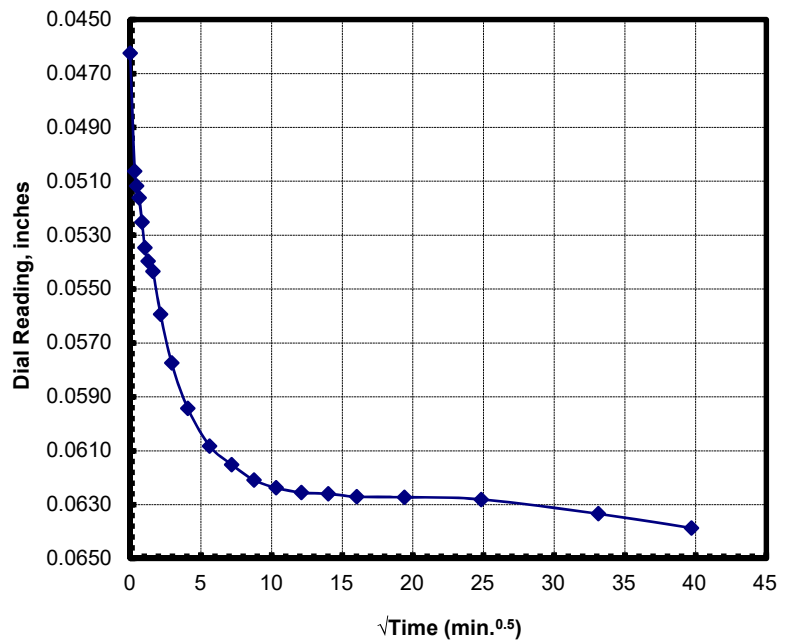
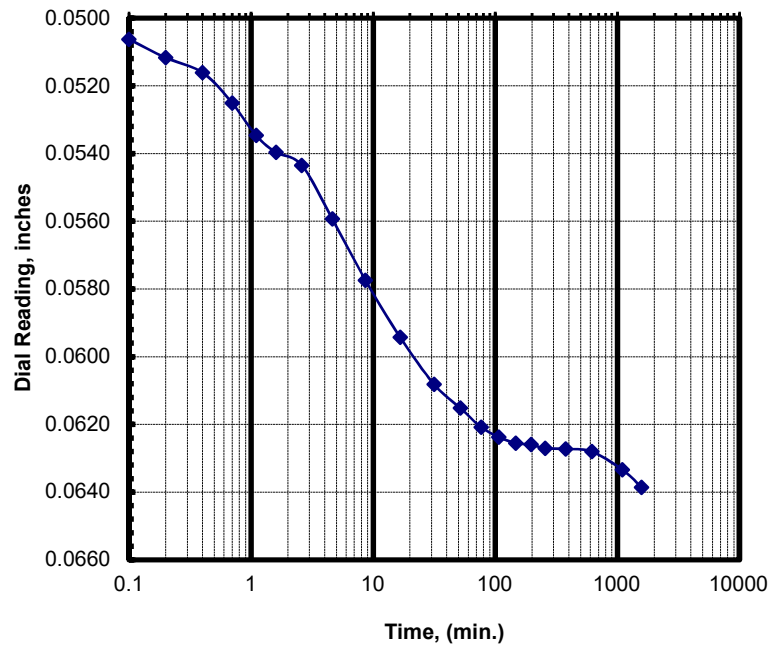
Project Number: 02541139	LL: 54	D _{0(min)} (in): 0.0327	d ₀ (in): 0.0327
Project Name: CPRA PO 181	PL: 22	D _{end} (in): 0.0375	d ₁₀₀ (in): 0.0371
Boring & Depth: M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): 0.0048	δ _{exp} (in.): 0.0044
Sample: Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 1	t ₉₀ (min): 40

CONSOLIDATION TEST RESULTS - ASTM D 2435 t50 & t90 Curves for the Vertical Stress of 1 tsf

[illegible]

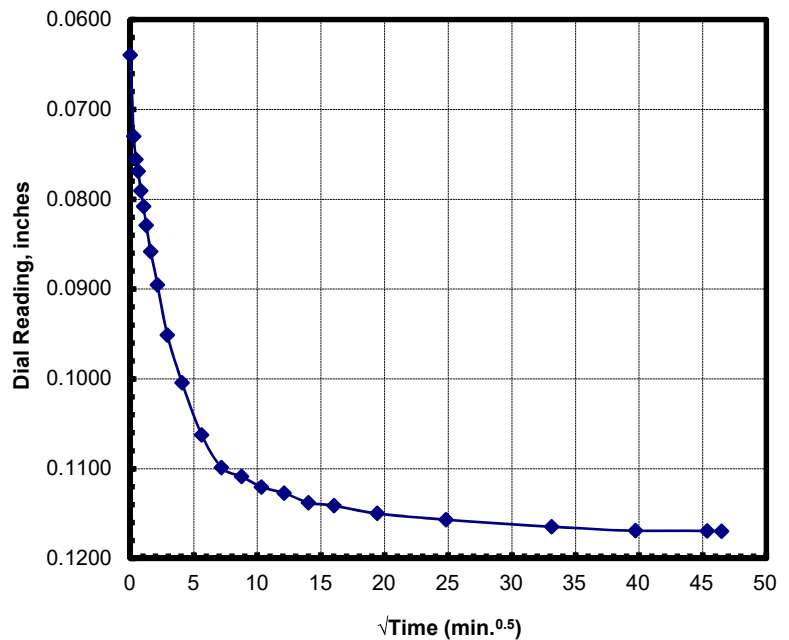
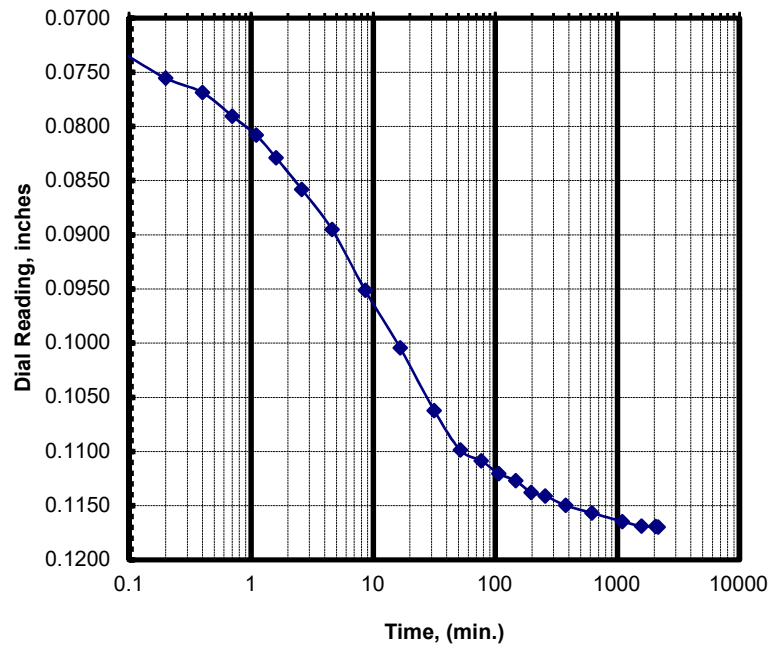
Project Number: 02541139	LL: 54	D _{0(min)} (in): 0.0376	d ₀ (in): 0.0376
Project Name: CPRA PO 181	PL: 22	D _{end} (in): 0.0462	d ₁₀₀ (in): 0.0450
Boring & Depth: M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): 0.0087	δ _{oop} (in.): 0.0074
Sample: Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 2.2	t ₉₀ (min): 43

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

[illegible]

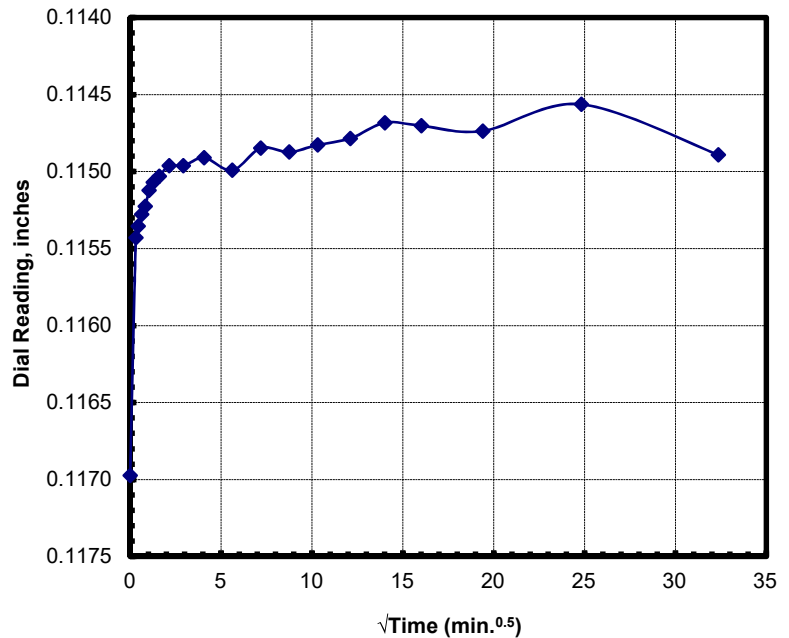
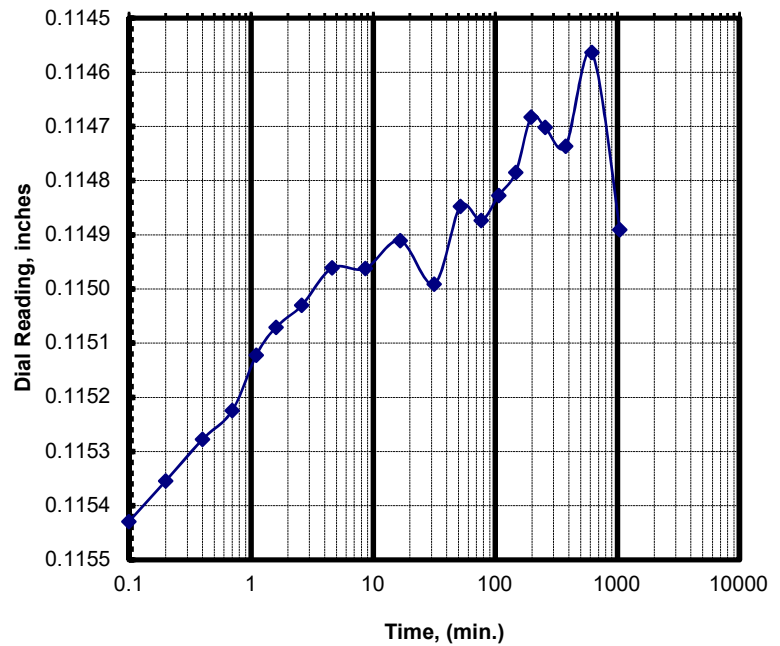
Project Number: 02541139	LL: 54	D _{0(min)} (in): 0.0462	d ₀ (in): 0.0462
Project Name: CPRA PO 181	PL: 22	D _{end} (in): 0.0639	d ₁₀₀ (in): 0.0624
Boring & Depth: M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): 0.0177	δ _{oop} (in.): 0.0162
Sample: Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 2.6	t ₉₀ (min): 31

CONSOLIDATION TEST RESULTS - ASTM D 2435 t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

Project Number:	02541139	LL: 54	D _{0(min)} (in): 0.0639	d ₀ (in): 0.0639
Project Name:	CPRA PO 181	PL: 22	D _{end} (in): 0.1170	d ₁₀₀ (in): 0.1160
Boring & Depth:	M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): 0.0530	δ _{oop} (in.): 0.0521
Sample:	Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 4	t ₉₀ (min): 40

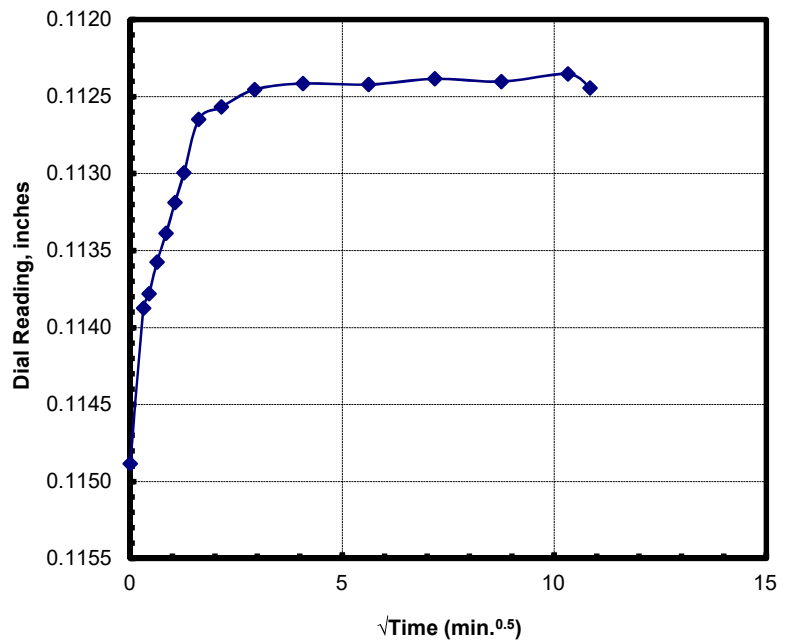
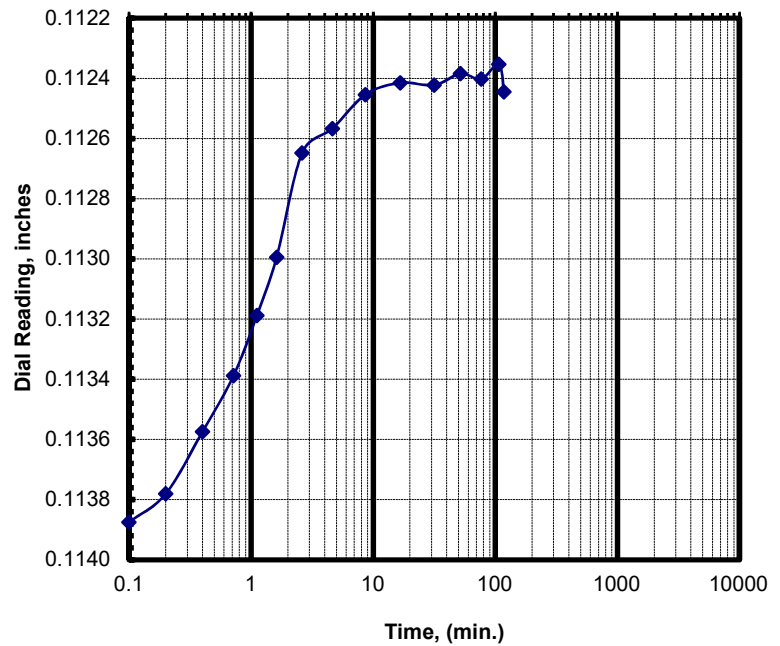
CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 2 tsf

[illegible]

Project Number: 02541139	LL: 54	D _{0(min)} (in): 0.1170	d ₀ (in): 0.1170
Project Name: CPRA PO 181	PL: 22	D _{end} (in): 0.1149	d ₁₀₀ (in): 0.1146
Boring & Depth: M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): -0.0021	δ _{exp} (in.): -0.0024
Sample: Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

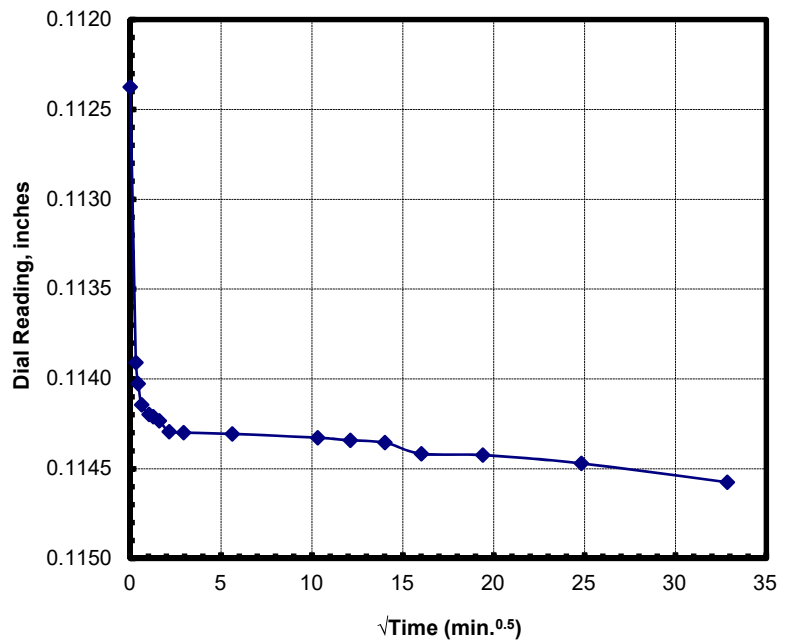
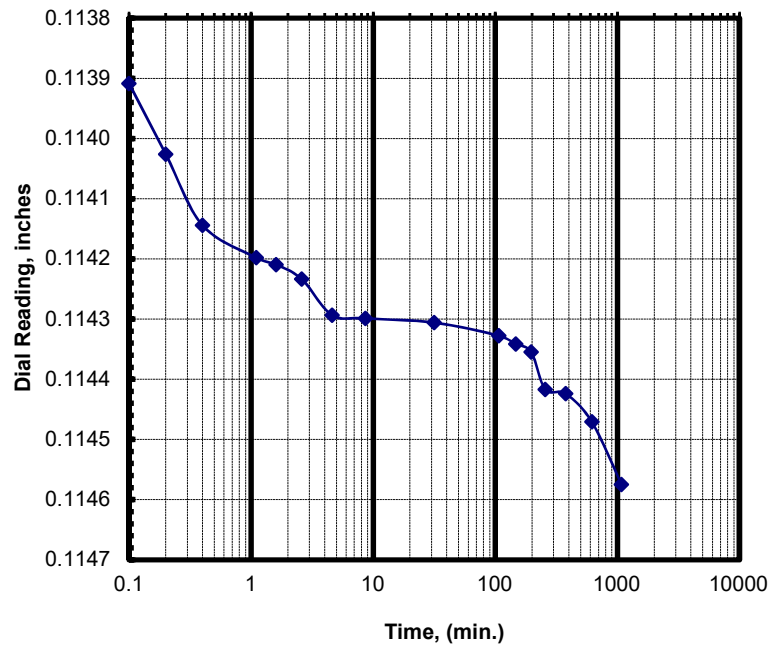
CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 1 tsf

Date & Time	Elaspsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
11/30/20 1:08:48 PM	0.00	0.00	0.1149
11/30/20 1:08:54 PM	0.10	0.32	0.1139
11/30/20 1:09:00 PM	0.20	0.45	0.1138
11/30/20 1:09:12 PM	0.40	0.63	0.1136
11/30/20 1:09:31 PM	0.72	0.85	0.1134
11/30/20 1:09:55 PM	1.12	1.06	0.1132
11/30/20 1:10:25 PM	1.62	1.27	0.1130
11/30/20 1:11:25 PM	2.62	1.62	0.1126
11/30/20 1:13:25 PM	4.62	2.15	0.1126
11/30/20 1:17:25 PM	8.62	2.94	0.1125
11/30/20 1:25:25 PM	16.62	4.08	0.1124
11/30/20 1:40:25 PM	31.62	5.62	0.1124
11/30/20 2:00:25 PM	51.62	7.18	0.1124
11/30/20 2:25:26 PM	76.63	8.75	0.1124
11/30/20 2:55:26 PM	106.63	10.33	0.1124
11/30/20 3:06:22 PM	117.57	10.84	0.1124



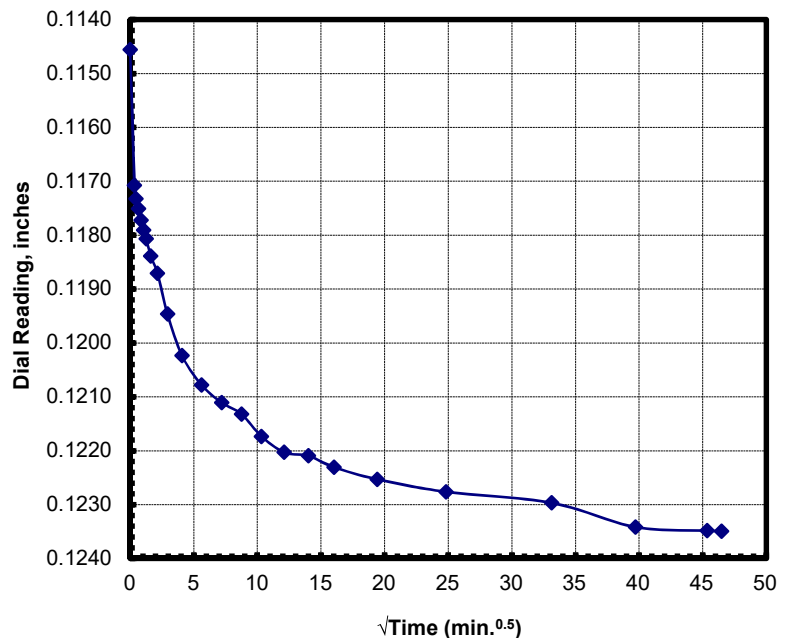
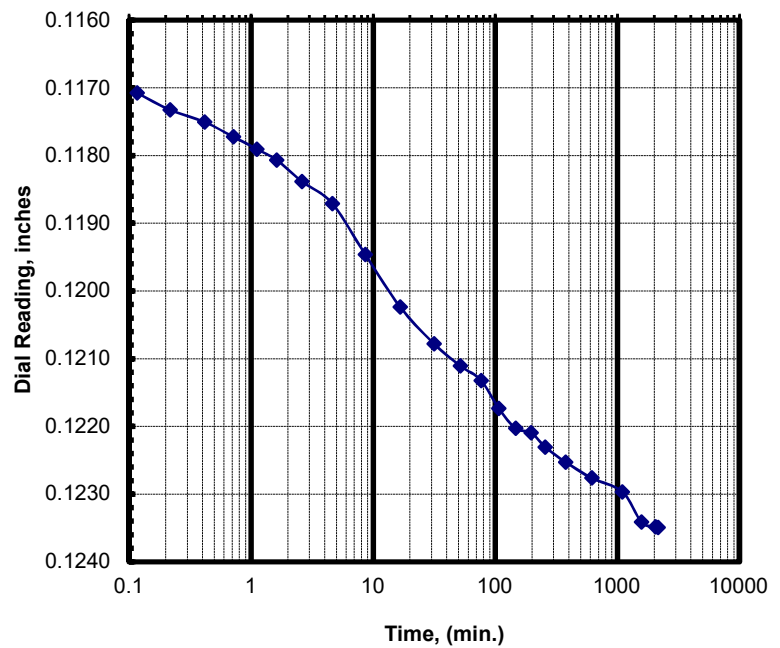
Project Number: 02541139	LL: 54	D _{0(min)} (in): 0.1149	d ₀ (in): 0.1149
Project Name: CPRA PO 181	PL: 22	D _{end} (in): 0.1124	d ₁₀₀ (in): 0.1124
Boring & Depth: M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): -0.0024	δ _{oop} (in.): -0.0024
Sample: Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 2 tsf

[illegible]

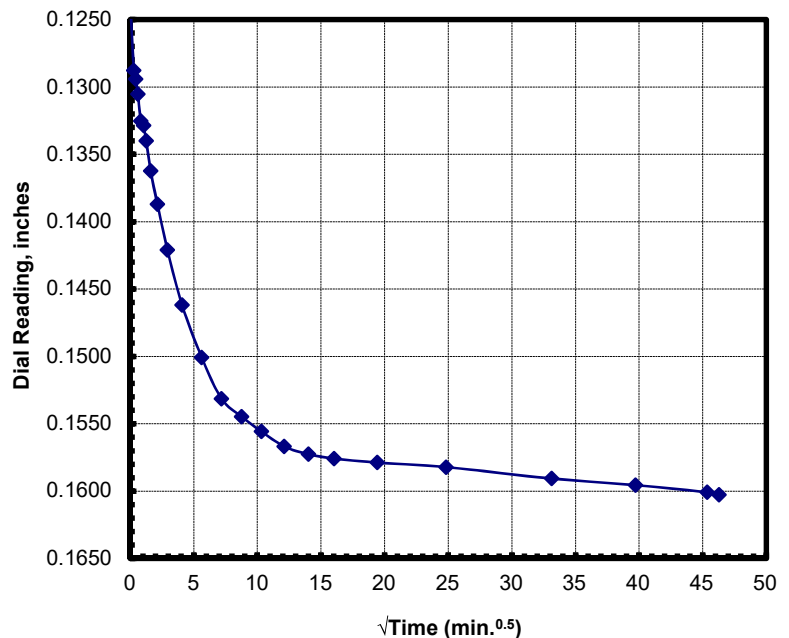
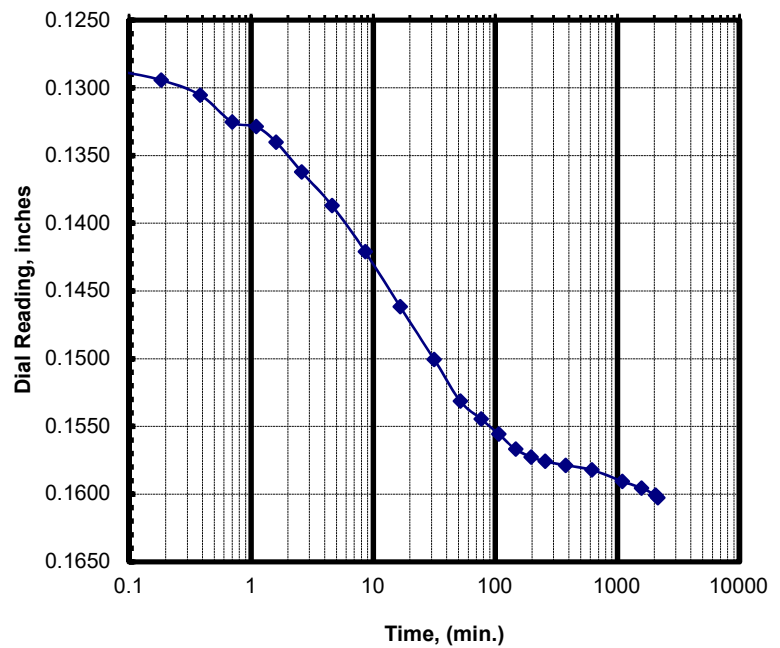
Project Number: 02541139	LL: 54	D _{0(min)} (in): 0.1124	d ₀ (in): 0.1124
Project Name: CPRA PO 181	PL: 22	D _{end} (in): 0.1146	d ₁₀₀ (in): 0.1143
Boring & Depth: M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): 0.0022	δ _{oop} (in.): 0.0019
Sample: Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

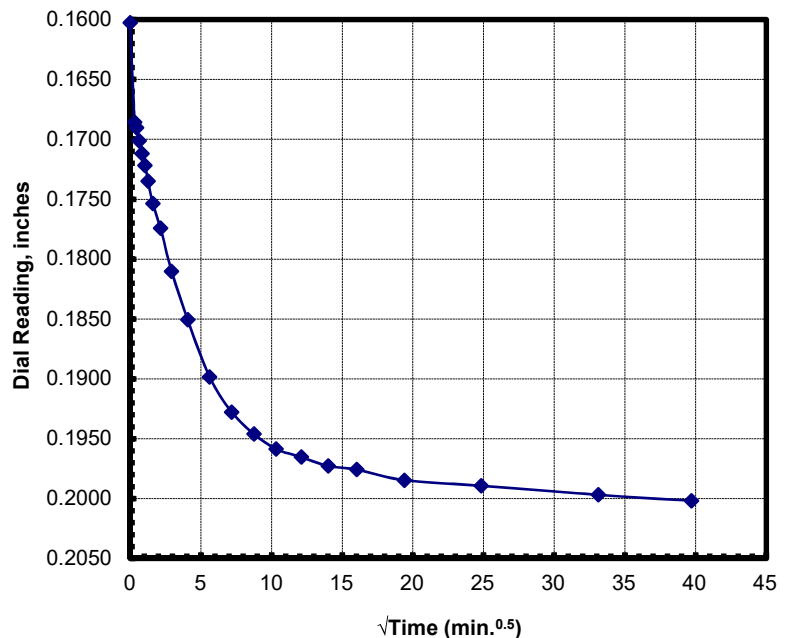
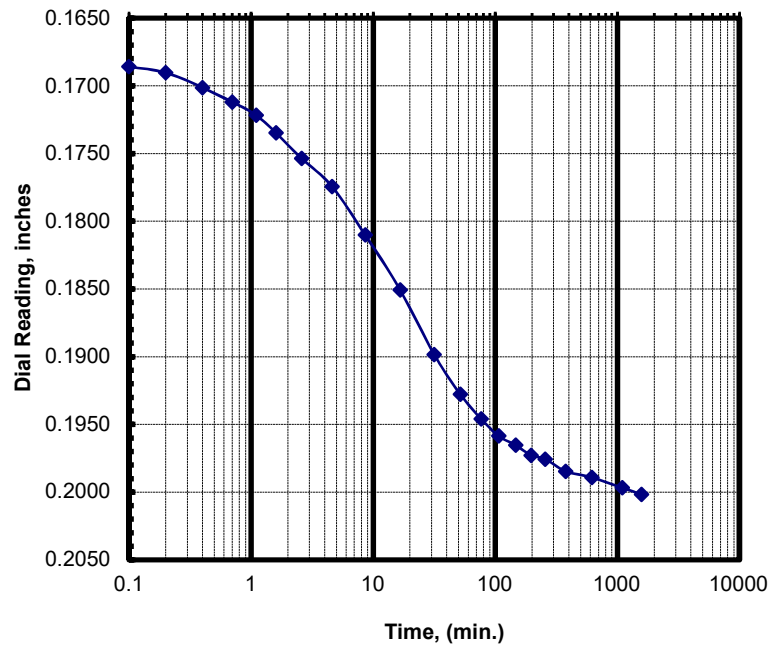
Project Number:	02541139	LL: 54	D _{0(min)} (in): 0.1146	d ₀ (in): 0.1146
Project Name:	CPRA PO 181	PL: 22	D _{end} (in): 0.1235	d ₁₀₀ (in): 0.1210
Boring & Depth:	M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): 0.0089	δ _{oop} (in.): 0.0064
Sample:	Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 8 tsf

[illegible]

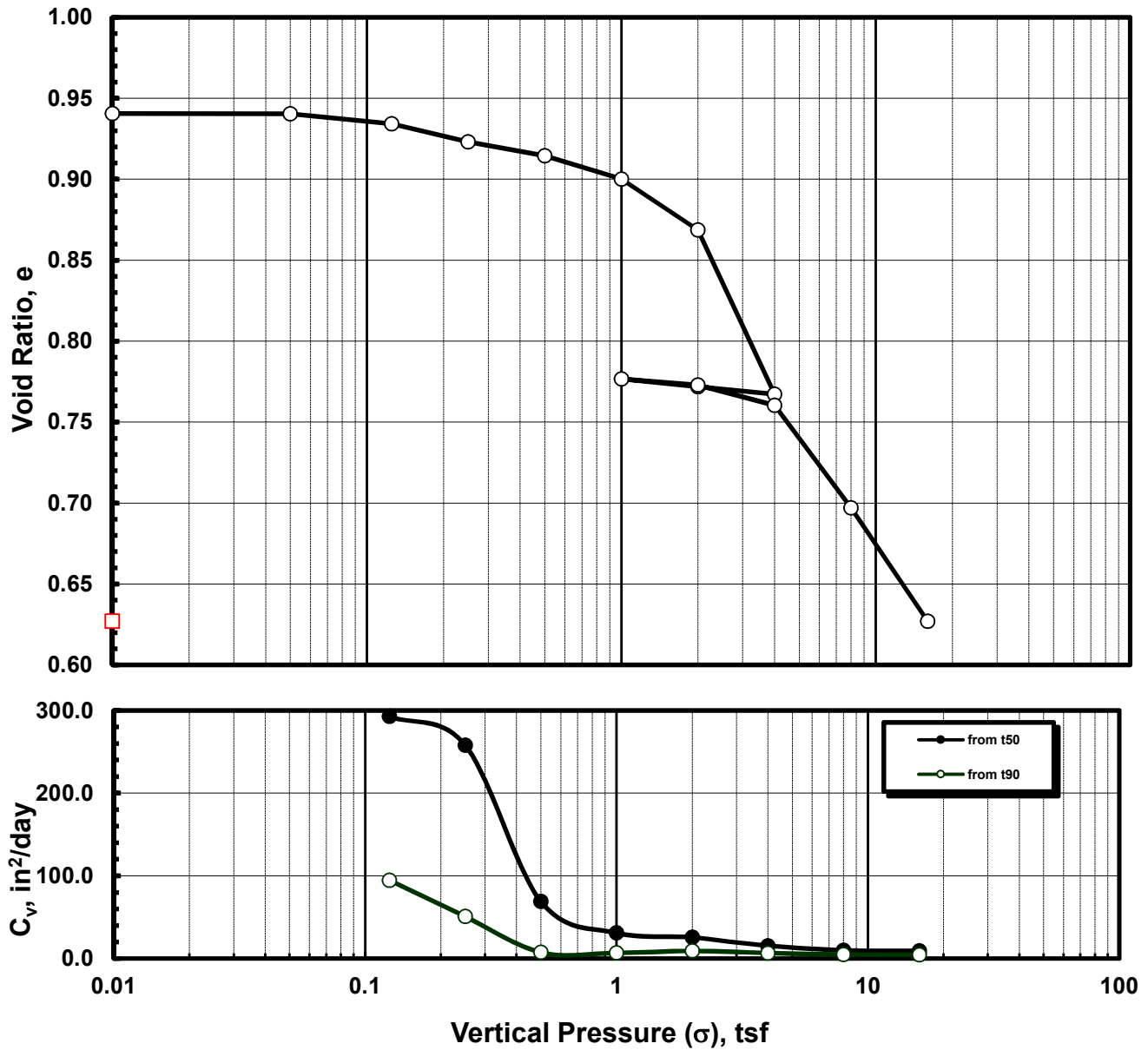
Project Number:	02541139	LL: 54	D _{0(min)} (in): 0.1235	d ₀ (in): 0.1235
Project Name:	CPRA PO 181	PL: 22	D _{end} (in): 0.1603	d ₁₀₀ (in): 0.1560
Boring & Depth:	M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): 0.0368	δ _{oop} (in.): 0.0325
Sample:	Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 5.6	t ₉₀ (min): 49

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 16 tsf

[illegible]

Project Number: 02541139	LL: 54	D _{0(min)} (in): 0.1603	d ₀ (in): 0.1603
Project Name: CPRA PO 181	PL: 22	D _{end} (in): 0.2002	d ₁₀₀ (in): 0.1963
Boring & Depth: M-6, S-5, 8-10 ft	PL: 32	Δ _{hi} (in.): 0.0399	δ _{oop} (in.): 0.0360
Sample: Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 5.6	t ₉₀ (min): 49

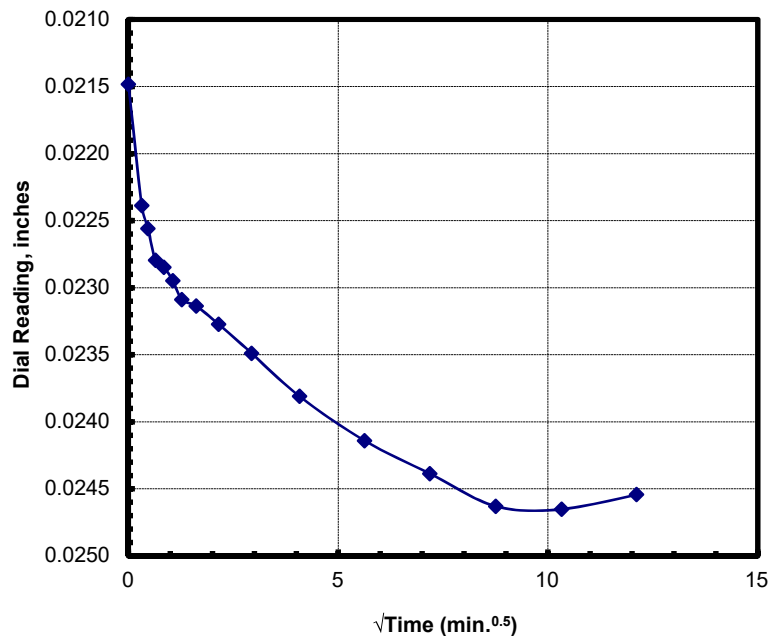
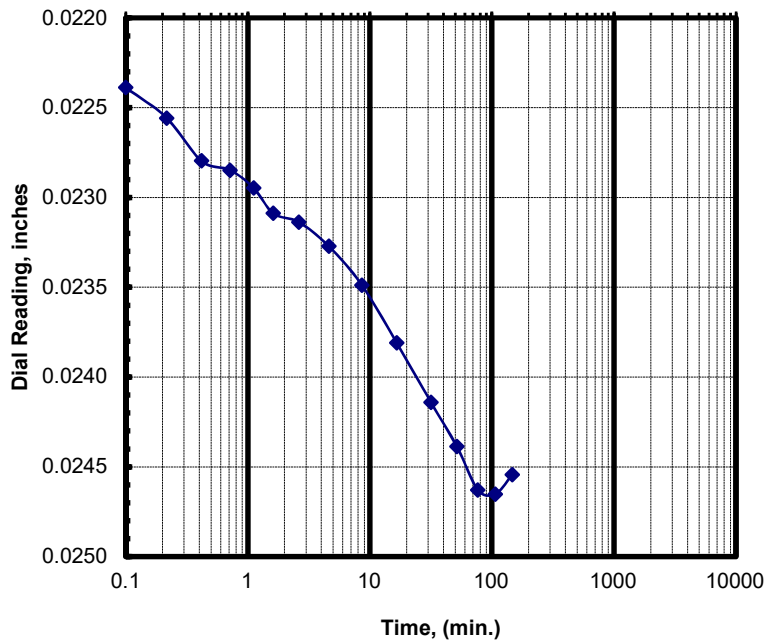
CONSOLIDATION TEST RESULTS - ASTM D 2435



Sample Condition	Before	After	Consolidation Parameters	
Moisture Content, %:	33.2%	23.3%	Overburden Pressure, tsf:	0.3
Sample Height, in.:	0.9970	0.8359	Preconsolidation Pressure, tsf:	1.5
Void Ratio, e:	0.9406	0.6270	Compression Index, (Cc):	0.268
Unit Weight, pcf:	86.8	103.6	Re-Compression Index, (Cr)	0.027
Degree of Saturation:	0.95	1.00	Swell Index, Cs:	-
Project Number: 02541139			Liquid Limit: 54	Hand Pen: 0.25 tsf
Project Name CPRA PO 181			Plastic Limit: 22	-200 %: NA
Boring No.: M-6			Plasticity Index: 32	SPG: 2.7
Sample ID, Depth: S-5, 8-10 ft			Classification: Gray FAT CLAY	

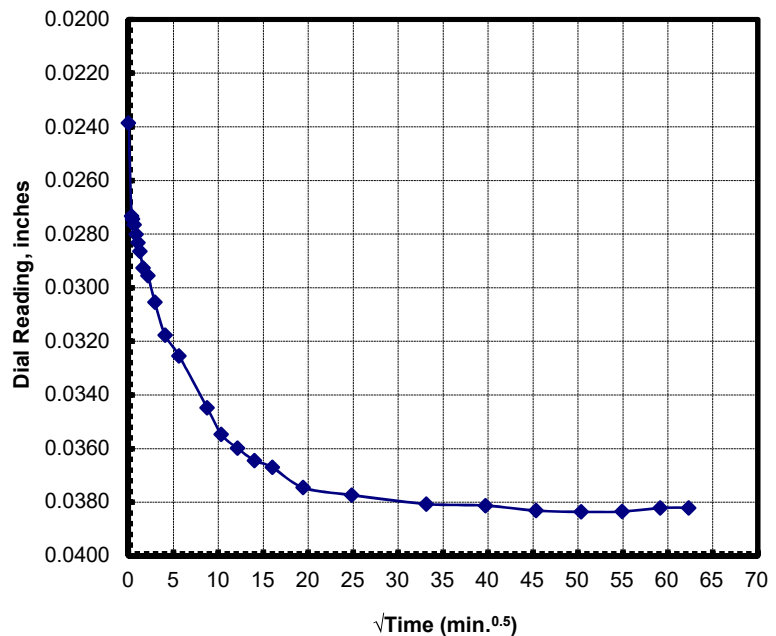
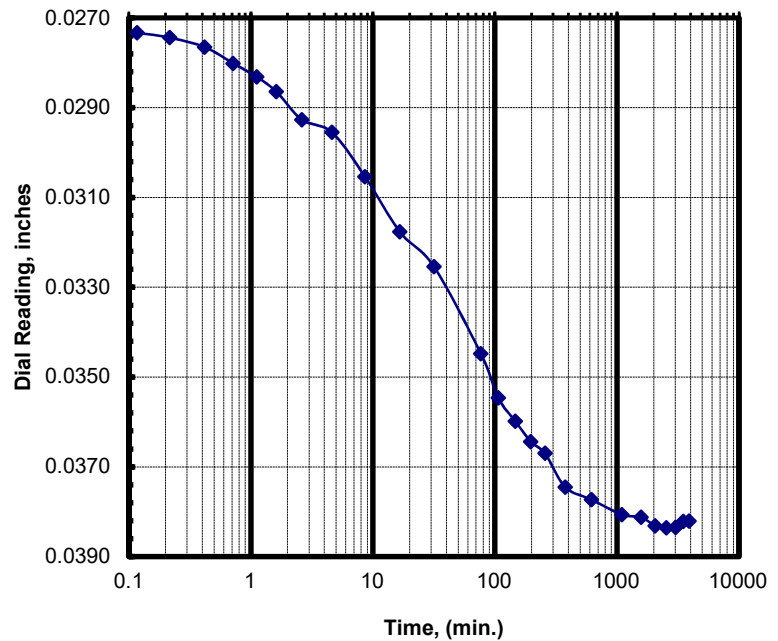
CONSOLIDATION TEST RESULTS - ASTM D 2435 ***t₅₀ & t₉₀ Curves for the Vertical Stress of 0.25 tsf***

Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
11/13/20 1:06:10 PM	0.00	0.00	0.0215
11/13/20 1:06:16 PM	0.10	0.32	0.0224
11/13/20 1:06:23 PM	0.22	0.47	0.0226
11/13/20 1:06:35 PM	0.42	0.65	0.0228
11/13/20 1:06:53 PM	0.72	0.85	0.0228
11/13/20 1:07:17 PM	1.12	1.06	0.0229
11/13/20 1:07:47 PM	1.62	1.27	0.0231
11/13/20 1:08:47 PM	2.62	1.62	0.0231
11/13/20 1:10:47 PM	4.62	2.15	0.0233
11/13/20 1:14:47 PM	8.62	2.94	0.0235
11/13/20 1:22:47 PM	16.62	4.08	0.0238
11/13/20 1:37:48 PM	31.63	5.62	0.0241
11/13/20 1:57:48 PM	51.63	7.19	0.0244
11/13/20 2:22:48 PM	76.63	8.75	0.0246
11/13/20 2:52:49 PM	106.65	10.33	0.0247
11/13/20 3:32:49 PM	146.65	12.11	0.0245
11/13/20 4:22:49 PM	196.65	14.02	0.0244
11/13/20 4:22:59 PM	196.82	14.03	0.0244



Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0215	d ₀ (in): 0.0215
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0244	d ₁₀₀ (in): 0.0246
Boring & Depth:	M-7, S-3, 4-6 ft	PL: NA	Δ _{hj} (in.): 0.0029	δ _{eop} (in.): 0.0031
Sample:	Light Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 1.7	t ₉₀ (min): 43

CONSOLIDATION TEST RESULTS - ASTM D 2435
***t*50 & *t*90 Curves for the Vertical Stress of 0.5 tsf**

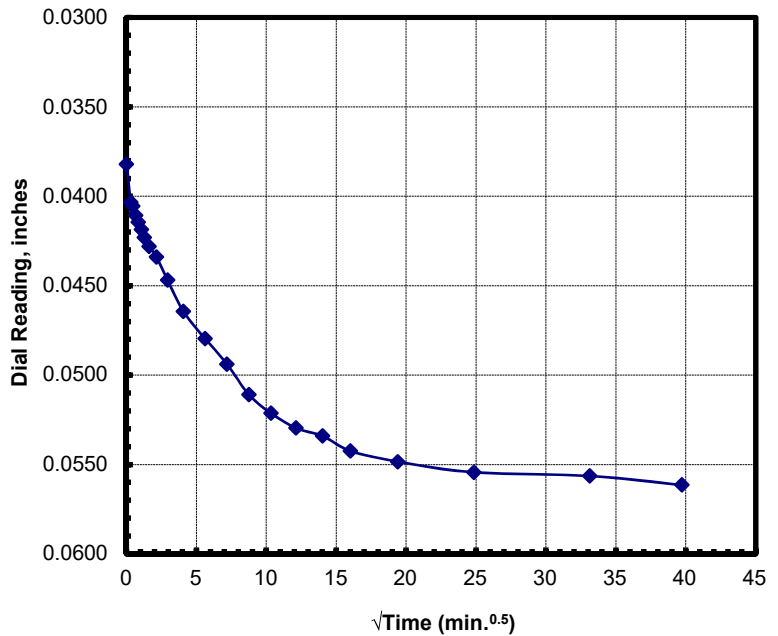
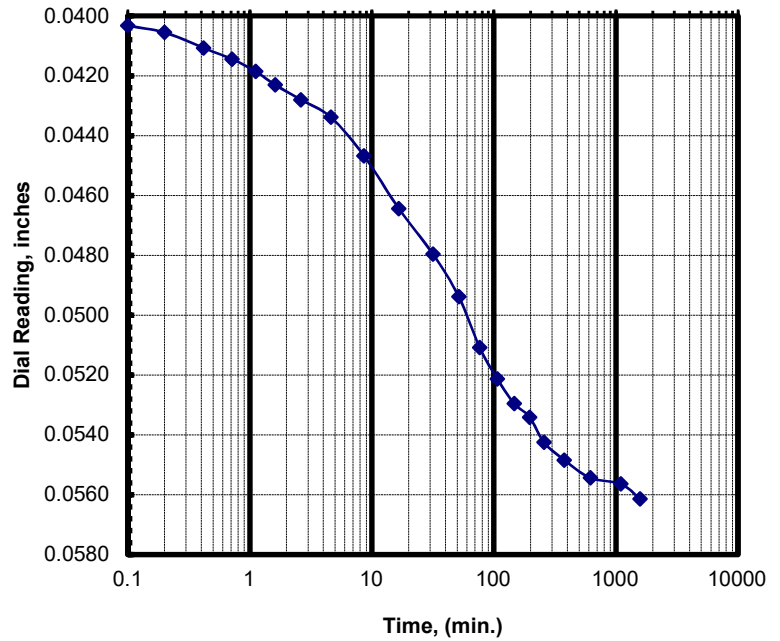
[illegible]

Project Number: 02541139	LL: NA	$D_{0(\min)}$ (in): 0.0239	d_0 (in): 0.0239
Project Name: CPRA PO 181	PL: NA	D_{end} (in): 0.0382	d_{100} (in): 0.0374
Boring & Depth: M-7, S-3, 4-6 ft	PL: NA	Δ_{hi} (in.): 0.0143	δ_{eop} (in.): 0.0135
Sample: Light Gray FAT CLAY	- 200 (%): NA	t_{50} (min): 8.9	t_{90} (min): 150

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02541139 M-7 S-3 (B) 4-6'

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 1 tsf

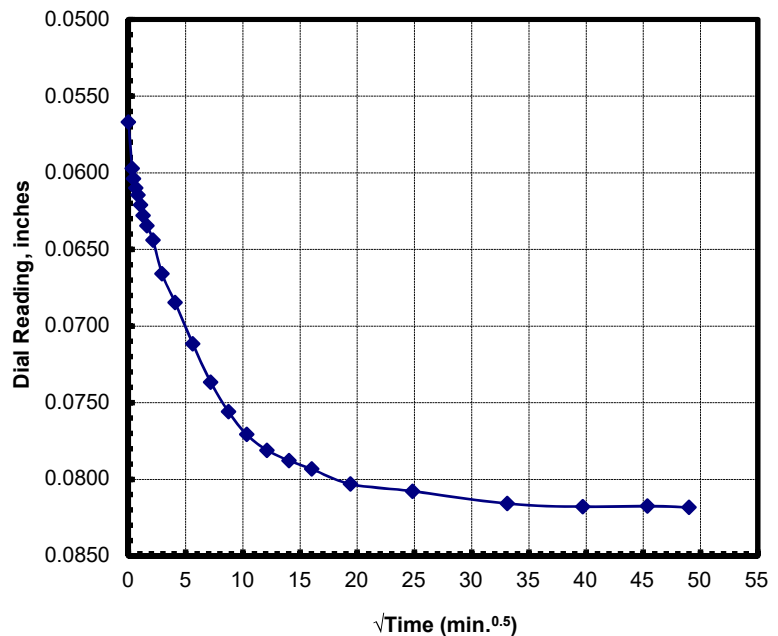
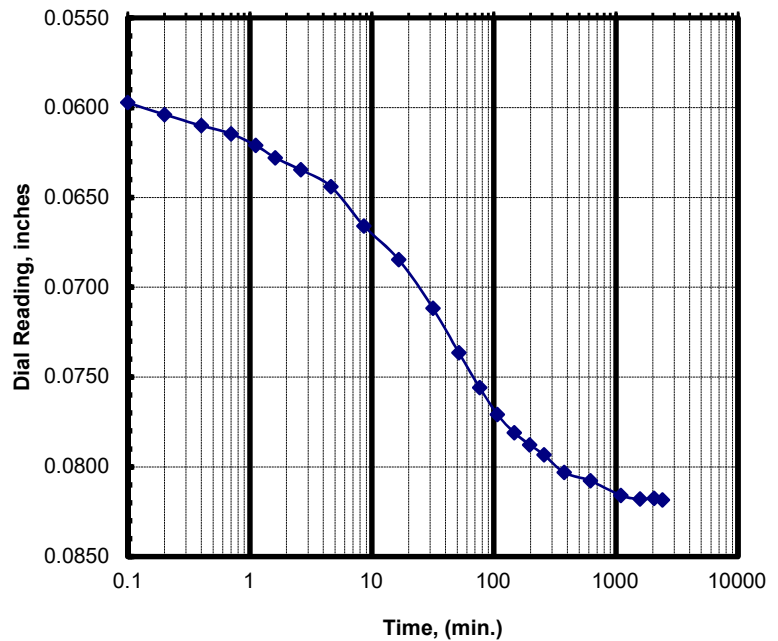
[illegible]

Project Number: 02541139	LL: NA	D _{0(min)} (in): 0.0382	d ₀ (in): 0.0382
Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.0567	d ₁₀₀ (in): 0.0552
Boring & Depth: M-7, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0185	δ _{eop} (in.): 0.0170
Sample: Light Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 18	t ₉₀ (min): 200

intertek
psi

02541139 M-7 S-3 (B) 4-6'

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

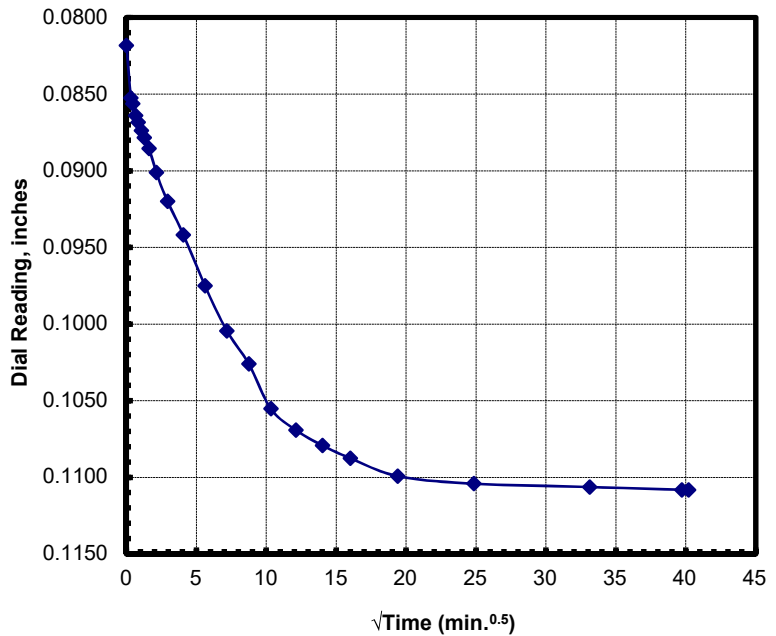
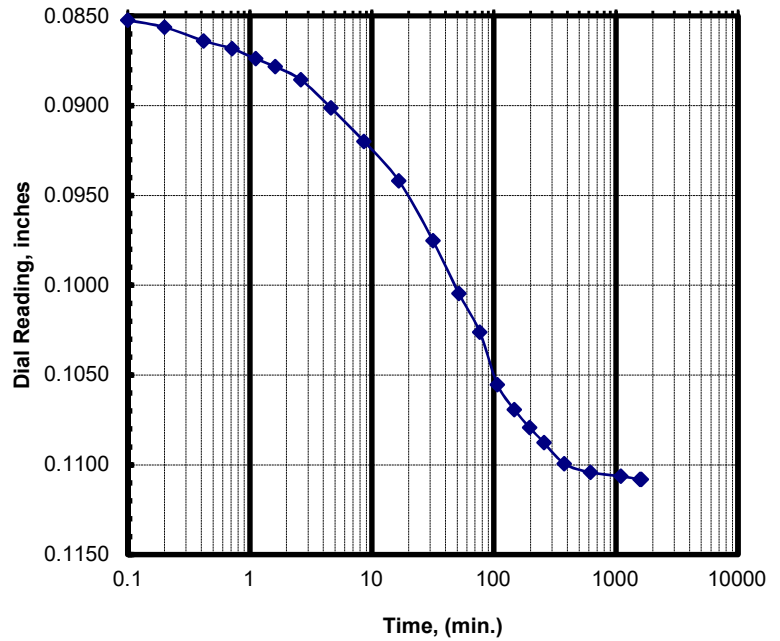
[illegible]

Project Number: 02541139	LL: NA	D _{0(min)} (in): 0.0567	d ₀ (in): 0.0567
Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.0818	d ₁₀₀ (in): 0.0803
Boring & Depth: M-7, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0252	δ _{eop} (in.): 0.0236
Sample: Light Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 17	t ₉₀ (min): 120

intertek
psi

02541139 M-7 S-3 (B) 4-6'

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 4 tsf

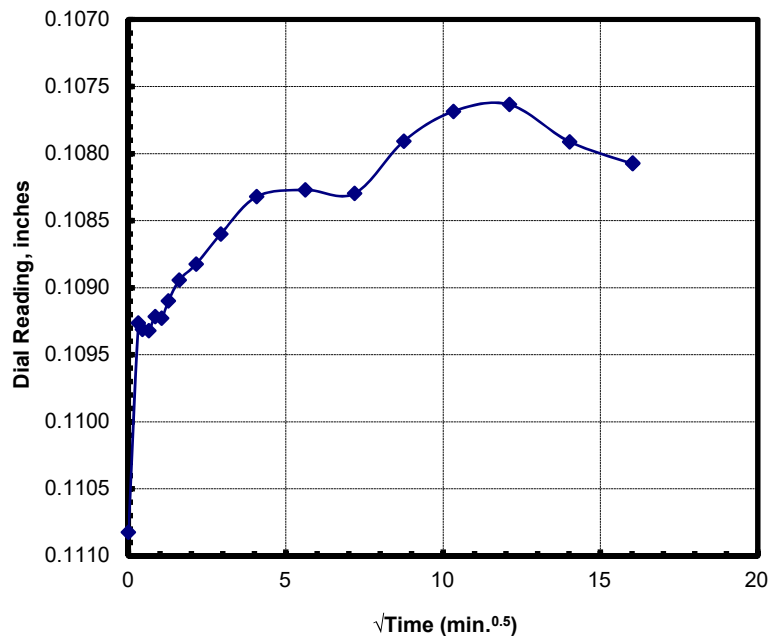
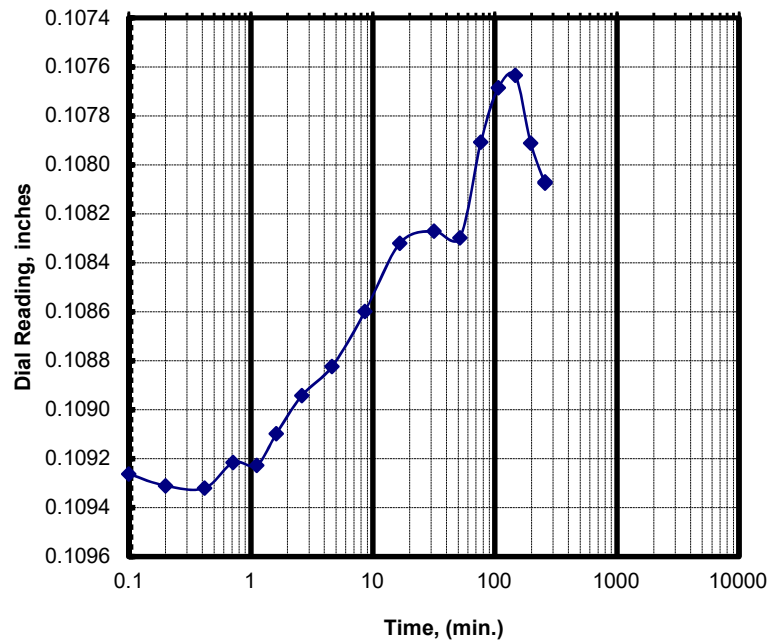
[illegible]

Project Number: 02541139	LL: NA	D _{0(min)} (in): 0.0818	d ₀ (in): 0.0818
Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.1108	d ₁₀₀ (in): 0.1098
Boring & Depth: M-7, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0290	δ _{exp} (in.): 0.0280
Sample: Light Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 22	t ₉₀ (min): 150

intertek
psi

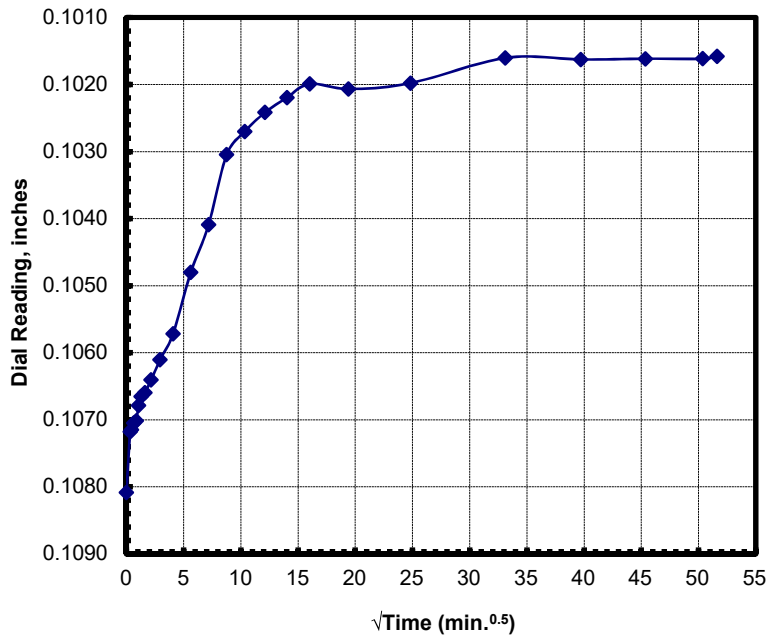
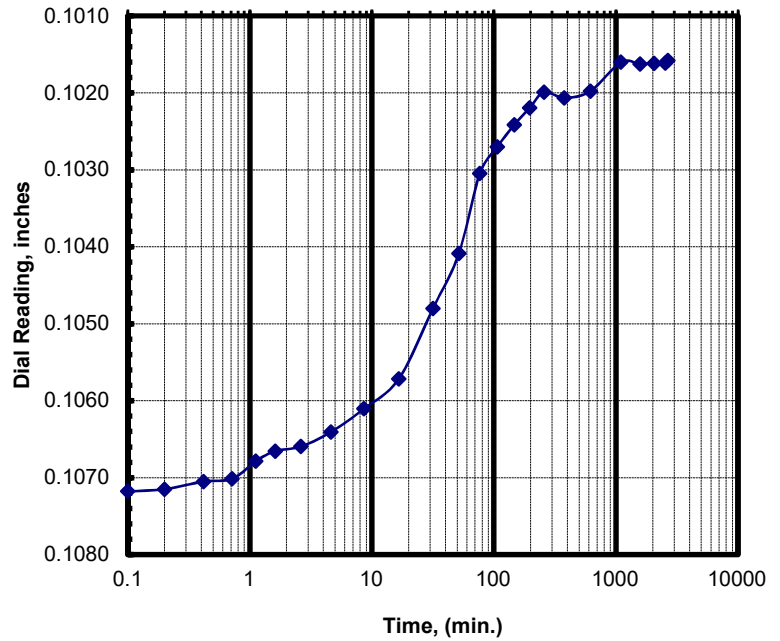
02541139 M-7 S-3 (B) 4-6'

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

[illegible]

Project Number: 02541139	LL: NA	D _{0(min)} (in): 0.1108	d ₀ (in): 0.1108
Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.1081	d ₁₀₀ (in): 0.1081
Boring & Depth: M-7, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): -0.0028	δ _{oop} (in.): -0.0028
Sample: Light Gray FAT CLAY	- 200 (%) : NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 1 tsf

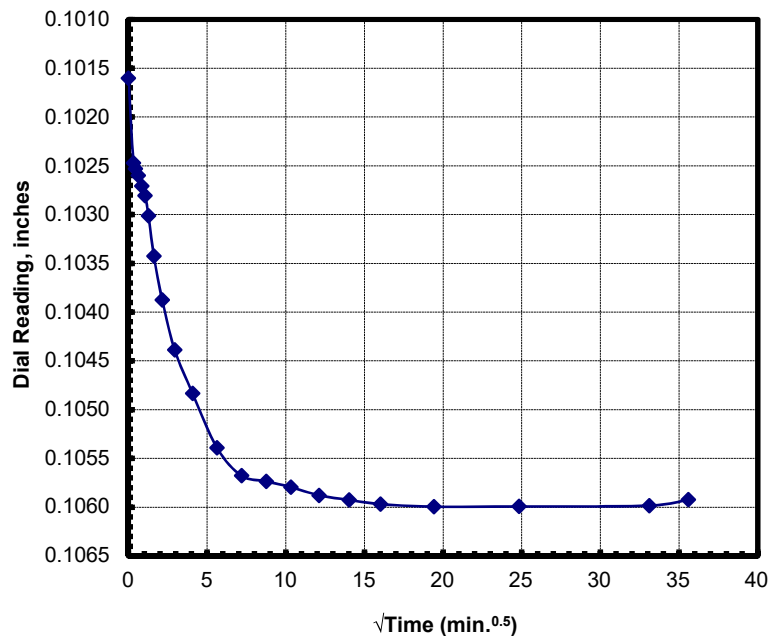
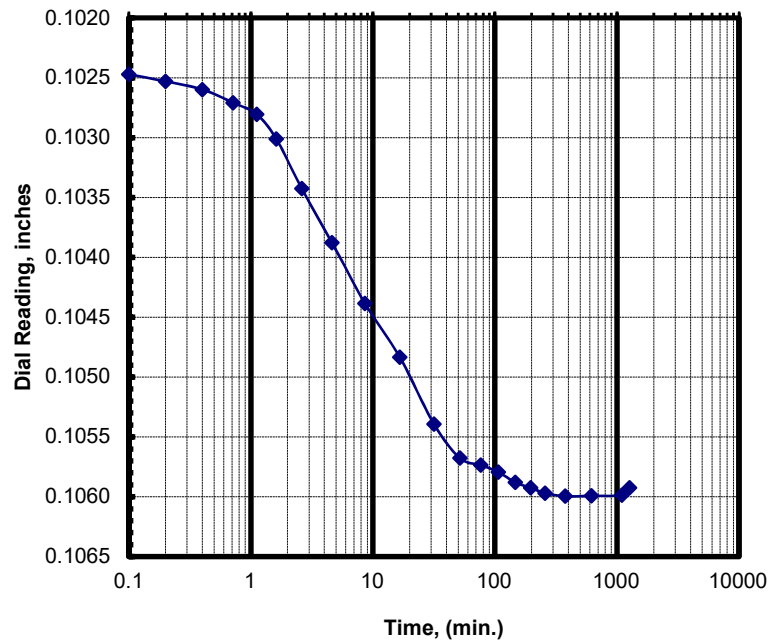
[illegible]

Project Number: 02541139	LL: NA	D _{0(min)} (in): 0.1081	d ₀ (in): 0.1081
Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.1016	d ₁₀₀ (in): 0.1016
Boring & Depth: M-7, S-3, 4-6 ft	PL: NA	Δ _{hl} (in.): -0.0065	δ _{exp} (in.): -0.0065
Sample: Light Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

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psi

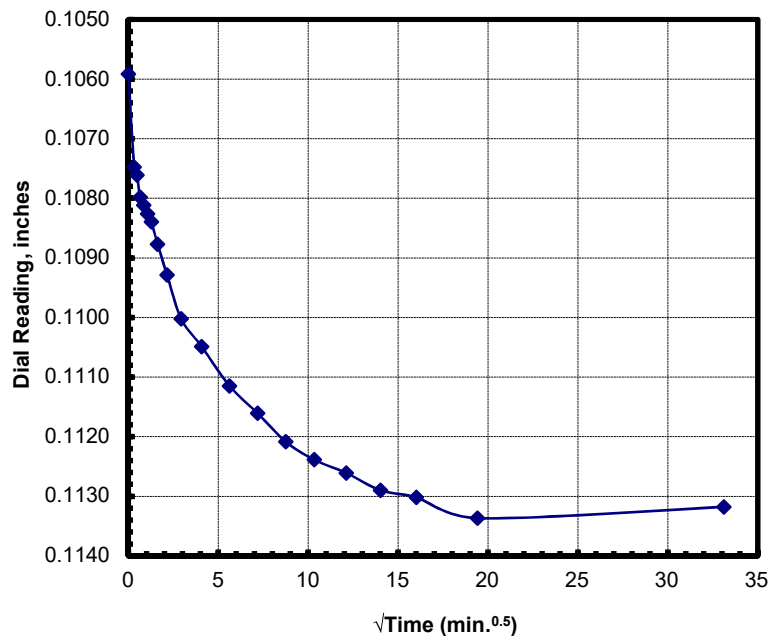
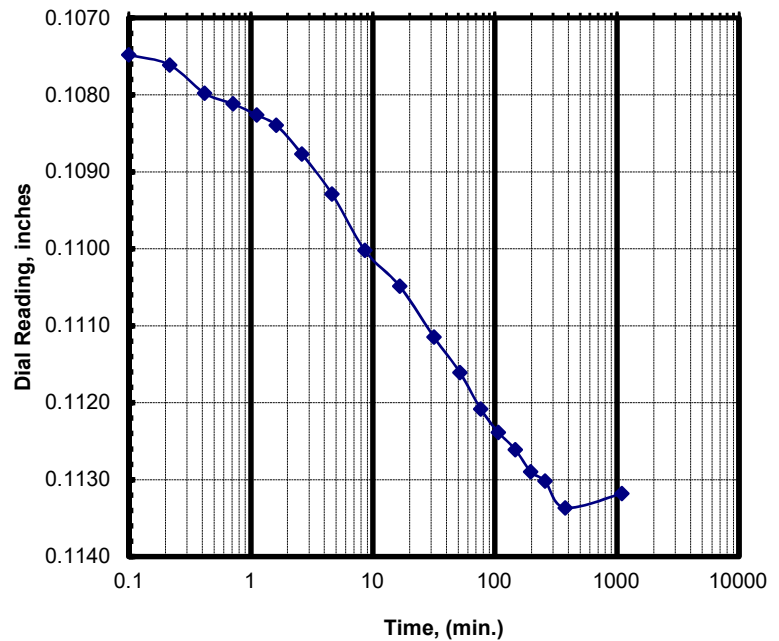
02541139 M-7 S-3 (B) 4-6'

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

[illegible]

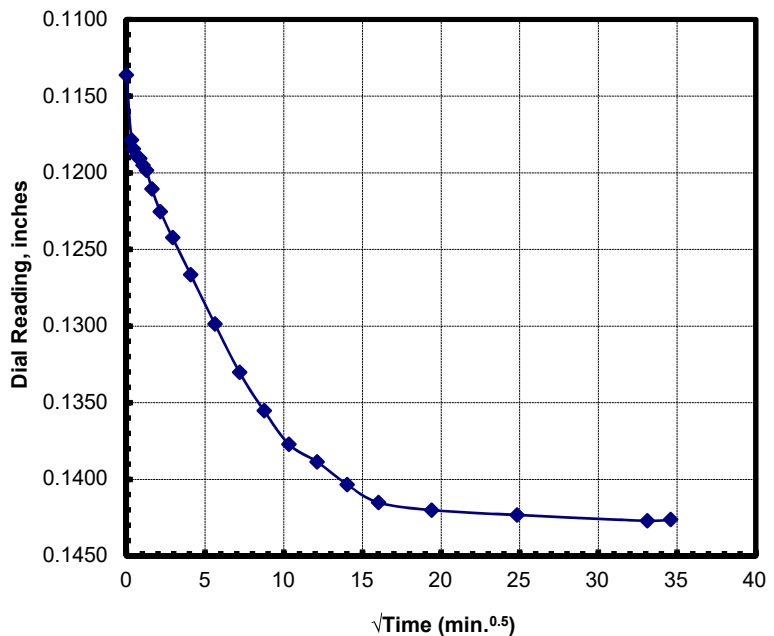
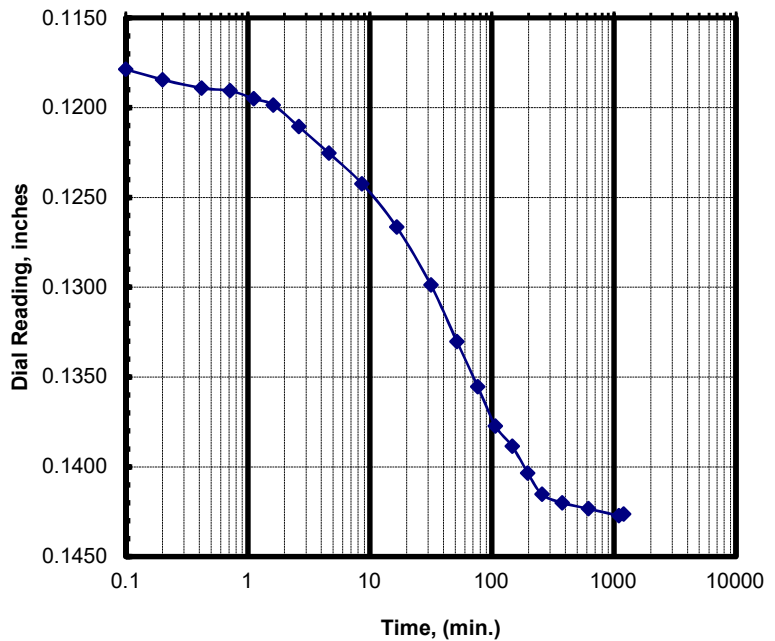
Project Number: 02541139	LL: NA	D _{0(min)} (in): 0.1016	d ₀ (in): 0.1016
Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.1059	d ₁₀₀ (in): 0.1059
Boring & Depth: M-7, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0043	δ _{oop} (in.): 0.0043
Sample: Light Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

Project Number: 02541139	LL: NA	D _{0(min)} (in): 0.1059	d ₀ (in): 0.1059
Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.1136	d ₁₀₀ (in): 0.1129
Boring & Depth: M-7, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0077	δ _{eop} (in.): 0.0070
Sample: Light Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

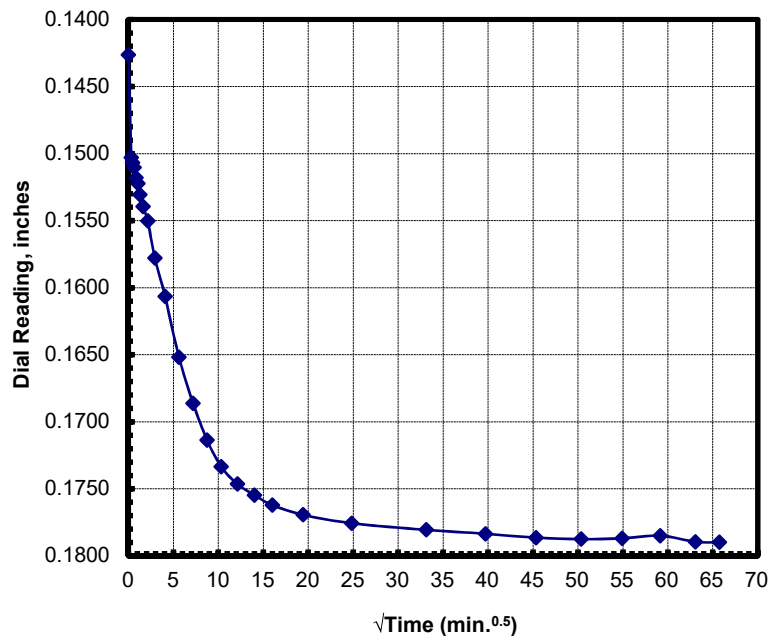
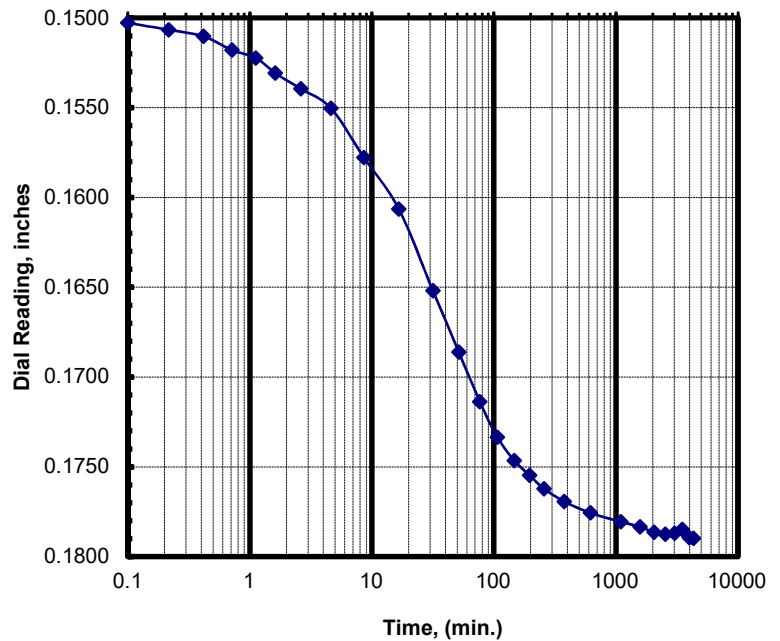
CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 8 tsf

[illegible]

Project Number: 02541139	LL: NA	D _{0(min)} (in): 0.1136	d ₀ (in): 0.1136
Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.1426	d ₁₀₀ (in): 0.1415
Boring & Depth: M-7, S-3, 4-6 ft	PL: NA	Δ _{hl} (in.): 0.0290	δ _{eop} (in.): 0.0279
Sample: Light Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 20	t ₉₀ (min): 140

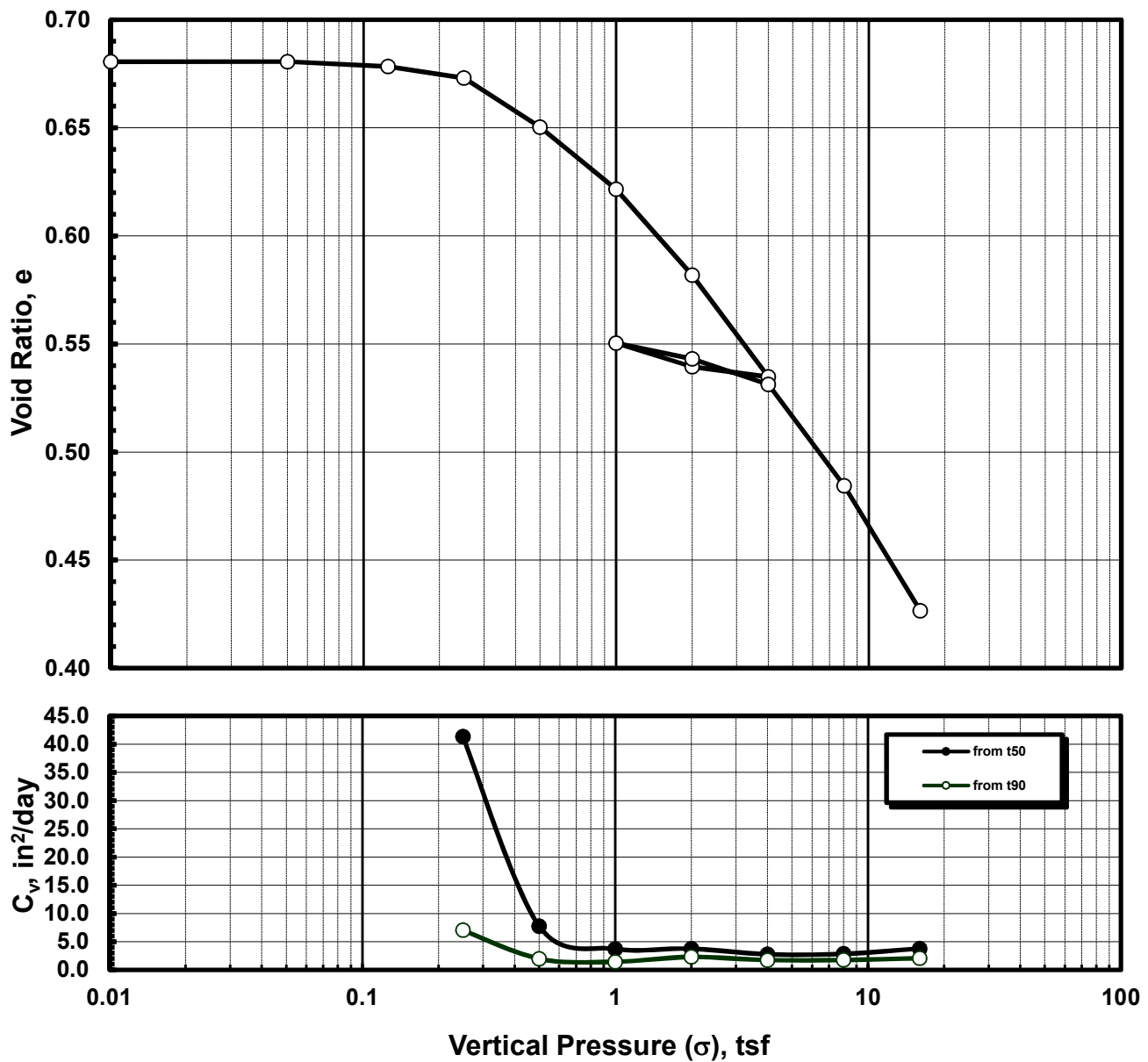
CONSOLIDATION TEST RESULTS - ASTM D 2435 ***t₅₀ & t₉₀ Curves for the Vertical Stress of 16 tsf***

Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
11/25/20 7:40:58 AM	0.00	0.00	0.1426
11/25/20 7:41:04 AM	0.10	0.32	0.1503
11/25/20 7:41:11 AM	0.22	0.47	0.1507
11/25/20 7:41:23 AM	0.42	0.65	0.1510
11/25/20 7:41:41 AM	0.72	0.85	0.1518
11/25/20 7:42:05 AM	1.12	1.06	0.1522
11/25/20 7:42:35 AM	1.62	1.27	0.1531
11/25/20 7:43:35 AM	2.62	1.62	0.1540
11/25/20 7:45:35 AM	4.62	2.15	0.1550
11/25/20 7:49:35 AM	8.62	2.94	0.1578
11/25/20 7:57:35 AM	16.62	4.08	0.1606
11/25/20 8:12:35 AM	31.62	5.62	0.1652
11/25/20 8:32:36 AM	51.63	7.19	0.1686
11/25/20 8:57:36 AM	76.63	8.75	0.1714
11/25/20 9:27:36 AM	106.63	10.33	0.1733
11/25/20 10:07:36 AM	146.63	12.11	0.1746
11/25/20 10:57:36 AM	196.63	14.02	0.1755
11/25/20 11:57:36 AM	256.63	16.02	0.1762
11/25/20 1:57:37 PM	376.65	19.41	0.1769
11/25/20 5:57:37 PM	616.65	24.83	0.1776
11/26/20 1:57:37 AM	1096.65	33.12	0.1781
11/26/20 9:57:38 AM	1576.67	39.71	0.1783
11/26/20 5:57:38 PM	2056.67	45.35	0.1786
11/27/20 1:57:39 AM	2536.68	50.37	0.1787
11/27/20 9:57:39 AM	3016.68	54.92	0.1787
11/27/20 5:57:40 PM	3496.70	59.13	0.1785
11/28/20 1:57:40 AM	3976.70	63.06	0.1789
11/28/20 7:41:23 AM	4320.42	65.73	0.1790



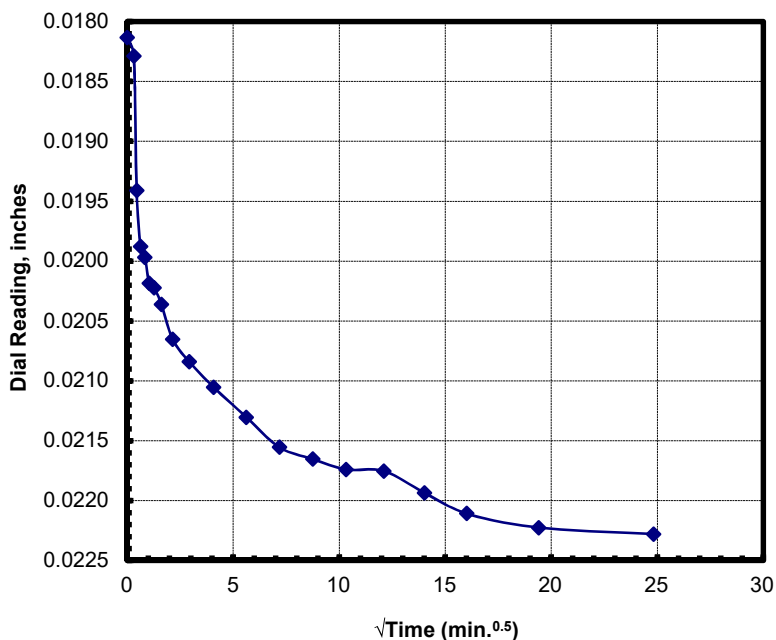
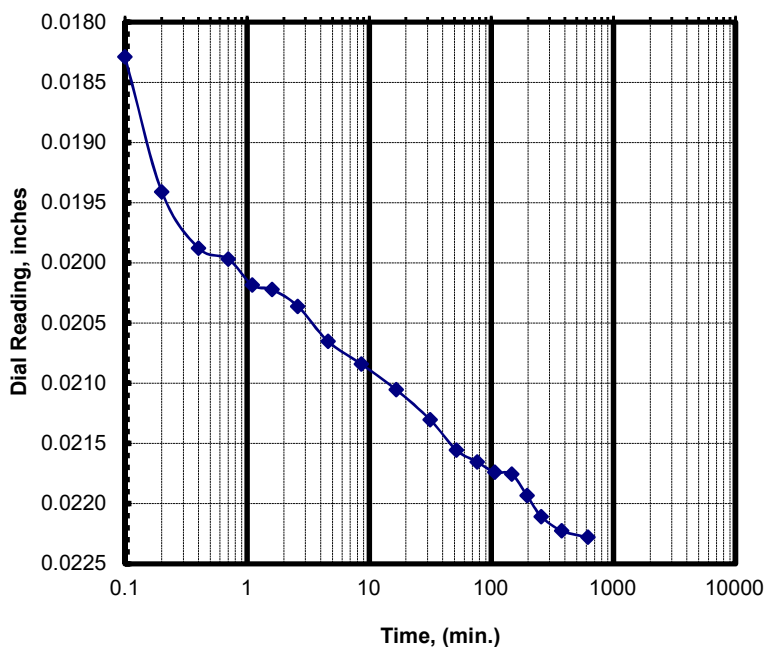
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1426	d ₀ (in): 0.1426
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1790	d ₁₀₀ (in): 0.1770
Boring & Depth:	M-7, S-3, 4-6 ft	PL: NA	Δ _{h1} (in.): 0.0363	δ _{eop} (in.): 0.0344
Sample:	Light Gray FAT CLAY	- 200 (%): NA	t ₅₀ (min): 14	t ₉₀ (min): 110

