Long Point Bayou Marsh Creation Project 95\% Design Report<br>Appendix G: Design Calculations

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### 1.0 Tidal Datum Evaluation

## A. Given:

Hourly hydrologic data was obtained from the following CRMS station using the CIMS database:

```
Station CRMS0687
Location Lat, Long: 29.9405823, -93.35432
Date of Record 5/1/2015-5/1/2020
```


## B. Calculations:

The MHW and MLW were determined for each day from the CRMS station. The MHW and MLW values were averaged over the 5-year period to compute the average MHW, MLW, MTL, and MR values as follows:

- Mean High Water (MHW) - the arithmetic mean of all the high water elevations observed over the 5-year period.
- Mean Low Water (MLW) - the arithmetic mean of all the low water elevations observed over the 5-year period.
- Mean Tide Level (MTL) - the tidal datum equivalent to the average of MHW and MLW observed over the 5-year period: $M T L=(M H W+M L W) / 2$
- Mean Tide Range (Mn) - the tidal range between the MHW and MLW elevations observed over the 5-year period: $M R=M H W-M L W$

Table 1: Tidal Datum Evaluation Calculations

| Known Variables | Equation | Elev. Ft, NAVD88 <br> GEOID18 |
| :--- | :--- | :--- |
| MHW=5- Year Mean <br> High Water | Measured from Raw Data | +1.05 |
| MLW=5-Year Mean Low <br> Water | Measured from Raw Data | +0.06 |
| MTL=5-Year Mean Tide <br> Level | (MHW+MLW)/2 | +0.56 |
| MR=5-Year Mean Tide <br> Range | MHW-MLW | +0.99 |

### 2.0 Percent Inundation Determination

## A. Given:

See Section 1.0A Above
Percent inundation refers to the percentage of the year a certain elevation of land would be flooded, by taking into account both tidal and non-tidal influences. Using percent inundation rather than tidal range as a proxy for marsh health (depending on the marsh type) can give a more accurate representation of the water levels found in the area. The hourly data collected in Section 1.0A) was used to compute the percent inundation levels.

## B. Methodology:

1. Collect the hourly hydrographic data from the CRMS stations.
2. Evaluate the marsh type present at CRMS 0687 to determine the ideal percent inundation range. For CS-0085, the CRMS stations show saline marsh, relating to a $20-80 \%$ optimal inundation range.

| Optimal marsh inundation ranges in Louisiana |  |
| :--- | :---: |
| Marsh Type | Optimal Inundation Range |
| Fresh | $10 \%-90 \%$ |
| Intermediate | $10 \%-90 \%$ |
| Brackish | $10 \%-65 \%$ |
| Saline | $20 \%-80 \%$ |

Figure 1: CPRA Marsh Creation Design Guidelines v1
3. For each inundation value, calculate the target percentile for the entire data set.

- $1 \%$ Inundated $=99$ th Percentile of water level elevations
- $10 \%$ Inundated $=90$ th Percentile of water level elevations
- $20 \%$ Inundated $=80$ th Percentile of water level elevations
- $30 \%$ Inundated $=70$ th Percentile of water level elevations
- $40 \%$ Inundated $=60$ th Percentile of water level elevations
- $50 \%$ Inundated $=50$ th Percentile of water level elevations
- $60 \%$ Inundated $=40$ th Percentile of water level elevations
- $70 \%$ Inundated $=30$ th Percentile of water level elevations
- $80 \%$ Inundated $=20$ th Percentile of water level elevations
- $90 \%$ Inundated $=10$ th Percentile of water level elevations

4. Apply RSLR to the computed inundation elevations to determine the target year 20 (TY20) tidal elevations.
C. Calculations:

| Percent Inundated | Equation | Inundation Elev. Ft. <br> NAVD88 GEOID18 |
| :---: | :---: | :---: |
| 10 | $0.9^{*}$ Raw Data Elevations | 1.36 |
| 20 | $0.8^{*}$ Raw Data Elevations | 1.11 |
| 30 | $0.7^{*}$ Raw Data Elevations | 0.94 |
| 40 | $0.6^{*}$ Raw Data Elevations | 0.79 |
| 50 | 0.5 Raw Data Elevations | 0.66 |
| 60 | 0. R $^{*}$ Raw Data Elevations | 0.51 |
| 65 | $0.35^{*}$ Raw Data Elevations | 0.43 |
| 70 | 0.3 Raw Data Elevations | 0.35 |
| 80 | 0.2 R Raw Data Elevations | 0.14 |
| 90 | 0.1 Raw Data Elevations | -0.15 |

### 3.0 Sea Level Rise and Subsidence

The 2017 Coastal Master Plan provides predicted sea level rise rates for use in the design of marsh creation projects. These rates range from 0.5 to 1.98 meters of sea level rise by 2100 and are bracketed in various scenarios to account for uncertainty. CPRA's Planning and Research Division recommends the use of the $1.0-\mathrm{m}$ gulf sea-level rise scenario shown below (Demarco, 2012). This accounts for nearly 6 inches of sea-level rise over the 20 -year project design life.