CONSOLIDATION TEST RESULTS - ASTM D 2435



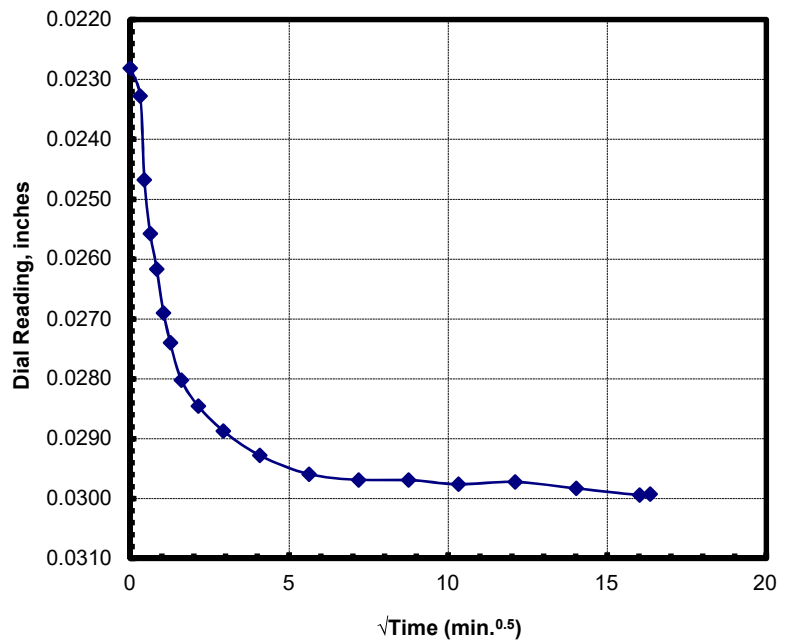
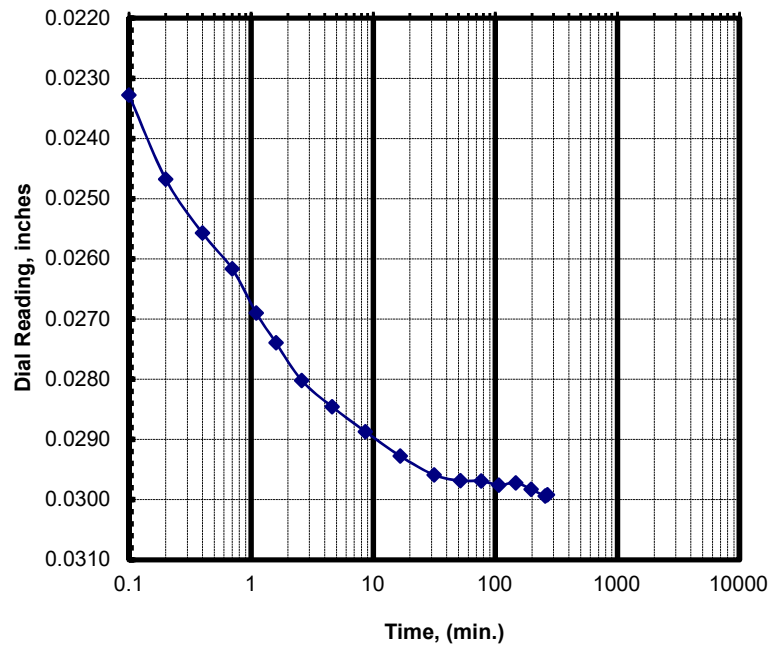
Sample Condition	Before	After	Consolidation Parameters		
Moisture Content, %:	24.6%	15.8%	Overburden Pressure, tsf:	0.2	
Sample Height, in.:	0.9980	0.8471	Preconsolidation Pressure, tsf:	1.0	
Void Ratio, e:	0.6806	0.4265	Compression Index, (Cc):	0.192	
Unit Weight, pcf:	100.2	118.1	Re-Compression Index, (Cr)	0.024	
Degree of Saturation:	0.97	1.00	Swell Index, Cs:	-	
Project Number:	02541139		Liquid Limit:	NA	Hand Pen: 0.20 tsf
Project Name	CPRA PO 181		Plastic Limit:	NA	-200 %: NA
Boring No.:	M-7		Plasticity Index:	NA	SPG: 2.7
Sample ID, Depth:	S-3, 4-6 ft		Classification:	Light Gray FAT CLAY	

t50 & t90 Curves for the Vertical Stress of 0.125 tsf

[illegible]

Project Number: 02541139	LL: NA	$D_{0(\min)}$ (in): 0.0181	d_0 (in): 0.0181
Project Name: CPRA PO 181	PL: NA	D_{end} (in): 0.0228	d_{100} (in): 0.0218
Boring & Depth: M-8, S-8, 14-16 ft	PL: NA	Δ_{hl} (in.): 0.0047	δ_{eop} (in.): 0.0037
Sample: Gray SANDY LEAN CLAY	- 200 (%): NA	t_{50} (min): 0.73	t_{90} (min): 39

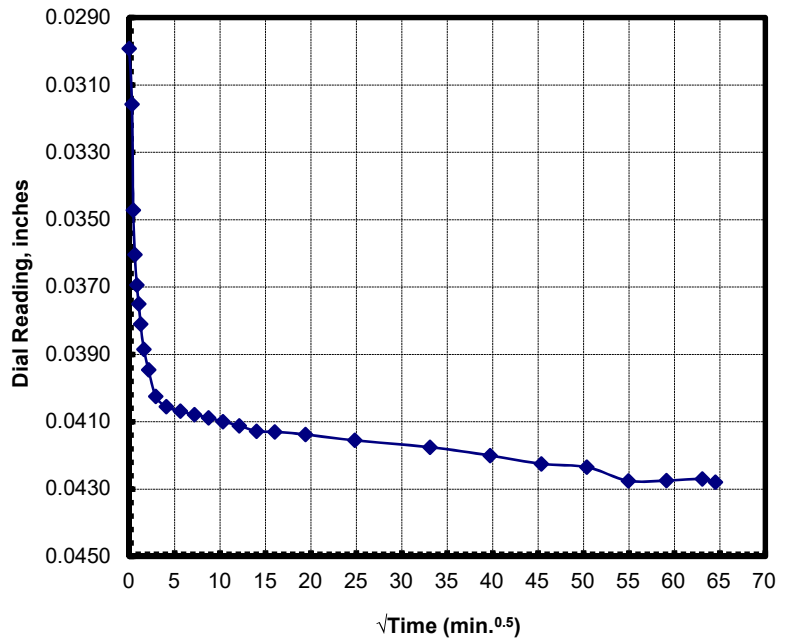
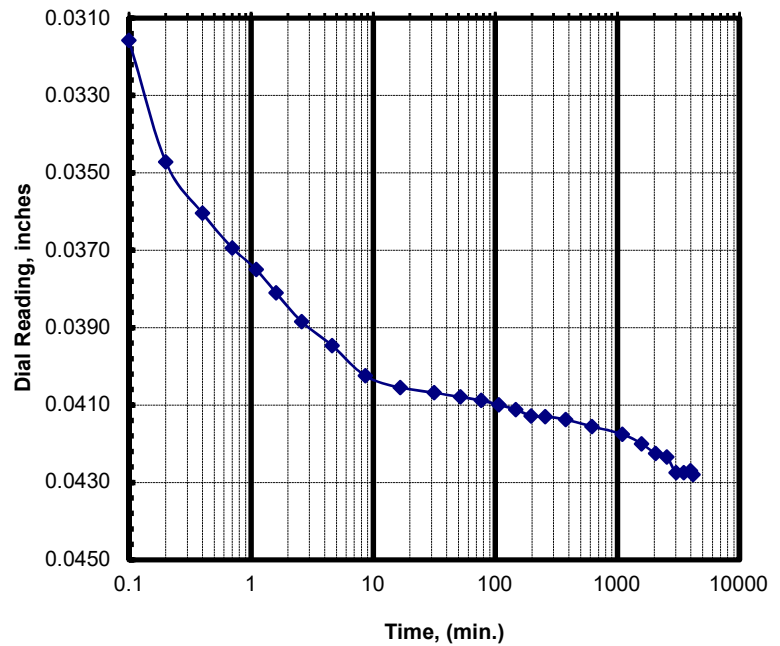
CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.25 tsf

[illegible]

Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0228	d ₀ (in): 0.0228
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0299	d ₁₀₀ (in): 0.0294
Boring & Depth:	M-8, S-8, 14-16 ft	PL: NA	Δ _{hi} (in.): 0.0071	δ _{eop} (in.): 0.0066
Sample:	Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 0.68	t ₉₀ (min): 6.6

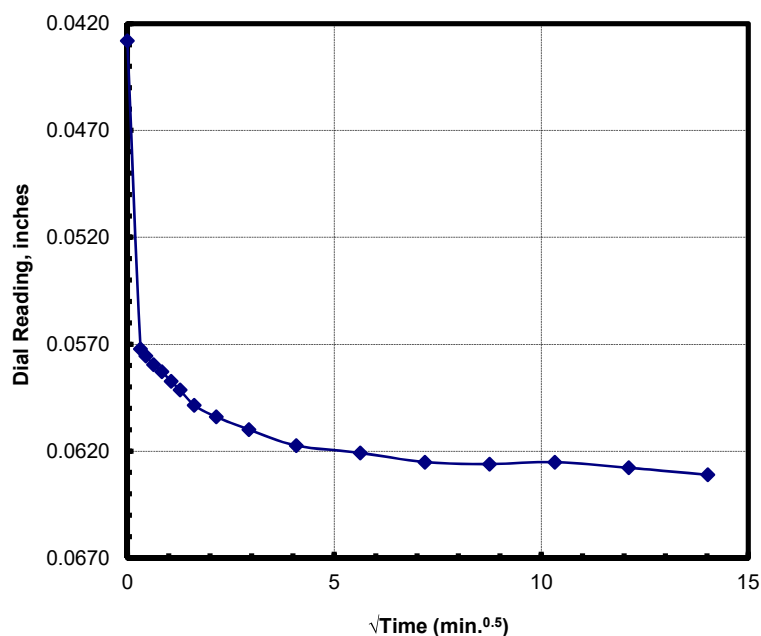
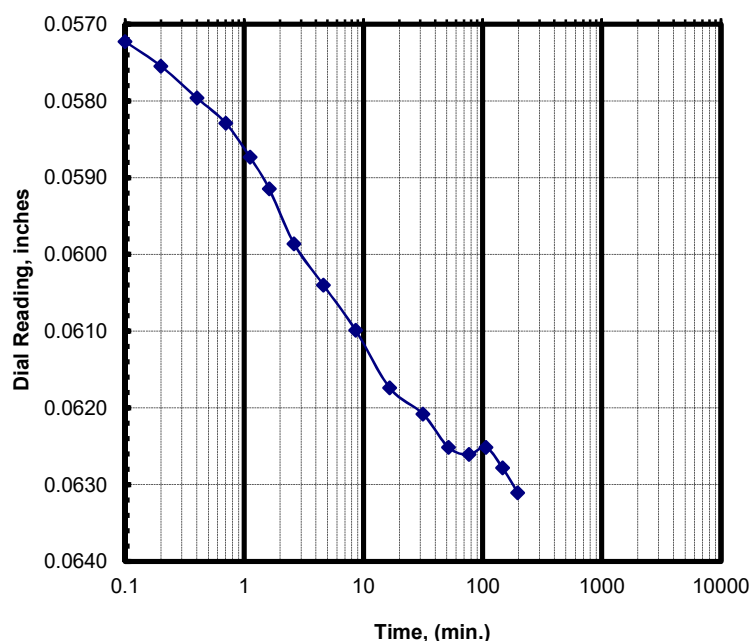
CONSOLIDATION TEST RESULTS - ASTM D 2435 ***t₅₀ & t₉₀ Curves for the Vertical Stress of 0.5 tsf***

Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
11/7/20 12:57:11 PM	0.00	0.00	0.0299
11/7/20 12:57:17 PM	0.10	0.32	0.0316
11/7/20 12:57:23 PM	0.20	0.45	0.0347
11/7/20 12:57:35 PM	0.40	0.63	0.0360
11/7/20 12:57:53 PM	0.70	0.84	0.0369
11/7/20 12:58:17 PM	1.10	1.05	0.0375
11/7/20 12:58:47 PM	1.60	1.26	0.0381
11/7/20 12:59:47 PM	2.60	1.61	0.0388
11/7/20 1:01:47 PM	4.60	2.14	0.0395
11/7/20 1:05:47 PM	8.60	2.93	0.0402
11/7/20 1:13:47 PM	16.60	4.07	0.0405
11/7/20 1:28:47 PM	31.60	5.62	0.0407
11/7/20 1:48:47 PM	51.60	7.18	0.0408
11/7/20 2:13:47 PM	76.60	8.75	0.0409
11/7/20 2:43:47 PM	106.60	10.32	0.0410
11/7/20 3:23:47 PM	146.60	12.11	0.0411
11/7/20 4:13:47 PM	196.60	14.02	0.0413
11/7/20 5:13:47 PM	256.60	16.02	0.0413
11/7/20 7:13:48 PM	376.62	19.41	0.0414
11/7/20 11:13:48 PM	616.62	24.83	0.0416
11/8/20 7:13:48 AM	1096.62	33.12	0.0418
11/8/20 3:13:48 PM	1576.62	39.71	0.0420
11/8/20 11:13:48 PM	2056.62	45.35	0.0422
11/9/20 7:13:48 AM	2536.62	50.36	0.0423
11/9/20 3:13:48 PM	3016.62	54.92	0.0427
11/9/20 11:13:48 PM	3496.62	59.13	0.0427
11/10/20 7:13:48 AM	3976.62	63.06	0.0427
11/10/20 10:19:32 AM	4162.35	64.52	0.0428



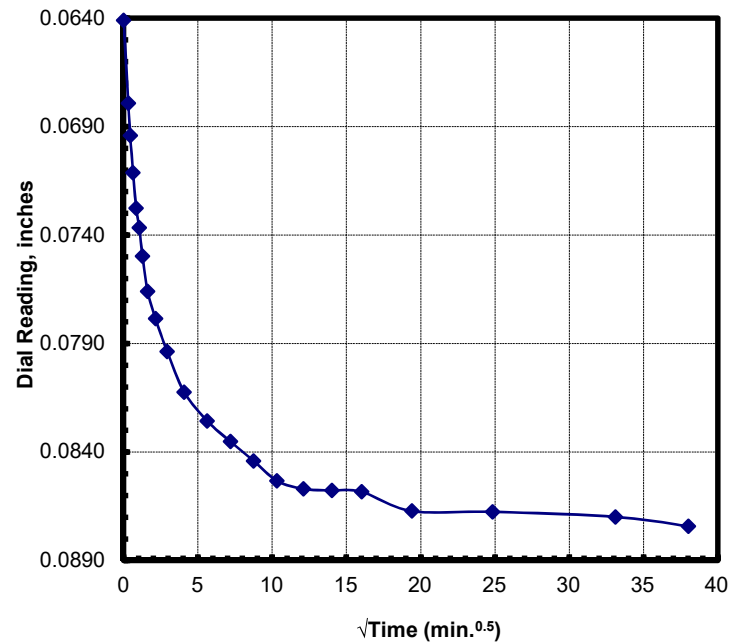
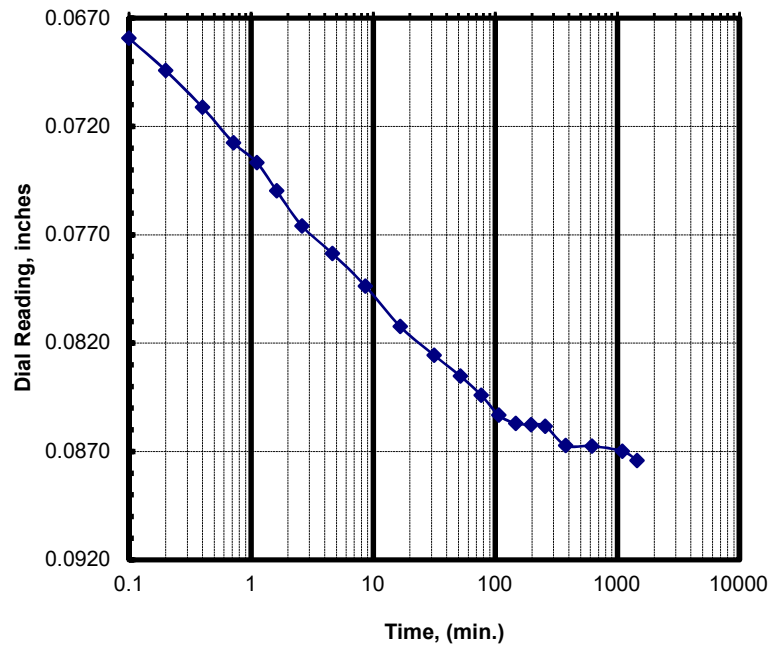
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0299	d ₀ (in): 0.0299
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0428	d ₁₀₀ (in): 0.0403
Boring & Depth:	M-8, S-8, 14-16 ft	PL: NA	Δ _h (in.): 0.0129	δ _{eop} (in.): 0.0104
Sample:	Gray SANDY LEAN CLAY	- 200 (%) : NA	t ₅₀ (min): 0.22	t ₉₀ (min): 3.9

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 1 tsf

[illegible]

Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0393	d ₀ (in): 0.0428
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0641	d ₁₀₀ (in): 0.0626
Boring & Depth:	M-8, S-8, 14-16 ft	PL: NA	Δ _{hi} (in.): 0.0248	δ _{eop} (in.): 0.0198
Sample:	Gray SANDY LEAN CLAY	- 200 (%) : NA	t ₅₀ (min): 2.5	t ₉₀ (min): 31

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 2 tsf

[illegible]

Project Number: 02541139	LL: NA	D _{0(min)} (in): 0.0641	d ₀ (in): 0.0641
Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.0874	d ₁₀₀ (in): 0.0852
Boring & Depth: M-8, S-8, 14-16 ft	PL: NA	Δ _{hi} (in.): 0.0233	δ _{oop} (in.): 0.0211
Sample: Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 1.6	t ₉₀ (min): 46

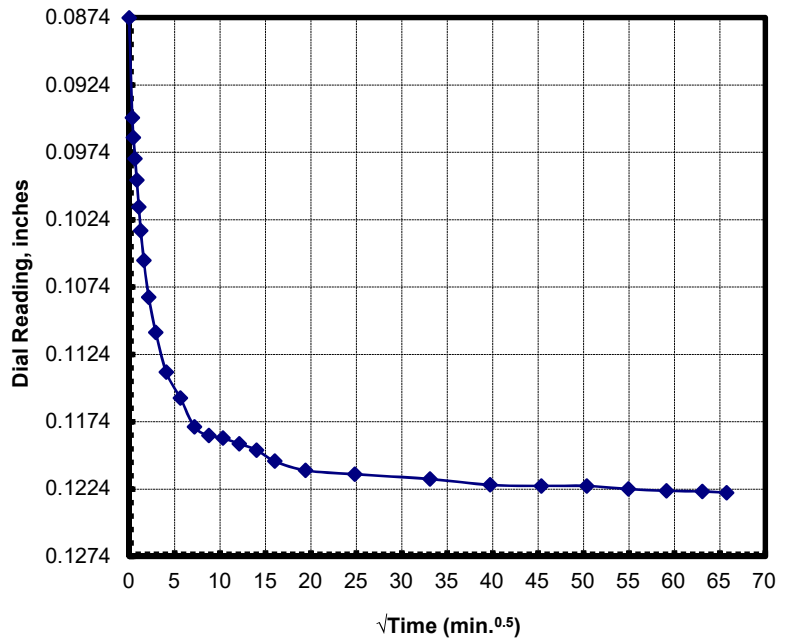
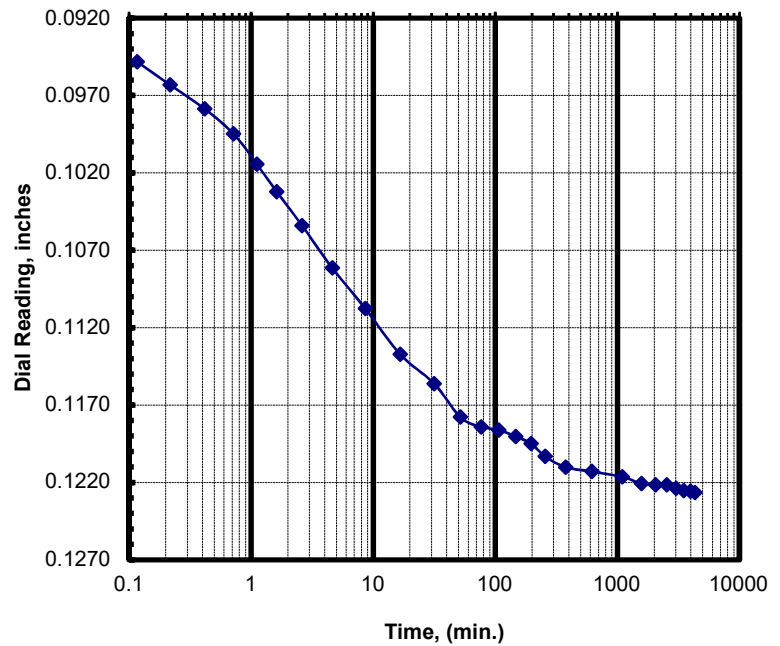
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02541139 M-8 S-8 (T) 14-16'

CONSOLIDATION TEST RESULTS - ASTM D 2435

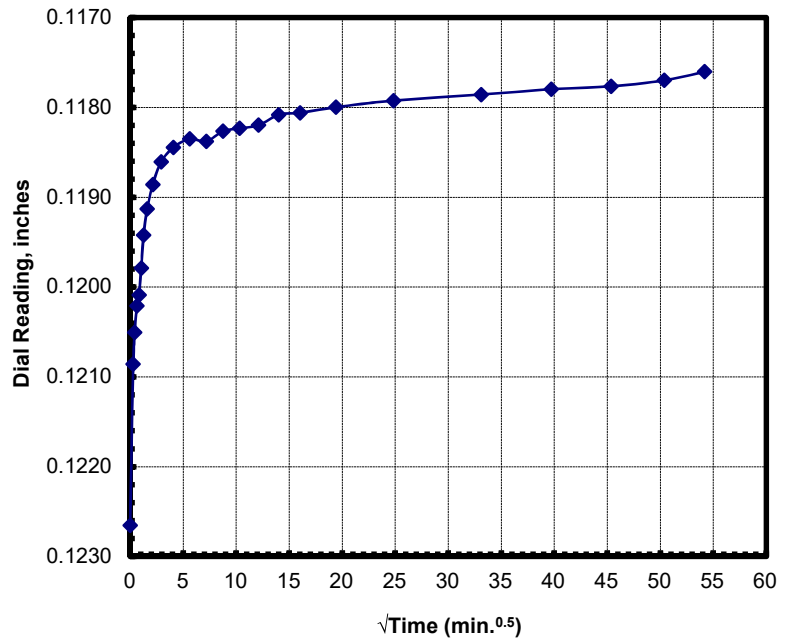
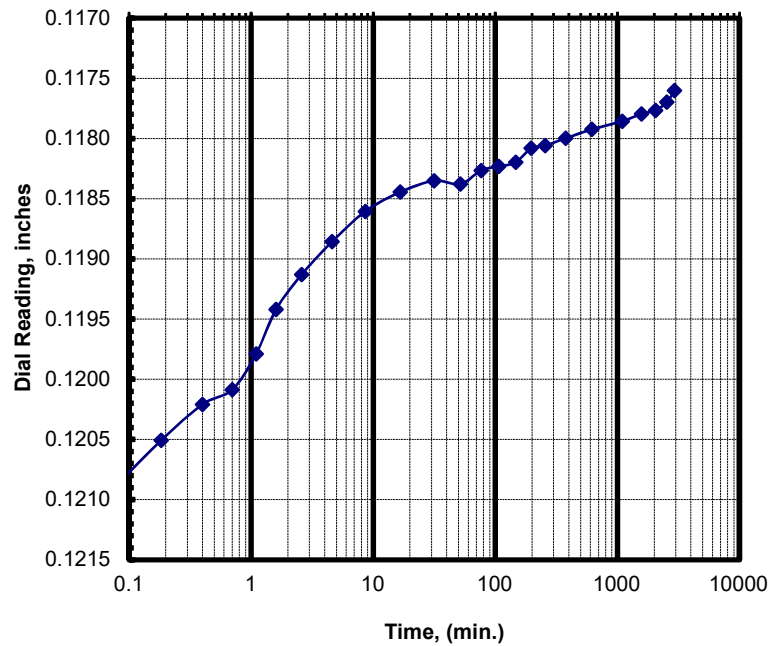
t₅₀ & t₉₀ Curves for the Vertical Stress of 4 tsf

Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
11/12/20 9:01:18 AM	0.00	0.00	0.0874
11/12/20 9:01:25 AM	0.12	0.34	0.0948
11/12/20 9:01:31 AM	0.22	0.47	0.0963
11/12/20 9:01:43 AM	0.42	0.65	0.0979
11/12/20 9:02:01 AM	0.72	0.85	0.0995
11/12/20 9:02:25 AM	1.12	1.06	0.1014
11/12/20 9:02:55 AM	1.62	1.27	0.1032
11/12/20 9:03:55 AM	2.62	1.62	0.1054
11/12/20 9:05:55 AM	4.62	2.15	0.1081
11/12/20 9:09:55 AM	8.62	2.94	0.1108
11/12/20 9:17:55 AM	16.62	4.08	0.1137
11/12/20 9:32:56 AM	31.63	5.62	0.1156
11/12/20 9:52:56 AM	51.63	7.19	0.1178
11/12/20 10:17:56 AM	76.63	8.75	0.1184
11/12/20 10:47:56 AM	106.63	10.33	0.1186
11/12/20 11:27:56 AM	146.63	12.11	0.1190
11/12/20 12:17:56 PM	196.63	14.02	0.1195
11/12/20 1:17:56 PM	256.63	16.02	0.1203
11/12/20 3:17:56 PM	376.63	19.41	0.1210
11/12/20 7:17:56 PM	616.63	24.83	0.1213
11/13/20 3:17:57 AM	1096.65	33.12	0.1216
11/13/20 11:17:57 AM	1576.65	39.71	0.1221
11/13/20 7:17:58 PM	2056.67	45.35	0.1222
11/14/20 3:17:58 AM	2536.67	50.37	0.1222
11/14/20 11:17:59 AM	3016.68	54.92	0.1224
11/14/20 7:17:59 PM	3496.68	59.13	0.1225
11/15/20 3:18:00 AM	3976.70	63.06	0.1226
11/15/20 9:01:33 AM	4320.25	65.73	0.1227



Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0874	d ₀ (in): 0.0874
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1227	d ₁₀₀ (in): 0.1180
Boring & Depth:	M-8, S-8, 14-16 ft	PL: NA	Δ _h (in.): 0.0352	δ _{eop} (in.): 0.0306
Sample:	Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 1.4	t ₉₀ (min): 22

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 2 tsf

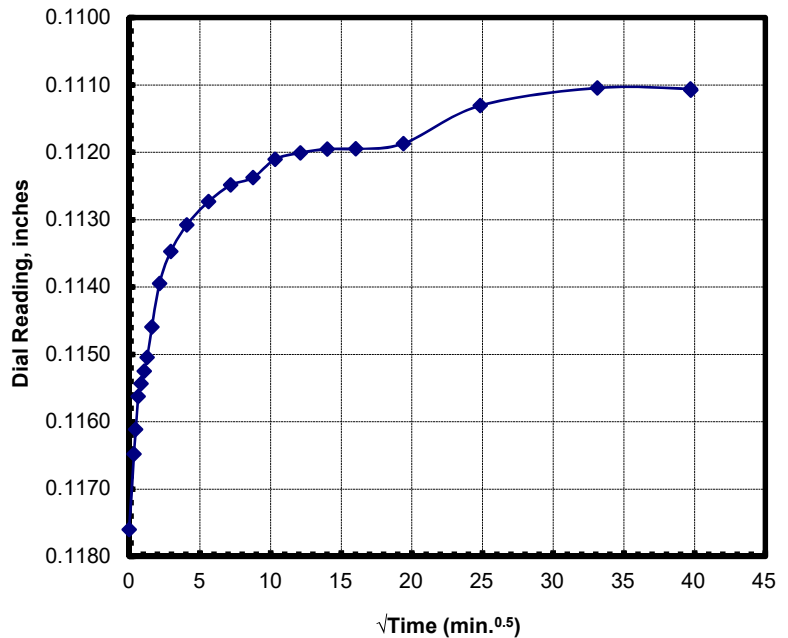
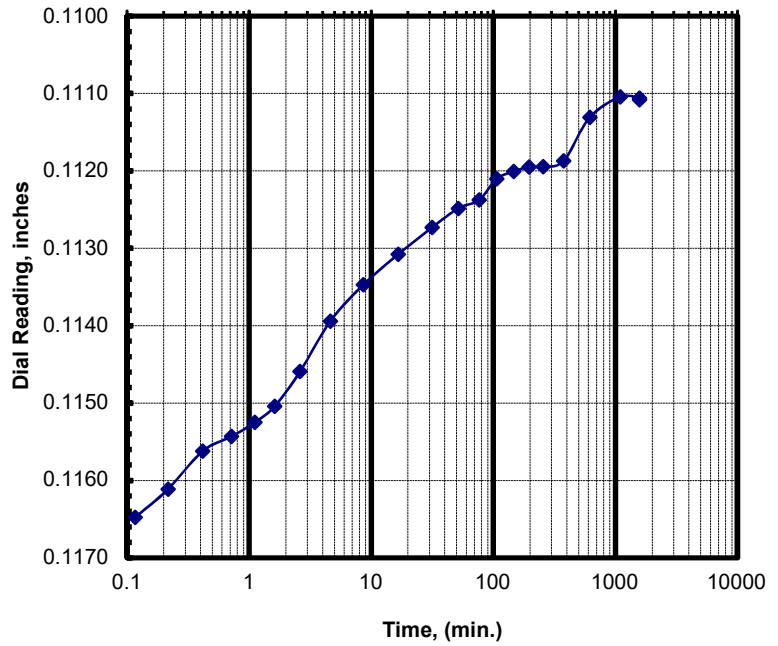
[illegible]

Project Number: 02541139	LL: NA	D _{0(min)} (in): 0.1227	d ₀ (in): 0.1227
Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.1176	d ₁₀₀ (in): 0.1179
Boring & Depth: M-8, S-8, 14-16 ft	PL: NA	Δ _{hi} (in.): -0.0050	δ _{exp} (in.): -0.0048
Sample: Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

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02541139 M-8 S-8 (T) 14-16'

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 1 tsf

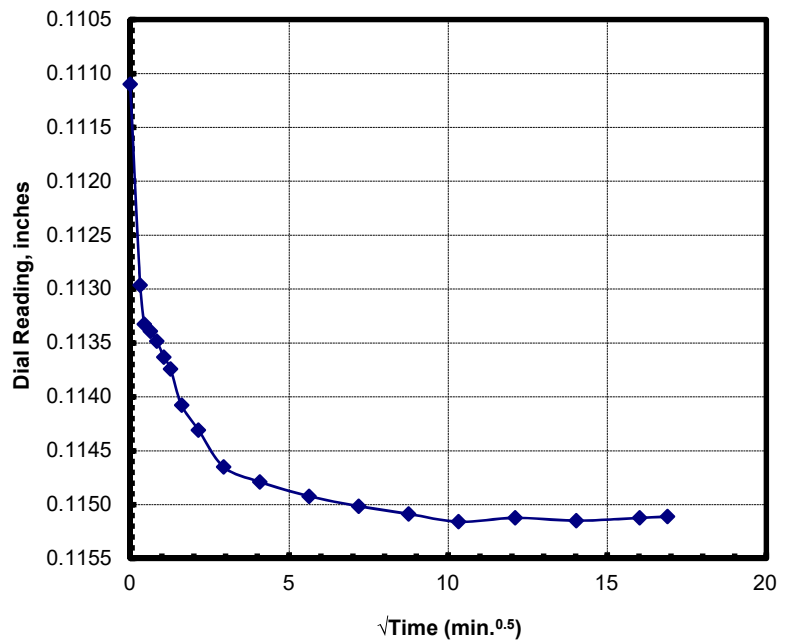
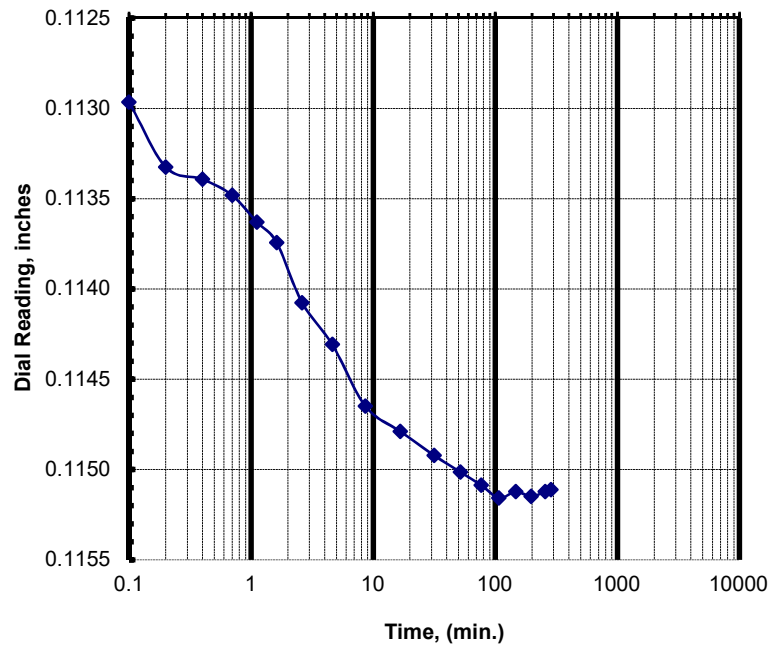
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Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1111	d ₁₀₀ (in): 0.1119
Boring & Depth:	M-8, S-8, 14-16 ft	PL: NA	Δ _{hi} (in.): -0.0065	δ _{exp} (in.): -0.0057
Sample:	Gray SANDY LEAN CLAY	- 200 (%) : NA	t ₅₀ (min):	t ₉₀ (min):

intertek
psi

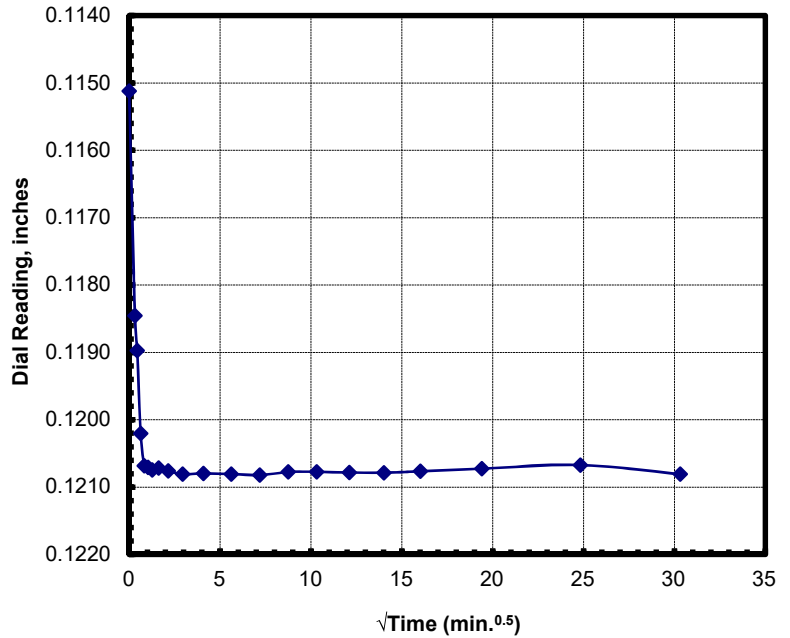
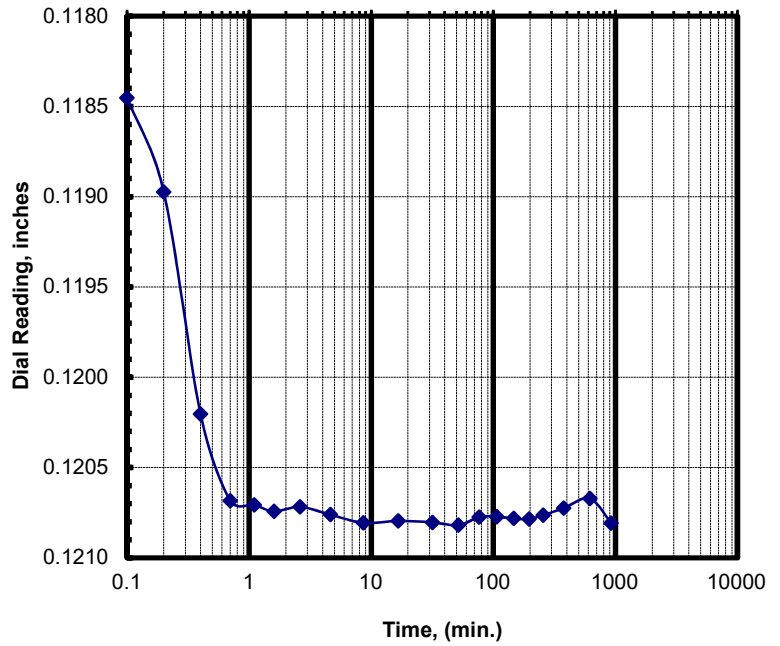
02541139 M-8 S-8 (T) 14-16'

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 2 tsf

[illegible]

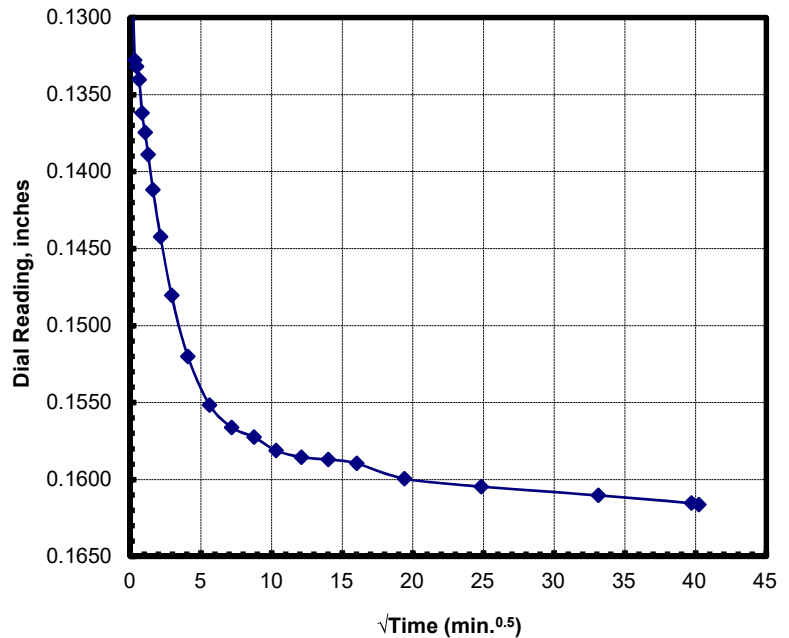
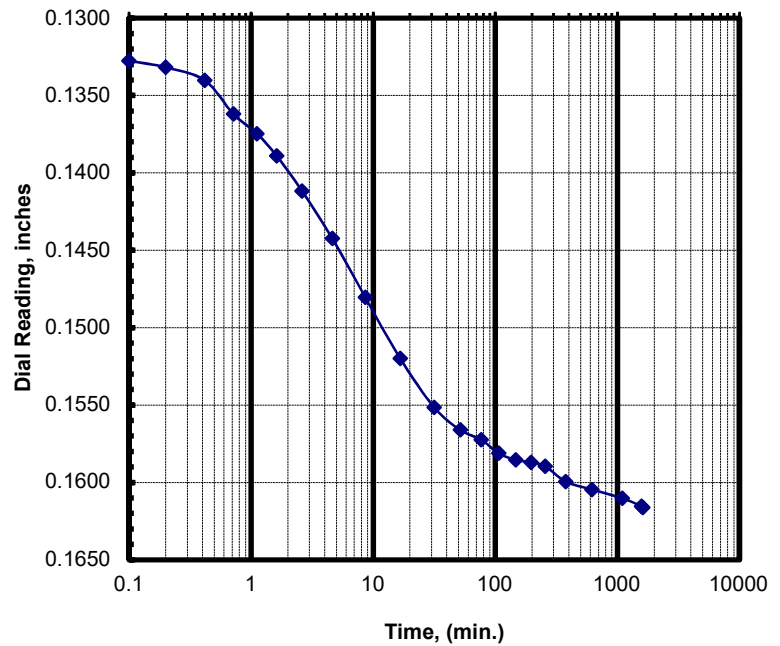
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Project Name: CPRA PO 181	PL: NA	D _{end} (in): 0.1151	d ₁₀₀ (in): 0.1150
Boring & Depth: M-8, S-8, 14-16 ft	PL: NA	Δ _{hi} (in.): 0.0040	δ _{oep} (in.): 0.0039
Sample: Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

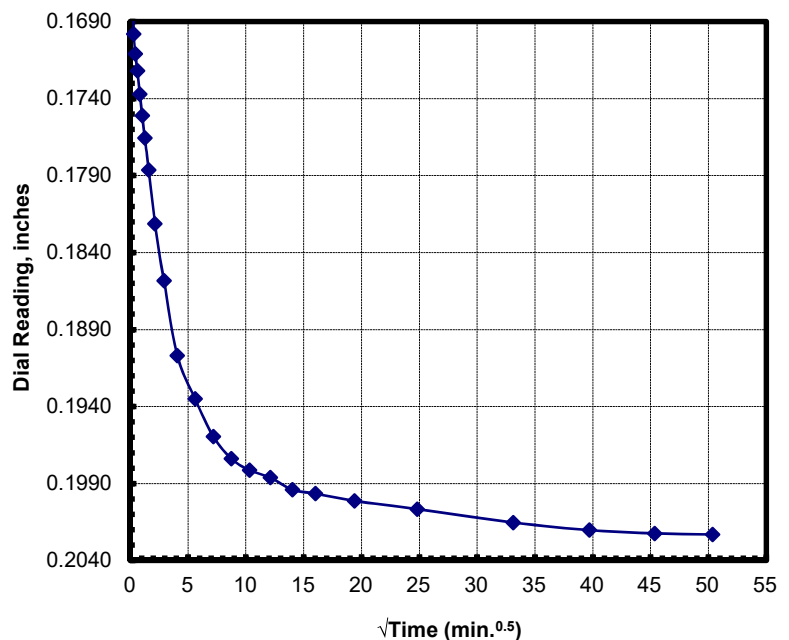
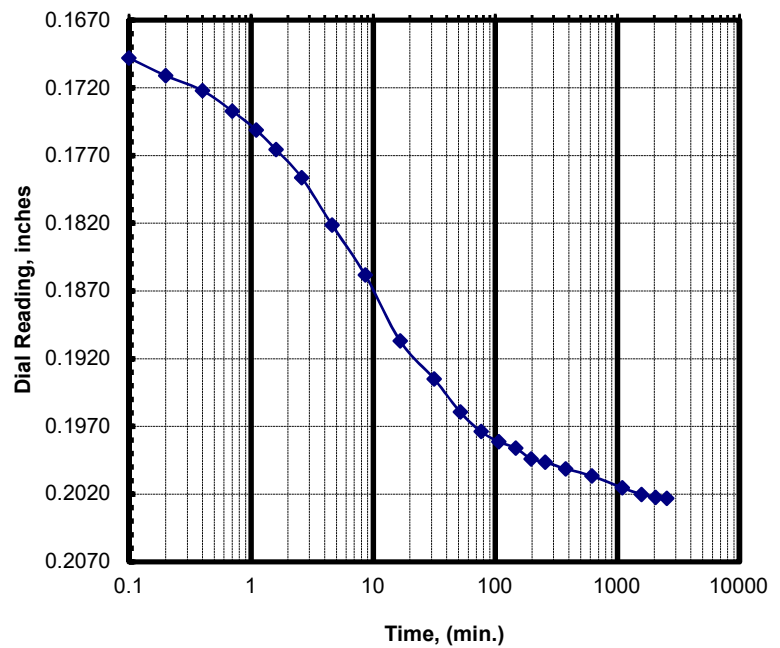
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Boring & Depth:	M-8, S-8, 14-16 ft	PL: NA	Δ _{hi} (in.): 0.0057	δ _{exp} (in.): 0.0056
Sample:	Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 8 tsf

[illegible]