Figure 2: Gulf Sea-Surface Change Relative to 1992

Subsidence rates in this region are based on the 2017 Coastal Master Plan which uses the same rates from the 2012 Master Plan. The subsidence rates in the Calcasieu/Sabine basin are approximately 4.3 mm per year ( $0.014 \mathrm{ft} /$ year) (Reed, Yuill 2016).


Figure 3: 2012 Coastal Master Plan - subsidence ranges in Coastal Louisiana
The rates of eustatic sea level rise (ESLR) and subsidence were used to determine the annual incremental relative sea level rise (RSLR) for the CS-0079 project area over the 20-year project life.

$$
E(t)=a t+b t^{2}+S t
$$

Where $E$ is the change in relative sea level at time, $t$
a is the rate of ESLR
$b$ is an acceleration factor, and
S is the rate of subsidence
The annual incremental ESLR and RSLR is shown in the following table.

Table 2: Annual Incremental ESLR and RSLR (ft NAVD88, GEOID18)

| Year | Annual Incremental Subsidence (St) (ft) | Annual Incremental Eustatic Sea Level Rise $(a t+b t)(\mathbf{f t})$ | Annual Incremental Relative Sea Level Rise $\left(a t+b t^{2}+S t\right)(\mathbf{f t})$ |
| :---: | :---: | :---: | :---: |
| 2020 | 0.000 | 0.404 | 0.404 |
| 2021 | 0.014 | 0.424 | 0.439 |
| 2022 | 0.028 | 0.445 | 0.473 |
| 2023 (TY0) | 0.042 | 0.466 | 0.508 |
| 2024 | 0.056 | 0.488 | 0.544 |
| 2025 | 0.071 | 0.509 | 0.580 |
| 2026 | 0.085 | 0.532 | 0.616 |
| 2027 | 0.099 | 0.554 | 0.653 |
| 2028 | 0.113 | 0.577 | 0.113 |
| 2029 | 0.127 | 0.601 | 0.728 |
| 2030 | 0.141 | 0.624 | 0.765 |
| 2031 | 0.155 | 0.649 | 0.804 |
| 2032 | 0.169 | 0.673 | 0.842 |
| 2033 | 0.183 | 0.698 | 0.882 |
| 2034 | 0.198 | 0.724 | 0.921 |
| 2035 | 0.212 | 0.749 | 0.961 |
| 2036 | 0.226 | 0.776 | 1.001 |
| 2037 | 0.240 | 0.802 | 1.042 |
| 2038 | 0.254 | 0.829 | 1.083 |
| 2039 | 0.268 | 0.856 | 1.125 |
| 2040 | 0.282 | 0.884 | 1.166 |
| 2041 | 0.296 | 0.912 | 1.209 |
| 2042 | 0.310 | 0.941 | 1.251 |
| 2043 | 0.324 | 0.970 | 1.295 |

### 4.0 Containment Dike Design

1. Crown Width: 5.0 ft
2. Side Slope: $3 \mathrm{H}: 1 \mathrm{~V}$
3. Freeboard: minimum 1.0 ft above target marsh elevation
4. Containment Dike Crown Elevation: +4.75 ft . NAVD88 GEOID18
5. Internal Training Dike Elevation: +2.75 ft . NAVD88 GEOID18
6. Survey Data: XYZ points from transects
7. Cut-to-fill Ratio: 1.4:1


Figure 4: Earthen Containment Dike Geometry
Where: $\mathrm{H}=$ Dike Height
B= Base Width
C $=$ Crown Width
$\mathrm{EB}=$ Base Elevation
$\mathrm{EC}=$ Crown Elevation
$\mathrm{A}_{\mathrm{A}}=$ Cross-Sectional Area of dike at Point A
$A_{B}=$ Cross-Sectional Area of dike at Point $B$
$\mathrm{A}_{\mathrm{A}-\mathrm{B}}=$ Average Cross-Sectional Area of dike between points A \& B
$L_{A-B}=$ Length between points A \& B
$\mathrm{S}_{\mathrm{H}}=$ Side Slope
The dike lengths and volumes were calculated in Civil 3D and are shown in the table below:

| Reach | Borrow | Centerline Length <br> (ft) | Fill Volume <br> (CY) | Cut Volume <br> (CY) | Dimension |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Containment <br> Dike - North | External <br> Borrow | 6,970 | 29,739 | 41,635 | $4.75^{\prime}$ EL.; <br> $5^{\prime}$ crown <br> width; $3: 1$ <br> side slope |
| Containment <br> Dike- <br> Southeast and <br> West | Internal <br> Borrow* | 12,709 | 42,245 | 59,143 |  |
| Internal <br> Training Dike | Internal <br> Borrow* | 3,861 | 9,501 | 13,301 | $2.75^{\prime}$ EL.; <br> 5 crown <br> width; $3: 1$ <br> side slope |
|  |  |  |  |  |  |

*Internal Borrow Cut volume (59,143+13,301) $=72,444$ CY will be backfilled by hydraulically dredged sediment from the Calcasieu Ship Channel.