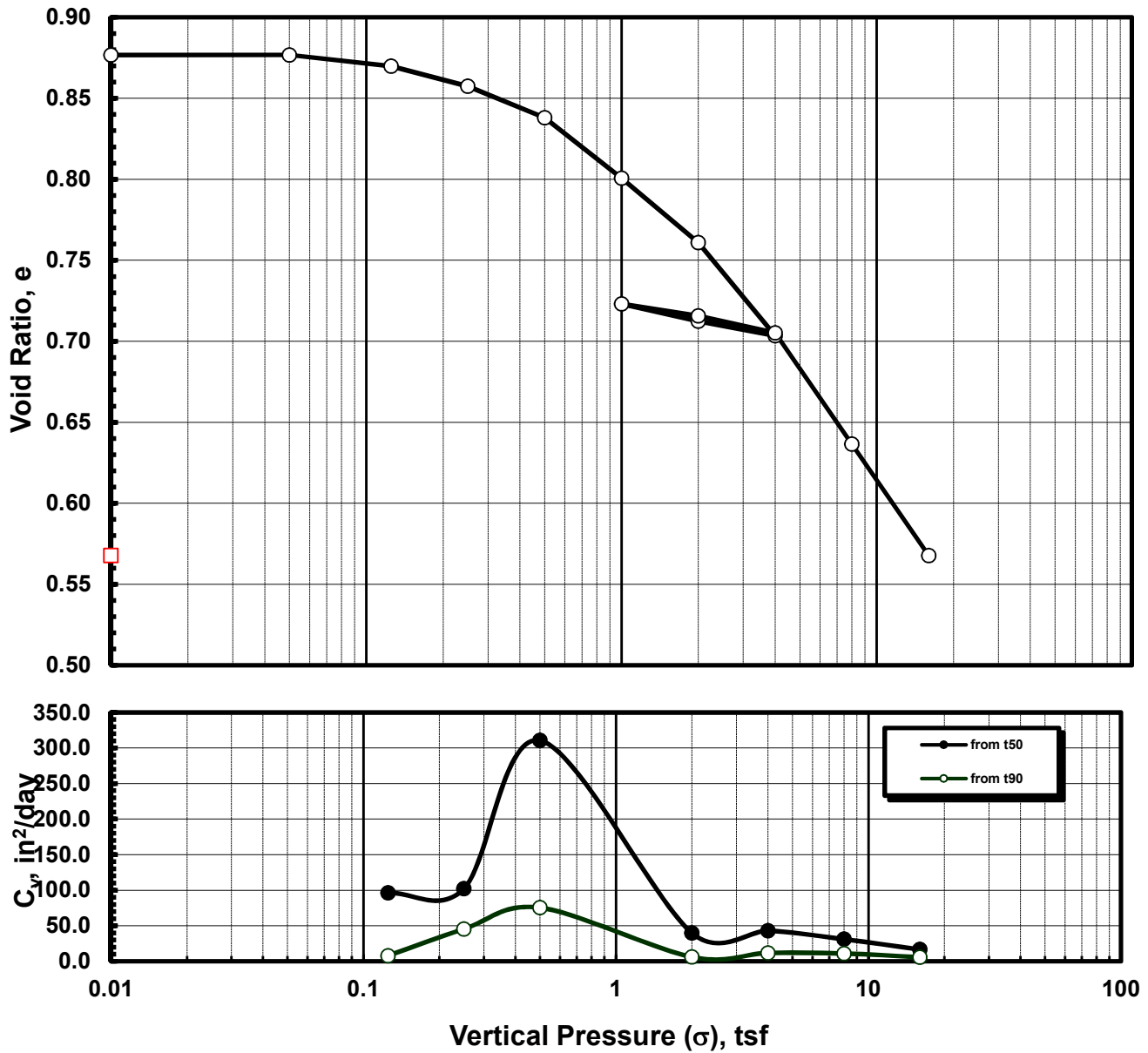
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Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1616	d ₁₀₀ (in): 0.1573
Boring & Depth:	M-8, S-8, 14-16 ft	PL: NA	Δ _{hi} (in.): 0.0497	δ _{exp} (in.): 0.0365
Sample:	Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 1.8	t ₉₀ (min): 22

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 16 tsf

[illegible]

Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.1616	d ₀ (in): 0.1616
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.2023	d ₁₀₀ (in): 0.1981
Boring & Depth:	M-8, S-8, 14-16 ft	PL: NA	Δ _{hi} (in.): 0.0407	δ _{oop} (in.): 0.0365
Sample:	Gray SANDY LEAN CLAY	- 200 (%): NA	t ₅₀ (min): 3.1	t ₉₀ (min): 39

CONSOLIDATION TEST RESULTS - ASTM D 2435

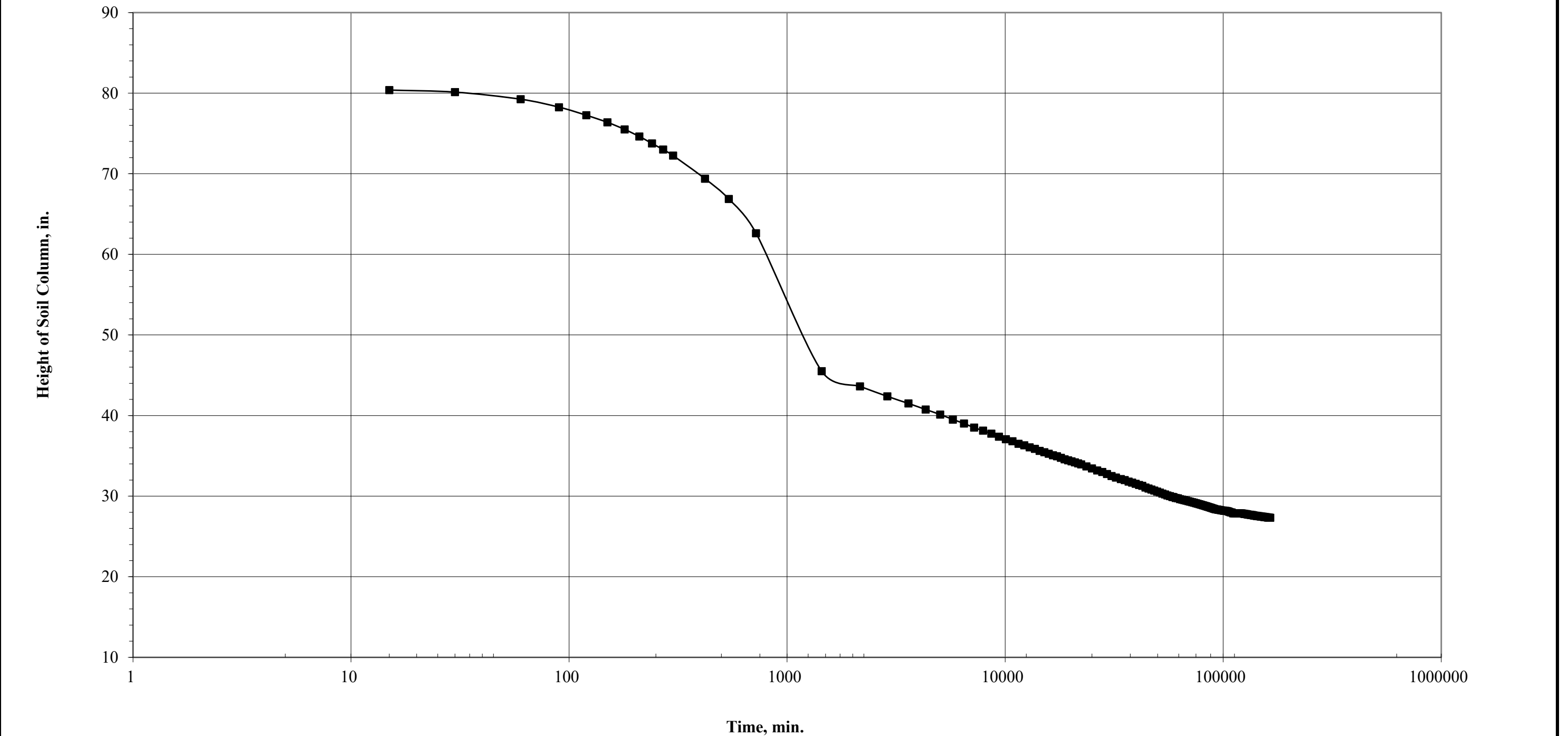


Sample Condition	Before	After	Consolidation Parameters		
Moisture Content, %:	29.4%	21.2%	Overburden Pressure, tsf:	0.5	
Sample Height, in.:	0.9970	0.8328	Preconsolidation Pressure, tsf:	1.0	
Void Ratio, e:	0.8768	0.5677	Compression Index, (Cc):	0.228	
Unit Weight, pcf:	89.2	106.8	Re-Compression Index, (Cr)	0.030	
Degree of Saturation:	0.90	1.00	Swell Index, Cs:	-	
Project Number: 02541139			Liquid Limit:	NA	Hand Pen: 0.50 tsf
Project Name CPRA PO 181			Plastic Limit:	NA	-200 %: NA
Boring No.: M-8			Plasticity Index:	NA	SPG: 2.683
Sample ID, Depth: S-8, 14-16 ft			Classification:	Gray SANDY LEAN CLAY	

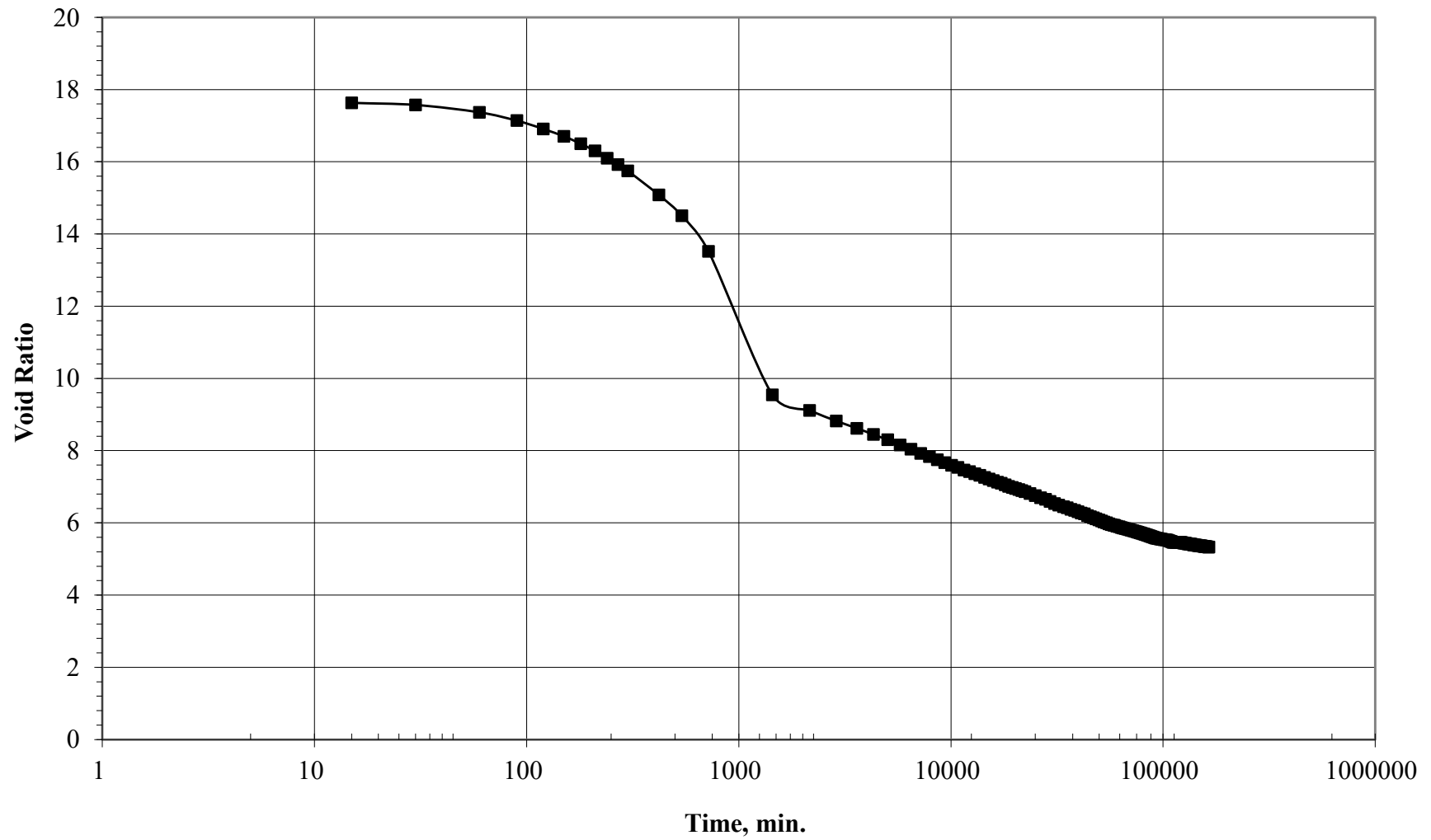


Appendix Figure No. G.1-236

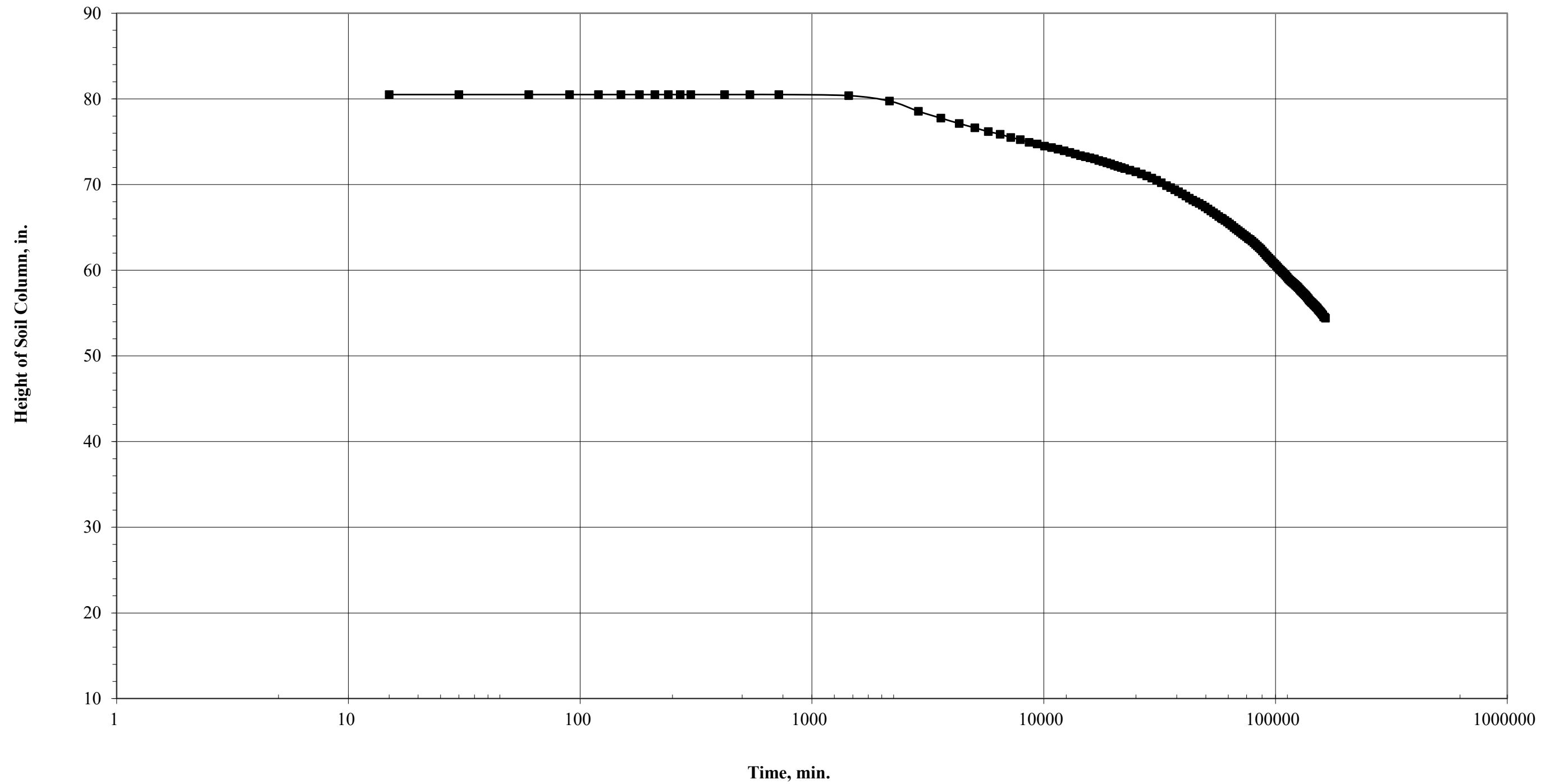
SETTLING COLUMN #1, 150 g/l



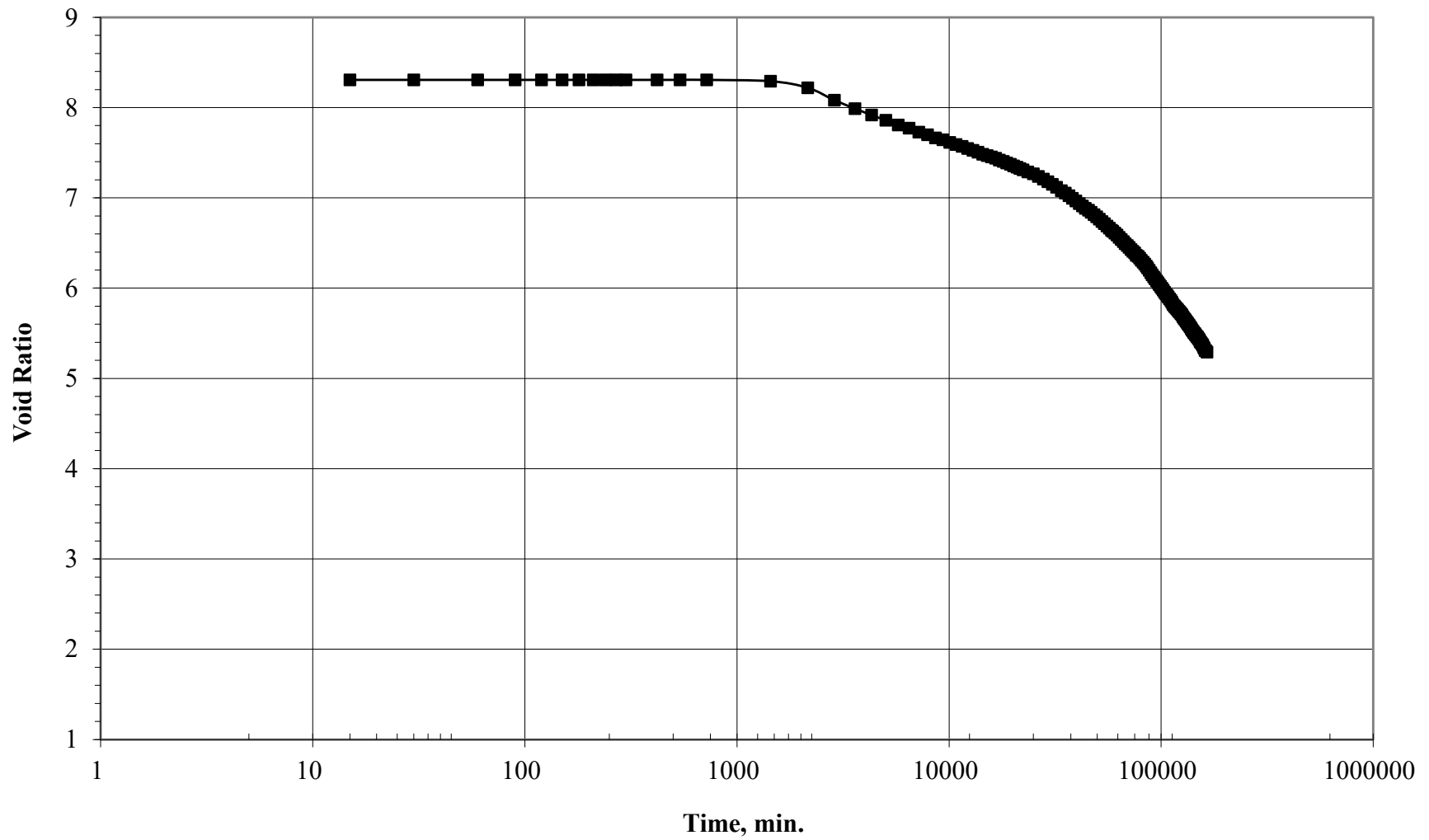
SETTLING COLUMN #1, 150 g/l



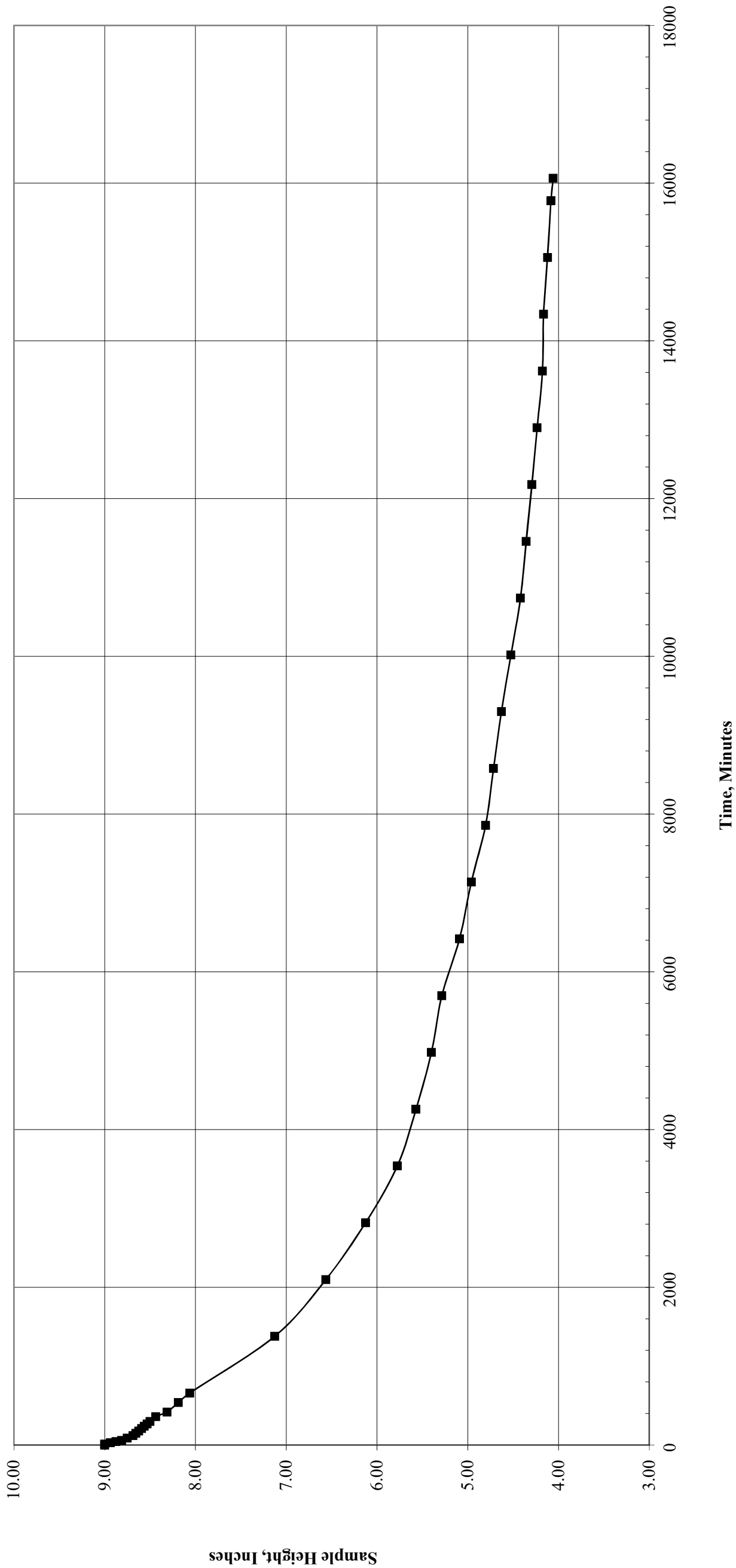
SETTLING COLUMN #2, 300 g/l



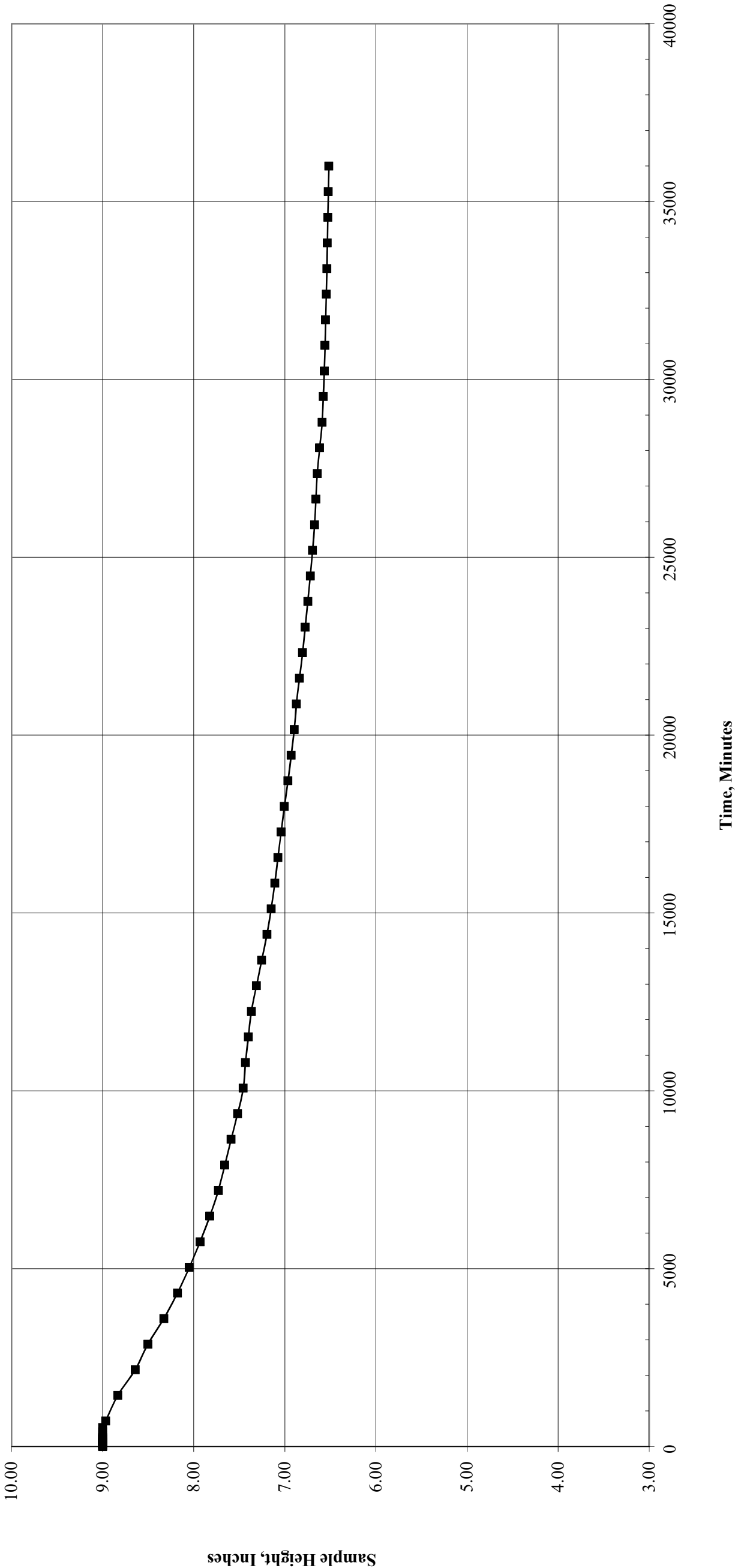
SETTLING COLUMN #2, 300 g/l



SELF-WEIGHT TEST#1, 150 g/l
Initial Sample Consolidation



SELF-WEIGHT TEST#2, 300 g/l
Initial Sample Consolidation





Professional Service Industries, Inc.
724 Central Avenue
Jefferson, Louisiana 70121
Phone: (504) 733-9411

November 8, 2021

Coastal Protection and Restoration Authority (CPRA)
150 Terrace Avenue
Baton Rouge, Louisiana 70802

Attn: Ms. April Newman
Project Manager
Phone: (225) 342-6412
Email: April.Newman@la.gov

Re: **Geotechnical Engineering Services Data Report**
Bayou Cane Marsh Creation Project (PO-181)
St. Tammany Parish, LA
PSI Project No. 02541139-1 Addendum #1

Dear Ms. Newman:

Professional Service Industries, Inc. (PSI), an Intertek company, is pleased to submit our Geotechnical Data Report for the above-referenced project. This report presents the results of our field exploration and laboratory testing performed for the proposed construction. A final geotechnical report will be submitted once the engineering analyses are performed.

If you have any questions pertaining to this report, please contact our office at (504) 733-9411. PSI would be pleased to continue providing geotechnical and construction material testing services throughout the construction of the project, and we look forward to working with you and your organization on this and future projects.

Respectfully submitted,

PROFESSIONAL SERVICE INDUSTRIES, INC.

Prasoontiwari, P.E. (TX)
Geotechnical Department Manager

11.3.2021
Nabil S. Mikhail, P.E., D. GE.
Chief Engineer



PSI Project No: 02541139-1 Addendum #1
Bayou Cane Marsh Creation Project (PO-181)
St. Tammany Parish, LA
November 8, 2021

GEOTECHNICAL ENGINEERING SERVICES DATA REPORT

**BAYOU CANE MARSH CREATION PROJECT (PO-181)
ST. TAMMANY PARISH, LA**

PSI PROJECT NO. 02541139-1

PREPARED FOR

**COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA)
150 TERRACE AVENUE
BATON ROUGE, LA 70802**

November 16, 2021

BY

**PROFESSIONAL SERVICE INDUSTRIES, INC.
724 CENTRAL AVENUE
JEFFERSON, LOUISIANA 70121
PHONE: 504-733-9411**



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1. PROJECT INFORMATION

1.1. PROJECT AUTHORIZATION

Professional Service Industries, Inc. (PSI), an Intertek company, has completed the field and laboratory portions of a geotechnical investigation for the proposed Bayou Cane Marsh Creation Project (PO-181) in St. Tammany Parish, Louisiana. Our scope of services is outlined in PSI Proposal 0254-332404- Addendum #2, dated May 13, 2021. Our geotechnical services were authorized by Mr. Jerry Carroll, P.E., Lafayette Regional Operations Manager with CPRA, by issuing a Notice to Proceed as Task #3 dated May 25, 2021 under our existing Contract No. 4400015386. A final geotechnical report will be submitted once the engineering analyses are performed.

1.2. PROJECT DESCRIPTION

In addition to the services that were proposed in PSI's original Proposal Number 0254-282343 Rev.6, CPRA has requested additional geotechnical services for the Bayou Cane Marsh Creation Project (PO-181). Ms. April Newman with CPRA provided scope for these additional services in email correspondences dated December 17, 2020 and May 3, 2021. The project information included scope of services for geotechnical investigation and engineering services for Bayou Cane Marsh Creation Project (PO-181) in St. Tammany Parish, Louisiana.

A modified version of the original permitting documents (dated December 2020) pertaining to the additional CPT soundings was provided to PSI by Ms. April Newman in an email correspondence dated December 17, 2020. Based on the modified permitting document, PSI understood that seven (7) additional CPT soundings are required to support the potential expansion of Marsh Creation Area-2 through Marsh Creation Area-6.

In addition to these seven (7) CPT soundings, additional boring and Cone Penetration Test (CPT) locations for Marsh Creation Area -7 and new borrow area were also included in the scope and information was provided to PSI by Ms. April Newman of CPRA in an email correspondence dated May 3, 2021. A Google Earth file presenting additional borings and CPT soundings was provided to PSI.

Based on the provided information, PSI understands that one (1) soil boring and three (3) CPT soundings in the marsh area are required to support the potential expansion of Marsh Creation Area -7. Six (6) soil borings in the Lake Pontchartrain area are required to explore the new borrow area for this project.

1.3. SITE LOCATION AND DESCRIPTION

The PO-181 project area is located in CWPPRA Planning Region 1 of Pontchartrain, St. Tammany Parish, along the north shore of Lake Pontchartrain between Mandeville and Lacombe on property owned by either the State of Louisiana (Fontainebleau State Park) or the Big Branch National Wildlife Refuge. The project area spans approximately six miles along the shoreline of Lake Pontchartrain and is bordered to the southeast by the existing Goose Point/Point Platte Marsh Creation Project (PO-0033) and to the west by the facilities of Fontainebleau State Park. Approximate coordinates for the center of marsh fill area of the project are shown in the **Table 1**.



Table 1: Centerline Coordinates of the PO-181 Project Area

Latitude (DMS)	Longitude (DMS)
30° 18'46.03" N	89° 59'42.42" W

The approximate site location is shown on the Site Vicinity Map included in the **Appendix A**, which is based on Appendix B of the CPRA Scoping document for Project (PO-181), Proposed Soil Sampling Layout and Permit Drawings.

2. FIELD AND LABORATORY PROCEDURES

2.1. FIELD EXPLORATION

2.1.1. PERMIT ACCESS

Prior to mobilizing for the field work to survey the boring locations and conduct site reconnaissance, PSI coordinated and acquired documented access permissions to conduct any work at the subject site. A Special Use Permit for Sabine National Wildlife Refuge was secured prior to mobilizing to the project site. Wildlife Refuge Manager for Atchafalaya, Big Branch Marsh, and Bogue Chitto NWR's and Park Manager for Fontainebleau State Park were notified at least one day prior to the mobilization to the project site.

2.1.2. SURVEY POSITIONING AND MAGNETOMETER SURVEY

Prior to mobilizing the drilling equipment to perform the soil borings and Cone Penetration Tests (CPT's), Chustz surveying, LLC performed survey positioning and magnetometer survey at each boring/CPT location in the marsh area and borrow area, as per CPRA guidelines. The horizontal and vertical controls for survey positioning were performed in accordance with secondary monument and data sheets presented in Appendix D of the attached Scope Document. The magnetometer survey at each boring/CPT location in the marsh and borrow area was performed to ensure there are no pipelines or obstructions in the area. The locations in the marsh area were staked in the field by the survey crew. Based on information obtained from the survey work, details for the soil borings and CPT soundings details performed in the borrow area and the marsh area are presented in **Table 2** and **Table 3** respectively. The survey report is presented in **Appendix A**.



Table 2: Soil Borings details performed in Borrow area (Datum: NAVD88 GEOID12B)

Boring Location	Northing	Easting	Mudline Elevation (ft)	Water Surface Elevation (ft)
B-29	656677.18	3698381.97	-12.34	0.15
B-31	658345.27	3699211.64	-11.61	0.15
B-40	658408.23	3696787.19	-12.45	0.15
B-50	659294.35	3694769.91	-13.09	0.15
B-54	660589.97	3694205.99	-11.37	0.15
B-56	658852.89	3693376.93	-13.37	0.15

Note: A water depth of approximately 11 to 13 feet was recorded at boring locations in the Borrow area during our field operations.

Table 3: Soil Borings and CPT Sounding details performed in Marsh area (Datum: NAVD88 GEOID12B)

Boring Location	Northing	Easting	Mudline Elevation (ft)	Water Surface Elevation (ft)
C-38	667840.42	3691807.23	-1.49	0.15
C-39	667322.60	3692674.56	-1.53	0.15
C-40	666694.31	3692750.53	-1.63	0.15
C-41	662583.11	3703037.87	-2.91	0.15
C-42	661126.44	3704753.21	-1.12	0.15
C-43	655140.26	3707793.74	-1.43	0.15
C-44	654804.73	3708765.89	-1.68	0.15
C-46	649343.42	3711958.38	-2.13	0.15
C-47	650534.68	3712057.87	0.74	0.15
C-48	651024.22	3710687.76	-1.13	0.15
M-10	650022.94	3710882.72	-2.09	0.15

2.1.3. SOIL BORINGS & CPTS

Based on furnished information and discussions with CPRA, the soil boring and CPT sounding locations were established. Boring Location Plans for marsh area and borrow area are attached in **Appendix B**.

A total of eleven (11) soil boring and CPT soundings were performed for the marsh creation area. As discussed with CPRA, the soil boring and CPT soundings were explored using airboat mounted drilling equipment. All the soil borings and CPT soundings were performed between July 30 and August 4, 2021. Detailed descriptions of these activities are provided below:

- One (1) soil boring was drilled to a depth of 20 feet below mudline continuously and then at 5 feet intervals to a depth of 40 feet below the mudline. Drilling and sampling were performed in accordance with applicable ASTM Standards and the furnished Scope of Services Document.

The soil boring was advanced with a 4-inch diameter bit using the rotary wash drilling method. Undisturbed samples of cohesive soils were obtained by using three-inch diameter thin-wall tube



samplers (Shelby Tube) in general accordance with the procedures for “Thin-Walled Tube Geotechnical Sampling of Soils” (ASTM D1587). For cohesionless soils and semi-cohesive soils, Standard Penetration Test (SPT) was performed to obtain standard penetration values of the soil. The standard penetration value (N) is defined as the number of blows of a 140-pound hammer falling 30 inches that is required to advance the split-barrel sampler one (1) ft. into the soil. The number of blows is recorded for each of three (3) successive increments of six (6) inches penetration. The “N” value is obtained by adding the second and third incremental numbers. The results of the standard penetration test indicate the relative density of cohesionless soils and thereby provide a basis for estimating the relative strength of the soil profile components. Samples of granular soils were obtained utilizing a two (2) inch O.D. split-barrel sampler in general accordance with procedures for “Penetration Test and Split-Barrel Sampling of Soils” (ASTM D1586). The boreholes were grouted upon their completion per LDEQ and LA DOTD regulations.

- Ten (10) Cone Penetration Test (CPT) soundings were conducted to a refusal depth varying from 7 feet to 17 feet. Our CPT utilizes a 60° apex cone, 1.4-inch diameter, Type 2 electric cone penetrometer that meets ASTM D5778 specification. The soil information obtained from CPT soundings are presented in the **Appendix B**. The CPT was advanced to the shallow refusal depth due to the presence of the Pleistocene clays and relatively thick sand deposits. Refusal prior to the depth specified occurs due to achieving maximum tip resistance, a high frictional resistance of the sleeve, or excessive tilt angle of the CPT probe. To prevent damage to the CPT, no attempt was made to advance beyond refusal depth. Due to low penetration depths for CPT soundings, a cut off value of 1.7 was used for overburden correction factors for estimating normalized soil behavior type. For cohesive soils, Shear strength (S_u) was estimated using the total cone resistance (q_t) and cone factor (N_{kt}). An N_{kt} value equal to 16, used for marsh area CPT's.

$$s_u = \frac{q_t - \sigma_v}{N_{kt}}$$

A total of six (6) soil borings were performed for the borrow area. The borings were performed between July 8 to July 10. Detailed descriptions of these activities are provided below:

- Six (6) soil borings were sampled to a depth of 20 feet below mudline continuously. Drilling and sampling were performed in accordance with applicable ASTM Standards and the furnished Scope of Services Document.

The proposed soil borings were advanced with a 4-inch diameter rotary wash drilling method. Undisturbed samples of cohesive soils were obtained by using three-inch diameter thin-wall tube samplers (Shelby Tube) in general accordance with the procedures for “Thin-Walled Tube Geotechnical Sampling of Soils” (ASTM D1587). For cohesionless soils and semi-cohesive soils, Standard Penetration Test (SPT) was performed to obtain standard penetration values of the soil. The standard penetration value (N) is defined as the number of blows of a 140-pound hammer falling 30 inches that is required to advance the split-barrel sampler one (1) ft. into the soil. The number of blows is recorded for each of three (3) successive increments of six (6) inches penetration. The “N” value is obtained by adding the second and third incremental numbers. The results of the standard penetration test indicate the relative density of cohesionless soils and



thereby provide a basis for estimating the relative strength of the soil profile components. Samples of granular soils were obtained utilizing a two (2) inch O.D. split-barrel sampler in general accordance with procedures for “Penetration Test and Split-Barrel Sampling of Soils” (ASTM D1586). The boreholes were grouted upon their completion per LDEQ and LA DOTD regulations.

2.2. GEOTECHNICAL LABORATORY TESTING

During the field work, the samples were identified according to boring number and depth and were transported to PSI’s laboratory for classification and testing. A geotechnical laboratory testing program was developed after reviewing the field logs. The laboratory testing program was provided to CPRA for review and approval prior to testing. After approval, laboratory tests were performed in general accordance with applicable ASTM procedures and the furnished Scope Document. Laboratory testing of the obtained samples has been performed to evaluate the classification, strength and other engineering characteristics of the subsurface materials.

The Shelby tube samples were removed from the sampling tubes in the laboratory using a specially fabricated hydraulic piston-type extruder. The tube samples were extruded, classified and subjected to index and compressive strength tests. Additional estimates of shear strength were obtained through the use of a hand penetrometer (HP) and Torvane (TV), respectively. The results of the laboratory tests are presented in the Boring Logs given in the **Appendix B**. A key to the terms and symbols used on the boring logs is also presented in **Appendix B**.

An overview of the scope of the laboratory testing phase for both the marsh area borings and borrow area borings are tabulated in **Table 4** and **Table 5** respectively below. A detailed discussion on laboratory test results is presented in subsequent sections of the data report.

Table 4: Laboratory Testing Summary for Marsh area borings

Test Method	ASTM Reference	No. of Tests performed
Moisture Content	(ASTM D2216)	14
Visual Classification	(ASTM D2488)	14
Organic Content	(ASTM D2974)	1
Atterberg limits	(ASTM D4318)	4
Percent Passing #200 sieve	(ASTM D1140)	6
Specific Gravity	(ASTM D854)	1
Consolidation Test	(ASTM D2435)	1



Table 5: Laboratory Testing Summary for Borrow area borings

Test Method	ASTM Reference	No. of Tests performed
Moisture Content	(ASTM D2216)	60
Visual Classification	(ASTM D2488)	60
Atterberg limits	(ASTM D4318)	12
Hydrometer Analysis	(ASTM D7928)	6
Total Unit Weight Determination	(ASTM D2937)	12
Column Settling Test	USACE 1110-2-5027	1
Low Stress Consolidation Test	ASTM D2345 (mod.)	1

2.2.1. CLASSIFICATION AND INDEX TESTS

To determine the soil classification and geotechnical index properties, various classification and index tests were performed. The following classification and index tests for the marsh and borrow area were performed:

- Visual Classification (ASTM D2488)
- Moisture Content (ASTM D2216)
- Atterberg Limits (ASTM D4318)
- Percent Passing #200 sieve (ASTM D1140)
- Particle-Size Analysis and Hydrometer (ASTM D7928)
- Specific Gravity (ASTM D854)
- Organic Content (ASTM D2974)
- Total Unit Weight Determination (ASTM D2937)

1. Visual Classification

Visual classification was performed based on ASTM D2488 and incorporated into the soil boring logs in **Appendix B**. It includes the description of soil color, consistency and type, and identification of variations (organics, layers, seams, etc.).

2. Moisture Content

More than seventy-four (74) moisture content determinations (ASTM D2216) were performed in conjunction with the sample extrusion process and preparation of test specimens. Moisture content determinations were made for each extruded sample, and total densities were computed for each sample. Moisture content for each sample are included on the soil boring logs in **Appendix B**.

3. Atterberg Limits

A total of sixteen (16) Atterberg limit determinations (ASTMD4318) were performed on selected samples to assist in soil classification and to enable correlation to pertinent clay behavior properties. The Atterberg limit data consist of measured liquid limit (LL) and plastic limit (PL) values from which the plasticity index ($PI = LL - PL$) is derived. The individual test data are included on the boring logs in **Appendix B**.



4. Percent Passing #200 sieve and Hydrometer Analysis

A total of six (6) hydrometer particle size analysis tests (ASTM D7928) along with sieve analyses and six (6) fines content determinations (ASTM D 1140) were performed on selected samples. The test results, in terms of percent fines (i.e., percent by dry weight finer than the U.S. No. 200 sieve size, 0.074 mm, or silt and clay fraction) are included on the soil boring logs in **Appendix B**. The hydrometer test with sieve analyses results are included in **Appendix C**.

5. Specific Gravity

A total of one (1) specific gravity tests based on ASTM D854 were performed on selected samples and results are included in **Appendix C**.

6. Organic Content

A total of one (1) organic tests based on ASTM D2974 were performed on selected samples. The organic content results are included in **Appendix C**.

7. Total Unit Weight Determination

Total unit weights of the tube samples were computed based on (ASTM D2937) sample volume and weight measurements taken after exclusion of any materials that appeared to have been disturbed during the sampling or extrusion process.

2.2.2. STRENGTH TESTS

To determine the strength characteristics of the marsh area soil deposits, various compression tests and index strength tests were performed. The following tests for the selected marsh area soil samples were performed:

1. Hand penetrometer and Torvane Index Strength Tests

Hand-operated Torvane (TV) and hand penetrometer index strength tests were performed in conjunction with the sample extrusion process and are included in soil boring logs. These test results are considered to be index strengths in that the absolute value of the measured undrained shear strength is generally not considered adequately reliable for use in design. The test results, however, are useful in identifying soil strength variability and trends with respect to depth, material type, etc.

2.2.3. CONSOLIDATION TESTS

To determine the stress history, stress deformation, and time rate settlement characteristics of the marsh area soil deposits, one (1) consolidation test was performed on selected clay samples from marsh area soil borings.

1. Consolidation Test (ASTM D2435)

Incremental consolidation tests (ASTM D 2435) were included in the laboratory testing program to enable assessment of stress history and determination of stress history, stress deformation, and time rate settlement characteristics of the marsh clay deposits that will dictate post-construction settlement of the marsh creation area. one (1) consolidation test was conducted for M-10 soil boring sample.



During each load increment, the accumulation of vertical displacement with time is measured. In general, each load increment was sustained for a period of 12 to 36 hours. The vertical displacement versus time data was evaluated using the conventional log-time and square-root time curve fitting techniques to determine the end of primary consolidation (i.e., the point in time at which dissipation of load-induced excess pore water pressures in the sample had dissipated and drained creep ensued for each load increment). The individual test results, in terms of coefficient of consolidation versus effective vertical stress and void ratio versus effective vertical stress are presented in **Appendix D**.

2. Settling Column Test

One representative composite sample was prepared using samples obtained from the sampling locations performed within the proposed new borrow area. As requested, a settling column test with a concentration of 150g/l was performed in our geotechnical lab. As mentioned in the original Geotechnical Data Report (GDR) The settling tests consist of measuring the fall of the liquid-solid interface over time.

It should be noted that a settling column test with 150g/l concentration was also performed using the samples obtained from the previously selected borrow area and was presented in the original GDR dated December 8, 2020. The data obtained from both the settling column i.e. liquid-solid interface points over a period time, was plotted together to compare the soil settlement behavior from the two different borrow areas.

Based on the comparison of these two settling column tests, it can be concluded that the soil settlement behavior is similar for the soil with similar concentration from both the borrow areas. These data are plotted in terms of height of the interface over time in **Appendix E**.

3. DISCUSSION ON SUBSURFACE CONDITIONS

3.1. MARSH FILL AREA SUBSURFACE CONDITIONS

Based on the field observations, CPT soundings and results of the geotechnical laboratory testing, the soils were classified, and boring log and CPT sounding plots were developed.

Seven (7) additional CPT soundings (C-38 thru C-44) were performed to support the potential expansion of Marsh Creation Area-2 through Marsh Creation Area-6.

Based on the soil classification results, it can be concluded that the soil properties are in agreement with the soil data obtained from the previously performed soil borings and CPTs in the Marsh Creation Area-2 through Marsh Creation Area-6. Therefore, the original design properties utilized for these marsh areas will be implemented in the final Geotechnical Engineering Report. A generalized subsurface profile for marsh fill area is presented in **Table 6**.



Table 6: Generalized Soil Profile (CPT Soundings C-38 thru C-44)

Approximate Elevation Range (NAVD88 GEOID12B, feet)*	Consistency/Relative Density	Material Description
-1.5 to -5.5		Sensitive Fine-Grained Material
-2 to -10	Firm to Very Stiff	Clay and Silty Clay
-9.5 to -10.5		Sand and Silty Sand

**Referenced from existing mudline elevation at the boring locations performed in marsh fill area, at the time of drilling activities.*

One (1) soil boring (M-10) and three (3) CPT soundings (C-46 thru C-48) were performed to support the potential expansion of Marsh Creation Area -7. Based on the soil classification results, it can be concluded that the soil properties are in agreement with the soil data obtained from the previously performed soil boring M-9 and CPT-23 in the Marsh Creation Area-7. Therefore, the original design properties utilized for this marsh area will be implemented in the final Geotechnical Engineering Report.

A generalized subsurface profile for marsh fill area is presented in **Table 67**.

Table 7: Generalized Soil Profile (CPT Soundings C-46 thru C-48 & Boring M-10)

Approximate Elevation Range (NAVD88 GEOID12B, feet)*	Consistency/Relative Density	Material Description
0 to -3.5	Very Soft	Peat/ Sand and Silty Sand
-3 to -5		Clay and Silty Clay
-5 to -40	Medium Dense	Poorly Graded Sand with Silt (SP-SM) and Poorly Graded Sand (SP)

**Referenced from existing mudline elevation at the boring locations performed in marsh fill area, at the time of drilling activities.*

3.2. BORROW AREA SUBSURFACE CONDITIONS

Based on the field observations and results of the geotechnical laboratory testing, the soils were classified, and boring logs were developed. A generalized subsurface profile for borrow area is presented in **Table 8**.



Table 8: Generalized Soil Profile (Borrow Area)

Approximate Elevation Range (NAVD88 GEOID12B, feet)*	Consistency/Relative Density	Material Description**
-12 to -32**	Very Soft to Very Stiff	Fat Clay (CH), Sandy Fat Clay (CH), Lean Clay (CL), Fat Clay with Sand (CH)

**Referenced from existing mudline elevation at the boring locations performed in borrow area, at the time of drilling activities.*

***Silty Sand was observed in borehole B-29 from El. -16 to E. -18 feet below grade.*

Based on the soil classification results from these additional borings, the data confirms that this area is similar to that encountered previously in our original borrow area investigation. Furthermore, comparing the results from the settling column tests as well as the self-weight consolidation tests, it can be concluded that the soil settlement behavior for the soils obtained from the new borrow area is similar to the soil sampled in the original borrow area.

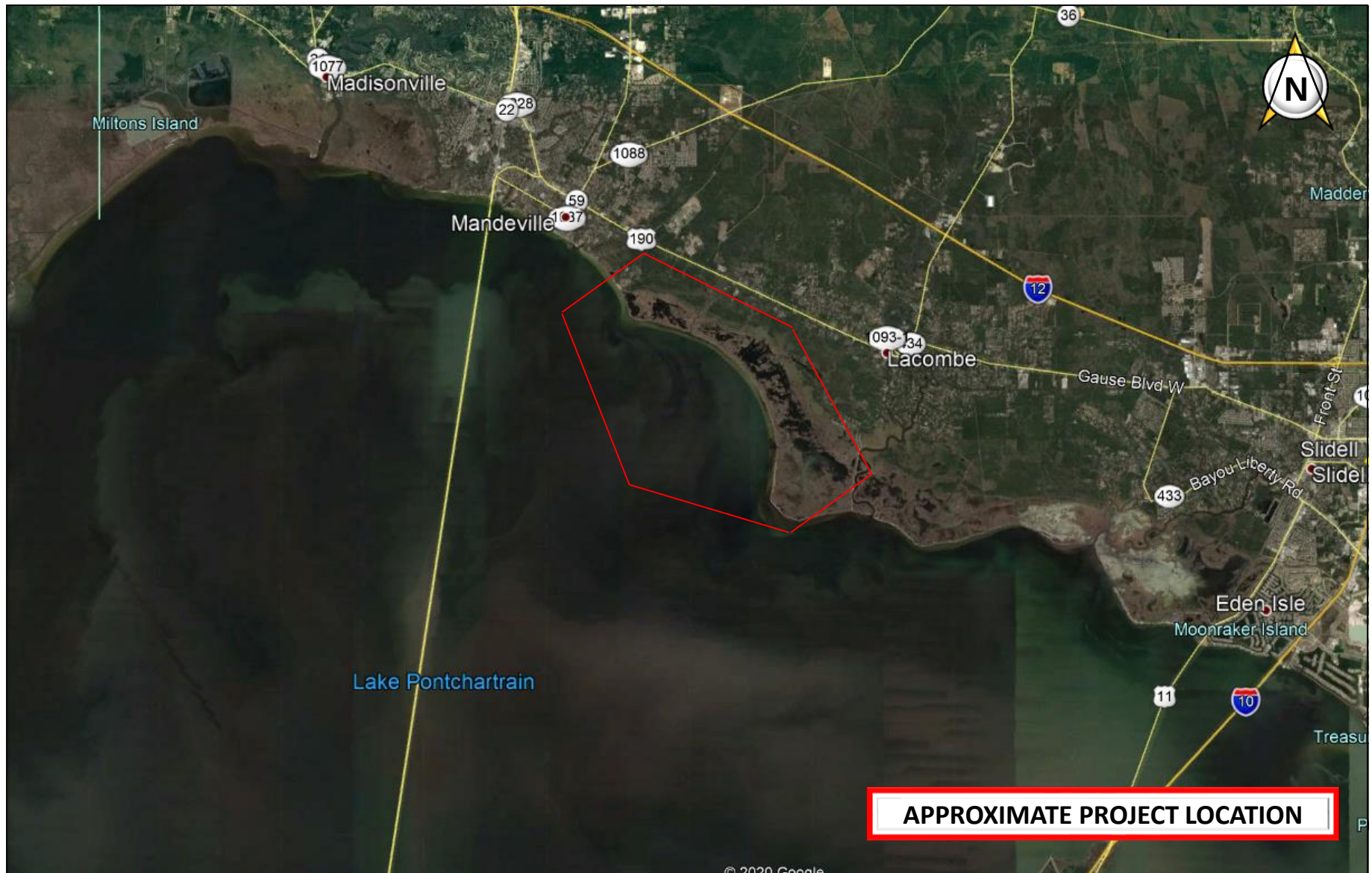
The above soil profile is of generalized nature to highlight the major subsurface stratification features and material characteristics of the marsh creation area. The boring logs and CPT sounding plots included in the **Appendix A** should be reviewed for specific information at individual locations. These records include soil descriptions, stratification, locations of the samples, and laboratory test data. The stratification shown on the boring logs represent the conditions only at the actual boring locations. Variations may occur and should be expected between and away from boring locations. The stratification represents the approximate boundary between subsurface materials and the actual transition may be more distinct or gradual.

3.3. FUTURE DESIGN REPORT

Based on the results from our field work and Laboratory test results it was concluded that the classification of the new borrow area samples are in agreement with the original borrow area exploration, and that the compressibility and sedimentation properties of these materials are also in agreement. Therefore, the original design properties utilized for the borrow area will be implemented in the final Geotechnical Engineering Report. Similarly, the data obtained within the marsh creation area was found to be in agreement with the soil data obtained from the previously performed soil borings and CPTs in the Marsh Creation Area-2 through Marsh Creation Area-6. Therefore, the original design properties utilized for these marsh areas will also be implemented in the final Geotechnical Engineering Report.

APPENDIX A

SITE VICINITY MAP SURVEY REPORT



APPROXIMATE PROJECT LOCATION

GEOTECHNICAL ENGINEERING SERVICES
Bayou Cane Marsh Creation
 St. Tammany Parish, Louisiana

SITE VICINITY MAP

PSI PROJECT NO.: 02541139 Addendum #1
 GOOGLE EARTH IMAGERY DATE: 11/2019
 Appendix Figure No. G.2-16



Survey Report

PO-181 Bayou Cane Boring Sites Survey St. Tammany Parish, Louisiana

Prepared for Intertek-PSI
724 Central Ave.
Jefferson, LA 70121



Chustz Surveying, LLC
211 Richy Street
New Roads, LA 70760
225-638-5949



June 2021

Section 1 General Project Description

In June of 2021, Chustz Surveying was tasked by Intertek-PSI to conduct location and magnetometer surveys at 17 boring sites in St. Tammany Parish. The surveys consisted of locating the sites utilizing RTK and multibeam hydrographic methods constrained to the provided project benchmarks (PO33 SM 01 and CRMSPO-SM-11) along with a magnetometer survey.

Vicinity Map



Section 2 Data Collection

On June 16, 2021, Chustz Surveying deployed two crews to the site to begin the survey. The first crew located a previously set 60D nail and the provided benchmark PO33 SM01. Next, utilizing two Trimble R10 GPS receivers, the crew set the base up on

the 60D nail and tied to PO33 SM01 to verify the control. Utilizing a Topcon ATG3 level, the other crew then ran a 3rd Order level loop from PO33 SM01 to the water surface and back to establish a temporary gauge for processing the hydrographic data. They then began the hydrographic and magnetometer data collection in the lake from aboard our 28 foot survey vessel equipped with an R2Sonic 2022 multibeam system and a Geometrics G882 cesium magnetometer. While the lake portion of the job was underway, the other crew began locating the boring sites in the marsh. They drove ¾" pvc pipes with pink flagging into the ground at each location in the marsh. In Hypack, they set up a grid around each location to cover a 25 ft. radius for the magnetometer survey and conducted that as well. Both crews completed the work that day and the data was transferred to the office for processing. No significant magnetometer anomalies were detected as part of this survey.

Section 3

Data Processing

The data was transferred to our office for initial processing once the surveys were complete. The RTK data files were loaded into Trimble Business Center where they were processed and checked for correct rod and instrument heights. All RTK ties were checked to ensure they were within the allowed accuracies. The hydrographic and magnetometer data was processed in Hypack utilizing the temporary gauge readings.

Section 6

Project Summary and Conclusion

The field effort was completed ahead schedule on Wednesday, June 16, 2021, and all data was processed and deliverables were completed for Tuesday, June 22, 2021. No safety incidents occurred as part of this work effort.





VICINITY MAP

1 inch = 3,000 feet

Reproduced from NAIP Louisiana 2013 1m Aerial Imagery

Station Name: "PO33 SM 01"

Location: From the intersection of I-12 and Hwy 434 go south 2.7 miles to Hwy 190. Turn right (west) for 0.3 miles to Hwy 434 (Lake road) for 3.4 miles to a bridge and a boat launch. The monument is 177 feet east of the centerline of Lake road, 59 feet N/E of the N/E corner of the concrete boat launch, and 13 feet westerly from the top bank of Bayou Lacome.

Monument Description: Monument is a 9/16 steel rod driven to ?? feet to refusal within a 6" PVC sleeve set in concrete with protective metal accesscover stamped 'PO SM 01'

Stamping: PO SM 01

Installation Date: 2005 **Date of Survey:** 13-May-14

Monument Established By: C&C Technologies

NAD83 (2011) Epoch 2010.00 Geodetic Position

Lat: 30°16'06.92777"N

Long: 89°57'20.77295"W

NAD83 (2011) Epoch 2010.00 Datum LSZ (1702) Ft

N= 645,784.85

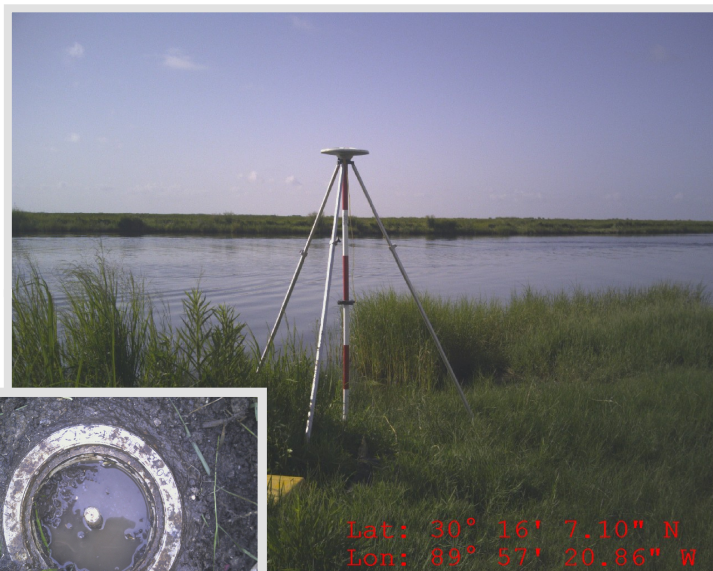
E= 3,715,691.24

Adjusted NAVD88 Height

Elevation = 1.19 feet (0.363 mtrs)

Ellipsoid Height (2011) = -26.241 mtrs.

Geoid12A Height = -26.604 mtrs.



FOR REFERENCE ONLY

LCZ Adjusted NAVD88 Height

Elevation (Geoid09)= N/A

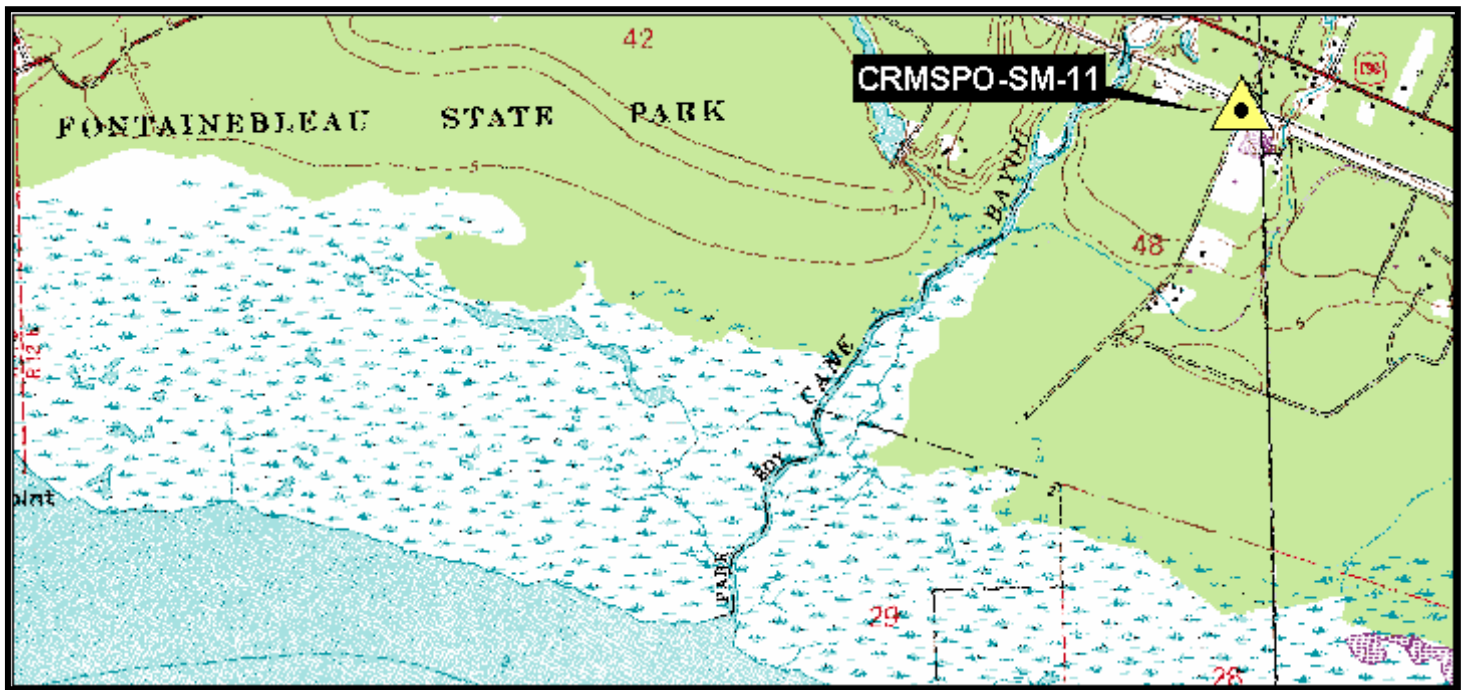
Ellipsoid Height = N/A

Elevation (Geoid03)= 1.76 feet (0.537 mtrs)

Ellipsoid Height = -26.253 mtrs.

Elevation (Geoid99)= N/A

Ellipsoid Height = N/A



VICINITY MAP

Scale: 1" = 2000'

Reproduced from USC&GS "MANDEVILLE & LACOMBE"

Quadrangle

Station Name: CRMSPO-SM-11

Monument Location: From the intersection of Hwy US Hwy 190 and the Lake Pontchartrain Causeway in Mandeville, go 6.8 miles west on Hwy 190 to Lemieux Blvd. Turn right on Lemieux Blvd. and go south 0.1 miles to a bike trail and the Monument. It is located 7' south of the centerline of a bicycle trail and 43' west of the centerline of Lemieux Blvd.

Monument Description: 9/16" steel rod driven to refusal within a 6" PVC sleeve set in concrete with protective metal access cover stamped **CRMSPO-SM-11**.

Date: July 2005

Monument Established By:

Chustz Surveying, Inc. for
Louisiana Department of
Natural Resources

Adjusted NAD 83 Geodetic Position*

Lat. 30°20'05.83357"N

Long. 90°00'02.48142"W

Adjusted NAD 1983 Datum*

LSZ (1702) Feet

N= 669750.54

E= 3701231.54

Adjusted NAVD88 (Feet) /Geoid99*

Elevation = 18.02



* As surveyed for LaDNR CRMS project, contract NO: 2503-05-56

PO-181 Bayou Cane Survey Locations (NAVD88-NAD83 US Survey Ft)

Site	Northing	Easting	Elevation	Water Surface	Depth
B-29	656677.18	3698381.97	-12.34	0.15	-12.19
B-31	658345.27	3699211.64	-11.61	0.15	-11.46
B-40	658408.23	3696787.19	-12.45	0.15	-12.30
B-50	659294.35	3694769.91	-13.09	0.15	-12.94
B-54	660589.97	3694205.99	-11.37	0.15	-11.22
B-56	658852.89	3693376.93	-13.37	0.15	-13.22
C-38	667840.42	3691807.23	-1.49	0.15	-1.34
C-39	667322.60	3692674.56	-1.53	0.15	-1.38
C-40	666694.31	3692750.53	-1.63	0.15	-1.48
C-41	662583.11	3703037.87	-2.91	0.15	-2.76
C-42	661126.44	3704753.21	-1.12	0.15	-0.97
C-43	655140.26	3707793.74	-1.43	0.15	-1.28
C-44	654804.73	3708765.89	-1.68	0.15	-1.53
C-46	649343.42	3711958.38	-2.13	0.15	-1.98
C-47	650534.68	3712057.87	0.74	0.15	0.89
C-48	651024.22	3710687.76	-1.13	0.15	-0.98
M-10	650022.94	3710882.72	-2.09	0.15	-1.94

ALL INFORMATION SHOWN HEREON IS BASED ON EVIDENCE FOUND DURING FIELD SURVEY AND PERMITTED INFORMATION; OTHER HAZARDS/PIPELINES MAY EXIST WITHIN THE SURVEYED AREA THAT WERE NOT DISCOVERED DURING THE COURSE OF THE SURVEY.

DATES OF SURVEY 06/16/2021

PREPARED FOR:

intertek

psi

BAYOU CANE MARSH CREATION (PO-181)

MAGNETOMETER SURVEY

Lake Pontchartrain Area

St. Tammy Parish, Louisiana

CSICHUSTZ

SURVEYING, LLC

211 Richy St. New

Rochelle, LA 70760

Office: (225) 638-5949

JOB NO. 200154

SIZE: 34x22

FILENAME: 200154-BASE-061721.dwg

CHK: BCH

DWG: JLR

DATE: 06/17/2021

SHEET: 1 OF 1

REV: 0

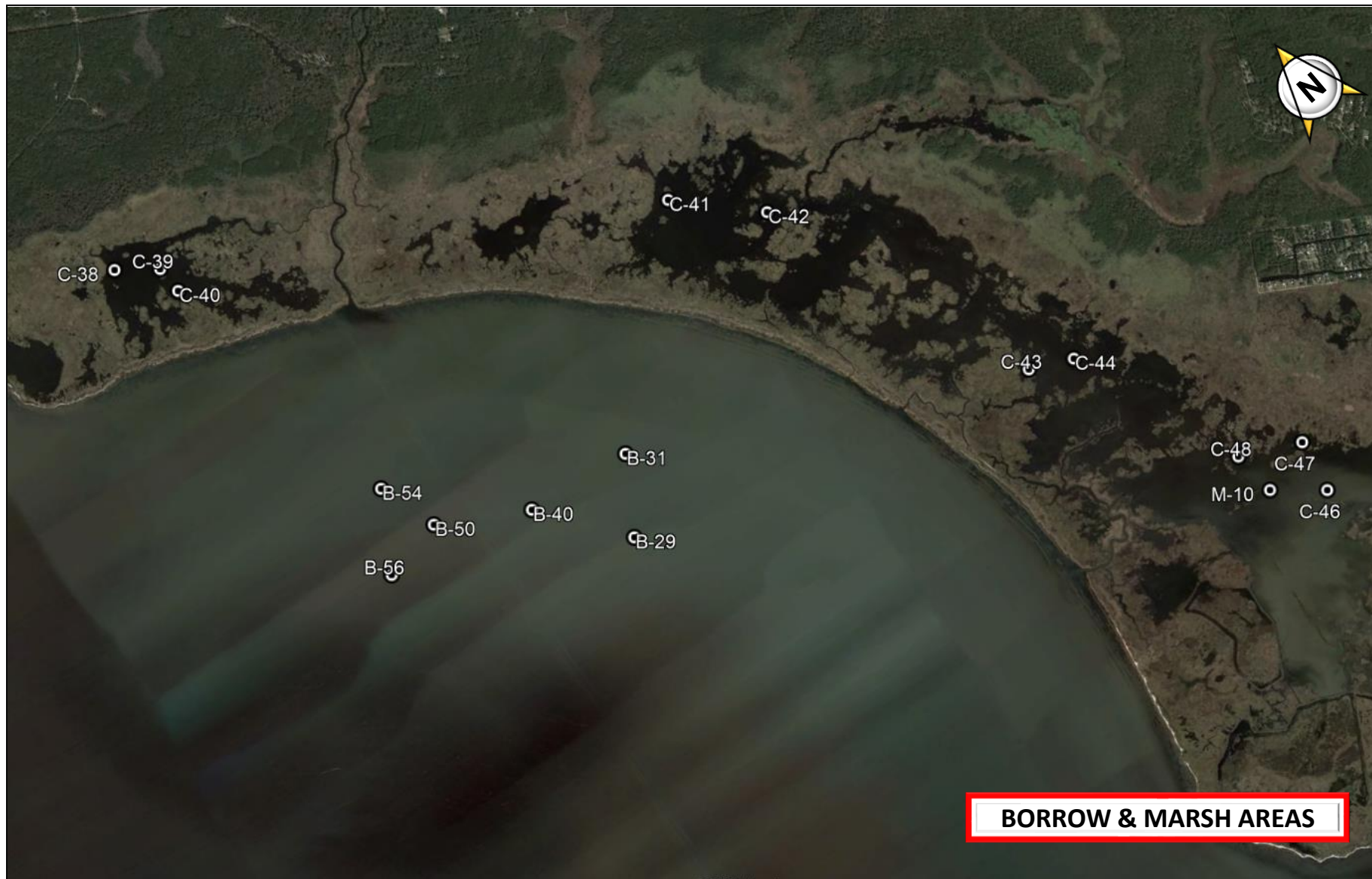
Mag Hit	Latitude	Longitude	X Coord.	Y Coord.	Peak Min	Peak Max	Peak Spread	Distance Over Ground	Time Elapsed
1	30 19 29.9534 N	090 01 42.6568 W	3,692,495	666,025	47,006.00	47,013.50	7.44	8.15	6.10
2	30 18 27.5485 N	089 59 43.9758 W	3,702,969	659,841	46,948.70	46,985.10	36.41	36.08	19.00
3	30 18 27.3648 N	089 59 44.4118 W	3,702,931	659,822	46,942.20	46,981.80	39.61	19.15	8.81
4	30 18 27.4583 N	089 59 43.8629 W	3,702,979	659,832	46,957.40	46,983.30	25.90	38.49	18.88
5	30 18 49.8561 N	089 59 49.2893 W	3,702,477	662,089	46,956.90	46,987.80	30.86	47.72	28.24
6	30 18 49.9790 N	089 59 48.7171 W	3,702,527	662,102	46,958.60	46,977.30	18.77	52.69	22.30

DATA BASED ON FIELD SURVEYS ON 07/15-16/2020
NO MAG CONTACTS DISCOVERED DURING 2021 SURVEY

- = Denotes Mag Hit
- = Denotes Bore Locations Surveyed during 2020 Survey
- = Denotes Bore Locations Surveyed during 2021 Survey

Appendix Figure No. G-2-23

APPENDIX B
BORING LOCATION PLAN
BORING LOGS AND CPT SOUNDING PLOTS
KEY TO TERMS AND SYMBOLS USED ON BORING LOGS



GEOTECHNICAL ENGINEERING SERVICES
Bayou Cane Marsh Creation
St. Tammany Parish, Louisiana

BORING LOCATION PLAN
PSI PROJECT NO.: 02541139 Addendum #1
GOOGLE EARTH IMAGERY DATE: 11/2019
Appendix Figure No. G.2-25

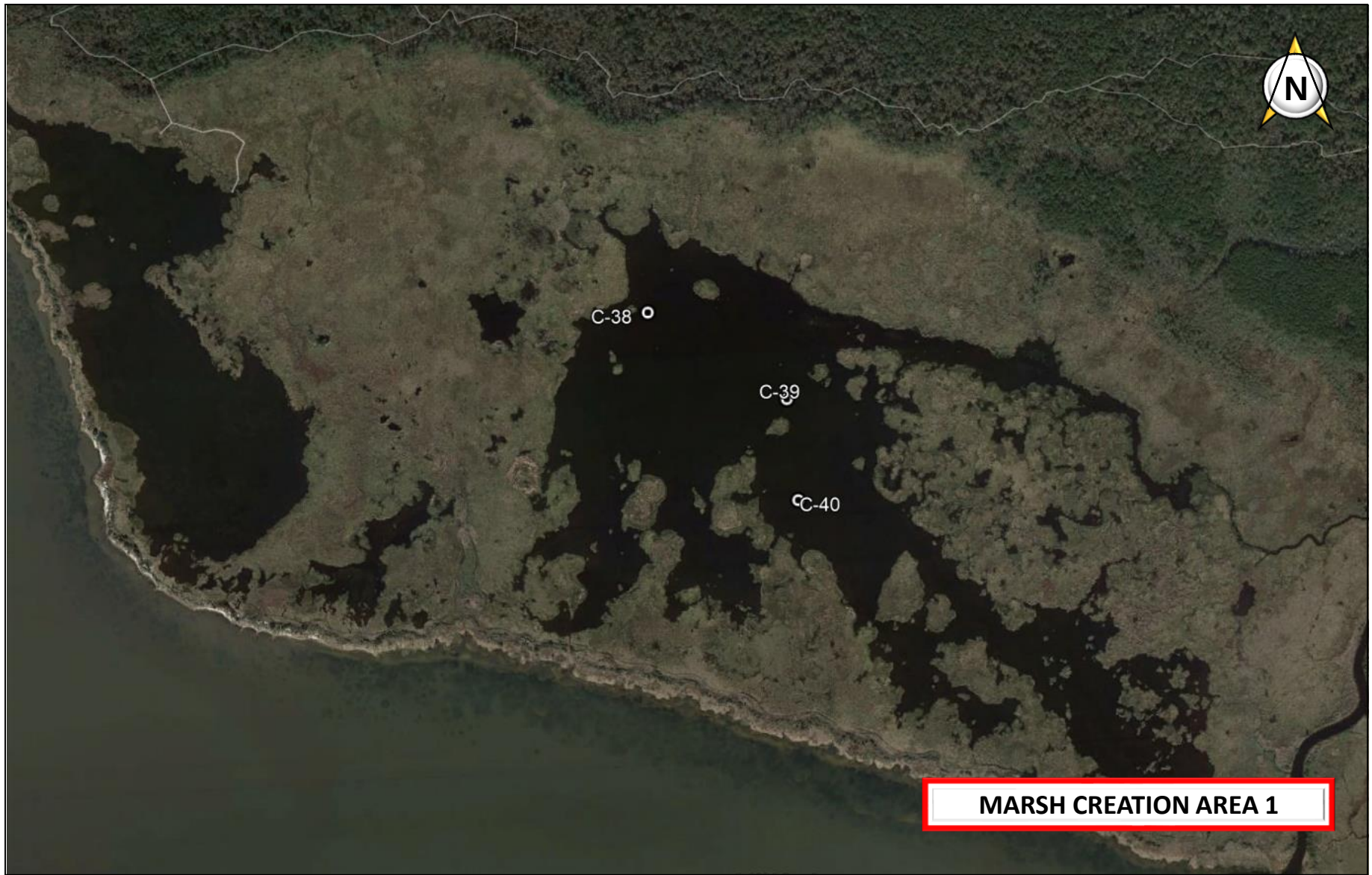




GEOTECHNICAL ENGINEERING SERVICES
Bayou Cane Marsh Creation
St. Tammany Parish, Louisiana

BORING LOCATION PLAN
PSI PROJECT NO.: 02541139 Addendum #1
GOOGLE EARTH IMAGERY DATE: 11/2019
Appendix Figure No. G.2-26





MARSH CREATION AREA 1

GEOTECHNICAL ENGINEERING SERVICES
Bayou Cane Marsh Creation
St. Tammany Parish, Louisiana

BORING LOCATION PLAN
PSI PROJECT NO.: 02541139 Addendum #1
GOOGLE EARTH IMAGERY DATE: 11/2019
Appendix Figure No. G.2-27

intertek
psi



GEOTECHNICAL ENGINEERING SERVICES
Bayou Cane Marsh Creation
St. Tammany Parish, Louisiana

BORING LOCATION PLAN
PSI PROJECT NO.: 02541139 Addendum #1
GOOGLE EARTH IMAGERY DATE: 11/2019
Appendix Figure No. G.2-28



Northing: 650022.94
 Easting: 3710882.72
 * Values are approximate

LOG OF BORING M-10 CPRA PO 181 ST. TAMMANY PARISH, LOUISIANA

LOCATION: BAYOU CANE

TYPE OF BORING: WET ROTARY

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)					DRY UNIT WEIGHT (pcf)		
											○ HP △ TV ▲ UU	● UC ◆ MV	0.0	0.5	1.0	1.5	HP	UC	TV		MV	UU
		PT		Very soft black/gray PEAT , sand layer																		
				- Organic Content: 22.3%, 0 to 2 feet		443	321	119	202			△							0.02			
		SM		Loose Gray/black SILTY SAND , peat layer		21				21		△						0.02				
		SM		Loose gray SILTY SAND , with clay pockets		19	NP	NP	NP	28		○					0.33					
5						17	NP	NP	NP	18		○					0.17					
						22	NP	NP	NP			○					0.33					
10				- with clay pockets		21				20												
		SP-SM		Medium dense gray POORLY GRADED SAND WITH SILT	3-5-6	24																
					11																	
					3-5-5	22																
15					10																	
					6-7-8	22				7												
					15																	
					6-9-9	20																
20					18																	

DEPTH OF BORING: 40 FEET

DATE DRILLED: 7/30/21

NOTE: ELEVATION OF BORING: -2.09 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▽ DELAYED GROUNDWATER: N / A

JEFFERSON - FULL SPT AND MV V/2 (N/E) - 11/2/21 09:16 - 0254



Professional Service Industries, Inc.
 724 Central Avenue
 Jefferson Louisiana 70121
 (504) 733-9411

LOG OF BORING M-10
CPRA PO 181
ST. TAMMANY PARISH, LOUISIANA

PSI Project No.: 02541139

JEFFERSON - FULL SPT AND MV V2 (N/E) - - 11/2/21 09:16 - 0254

intertek
psi

Page 2 of 2

Northing: 656677.18
 Easting: 3698381.97
 * Values are approximate

LOG OF BORING B-29 CPRA PO 181 ST. TAMMANY PARISH, LOUISIANA

LOCATION: BAYOU CANE

TYPE OF BORING: WET ROTARY

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)					DRY UNIT WEIGHT (pcf)						
							LL	PL	PI		○ HP △ TV ▲ UU	● UC ◆ MV	0.0	0.5	1.0	1.5	HP	UC	TV		MV	UU				
		CH		Very soft dark gray FAT CLAY , with silt seams and shell fragments		66																				
						45	58	16	42														67			
		SM		Dark gray SILTY SAND , with shell fragments of 2 to 3 inch long - Hydrometer Results (47.91% fines): 11.42% clay & 36.49% silt, 4 to 6 feet		30																				
5																										
		CH		Very soft gray/tan FAT CLAY , with shell fragments		37																				
						39																				
10		CH		Soft to firm light gray FAT CLAY		45	72	21	51															78		
										44																
15										45																
										50																
						39																				
20				BORING TERMINATED AT 20 FEET																						

DEPTH OF BORING: 20 FEET

DATE DRILLED: 7/8/21

NOTE: ELEVATION OF BORING: -12.34 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▽ DELAYED GROUNDWATER: N / A

JEFFERSON - FULL SPT AND MV V2 (NIE) - 11/2/21 09:20 - 0254

Northing: 658345.27
 Easting: 3699211.64
 * Values are approximate

LOG OF BORING B-31 CPRA PO 181 ST. TAMMANY PARISH, LOUISIANA

LOCATION: BAYOU CANE

TYPE OF BORING: WET ROTARY

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf) ○ HP ● UC △ TV ◆ MV ▲ UU	SHEAR STRENGTH (tsf)					DRY UNIT WEIGHT (pcf)
												HP	UC	TV	MV	UU	
							0.0	0.5	1.0			1.5					
		CH		Very soft gray SANDY FAT CLAY , with shell fragments		53					△			0.01			
		CH		Firm to very stiff gray/brown FAT CLAY		42						○	1.08				
5				- Hydrometer Results (98.57% fines): 43.86% clay & 54.71% silt, 4 to 6 feet		38						○	0.83				
				- with silt partings, 4 to 10 feet		39	81	26	55			○	0.58			84	
						37						⊕	0.50				
10						46						⊕	0.50				
						45						○	0.42				
15				- with silt partings, 14 to 20 feet		45	90	29	61			○	0.58			76	
						28						○	0.58				
						31						○	0.33				
20				BORING TERMINATED AT 20 FEET													

DEPTH OF BORING: 20 FEET

DATE DRILLED: 7/8/21

NOTE: ELEVATION OF BORING: -11.61 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▮ DELAYED GROUNDWATER: N / A

JEFFERSON - FULL SPT AND MV V2 (NIE) - 11/2/21 09:20 - 0254

Northing: 658408.23
 Easting: 3696787.19
 * Values are approximate

LOG OF BORING B-40 CPRA PO 181 ST. TAMMANY PARISH, LOUISIANA

LOCATION: BAYOU CANE

TYPE OF BORING: WET ROTARY

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	MOISTURE CONTENT (%)	SHEAR STRENGTH (tsf)			% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)					DRY UNIT WEIGHT (pcf)							
							LL	PL	PI		HP	UC	TV	MV	UU								
																	0.0	0.5	1.0	1.5			
		CH	Soft gray FAT CLAY , with silt pockets and shell fragments			50																	
		CH		Firm to stiff brown/gray FAT CLAY , with silt partings - Hydrometer Results (99.92% fines): 74.29% clay & 25.63% silt, 2 to 4 feet			43																
5						46	80	25	55														76
						48																	
						35																	
10				35																			
			CH	Soft gray FAT CLAY , with silt partings			49	74	21	53												75	
15							36																
							40																
			CH	Stiff gray FAT CLAY , with silt partings			31																
20			BORING TERMINATED AT 20 FEET																				

DEPTH OF BORING: 20 FEET

DATE DRILLED: 7/8/21

NOTE: ELEVATION OF BORING: -12.45 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▽ DELAYED GROUNDWATER: N / A

JEFFERSON - FULL SPT AND MV V/2 (N/E) - 11/2/21 09:20 - 0254

Northing: 659294.35
 Easting: 3694769.91
 * Values are approximate

LOG OF BORING B-50 CPRA PO 181 ST. TAMMANY PARISH, LOUISIANA

LOCATION: BAYOU CANE

TYPE OF BORING: WET ROTARY

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)				SHEAR STRENGTH (tsf)					DRY UNIT WEIGHT (pcf)	
											○ HP ● UC △ TV ◆ MV ▲ UU				HP	UC	TV	MV	UU		
											0.0	0.5	1.0	1.5							
		CL		Stiff gray LEAN CLAY		17	36	11	25							0.67					114
		CH		Firm brown/gray FAT CLAY WITH SAND		26										0.50					
5				- Hydrometer Results (75.99% fines): 29.52% clay & 46.47% silt, 4 to 6 feet		43										0.50					
						44										0.50					
		CH		Soft brown/gray FAT CLAY		35	65	19	46							0.17					86
10		CH		Firm to stiff brown/gray FAT CLAY		48										0.50					
						31										0.25					
15						55										0.25					
				- with silt partings and shell fragments, 16 to 20 feet		53										0.42					
						53										0.25					
20				BORING TERMINATED AT 20 FEET																	

DEPTH OF BORING: 20 FEET

DATE DRILLED: 7/10/21

NOTE: ELEVATION OF BORING: -13.09 feet

▽ GROUNDWATER DURING DRILLING: N / A

▼ GROUNDWATER UPON COMPLETION: N / A

▽ DELAYED GROUNDWATER: N / A

JEFFERSON - FULL SPT AND MV V2 (NIE) - 11/2/21 09:20 - 0254



Professional Service Industries, Inc.
 724 Central Avenue
 Jefferson Louisiana 70121
 (504) 733-9411

Northing: 660589.97
 Easting: 3694205.99
 * Values are approximate

LOG OF BORING B-54 CPRA PO 181 ST. TAMMANY PARISH, LOUISIANA

LOCATION: BAYOU CANE

TYPE OF BORING: WET ROTARY

PSI Project No.: 02541139

DEPTH, FT.	SOIL TYPE	USCS SYMBOL	SAMPLES	SOIL DESCRIPTION	N-BLOWS/FT.	MOISTURE CONTENT (%)	SHEAR STRENGTH (tsf)			% PASSING No. 200 SIEVE	SHEAR STRENGTH (tsf)					DRY UNIT WEIGHT (pcf)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																														
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		CH		Firm to stiff brown/gray FAT CLAY , with silt partings - Hydrometer Results (90.61% fines): 28.47% clay & 62.14% silt, 0 to 2 feet		27																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																								