### 5.0 Fill Area Design

## A. Given:

i. Cross-Sectional Survey Data of Marsh Fill Site: XYZ data for each fill area cross-section survey transect
ii. Volume Calculation Fill Elevation (plane height): +0.71 ft NAVD88 Geoid18 (20-year elevation from +2.75 ft CMFE) and +1.10 ft NAVD88 Geoid18 to offset the foundation settlement and subsidence of the exisiting surface.

## B. Methodology:

i. Transect survey data was used to generate a 3-dimensional surface called a Triangular Irregular Network (TIN) surface in Civil 3D.
ii. Volume Calculations: The volume was calculated by taking the surface difference between the flat +1.10 ft . NAVD88 plane height surface and the existing mud line TIN surface.

These calculations were performed in Civil 3D and the required fill volume was $\mathbf{1 , 2 6 1 , 7 0 6} \mathbf{C Y}$.
5.1 Cut to Fill Ratio Marsh

This calculation below details a method for predicting the cut-to-fill ratio to be applied to marsh fill quantities.
A. Given:
i. Fine-grained or Clay sediment fraction of borrow area is $100 \%$
ii. In-situ void ratio in borrow area $=3.11$
iii. Average void ratio in the MCA at year 20 from $\operatorname{PSDDF}=3.34$

## B. Methodology:

The cut-to-fill ratio for marsh fill was estimated 20 years after dredging using the following equation from EM1110-2-5025:

$$
V_{f}=V_{i}\left\{\left(\frac{e_{o}-e_{i}}{1+e_{i}}\right)+1\right\}
$$

Where:
$\mathrm{V}_{\mathrm{f}}=$ volume of fine-grained dredged material after placement, yd3
$\mathrm{V}_{\mathrm{i}}=$ volume of fine-grained sediments from borrow area, yd3
ei $=$ average in-situ void ratio of the borrow area
$e_{0}=$ void ratio after 20 years.
i. $\quad$ Cut to fill ratio $=V_{i} / V_{f}$

## C. Calculation:

Re-arrange the formula in 5.1B to get:

$$
\frac{V_{i}}{V_{f}}=\frac{1}{\left(\left(\frac{e_{o}-e_{i}}{1+e_{i}}\right)+1\right)}
$$

Where

$$
\begin{aligned}
& \mathrm{ei}=3.11 \\
& \mathrm{e}_{\mathrm{o}}=3.34
\end{aligned}
$$

$$
\begin{gathered}
\frac{V_{i}}{V_{f}}=\frac{1}{\left(\left(\frac{3.34-3.11}{1+3.11}\right)+1\right)} \\
\frac{V_{i}}{V_{f}}=\frac{1}{\left(\left(\frac{0.23}{4.11}\right)+1\right)} \\
\frac{V_{i}}{V_{f}}=\frac{1}{((0.055)+1)} \\
\frac{V_{i}}{V_{f}}=\frac{1}{1.055} \\
\frac{V_{i}}{V_{f}}=0.947
\end{gathered}
$$

Although the calculated cut-to-fill ration is less than 1, considering losses due to dredging and dewatering cut-to-fill ratio 1.10 is selected for this project. The total volume (including containment dike volume) needed based on this evaluation is presented in the table below:

| Marsh Fill <br> Volume Only <br> (CY) | Dike Borrow <br> Area Backfill <br> Volume (CY) | Total Fill <br> Volume (CY) | Marsh Cut- <br> To-Fill | Total Cut <br> Volume (CY) |
| :---: | :---: | :---: | :---: | :---: |
| $1,261,706$ | 72,444 | $1,334,150$ | $1.1: 1$ | $1,467,565$ |

### 6.0 References

Reed, D. and Yuill, B. (2016). 2017 Coastal Master Plan: Attachment C2-2: Subsidence. Version I. (p. 15). Baton Rouge, Louisiana: Coastal Protection and Restoration Authority

DeMarco, K.E., Mouton, J. J., and Pahl, J.W. (2012). Guidance for Anticipating SLR Impacts on Louisiana Coastal Resources during Project Planning and Design: Technical Report, Version 1.4. State of Louisiana, Coastal Protection and Restoration Authority, Baton Rouge, Louisiana. 121 pp.

United States Army Corps of Engineers, EM 1110-2-5025 Dredging and Dredge Material Management. July 2015