DEPTH OF BORING: 20 FEET

DATE DRILLED: 7/10/21

NOTE: ELEVATION OF BORING: -11.37 feet

▽ GROUNDWATER DURING DRILLING: N / A

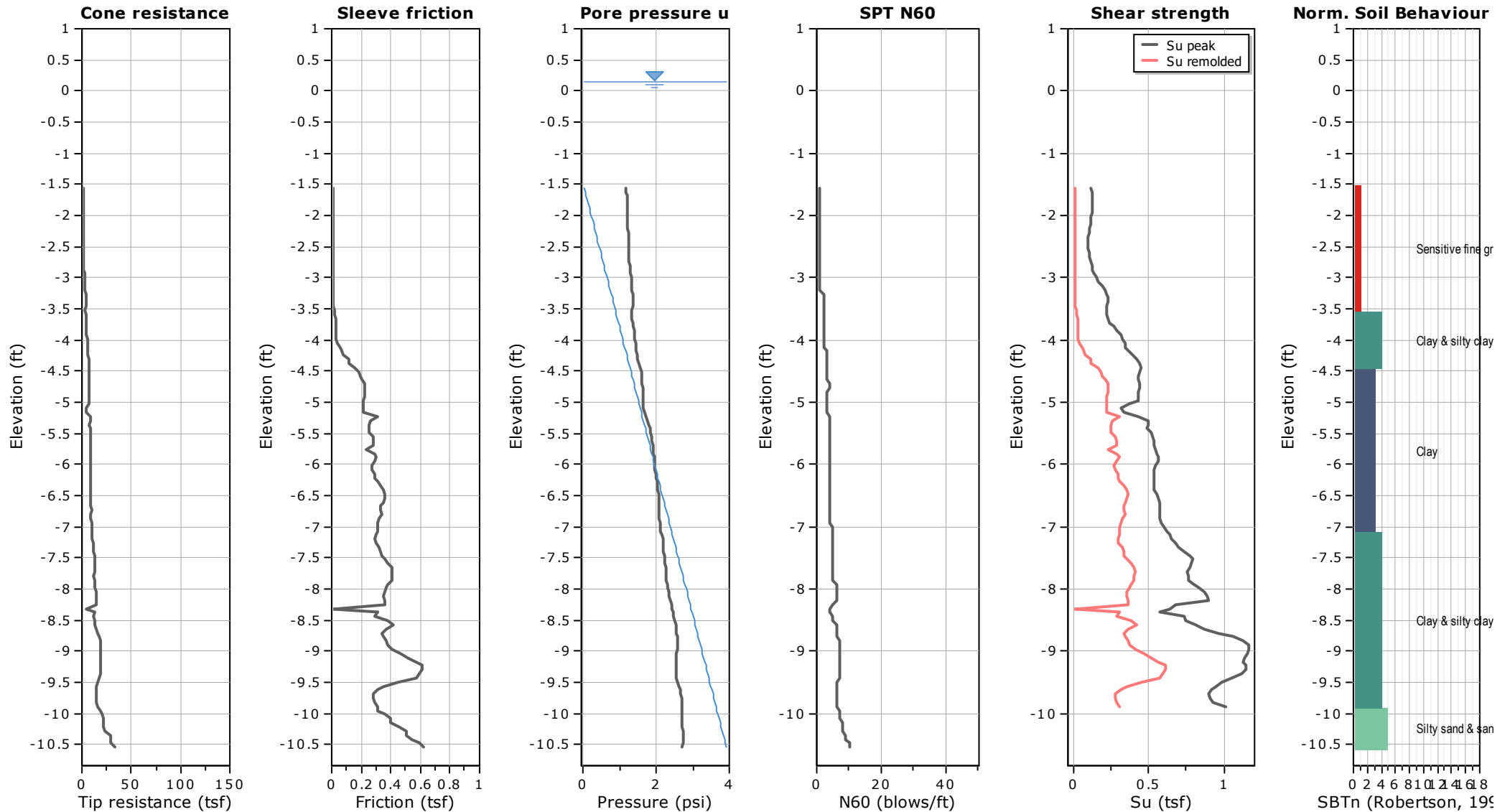
▼ GROUNDWATER UPON COMPLETION: N / A

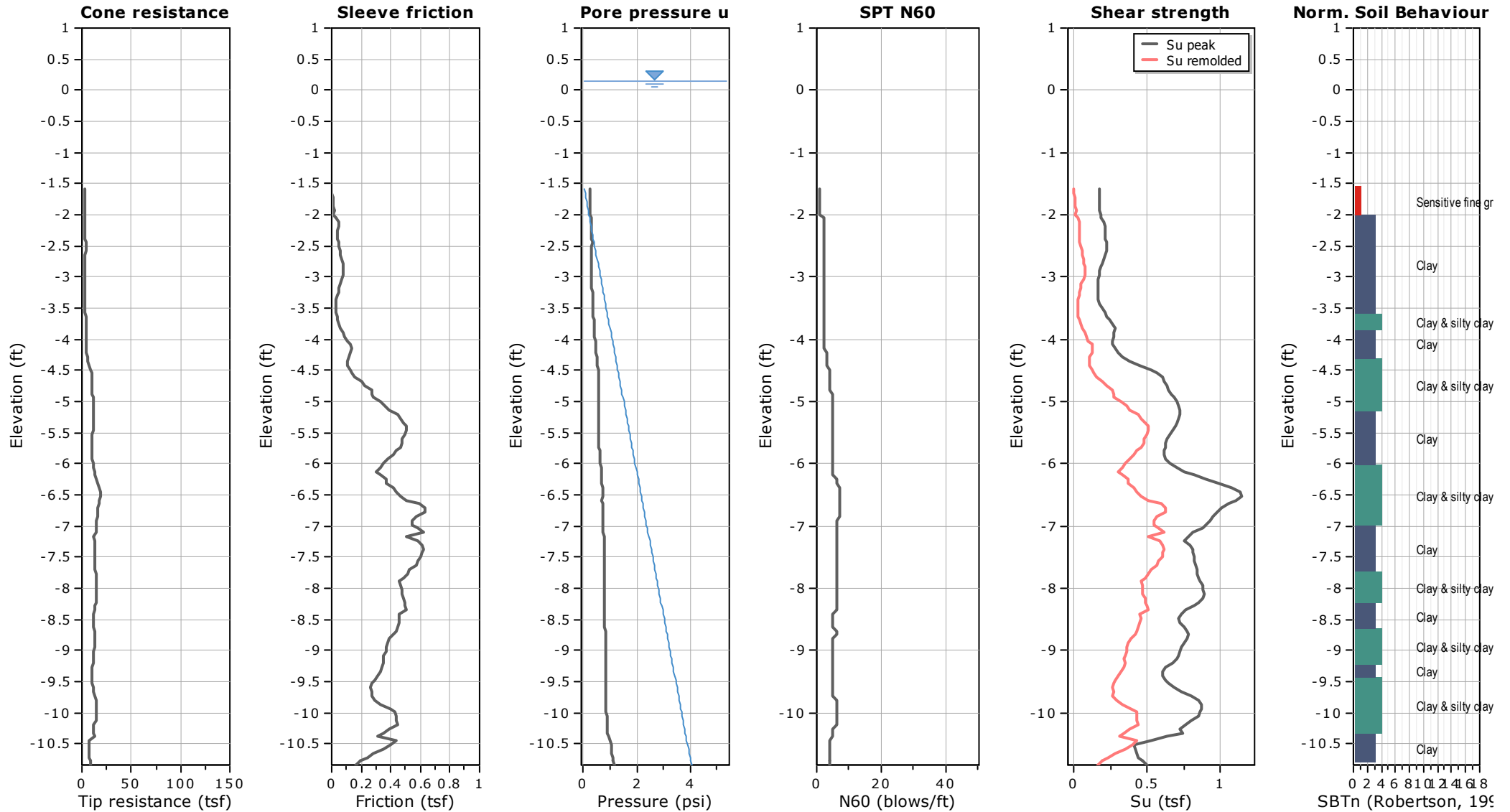
▮ DELAYED GROUNDWATER: N / A

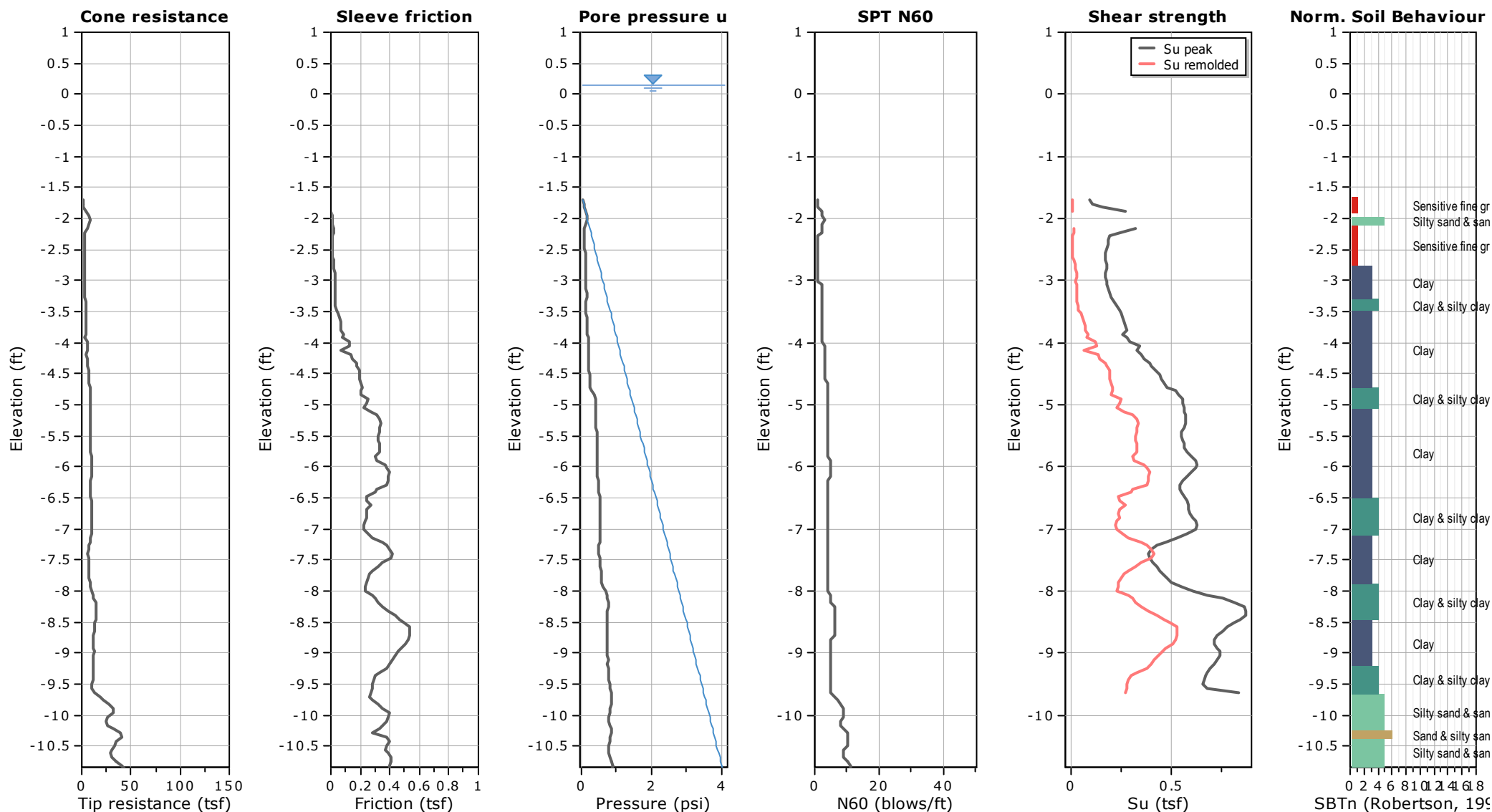
JEFFERSON - FULL SPT AND MV V2 (NIE) - 11/2/21 09:20 - 0254

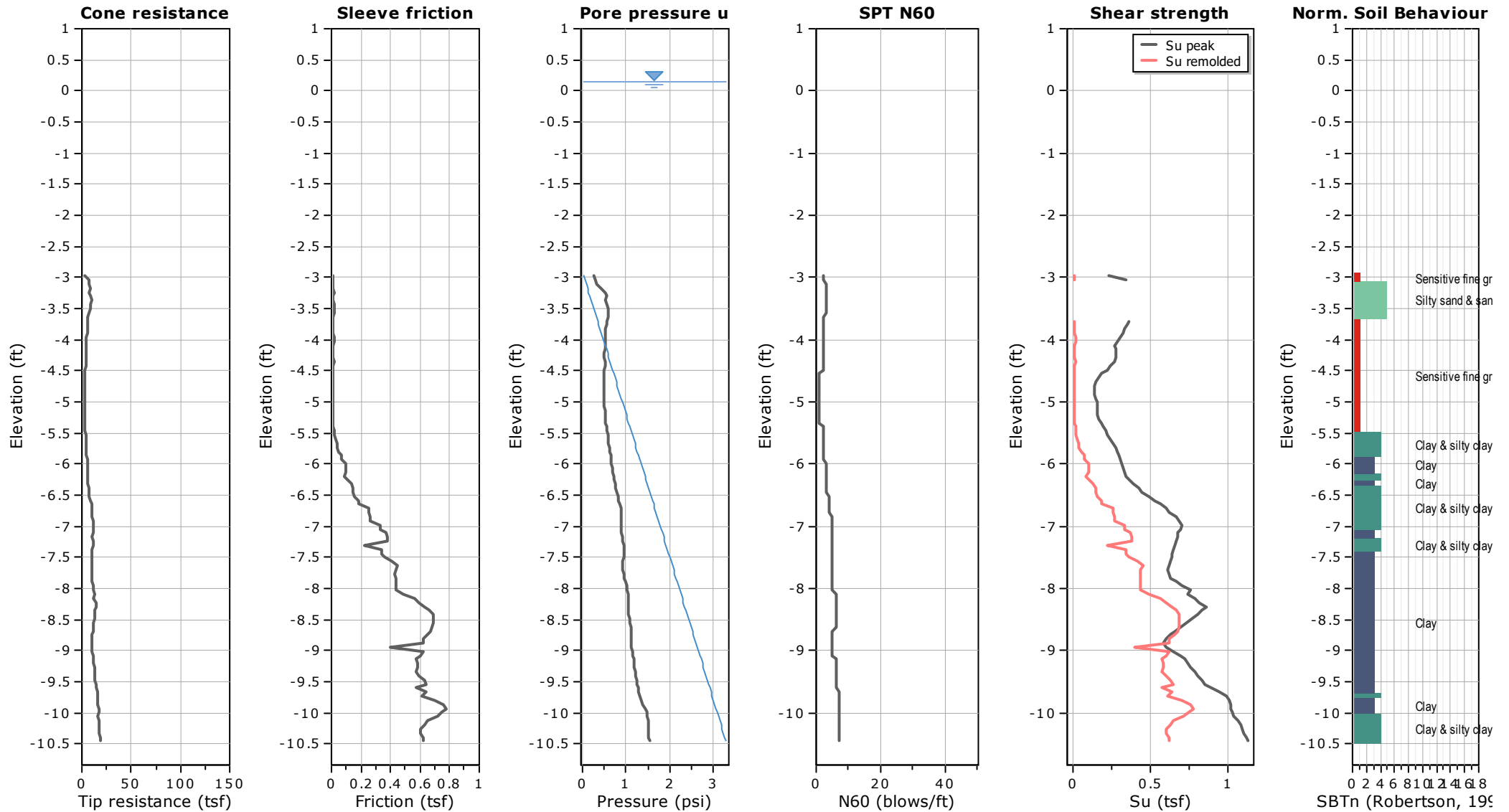


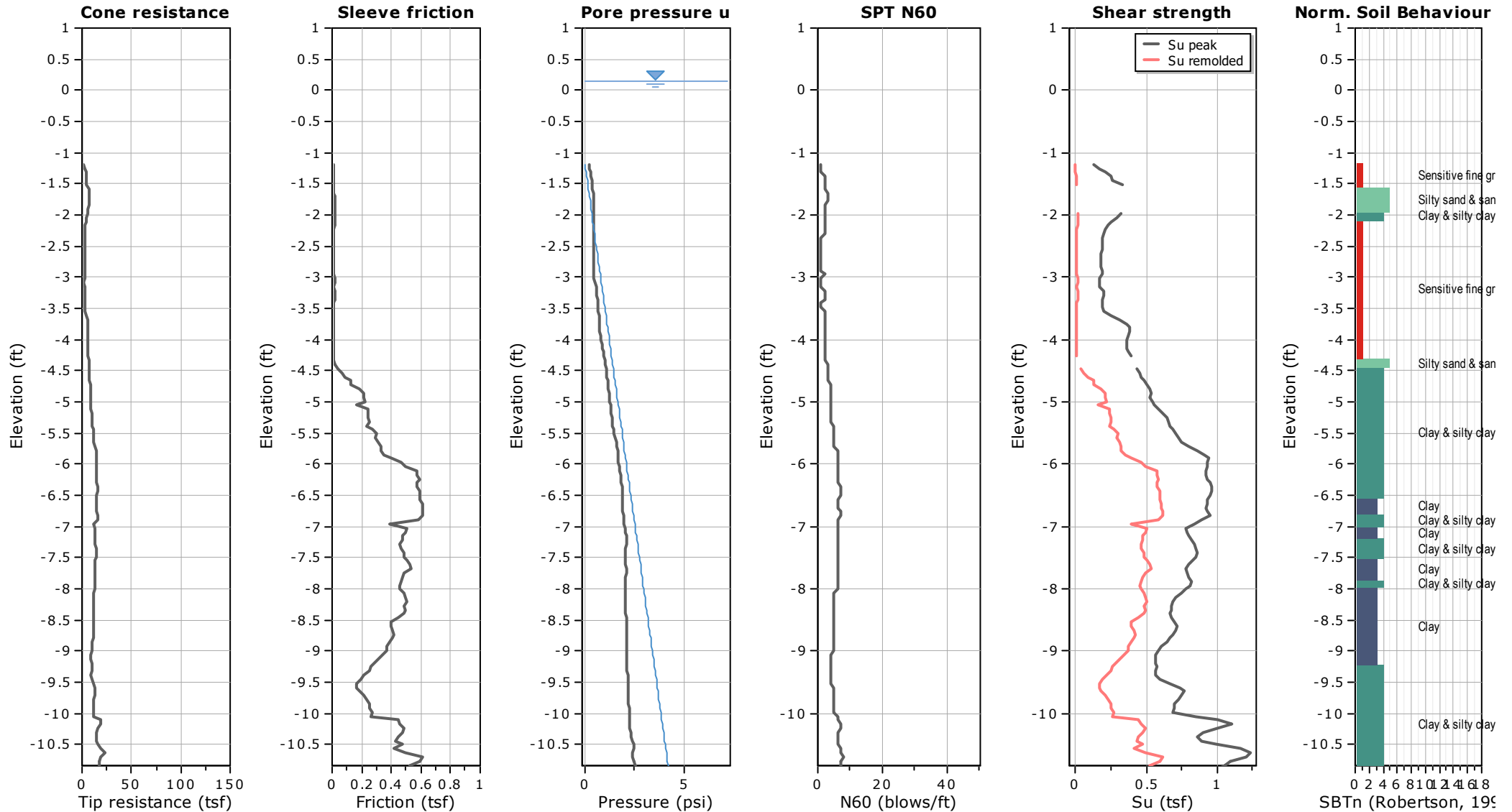
Professional Service Industries, Inc.
 724 Central Avenue
 Jefferson Louisiana 70121
 (504) 733-9411

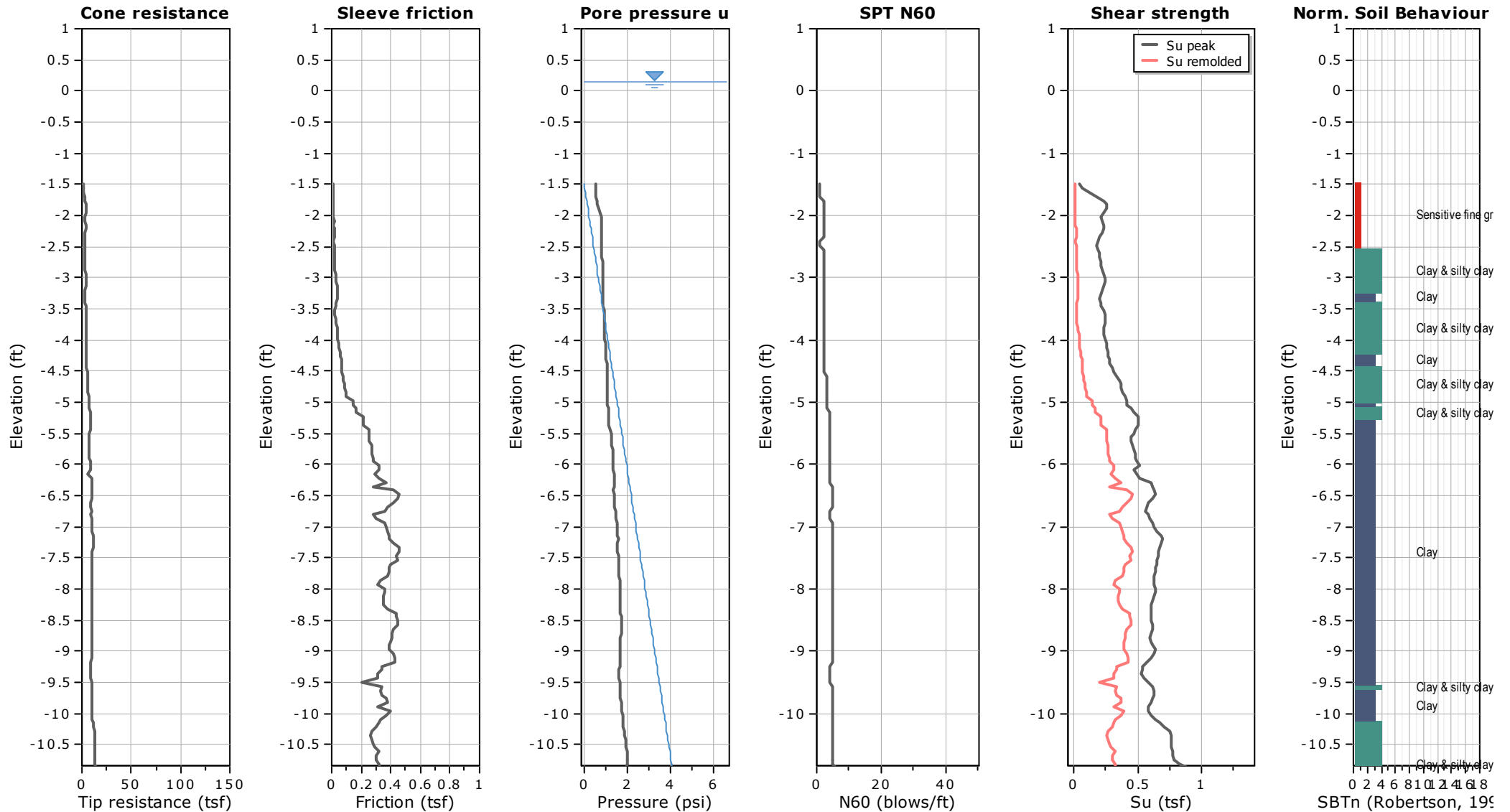


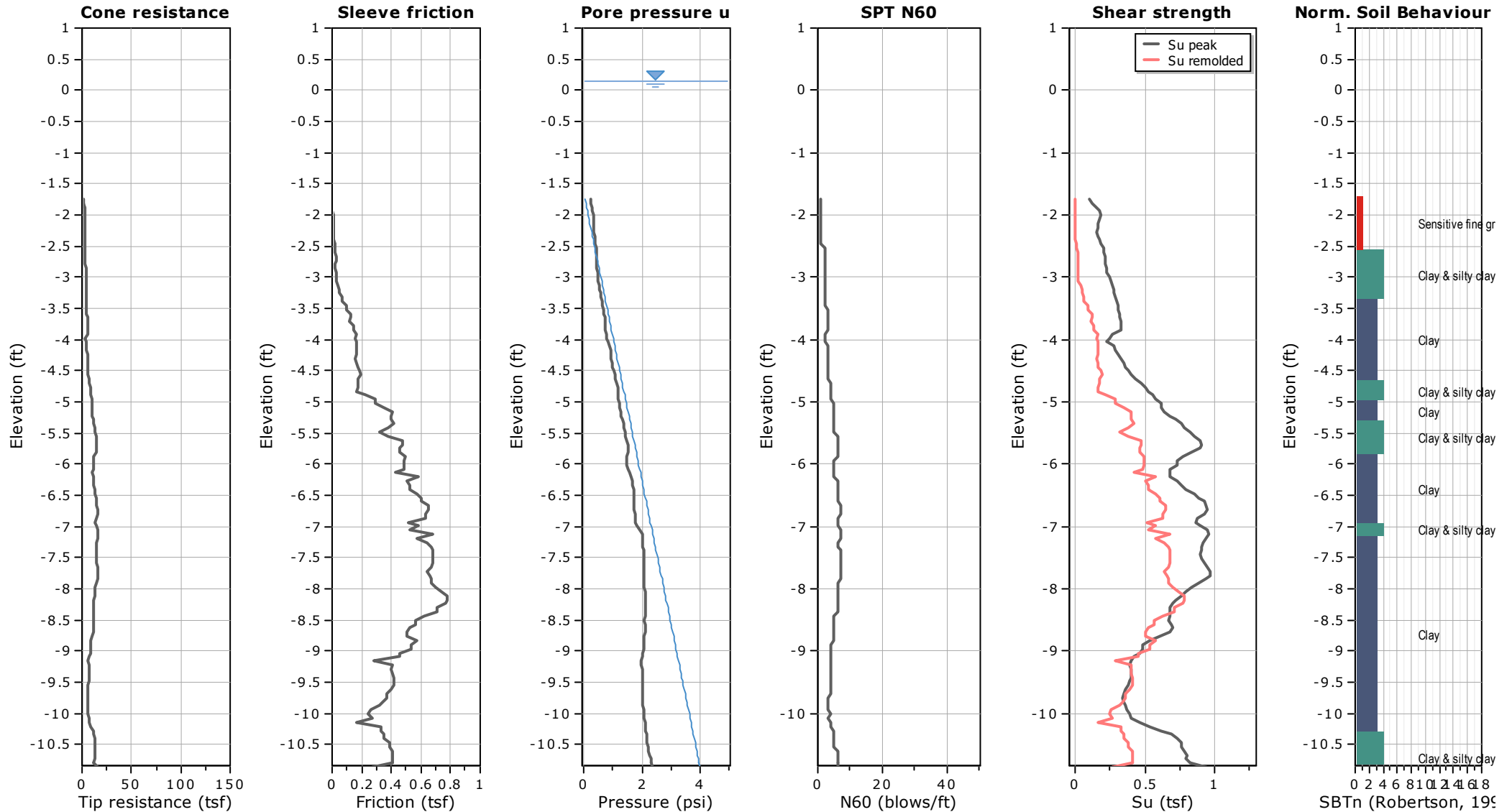


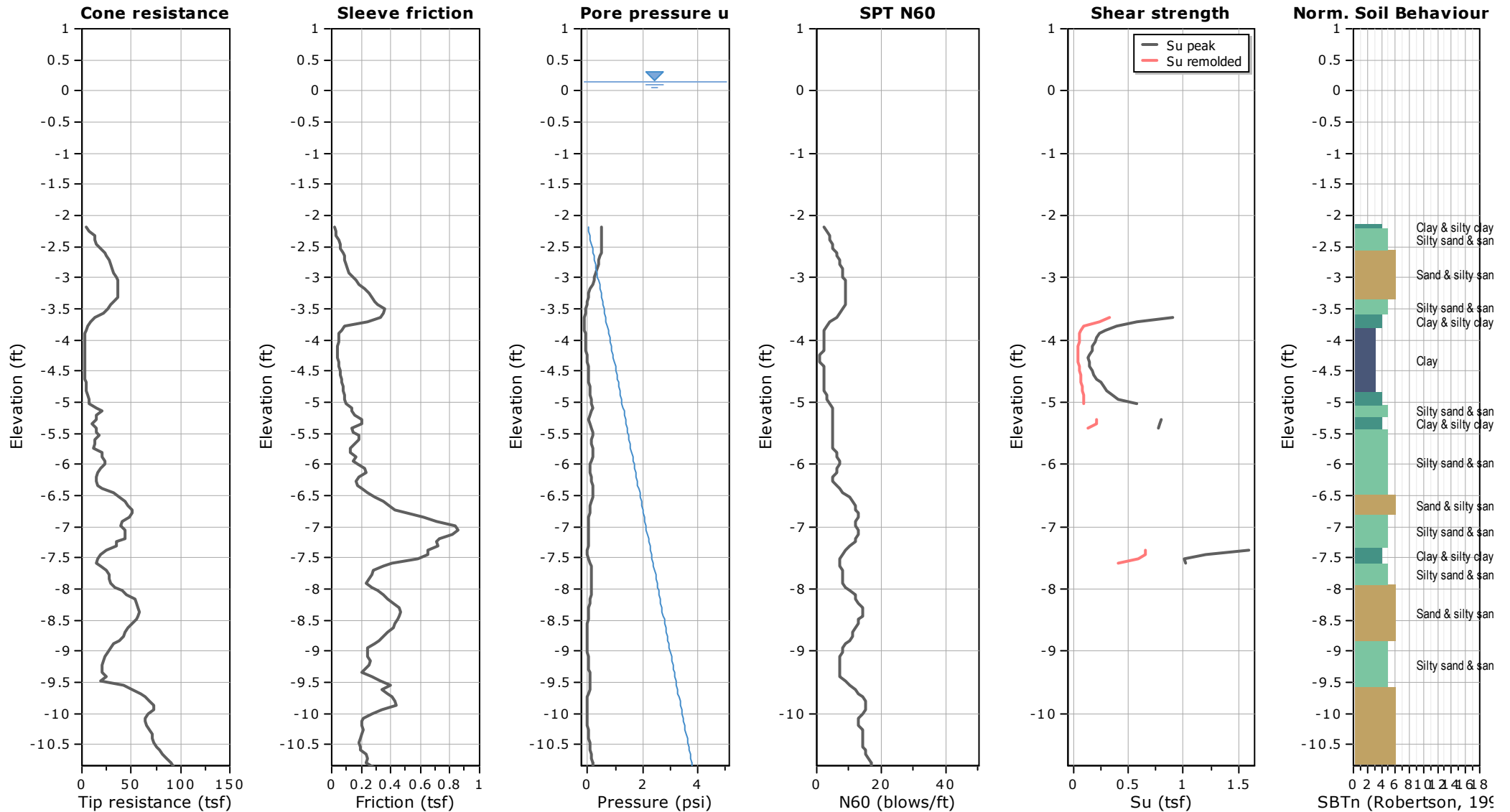


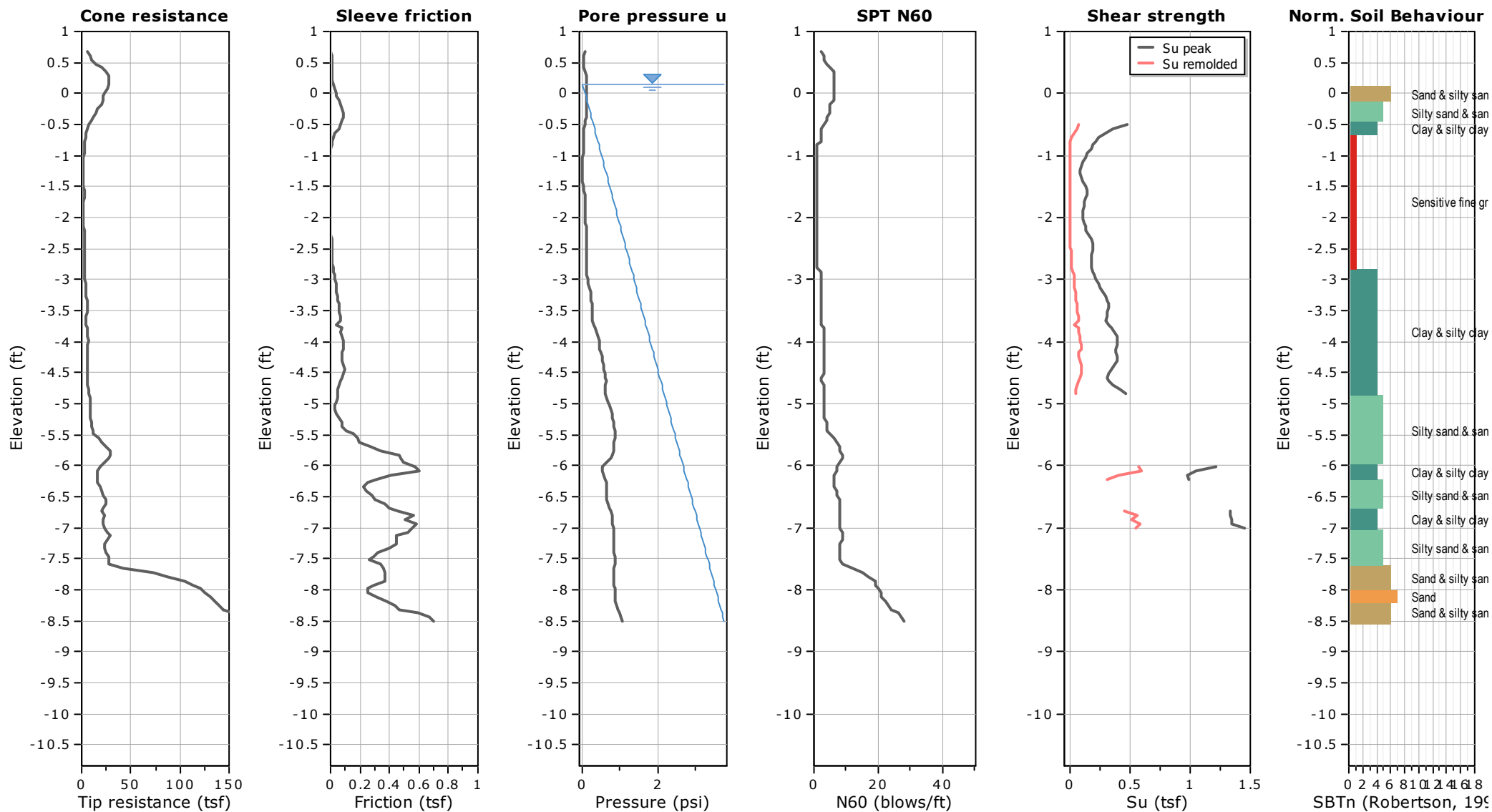


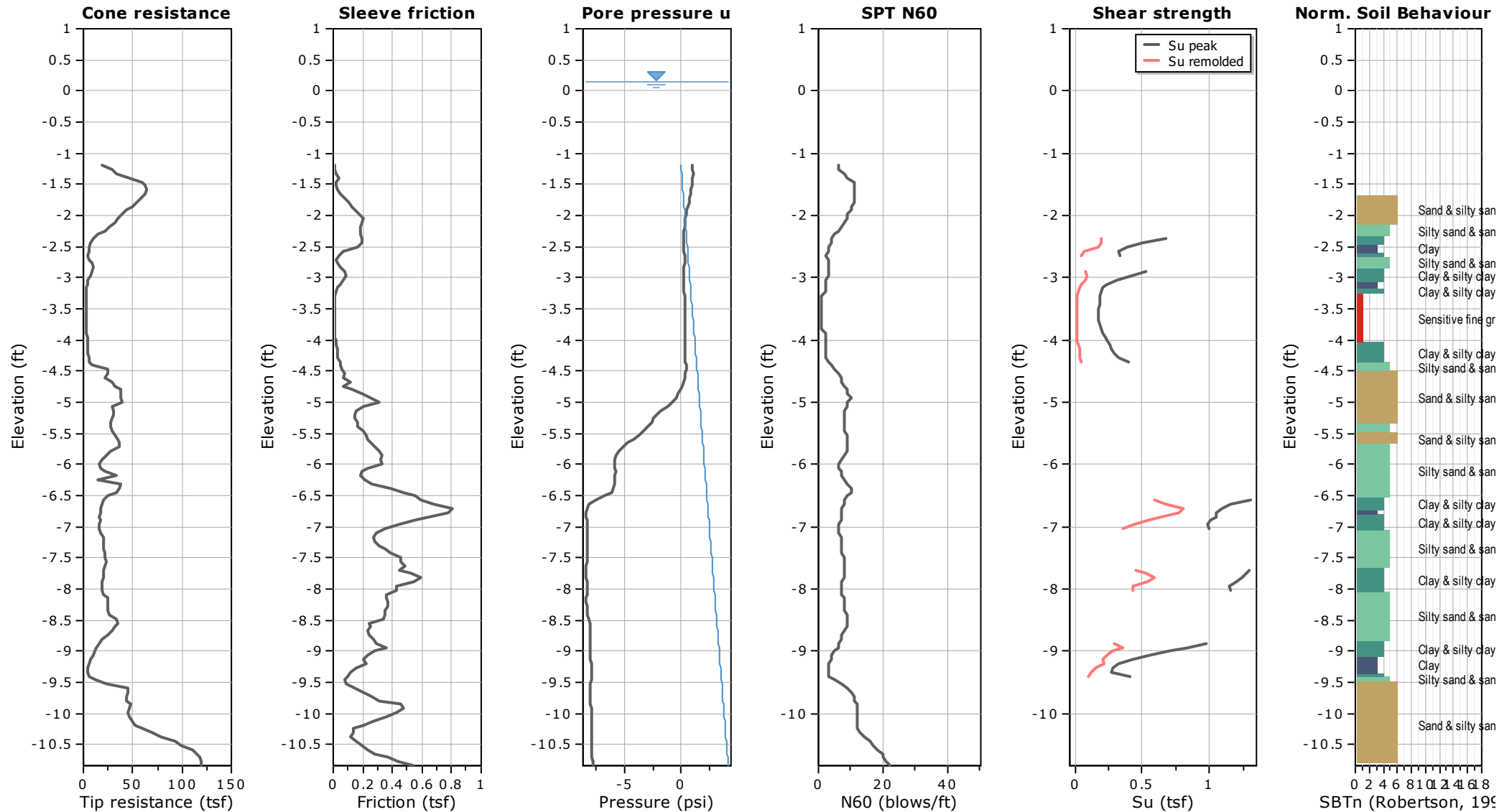












KEY TO TERMS AND SYMBOLS USED ON LOGS

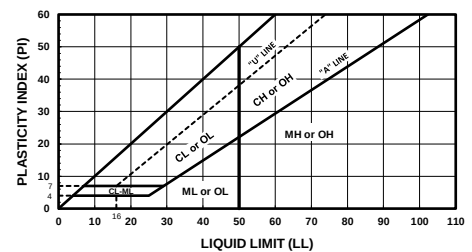
SOIL TYPE					
FAT CLAY	LEAN CLAY	ORGANIC CLAY	SAND	SILT	GRAVEL
SOIL TYPE		MODIFIERS			
TOPSOIL	FILL	CLAYEY	SANDY	SILTY	GRAVELLY

SAMPLER TYPE			
NO RECOVERY	AUGER SAMPLE	SHELBY TUBE	SPLIT SPOON
GROUNDWATER DURING DRILLING GROUNDWATER UPON COMPLETION			

UNIFIED SOIL CLASSIFICATION SYSTEM - ASTM D 2487 (1980)					
MAJOR DIVISIONS			LETTER SYMBOL	TYPICAL DESCRIPTIONS	
COARSE-GRAINED SOILS LESS THAN 50% PASSING NO. 200 SIEVE	GRAVEL & GRAVELLY SOILS LESS THAN 50% PASSING NO. 4 SIEVE	CLEAN GRAVEL (LITTLE OR NO FINES)	GW	WELL-GRADED GRAVEL, GRAVEL-SAND MIXTURES WITH LITTLE OR NO FINES	
			GP	POORLY GRADED GRAVEL, GRAVEL-SAND MIXTURES WITH LITTLE OR NO FINES	
		WITH APPRECIABLE FINES	GM	SILTY GRAVEL, GRAVEL-SAND-SILT MIXTURES	
			GC	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES	
	SANDS MORE THAN 50% PASSING NO. 4 SIEVE	CLEAN SANDS (LITTLE OR NO FINES)	SW	WELL-GRADED SAND	
			SP	POORLY-GRADED SANDS	
		WITH APPRECIABLE FINES	SM	SILTY SANDS	
			SC	CLAYEY SANDS	
FINE-GRAINED SOILS MORE THAN 50% PASSING NO. 200 SIEVE	SILTS AND CLAYS LIQUID LIMIT < 50		ML	INORGANIC SILTS & VERY FINE SANDS, CLAYEY SILT W/ LOW PLASTICITY INDEX	
			CL	INORGANIC LEAN CLAYS GRAVELLY, SANDY, OR SILTY LEAN CLAYS	
			OL	ORGANIC SILTS & ORGANIC SILTY CLAYS W/LOW PLASTICITY INDEX	
	SILTS AND CLAYS LIQUID LIMIT ≥ 50		MH	INORGANIC SILTS W/ HIGH PLASTICITY INDEX, ELASTIC SILTS	
			CH	INORGANIC FAT CLAYS GRAVELLY, SANDY, OR SILTY FAT CLAYS	
			OH	ORGANIC CLAYS OF MED TO HIGH PLASTICITY, ORGANIC SILTS	
HIGHLY ORGANIC SOIL			PT	PEAT AND OTHER HIGHLY ORGANIC SOILS	
UNCLASSIFIED FILL MATERIALS			ARTIFICIALLY DEPOSITED AND OTHER UNCLASSIFIED SOILS AND MAN-MADE SOIL MIXTURES		

CONSISTENCY - COHESIVE SOILS	
CONSISTENCY	SHEAR STRENGTH IN TONS/FT ²
VERY SOFT	0 TO 0.125
SOFT	0.125 TO 0.25
FIRM	0.25 TO .50
STIFF	0.50 TO 1.00
VERY STIFF	1.00 TO 2.00
HARD	> 2.00 OR 2.00+

RELATIVE DENSITY - GRANULAR SOILS	
DENSITY	N-VALUE (BLOWS/FT)
VERY LOOSE	0-4
LOOSE	4-9
MEDIUM DENSE	10-29
DENSE	30-49
VERY DENSE	> 50 OR 50+



ABBREVIATIONS

HP - HAND PENETROMETER

UC - UNCONFINED COMPRESSION TEST

TV - MINIATURE TORVANE

UU - UNCONSOLIDATED UNDRAINED TRIAXIAL

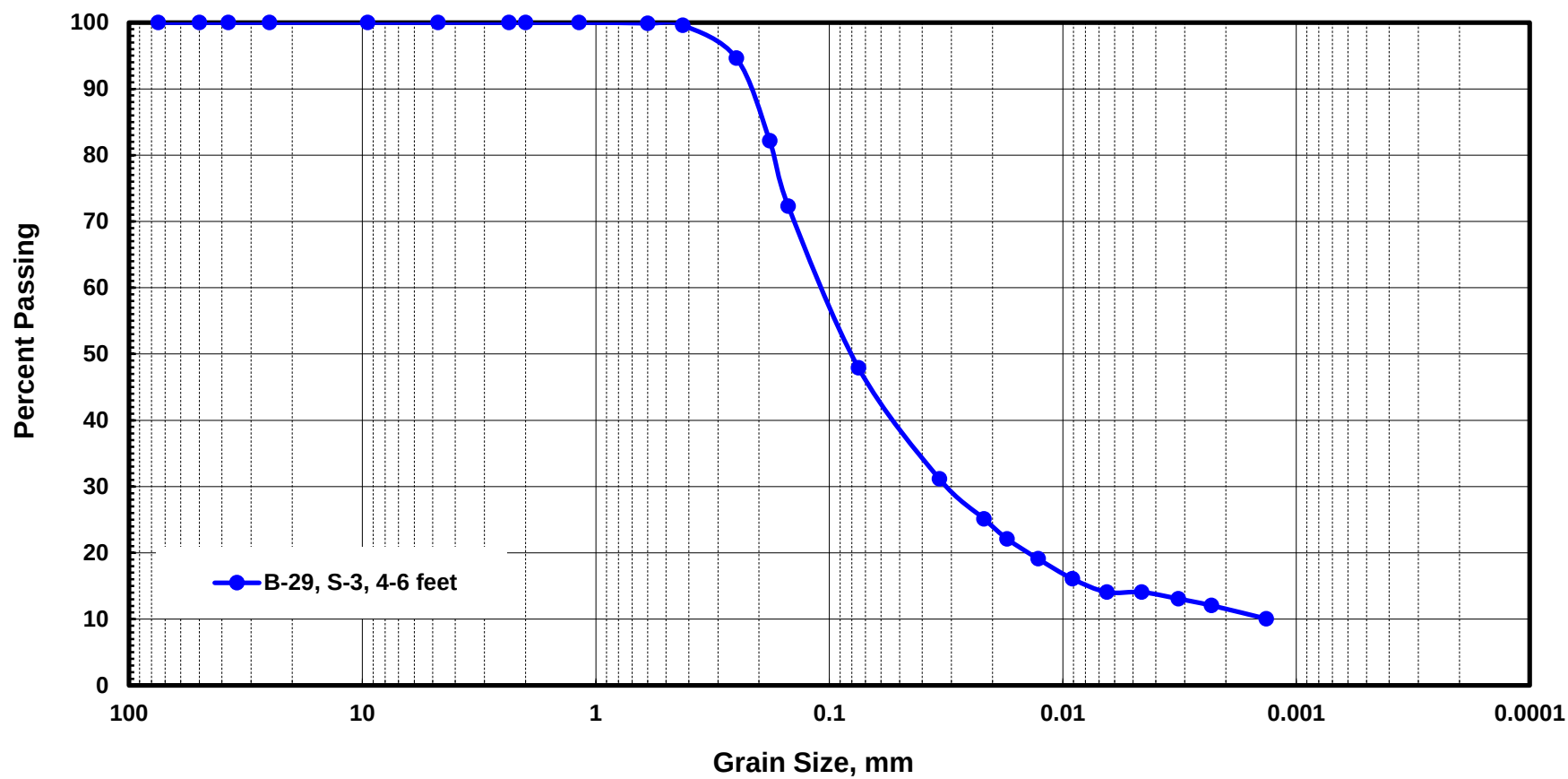
NOTE: BORING LOGS INDICATE SHEAR STRENGTH AS OBTAINED BY ABOVE TESTS

CLASSIFICATION OF GRANULAR SOILS

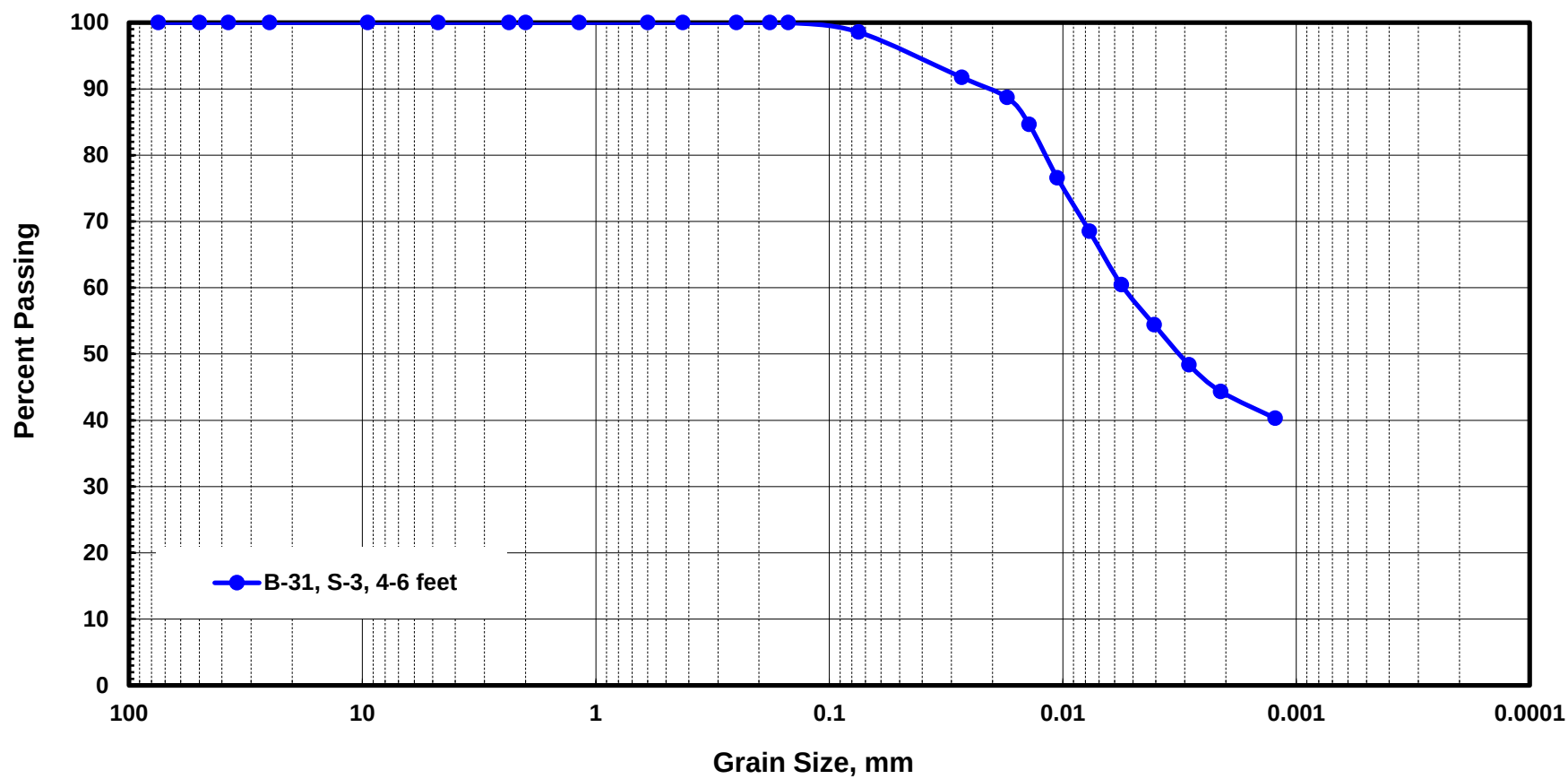
U.S. STANDARD SIEVE SIZE(S)									
12"		3"	3/4"	4	10	40	200		
BOULDERS	COBBLES	GRAVEL		SAND			SILT OR CLAY	CLAY	
		COARSE	FINE	COARSE	MEDIUM	FINE			
300	75	19	4.75	2.0	0.42	0.075	0.005		
GRAIN SIZE IN MM									

APPENDIX C

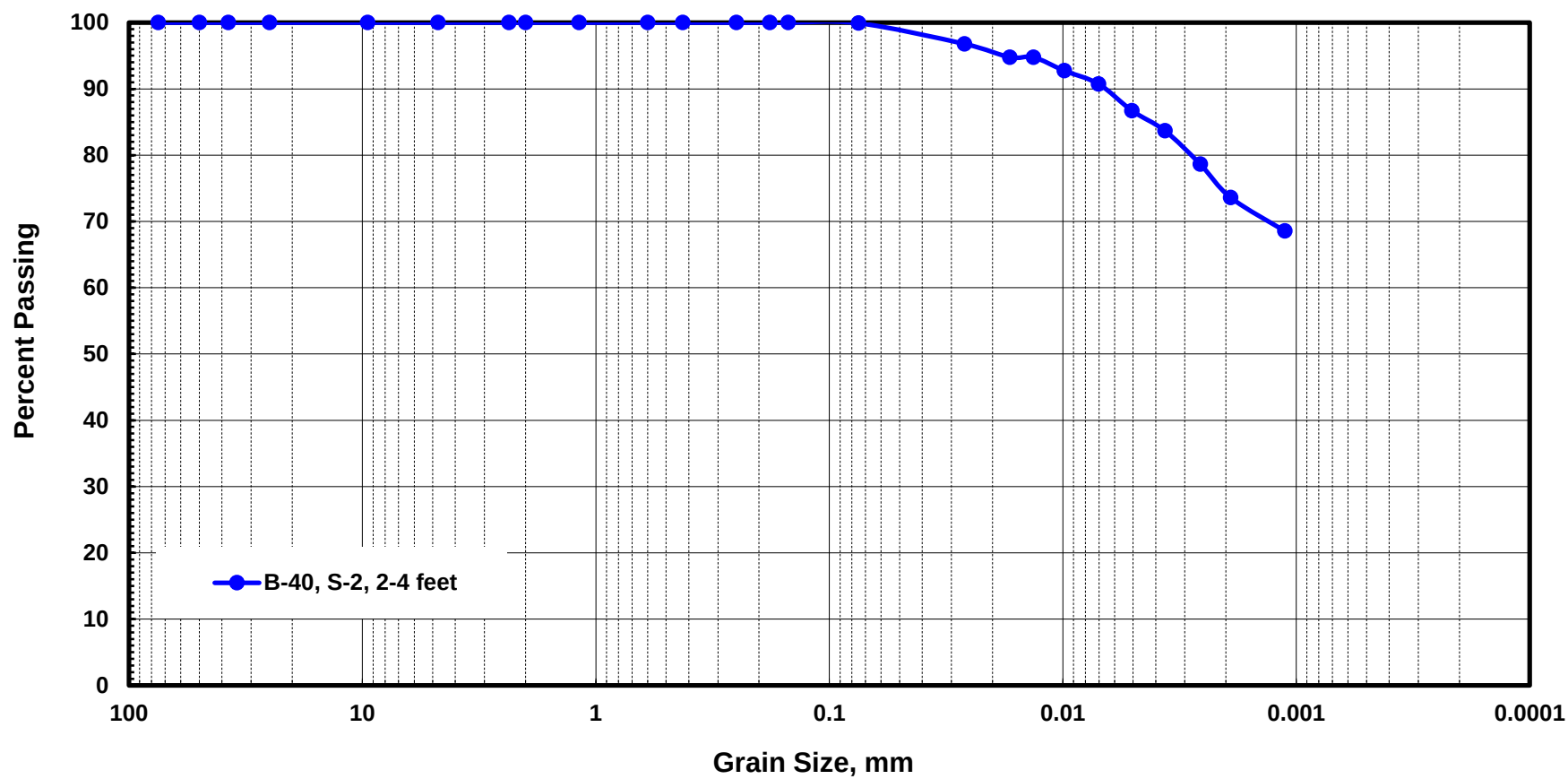
HYDROMETER TEST RESULTS
SPECIFIC GRAVITY TEST RESULTS
ORGANIC CONTENT RESULTS



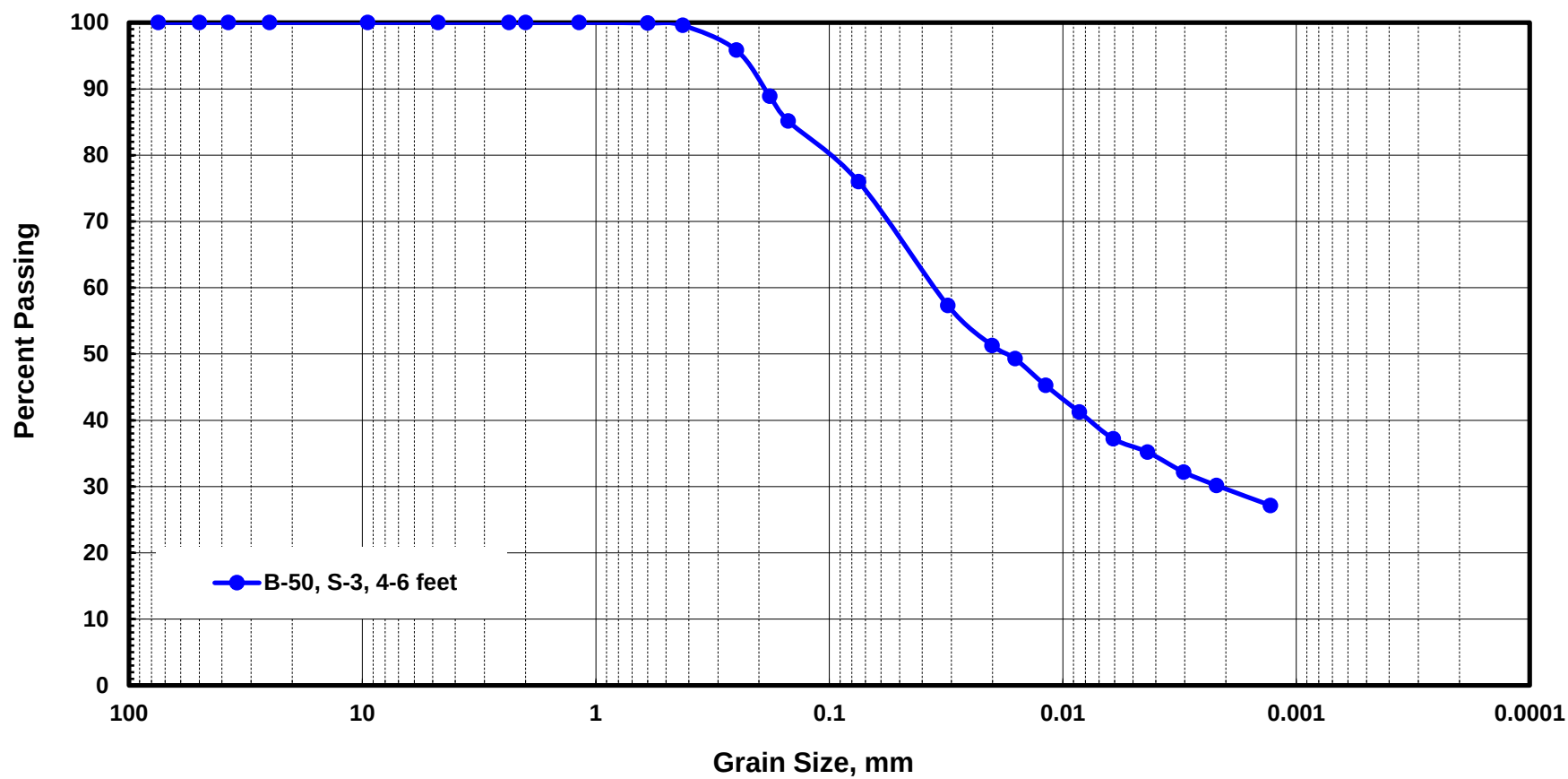
GRADATION INFORMATION	D ₁₀	D ₃₀	D ₅₀	D ₆₀	C _u	C _c	%Gravel	%Sand	%Fines	%Silt	%Clay
	mm	mm	mm	mm	(D ₆₀ /D ₁₀)	D ₃₀ ² /(D ₆₀ *D ₁₀)	Ret. #4	<#4 & >#200	Pass #200	0.075 mm < S > 0.002 mm	< 0.002 mm
B-29, S-3, 4-6 feet	NA	0.0310	0.0796	0.1057	NA	NA	0.0%	52.09	47.91	36.49	11.42
CPRA PO 181											
Silty Sand (SM), Dark Gray											



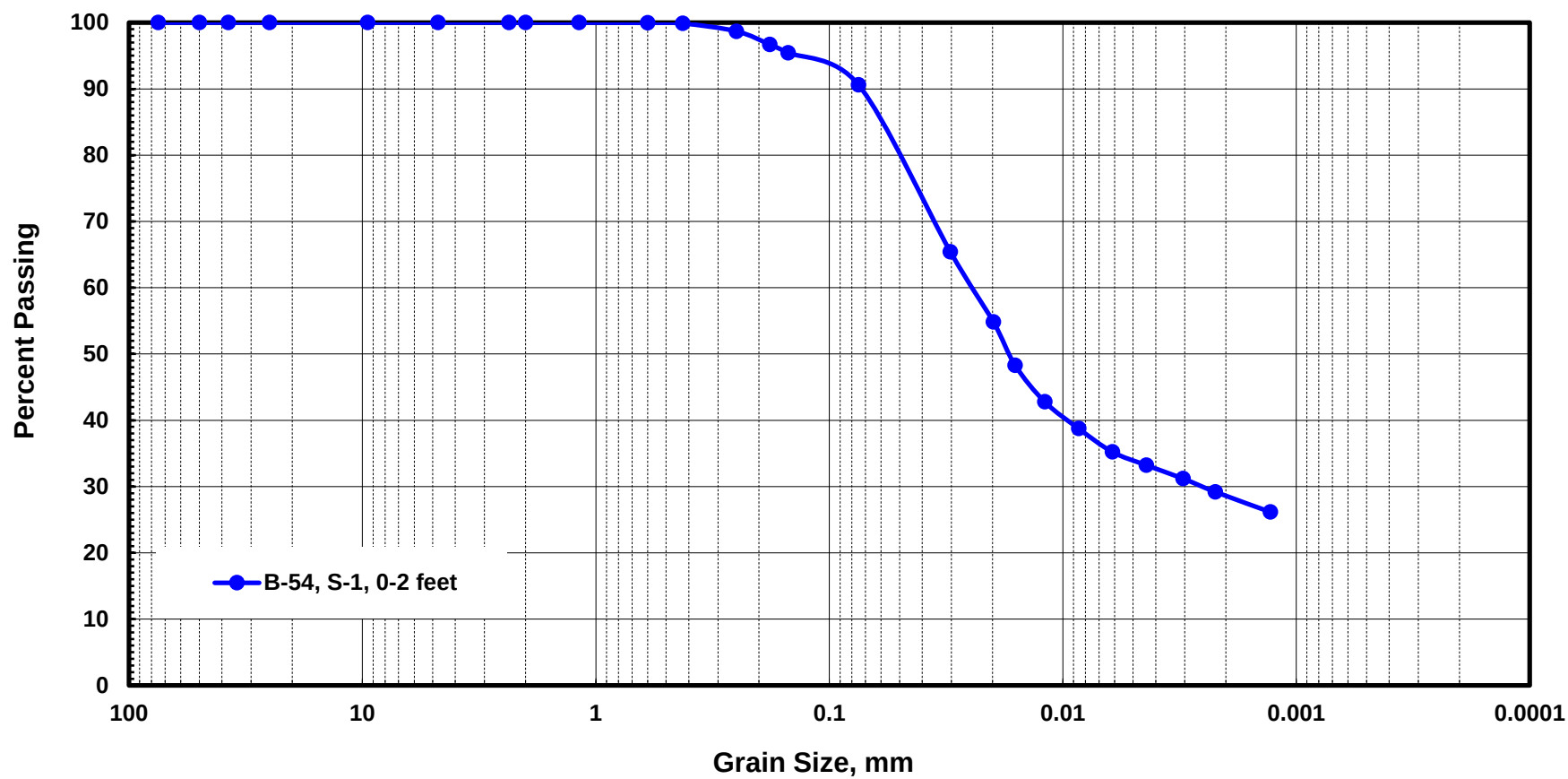
GRADATION INFORMATION	D ₁₀ mm	D ₃₀ mm	D ₅₀ mm	D ₆₀ mm	C _u (D ₆₀ /D ₁₀)	C _c D ₃₀ ² /(D ₆₀ *D ₁₀)	%Gravel Ret. #4	%Sand <#4 & >#200	%Fines Pass #200	%Silt 0.075 mm < S < 0.002 mm	%Clay < 0.002 mm
B-31, S-3, 4-6 feet	NA	NA	0.0032	0.0055	NA	NA	0.0%	1.43	98.57	54.71	43.86
CPRA PO 181											
Fat Clay (CH), Gray/Brown											



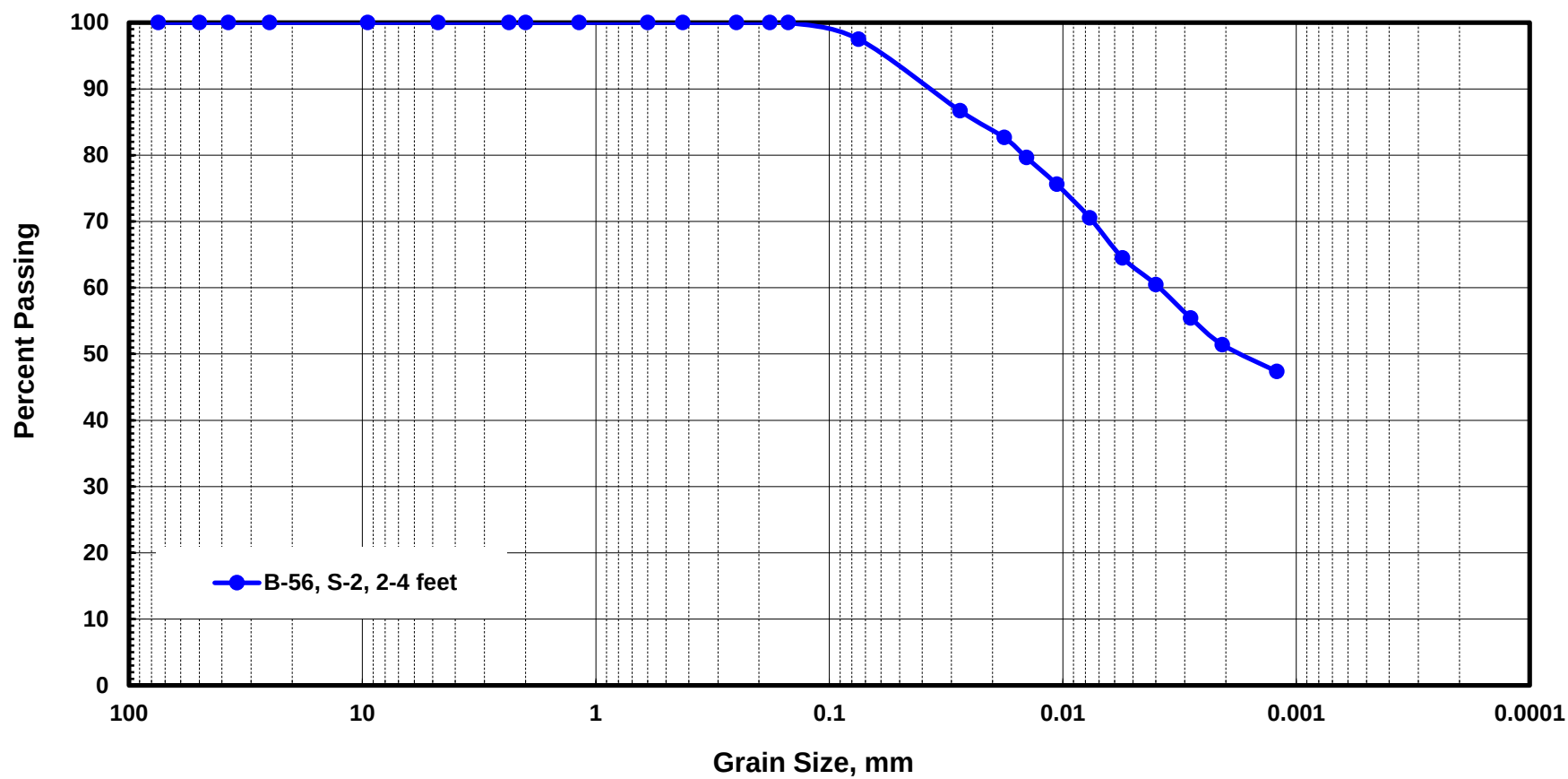
GRADATION INFORMATION	D ₁₀	D ₃₀	D ₅₀	D ₆₀	C _u	C _c	%Gravel	%Sand	%Fines	%Silt	%Clay
	mm	mm	mm	mm	(D ₆₀ /D ₁₀)	D ₃₀ ² /(D ₆₀ *D ₁₀)	Ret. #4	<#4 & >#200	Pass #200	0.075 mm < S > 0.002 mm	< 0.002 mm
B-40, S-2, 2-4 feet	NA	NA	NA	NA	NA	NA	0.0%	0.08	99.92	25.63	74.29
CPRA PO 181											
Fat Clay (CH), Brown/Gray											



GRADATION INFORMATION	D ₁₀	D ₃₀	D ₅₀	D ₆₀	C _u	C _c	%Gravel	%Sand	%Fines	%Silt	%Clay
	mm	mm	mm	mm	(D ₆₀ /D ₁₀)	D ₃₀ ² /(D ₆₀ *D ₁₀)	Ret. #4	<#4 & >#200	Pass #200	0.075 mm < S > 0.002 mm	< 0.002 mm
B-50, S-3, 4-6 feet	NA	0.0021	0.0174	0.0353	NA	NA	0.0%	24.01	75.99	46.47	29.52
CPRA PO 181											
Fat Clay with Sand (CH), Brown/Gray											



GRADATION INFORMATION		D ₁₀	D ₃₀	D ₅₀	D ₆₀	C _u	C _c	%Gravel	%Sand	%Fines	%Silt	%Clay
		mm	mm	mm	mm	(D ₆₀ /D ₁₀)	D ₃₀ ² /(D ₆₀ *D ₁₀)	Ret. #4	<#4 & >#200	Pass #200	0.075 mm < S > 0.002 mm	< 0.002 mm
B-54, S-1, 0-2 feet		NA	0.0025	0.0169	0.0244	NA	NA	0.0%	9.39	90.61	62.14	28.47
CPRA PO 181												
Fat Clay (CH), Brown/Gray												



GRADATION INFORMATION	D ₁₀	D ₃₀	D ₅₀	D ₆₀	C _u	C _c	%Gravel	%Sand	%Fines	%Silt	%Clay
	mm	mm	mm	mm	(D ₆₀ /D ₁₀)	D ₃₀ ² /(D ₆₀ *D ₁₀)	Ret. #4	<#4 & >#200	Pass #200	0.075 mm < S > 0.002 mm	< 0.002 mm
B-56, S-2, 2-4 feet	NA	NA	0.0017	0.0039	NA	NA	0.0%	2.5	97.5	46.42	51.08
CPRA PO 181											
Fat Clay (CH), Gray											

		Specific Gravity ASTM D854		
Project Name: CPRA PO 181		Checked By:		Computed By:
Project No: 02541139		Date tested:		
Date Prepared:		Scales Control Number:		03ES254
Tested by: DD		Oven Control Number:		02OV251
Boring Number :	M-10			
Sample Number:	S-3			
Depth, feet:	4-6' (B)			
Percent passing No. 4 sieve:	100%			
Description of Material:	Gray Silty Sand (SM)			
Pyconmeter Calibration Information				
Flask ID.	22PYC254			
Calibrated Mass of Flask (Mpc), gms:	164.840			
Calibrated Volume of the Flask (Vpc), mL:	499.140			
Check for Pyconmeter Calibration				
Mass of Flask, gms(Mp):	164.82	Is Mp within 0.06g of Mpc (Y/N):		Y
Mass of Flask + Water, (Mpw,t), gms:	662.45			
Temperature of Water (Ct), deg. C:	25.4	(Temperature of Water After Cooling)		
Density of Water at Tt, (pw,ct), g/mL:	0.99694	(From Table 1 of ASTM D 854)		
Volume of the Flask (Vp), mL:	499.157	$Vp = (Mpwct - Mp)/pw,ct$		
Test Data (Method B)				
Mass of Flask + Water at Test (Mpw, t), gms:	662.126	$(Mpw = Mp + (Vp * pw,t))$		
Mass of Dry Soil (Ms), gms:	100.81			
Mass of Flask + Water + Soil, (Mpws, t), gms:	725.15			
Temperature of Water (Tt), deg. C:	27.8	(Temperature of Water+Soil After Cooling)		
Density of Water at Tt, (pw,t), g/mL:	0.99629	(From Table 1 of ASTM D 854)		
Specific gravity of Water at T, (Gt):	2.668	$Gt = Ms / \{Mpw,t - (Mpwst - Ms)\}$		
Temperature Coefficient, K:	0.99808	(From Table 1 of ASTM D 854)		
Specific gravity of soil, Gs (20 deg. C):	2.663	$Gs (20C) = Gt * K$		
Remarks:				

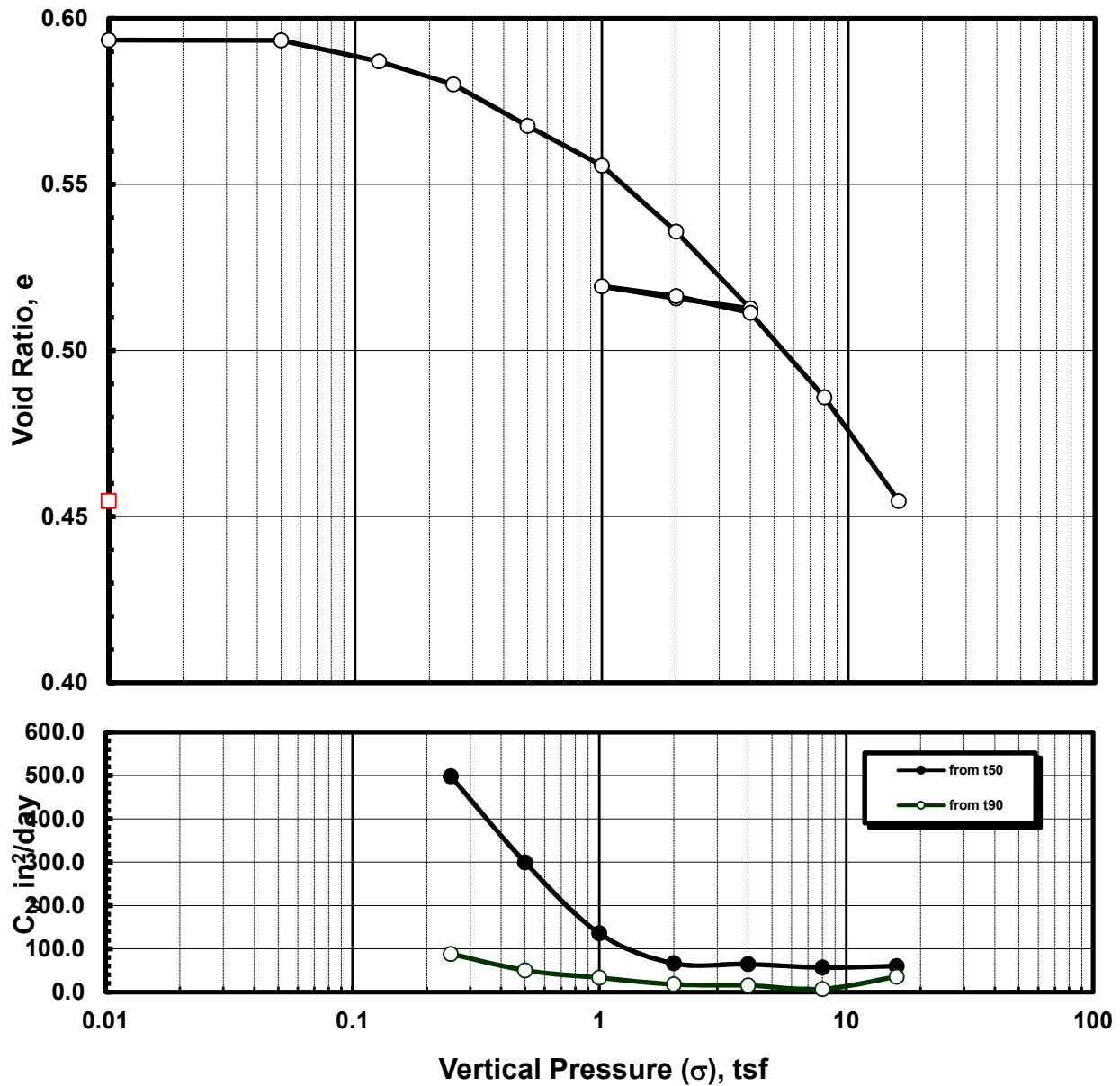
ORGANIC CONTENT

Boring No.	Depth (feet)	Organic Content (%)
M-10	1 to 2 feet	22.3%

APPENDIX D

CONSOLIDATION TEST RESULTS
SETTLING COLUMN TEST
SLURRY CONSOLIDATION TEST RESULTS

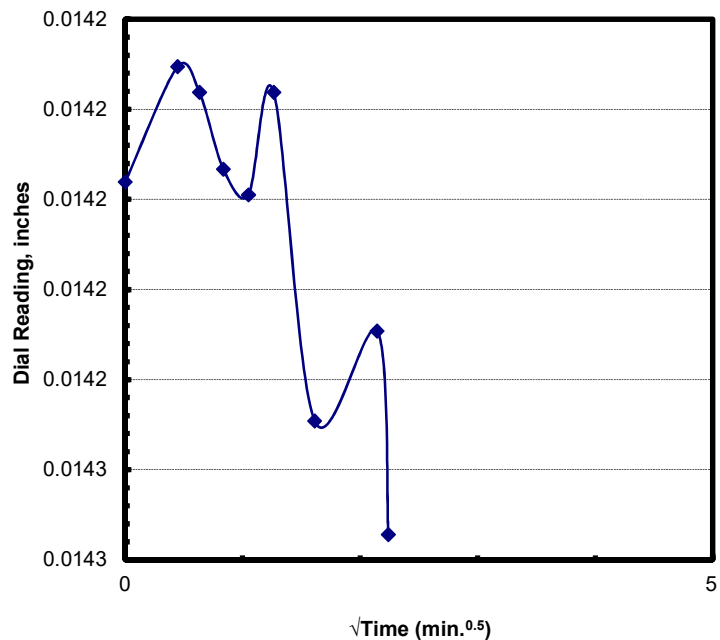
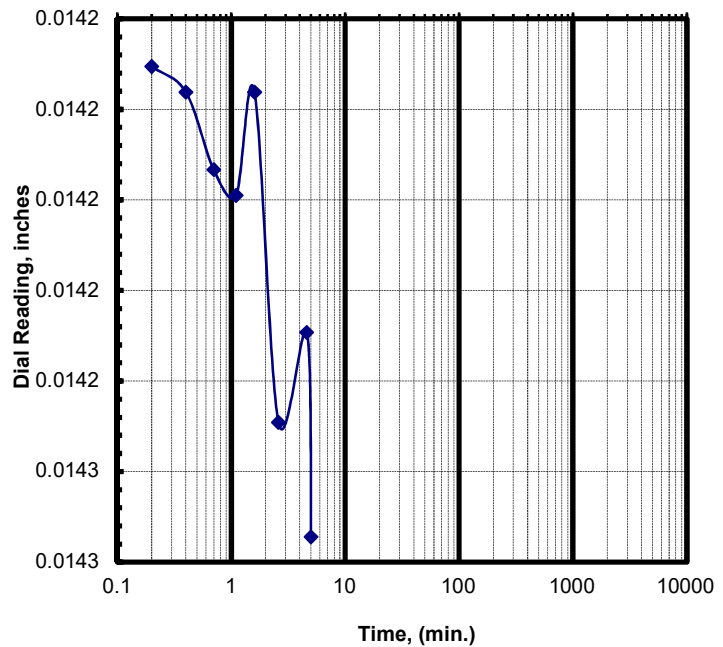
CONSOLIDATION TEST RESULTS - ASTM D 2435



Sample Condition	Before	After	Consolidation Parameters	
Moisture Content, %:	19.3%	16.8%	Overburden Pressure, tsf:	0.2
Sample Height, in.:	0.9980	0.9111	Preconsolidation Pressure, tsf:	1.1
Void Ratio, e:	0.5935	0.4547	Compression Index, (Cc):	0.103
Unit Weight, pcf:	105.7	115.8	Re-Compression Index, (Cr)	0.010
Degree of Saturation:	0.88	1.00	Swell Index, Cs:	-
Project Number: 02541139			Liquid Limit: NA	Hand Pen: 0.33 tsf
Project Name CPRA PO 181			Plastic Limit: NA	-200 %: NA
Boring No.: M-10			Plasticity Index: NA	SPG: 2.7
Sample ID, Depth: S-3, 4-6 ft			Classification: Clayey Sand (SC), Gray	

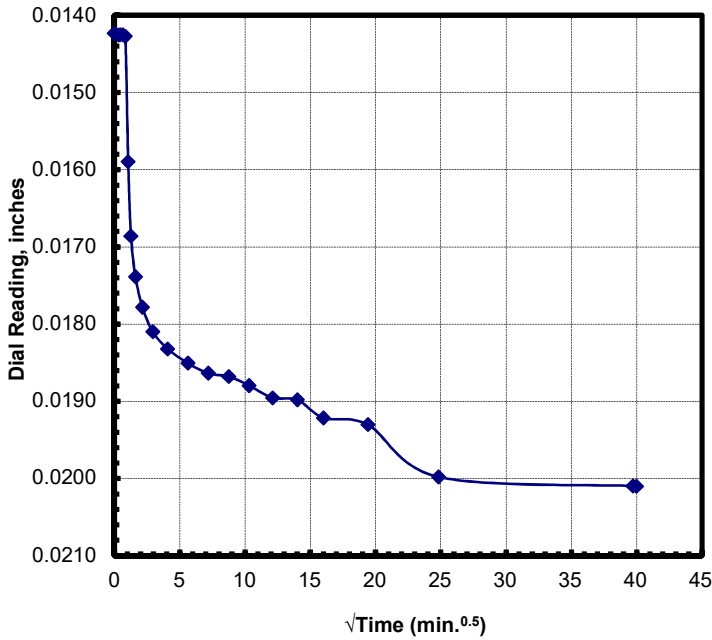
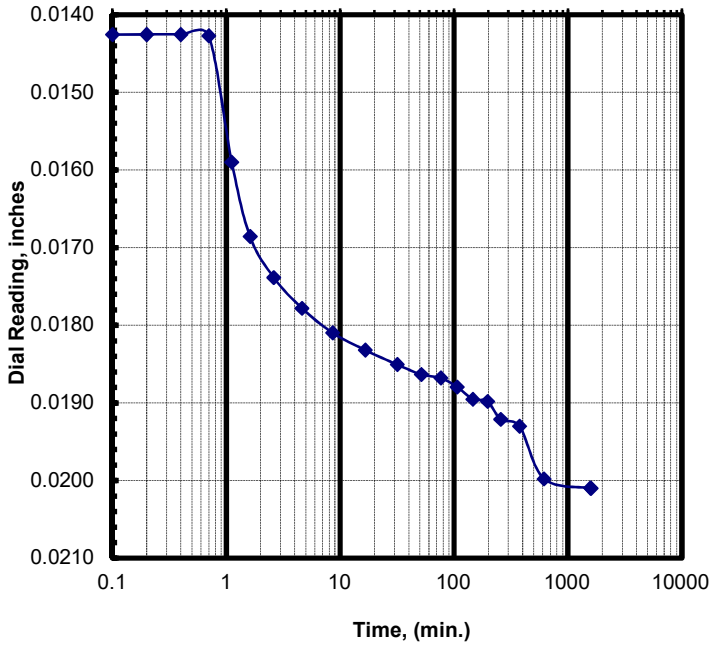
Consol M-10 S-3 4-6 (B) RR

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.05 tsf

[illegible]

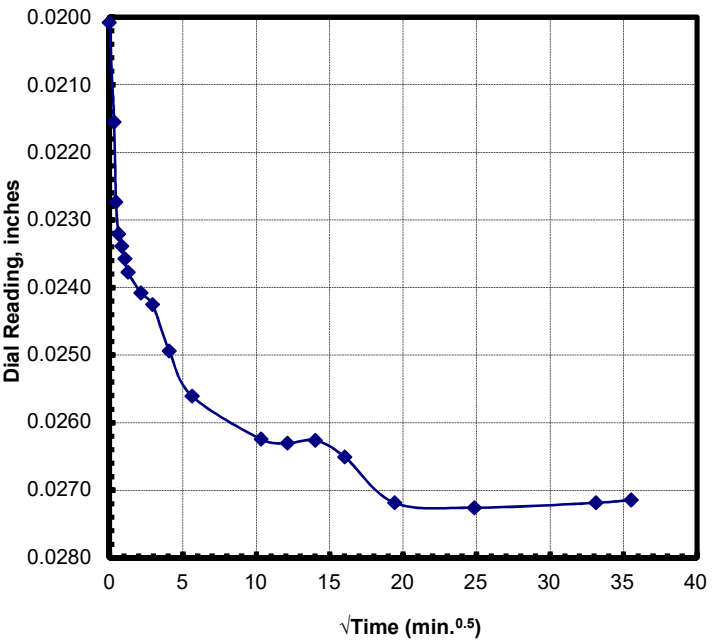
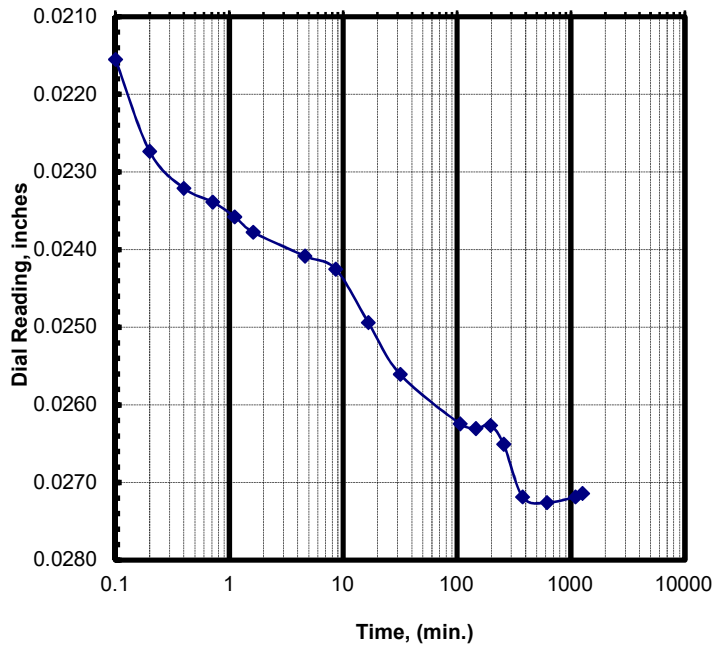
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0142	d ₀ (in): 0.0142
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0143	d ₁₀₀ (in): 0.0143
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0001	δ _{eop} (in.): 0.0001
Sample:	Clayey Sand (SC), Gray	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.125 tsf

[illegible]

Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0142	d ₀ (in): 0.0142
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0201	d ₁₀₀ (in): 0.0182
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _{hl} (in.): 0.0059	δ _{eop} (in.): 0.0040
Sample:	Clayey Sand (SC), Gray	- 200 (%): NA	t ₅₀ (min): 1.2	t ₉₀ (min): 4.8

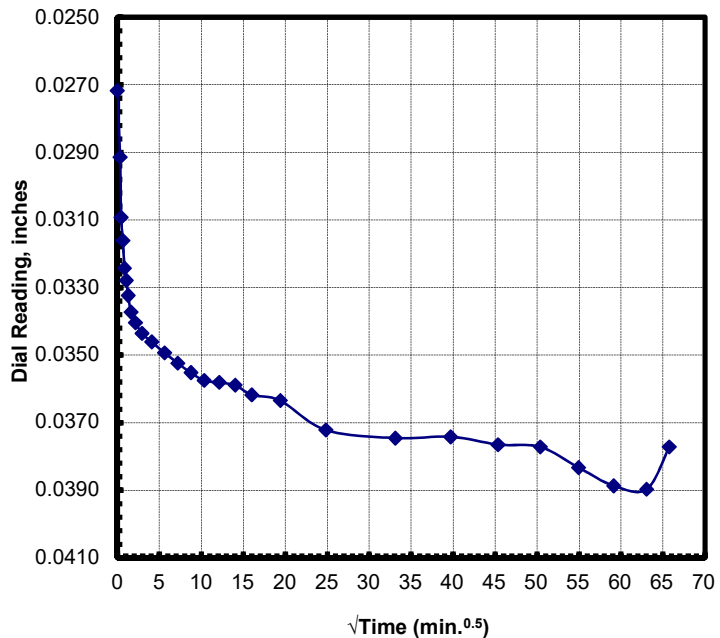
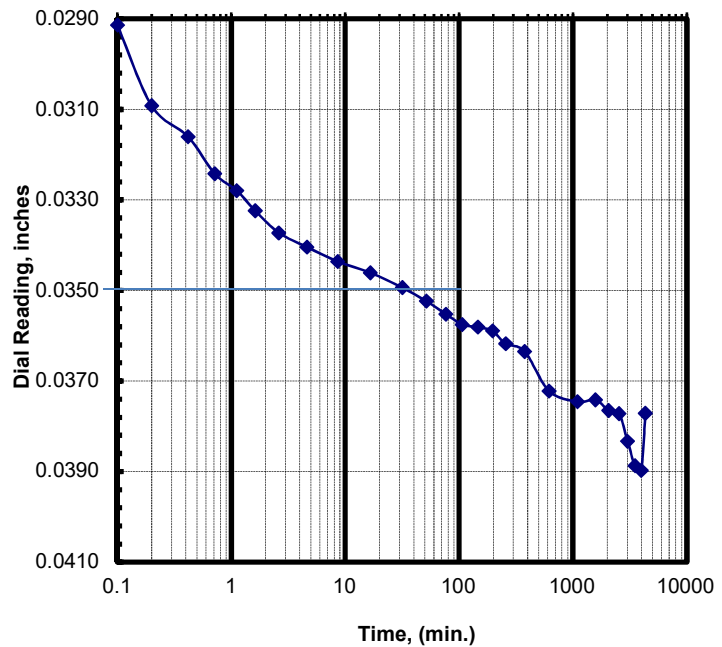
CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 0.25 tsf

[illegible]

Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0201	d ₀ (in): 0.0201
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0271	d ₁₀₀ (in): 0.0244
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _h (in.): 0.0071	δ _{eop} (in.): 0.0043
Sample:	Clayey Sand (SC), Gray	- 200 (%): NA	t ₅₀ (min): 0.14	t ₉₀ (min): 3.4

CONSOLIDATION TEST RESULTS - ASTM D 2435 ***t₅₀ & t₉₀ Curves for the Vertical Stress of 0.5 tsf***

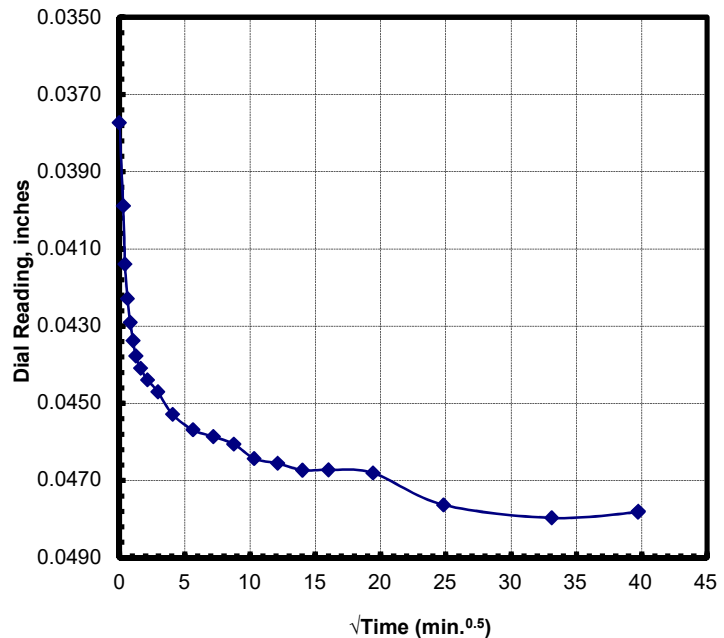
Date & Time	Elapsed Time (min)	Sqrt(Time) min ^{0.5}	Dial inches
8/6/21 9:38:31 AM	0.00	0.00	0.0272
8/6/21 9:38:37 AM	0.10	0.32	0.0291
8/6/21 9:38:43 AM	0.20	0.45	0.0309
8/6/21 9:38:56 AM	0.42	0.65	0.0316
8/6/21 9:39:14 AM	0.72	0.85	0.0324
8/6/21 9:39:38 AM	1.12	1.06	0.0328
8/6/21 9:40:08 AM	1.62	1.27	0.0332
8/6/21 9:41:08 AM	2.62	1.62	0.0337
8/6/21 9:43:08 AM	4.62	2.15	0.0340
8/6/21 9:47:08 AM	8.62	2.94	0.0344
8/6/21 9:55:08 AM	16.62	4.08	0.0346
8/6/21 10:10:08 AM	31.62	5.62	0.0349
8/6/21 10:30:08 AM	51.62	7.18	0.0352
8/6/21 10:55:09 AM	76.63	8.75	0.0355
8/6/21 11:25:09 AM	106.63	10.33	0.0357
8/6/21 12:05:09 PM	146.63	12.11	0.0358
8/6/21 12:55:09 PM	196.63	14.02	0.0359
8/6/21 1:55:09 PM	256.63	16.02	0.0362
8/6/21 3:55:09 PM	376.63	19.41	0.0363
8/6/21 7:55:10 PM	616.65	24.83	0.0372
8/7/21 3:55:10 AM	1096.65	33.12	0.0375
8/7/21 11:55:10 AM	1576.65	39.71	0.0374
8/7/21 7:55:11 PM	2056.67	45.35	0.0376
8/8/21 3:55:11 AM	2536.67	50.37	0.0377
8/8/21 11:55:12 AM	3016.68	54.92	0.0383
8/8/21 7:55:12 PM	3496.68	59.13	0.0389
8/9/21 3:55:13 AM	3976.70	63.06	0.0390
8/9/21 9:38:36 AM	4320.08	65.73	0.0377



Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0272	d ₀ (in): 0.0272
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0377	d ₁₀₀ (in): 0.0350
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0105	δ _{eop} (in.): 0.0078
Sample:	Clayey Sand (SC), Gray	- 200 (%) : NA	t ₅₀ (min): 0.23	t ₉₀ (min): 5.9

t50 & t90 Curves for the Vertical Stress of 1 tsf

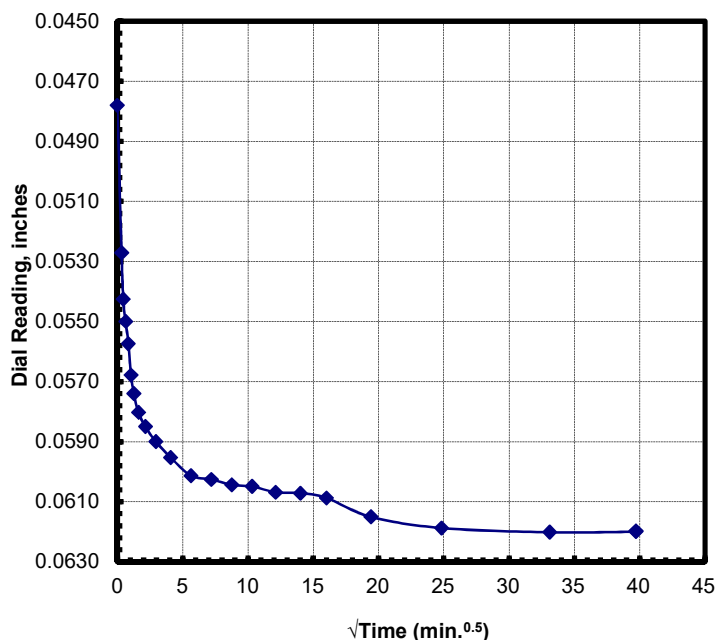
Project Number:	02541139
Project Name:	CPRA PO 181
Boring & Depth:	M-10, S-3, 4-6 ft
Sample:	Clayey Sand (SC), Gray



LL: NA	$D_{0(\min)}$ (in): 0.0377	d_0 (in): 0.0377
PL: NA	D_{end} (in): 0.0478	d_{100} (in): 0.0452
PL: NA	Δ_{hi} (in.): 0.0101	δ_{eop} (in.): 0.0075
- 200 (%): NA	t_{50} (min): 0.5	t_{90} (min): 8.8

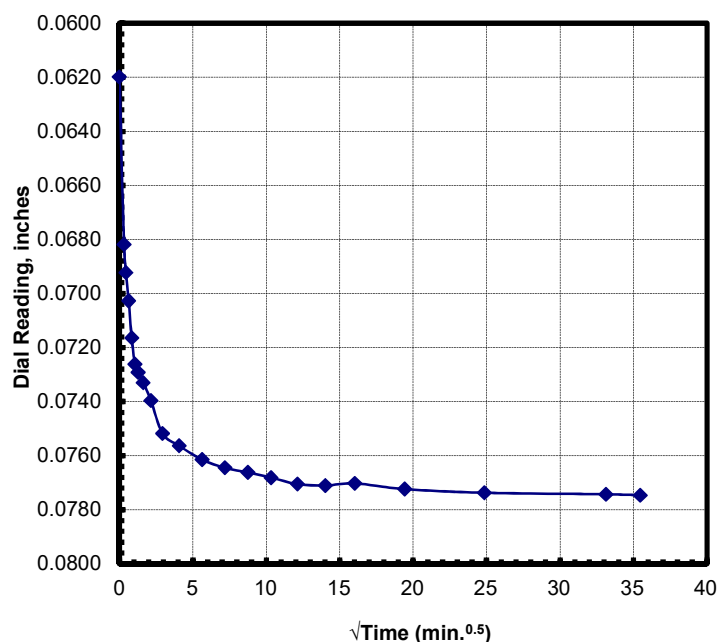
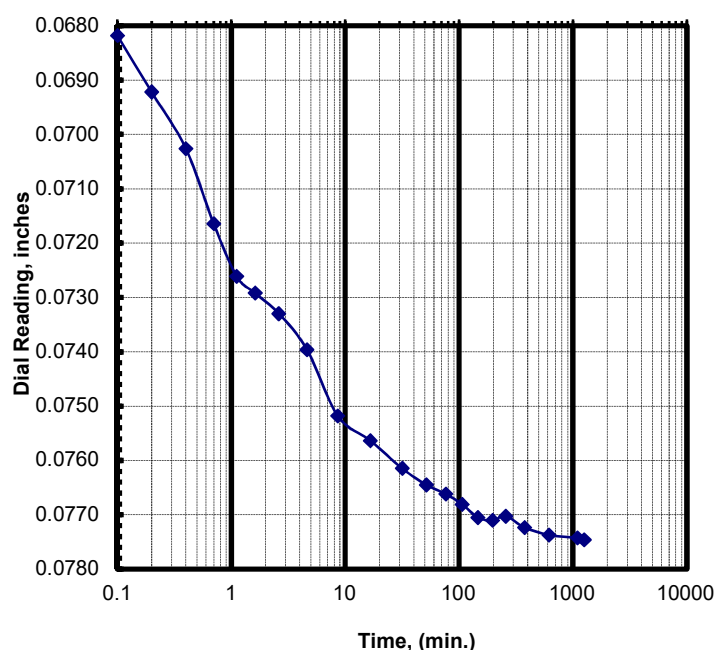
t50 & t90 Curves for the Vertical Stress of 2 tsf

Project Number:	02541139
Project Name:	CPRA PO 181
Boring & Depth:	M-10, S-3, 4-6 ft
Sample:	Clayey Sand (SC), Gray



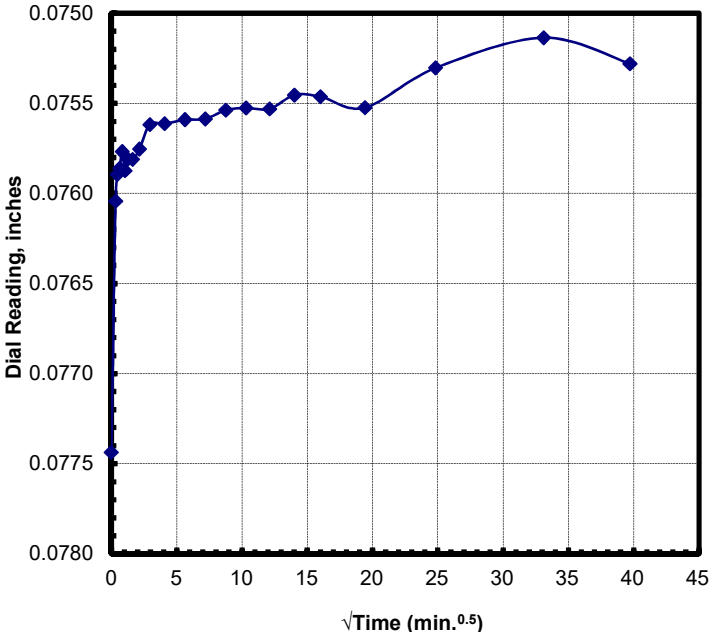
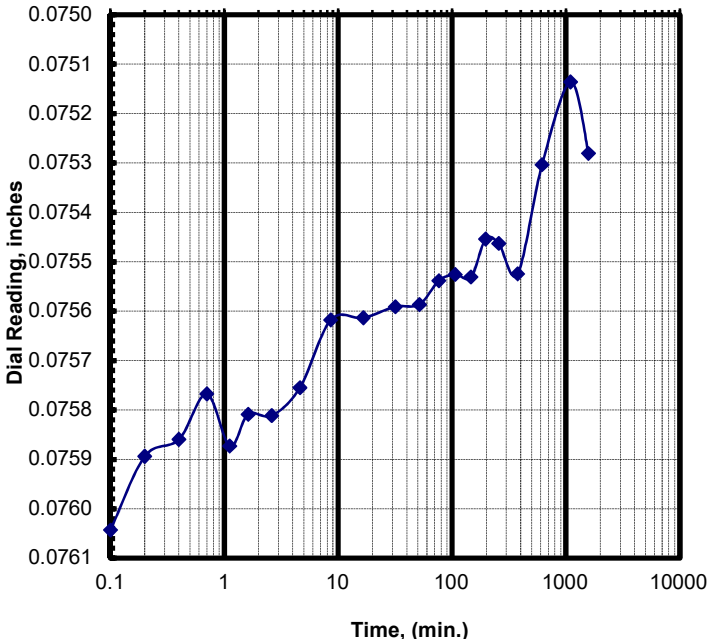
LL: NA	$D_{0(\min)}$ (in): 0.0478	d_0 (in): 0.0478
PL: NA	D_{end} (in): 0.0620	d_{100} (in): 0.0602
PL: NA	Δ_{hi} (in.): 0.0142	δ_{eop} (in.): 0.0124
- 200 (%): NA	t_{50} (min): 1	t_{90} (min): 16

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 4 tsf

[illegible]

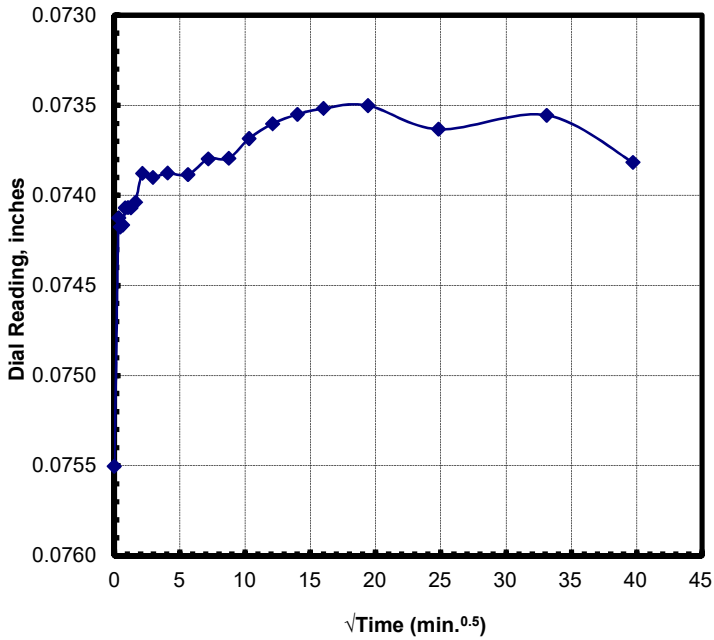
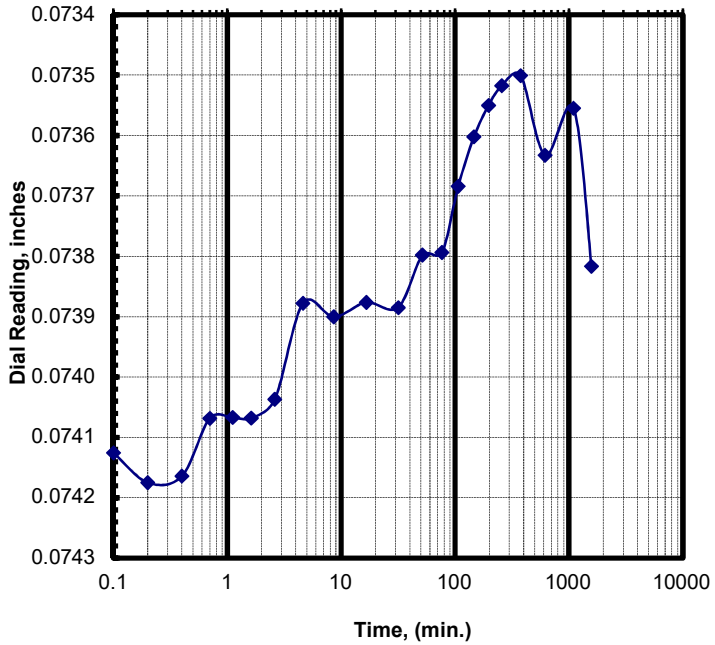
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0620	d ₀ (in): 0.0620
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0775	d ₁₀₀ (in): 0.0765
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0155	δ _{eop} (in.): 0.0145
Sample:	Clayey Sand (SC), Gray	- 200 (%): NA	t ₅₀ (min): 1	t ₉₀ (min): 18

CONSOLIDATION TEST RESULTS - ASTM D 2435 ***t₅₀ & t₉₀ Curves for the Vertical Stress of 2 tsf***

[illegible]

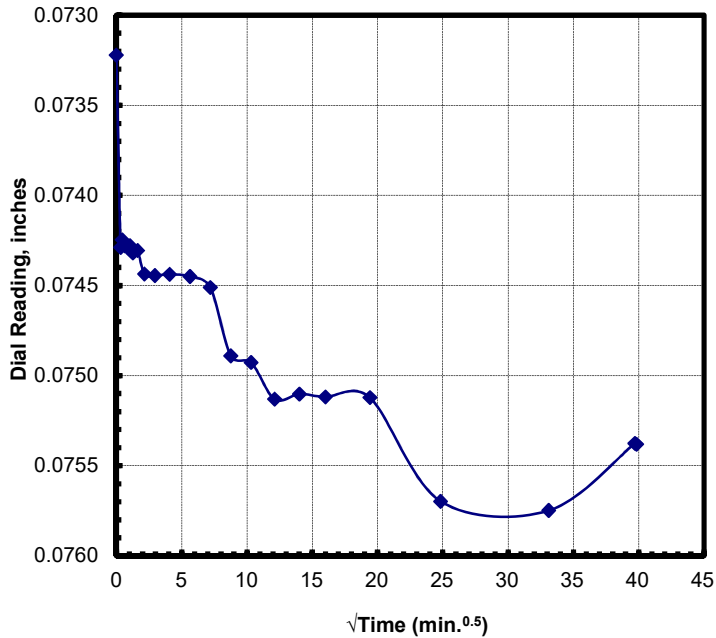
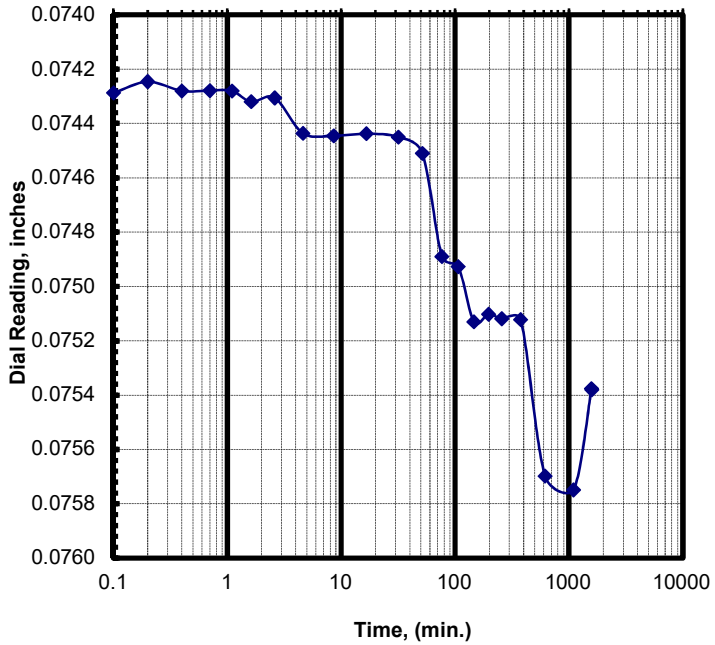
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0774	d ₀ (in): 0.0774
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0755	d ₁₀₀ (in): 0.0755
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): -0.0019	δ _{eop} (in.): -0.0019
Sample:	Clayey Sand (SC), Gray	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 1 tsf

[illegible]

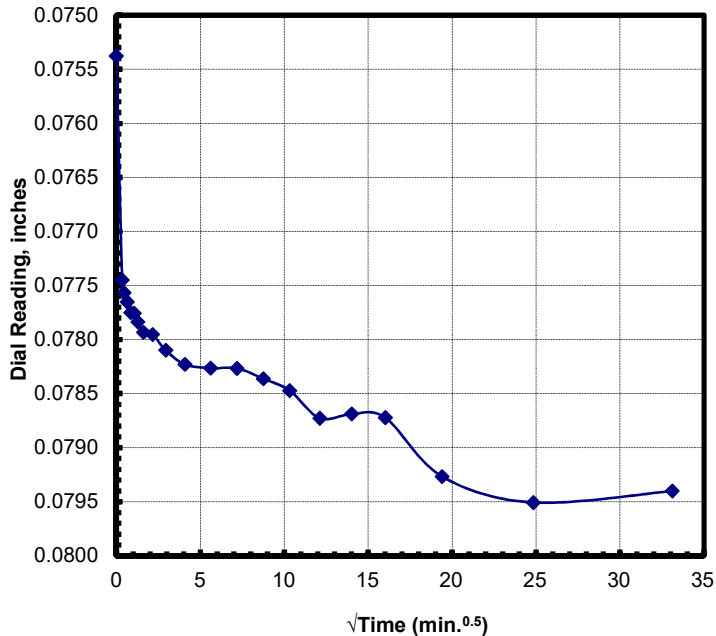
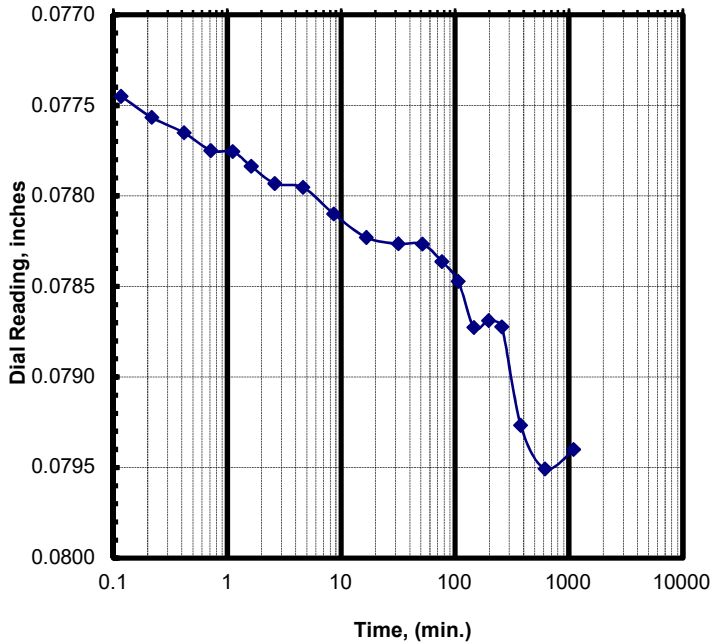
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0755	d ₀ (in): 0.0755
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0732	d ₁₀₀ (in): 0.0732
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): -0.0023	δ _{eop} (in.): -0.0023
Sample:	Clayey Sand (SC), Gray	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 2 tsf

[illegible]

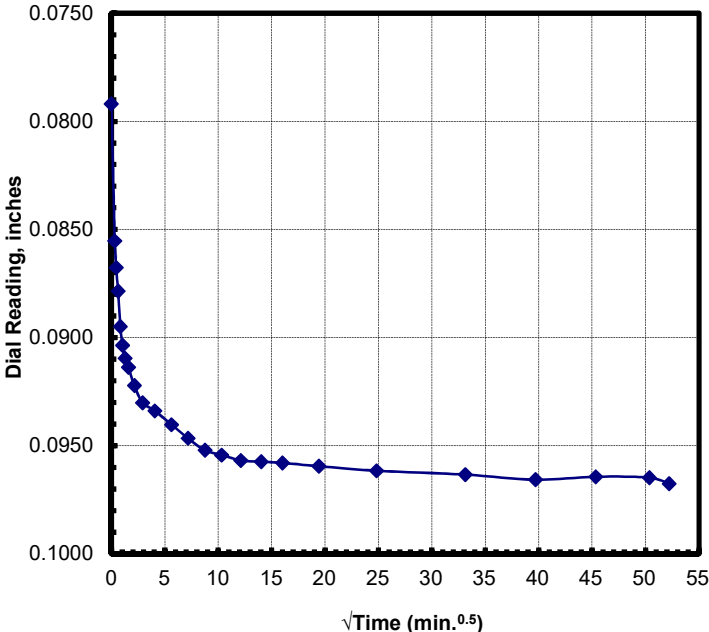
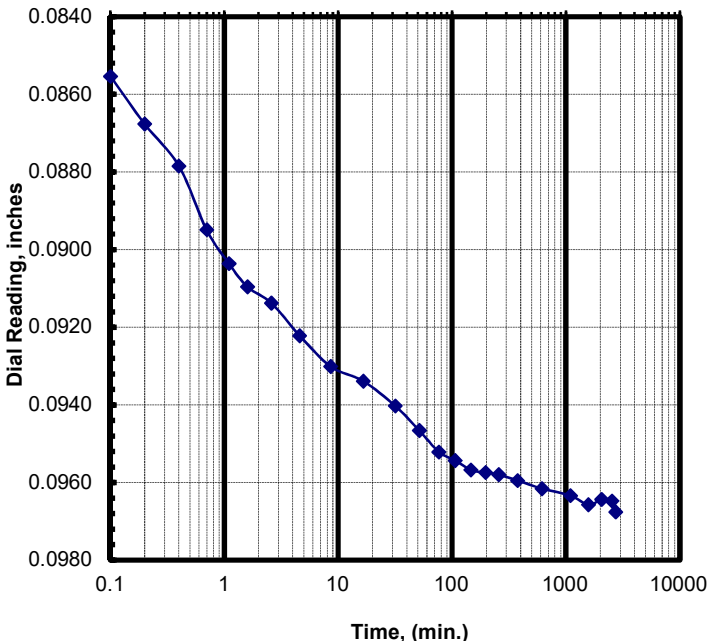
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0732	d ₀ (in): 0.0732
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0754	d ₁₀₀ (in): 0.0751
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0022	δ _{eop} (in.): 0.0019
Sample:	Clayey Sand (SC), Gray	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 4 tsf

[illegible]

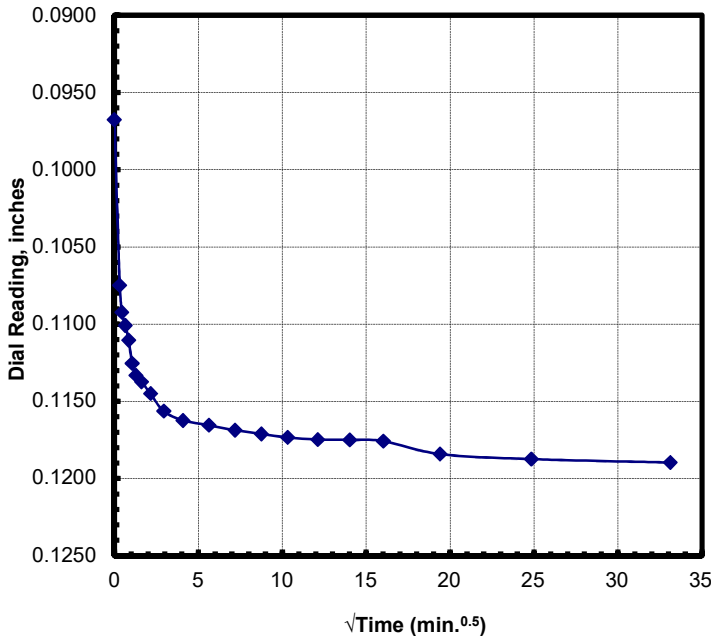
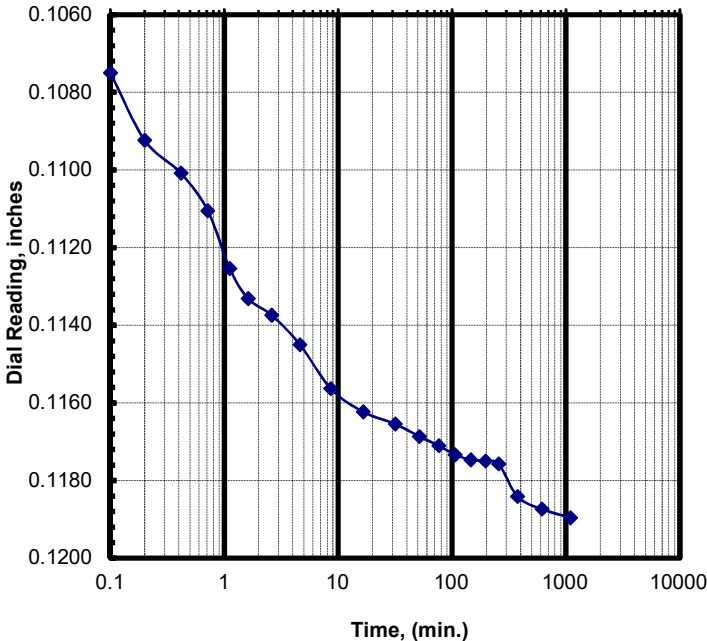
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0754	d ₀ (in): 0.0754
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0792	d ₁₀₀ (in): 0.0785
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0038	δ _{eop} (in.): 0.0031
Sample:	Clayey Sand (SC), Gray	- 200 (%): NA	t ₅₀ (min):	t ₉₀ (min):

CONSOLIDATION TEST RESULTS - ASTM D 2435
t₅₀ & t₉₀ Curves for the Vertical Stress of 8 tsf

[illegible]

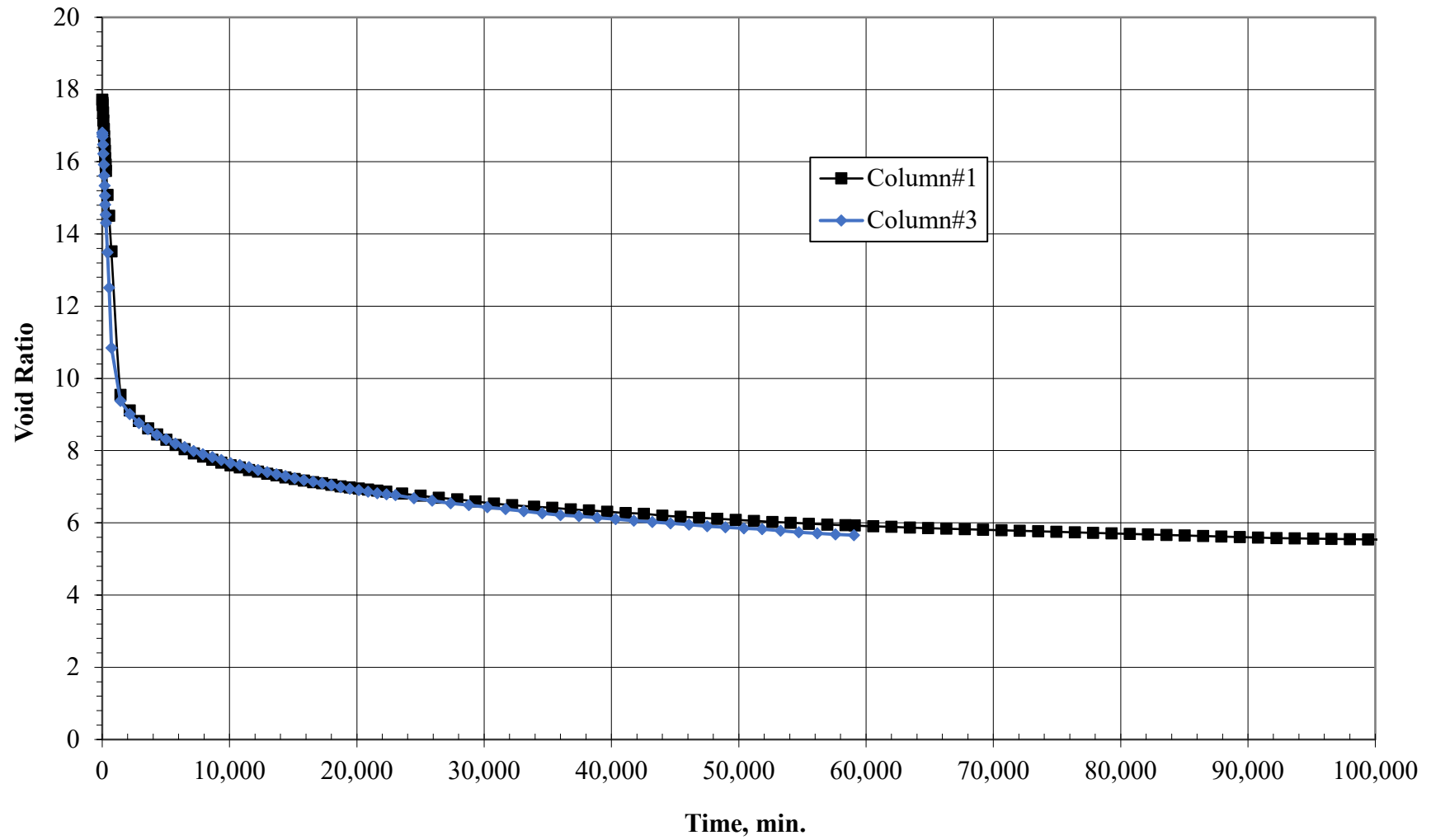
Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0792	d ₀ (in): 0.0792
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.0968	d ₁₀₀ (in): 0.0952
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0176	δ _{eop} (in.): 0.0160
Sample:	Clayey Sand (SC), Gray	- 200 (%): NA	t ₅₀ (min): 1.1	t ₉₀ (min): 38

CONSOLIDATION TEST RESULTS - ASTM D 2435
t50 & t90 Curves for the Vertical Stress of 16 tsf

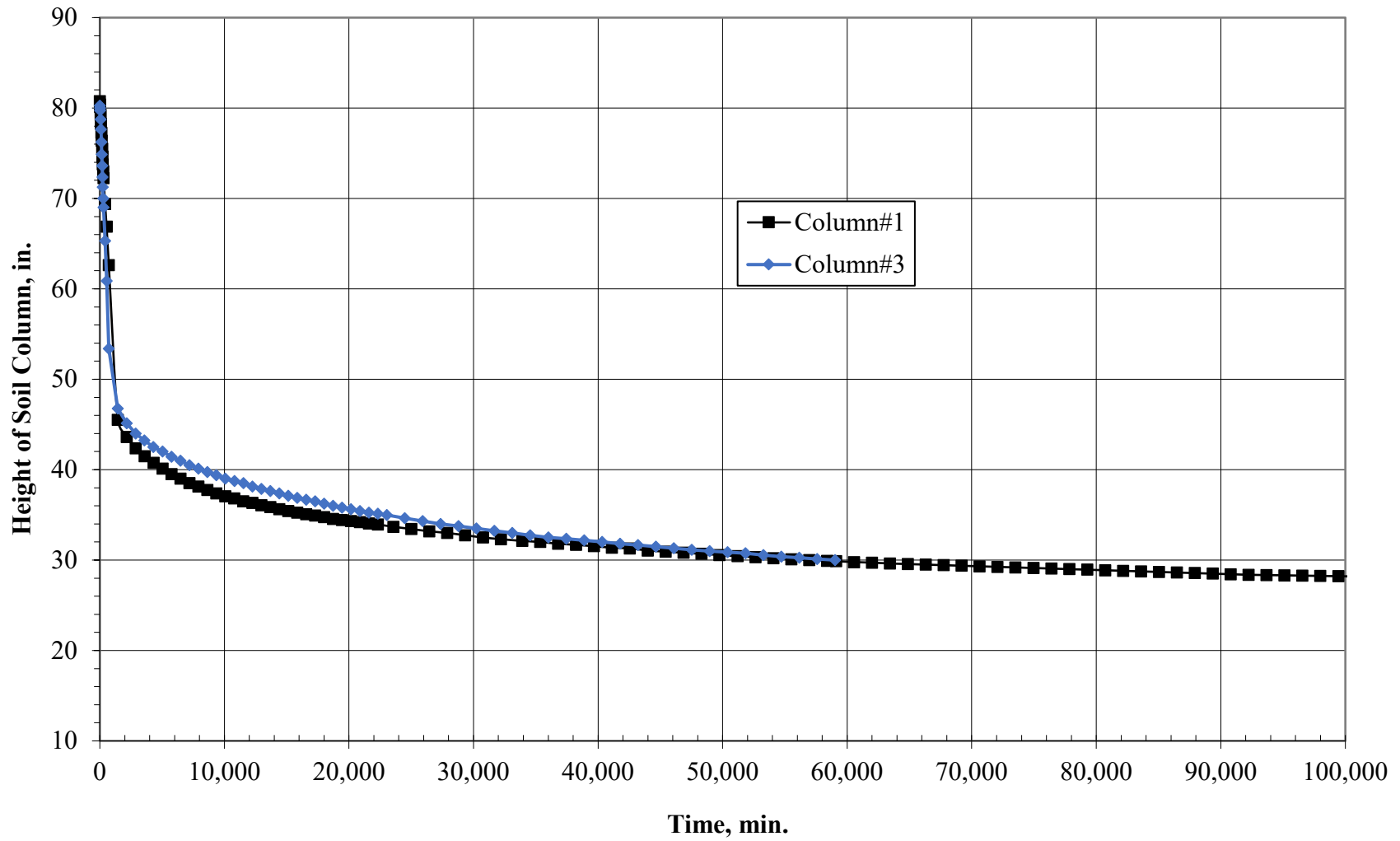
[illegible]

Project Number:	02541139	LL: NA	D _{0(min)} (in): 0.0968	d ₀ (in): 0.0968
Project Name:	CPRA PO 181	PL: NA	D _{end} (in): 0.1190	d ₁₀₀ (in): 0.1163
Boring & Depth:	M-10, S-3, 4-6 ft	PL: NA	Δ _{hi} (in.): 0.0222	δ _{eop} (in.): 0.0195
Sample:	Clayey Sand (SC), Gray	- 200 (%): NA	t ₅₀ (min): 1	t ₉₀ (min): 7.3

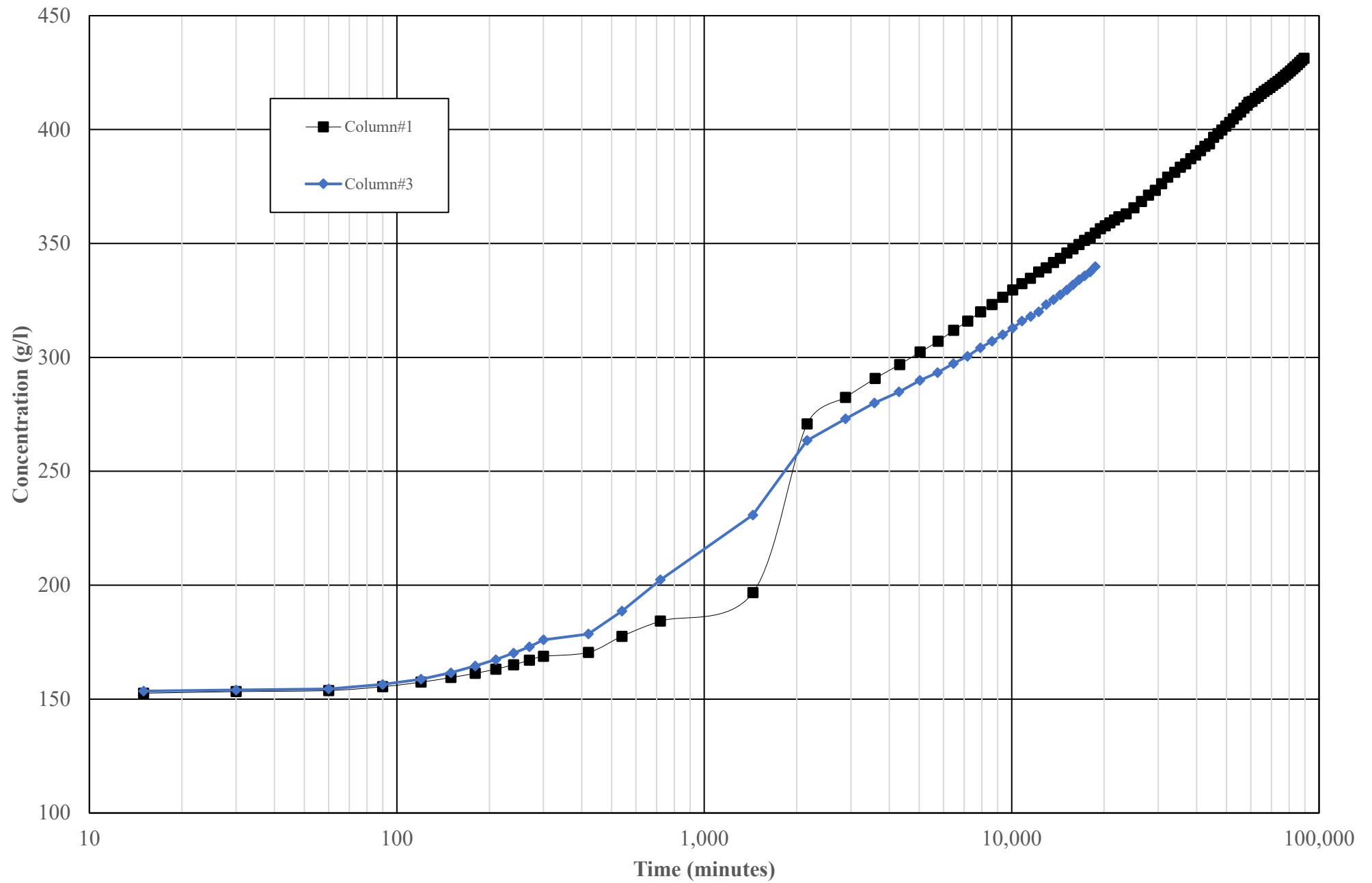
SETTLING COLUMN TEST RESULTS



SETTLING COLUMN TEST RESULTS



SETTLING COLUMN TEST RESULTS



SELF-WEIGHT TESTS

Initial Sample Consolidation

